Snipes-Dye associates

HYDROLOGY/DRAINAGE STUDY

For

TORREYANA CAMPUS

11085 Torreyana Rd, San Diego, CA 92121 APN: 430-010-30

> City of San Diego PRJ-1056938

> > Applicant:

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Dated: August 8, 2024

DECLARATION OF RESPONSIBLE CHARGE

I. HEREBY DECLARE THAT I AM THE CIVIL ENGINEER OF WORK FOR THIS PROJECT, THAT I HAVE EXERCISED RESPONSIBLE CHARGE OVER THE DESIGN OF THE PROJECT AS DEFINED IN SECTION 6703 OF THE BUSINESS AND PROFESSIONS CODE. AND THAT THE DESIGN IS CONSISTENT WITH CURRENT STANDARDS.

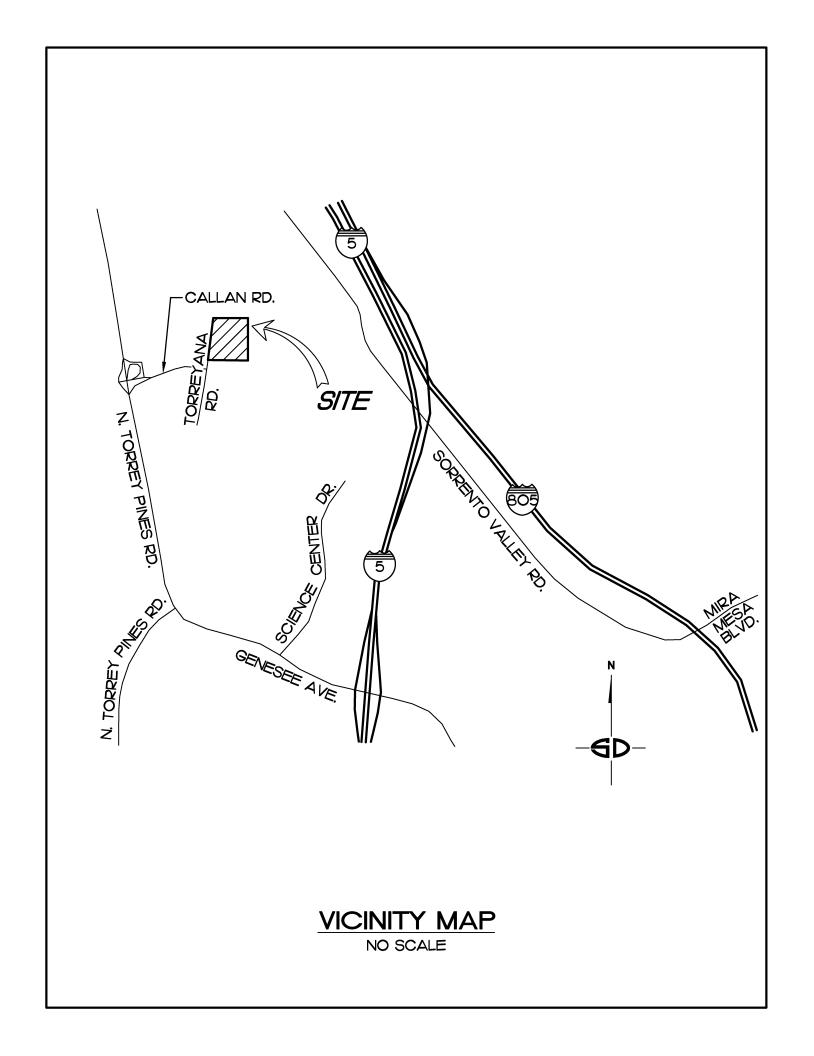
SPECIFICATIONS BY THE CONTROLL SPECIFICATIONS BY THE CONTROLL SPECIFICATIONS BY THE CONTROLL SPECIFICATION OF THE CONTROLL SPE I UNDERSTAND THAT THE CHECK OF PROJECT DRAWINGS AND SPECIFICATIONS BY THE CITY OF SAN DIEGO IS CONFINED TO A REVIEW ONLY AND DOES NOT RELIEVE ME, AS ENGINEER OF WORK, OF MY

REGISTERED Exp. 06-30-25

TE OF CALIFOR

WILLIAM A. SNIPES

R.C.E. 50477 EXP. 06-30-25



HYDROLOGY/DRAINAGE STUDY FOR TORREYANA CAMPUS CDP

INTRODUCTION: The project proposes to demolish two existing buildings and construct two new buildings, parking lot, and subsurface parking structure in La Jolla. The site is located at 11085-11095 Torreyana Rd, Assessor's Parcel No. 430-010-30.

PRE-DEVELOPMENT CONDITIONS: The existing site sits upon a graded mesa at the end of Torreyana Road. The drainage conveyance on the site is mostly impervious surfaces, with various small planters for trees scattered amongst the parking lot. The site has existing downward slopes along the north and south property boundaries and an upward slope to the east. The drainage area includes approximately 4.5 acres which consists several basins that sheet-flow and collect in a storm drain system on the site. The existing storm drain system confluences at a single point in an existing storm drain cleanout on the adjacent property to the north, which then flows down the hill towards Los Peñasquitos Lagoon. There are eight basins that make up the pre-development site. The runoff coefficient of the pre-construction site varies per basin and each has been weighted per the City of San Diego Drainage Design Manual (2017 Edition) for an Industrial site. The calculated peak discharge for the pre-construction site in the 100-year storm is approximately 13.07 cubic feet per second (cfs). See the attached calculations.

POST-DEVELOPMENT CONDITIONS: The proposed improvements consist of constructing two new buildings, parking lot, and a subsurface parking structure. The post-development site is divided into 5 basins. Basin A consists of undisturbed slopes to the east that flows downhill from the east towards the site, and is collected into an existing drainage ditch. A Type F Catch Basin will be constructed at the termination point of the ditch where it meets the proposed site to prevent run-on. Basin B consists of the upper easterly portion of the new site. Basin C consists of the lower easterly portion. Basin B is the upper westerly portion, while Basin E is the lower easterly portion. Basins B-E will each contain a Modular Wetlands System and an underground detention basin for hydromodification. All five basins will then connect to a main storm drain system in the driveway. This new main storm drain line will connect to the existing storm drain system on the site that runs down the northerly slope towards Los Peñasquitos Lagoon as described in the previous section. The runoff coefficient of the post-construction site varies per basin and each has been weighted per the City of San Diego Drainage Design Manual (2017 Edition) for an Industrial site.

The total peak discharge from post-construction site in the 100-year storm is 12.45 cfs, a decrease of 0.62 cfs from the pre-development condition. See the attached calculations. The following Summary Table 1 is a summary of the discharges for the pre and post-development conditions.

It should be noted that the proposed project does not discharge runoff directly into the Pacific Ocean, a navigable water, and is therefore not required to obtain approval from the Regional Water Quality Control Board under Federal Clean Water Act (CWA) section 401. There are no dredged or fill materials being discharged into the navigable waters of the United States and is not subject to CWA section 404 as a result.

PRE & POST DEVELOPMENT 100-YR. PEAK DISCHARGES						
	PRE-DEVELOPMENT				POST-DEVELOPME	ENT
BASIN	AREA (ACRE)	TIME OF CONCENTRATION (MIN.)	DISCHARGE (CFS)	AREA (ACRE)	TIME OF CONCENTRATION (MIN.)	DISCHARGE (CFS)
Α	1.44	15.66	2.02	0.51	14.07	0.76
В	0.04	1.78	0.09	0.84	8.67	1.88
С	0.81	2.77	3.39	1.02	3.02	4.17
D	0.51	6.56	1.79	0.76	6.17	2.55
E	0.97	3.59	3.71	1.31	7.20	3.09
F	0.35	2.65	1.40			
G	0.30	7.67	0.63			
Н	0.02	3.59	0.04			
TOTAL	4.44		13.07	4.44		12.45

CONCLUSION: The proposed construction will cause a decrease in 0.62 cfs for the overall site peak runoff in the 100-year storm. A decrease is expected as a result of altered flow paths and proposed detention facilities. It should be noted that the runoff discharge from the post-development condition are at the same locations as in the pre-development condition. The proposed project will not substantially alter the existing drainage pattern of the site or drainage basin areas.

Site runoff will be significantly less in the post-development condition due to lengthened flow paths and proposed detention systems for hydromodification.

In summary, the decrease in the peak 100-year discharge due to the construction of this project will not cause significant impacts to downstream facilities, streams, rivers, and/or adjacent properties.

PRE-DEVELOPMENT DRAINAGE CALCULATIONS

BASIN A: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.50	1.44 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.50

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

36.7 Feet

D= Water Course Distance

660 Feet

 $S = Slope = (\Delta E/D) \times 100\%$

5.56 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

15.66 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

City of San Diego- Drainage Design Manual 2017

= 2.80 Inches/Hour

PEAK DISCHARGES (Q):

A = Area of the basin

1.44 Acres

Q = CIA

2.02 ft³/sec

BASIN B: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.50	0.04 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.50

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

15 Feet

D= Water Course Distance

34 Feet

 $S = Slope = (\Delta E/D) \times 100\%$

44.12 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

1.78 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

City of San Diego- Drainage Design Manual 2017

4.40 Inches/Hour

PEAK DISCHARGES (Q):

A = Area of the basin

0.04 Acres

Q = CIA

0.09 ft³/sec

BASIN C: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.95	0.81 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= **0.95**

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

3.1 Feet

D= Water Course Distance

162 Feet

 $S = Slope = (\Delta E/D) \times 100\%$

: 1.91 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

2.77 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

= 4.40 Inches/Hour

City of San Diego- Drainage Design Manual 2017

PEAK DISCHARGES (Q):

A = Area of the basin

0.81 Acres

Q = CIA

= 3.39 ft³/sec

BASIN D: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.87	0.51 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.87

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

2.2 Feet

D= Water Course Distance

238 Feet

 $S = Slope = (\Delta E/D) \times 100\%$

0.92 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

6.56 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

= 4.03 Inches/Hour

City of San Diego- Drainage Design Manual 2017

PEAK DISCHARGES (Q):

A = Area of the basin

0.51 Acres

Q = CIA

1.79 ft³/sec

BASIN E: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.87	0.97 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.87

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

7.5 Feet

D= Water Course Distance

189 Feet

 $S= Slope = (\Delta E/D) X 100\%$

= 3.97 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

3.59 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

4.40 Inches/Hour

PEAK DISCHARGES (Q):

A = Area of the basin

0.97 Acres

Q = CIA

3.71 ft³/sec

BASIN F: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.91	0.35 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.91

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

3.1 Feet

D= Water Course Distance

116 Feet

 $S = Slope = (\Delta E/D) \times 100\%$

2.67 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

2.65 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

City of San Diego- Drainage Design Manual 2017

= 4.40 Inches/Hour

PEAK DISCHARGES (Q):

A = Area of the basin

0.35 Acres

Q = CIA

1.40 ft³/sec

BASIN G: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.55	0.30 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= **0.55**

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

19.6 Feet

D= Water Course Distance

242 Feet

 $S = Slope = (\Delta E/D) \times 100\%$

8.10 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

7.67 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

3.84 Inches/Hour

City of San Diego- Drainage Design Manual 2017

PEAK DISCHARGES (Q):

A = Area of the basin

0.30 Acres

Q = CIA

0.63 ft³/sec

BASIN H: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.50	0.02 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.50

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

3.9 Feet

D= Water Course Distance

: 46 Feet

 $S= Slope = (\Delta E/D) X 100\%$

8.48 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

3.59 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

4.40 Inches/Hour

City of San Diego- Drainage Design Manual 2017

PEAK DISCHARGES (Q):

A = Area of the basin

0.02 Acres

Q = CIA

0.04 ft³/sec

POST-DEVELOPMENT DRAINAGE CALCULATIONS

BASIN A: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.50	0.51 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.50

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

34 Feet

D= Water Course Distance

563 Feet

 $S = Slope = (\Delta E/D) \times 100\%$

6.04 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

14.07 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

= 2.98 Inches/Hour

City of San Diego- Drainage Design Manual 2017

PEAK DISCHARGES (Q):

A = Area of the basin

0.51 Acres

Q = CIA

0.76 ft³/sec

BASIN B: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.62	0.84 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.62

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

28.4 Feet

D= Water Course Distance

383 Feet

 $S = Slope = (\Delta E/D) \times 100\%$

7.42 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

8.67 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

City of San Diego- Drainage Design Manual 2017

= 3.61 Inches/Hour

PEAK DISCHARGES (Q):

A = Area of the basin

0.84 Acres

Q = CIA

1.88 ft³/sec

BASIN C: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.93	1.02 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.93

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

13.6 Feet

D= Water Course Distance

280 Feet

 $S = Slope = (\Delta E/D) \times 100\%$

4.86 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

3.02 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

City of San Diego- Drainage Design Manual 2017

= 4.40 Inches/Hour

PEAK DISCHARGES (Q):

A = Area of the basin

1.02 Acres

Q = CIA

4.17 ft³/sec

BASIN D: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.80	0.76 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.80

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope

12.3 Feet

D= Water Course Distance

320 Feet

 $S = Slope = (\Delta E/D) \times 100\%$

= 3.84 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^(1/3)] =$

6.17 Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4

= 4.20 Inches/Hour

City of San Diego- Drainage Design Manual 2017

PEAK DISCHARGES (Q):

A = Area of the basin

0.76 Acres

Q = CIA

2.55 ft³/sec

BASIN E: Q 100

COMBINED RUNOFF COEFFICIENT (C):

Land Use	Coefficient (C)	Tributary Area
INDUSTRIAL	0.67	1.31 AC

City of San Diego Drainage Design Manual - January 2017 Edition Table A-1: Runoff Coefficient for Rational Method. (See Enclosed Calculations)

C= Runoff Factor

= 0.67

RAINFALL INTENSITY (I):

 ΔE = Change in elevation along the Effective Slope = 13 Feet

D= Water Course Distance = 256 Feet

 $S = Slope = (\Delta E/D) \times 100\%$ = 5.08 %

Tc = Time of Concentration

 $T_c = [1.8(1.1-C)(D^1/2)]/[S^1/3] = 7.20$ Minutes

Urban Areas Overland Time of Flow Curves Pg. A-8

City of San Diego- Drainage Design Manual 2017

Intensity = Intensity-Duration-Frequency Curves Pg. A-4 = 3.52 Inches/Hour

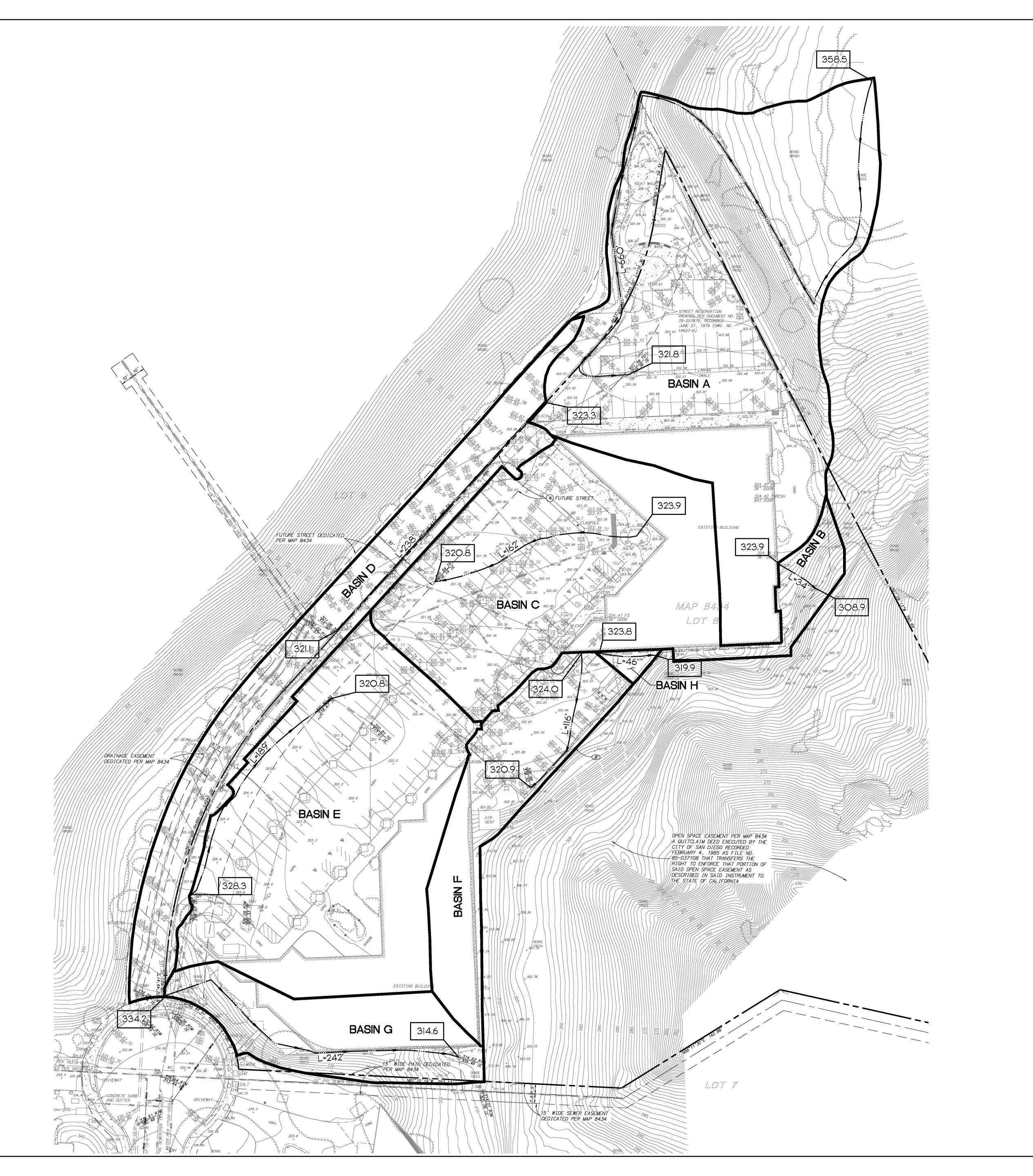
City of San Diego- Drainage Design Manual 2017

PEAK DISCHARGES (Q):

A = Area of the basin = 1.31 Acres

 $Q = CIA = 3.09 ft^3/sec$

DRAINAGE EXHIBITS



PRE	PRE & POST DEVELOPMENT 100-YR. PEAK DISCHARGES								
	PRE-DEVELOPMENT POST-DEVELOPMENT								
BASIN	AREA (ACRE)	TIME OF CONCENTRATION (MIN)	NCENTRATION DISCHARGE (CFS)		TIME OF CONCENTRATION (MIN)	DISCHARGE (CFS)			
Α	1.44	15.66	2.02	O.51	14.07	0.76			
В	0.04	1.78	0.09	0.84	8.67	1.88			
С	0.81	2.77	3.39	1.02	3.02	4.17			
D	0.51	6.56	1.79	0.76	6.17	2.55			
E	0.97	3.59	3.71	1.31	7.20	3.09			
F	O.35	2.65	1.40		-	Section 19			
G	0.30	7.67	0.63		1				
Н	0.02	3.59	0.04	ı					
TOTAL	4.44		13.07	4.44		12.45			

BASIN A - 63,000 SF IMPERVIOUS - 28,170 SF PERVIOUS - 34,830 SF SEMI-PERVIOUS - 0

<u>BASIN B -</u> 1,910 SF IMPERVIOUS - O PERVIOUS - 1,910 SF SEMI-PERVIOUS - O

BASIN C - 35,500 SF IMPERVIOUS - 34,920 SF PERVIOUS - 580 SF SEMI-PERVIOUS - 0

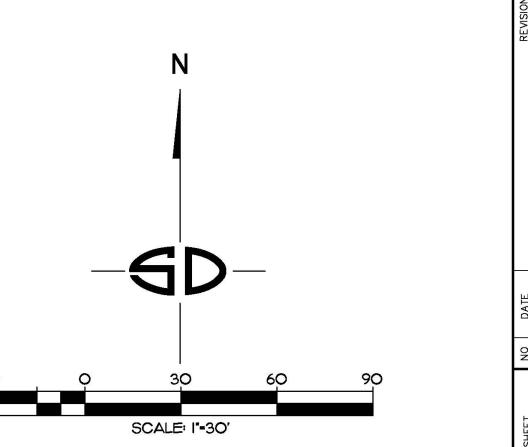
BASIN D - 22,085 SF IMPERVIOUS - 18,155 SF PERVIOUS - 3,930 SF SEMI-PERVIOUS - 0

BASIN E - 42,210 SF IMPERVIOUS - 39,800 SF PERVIOUS - 2,410 SF SEMI-PERVIOUS - 0

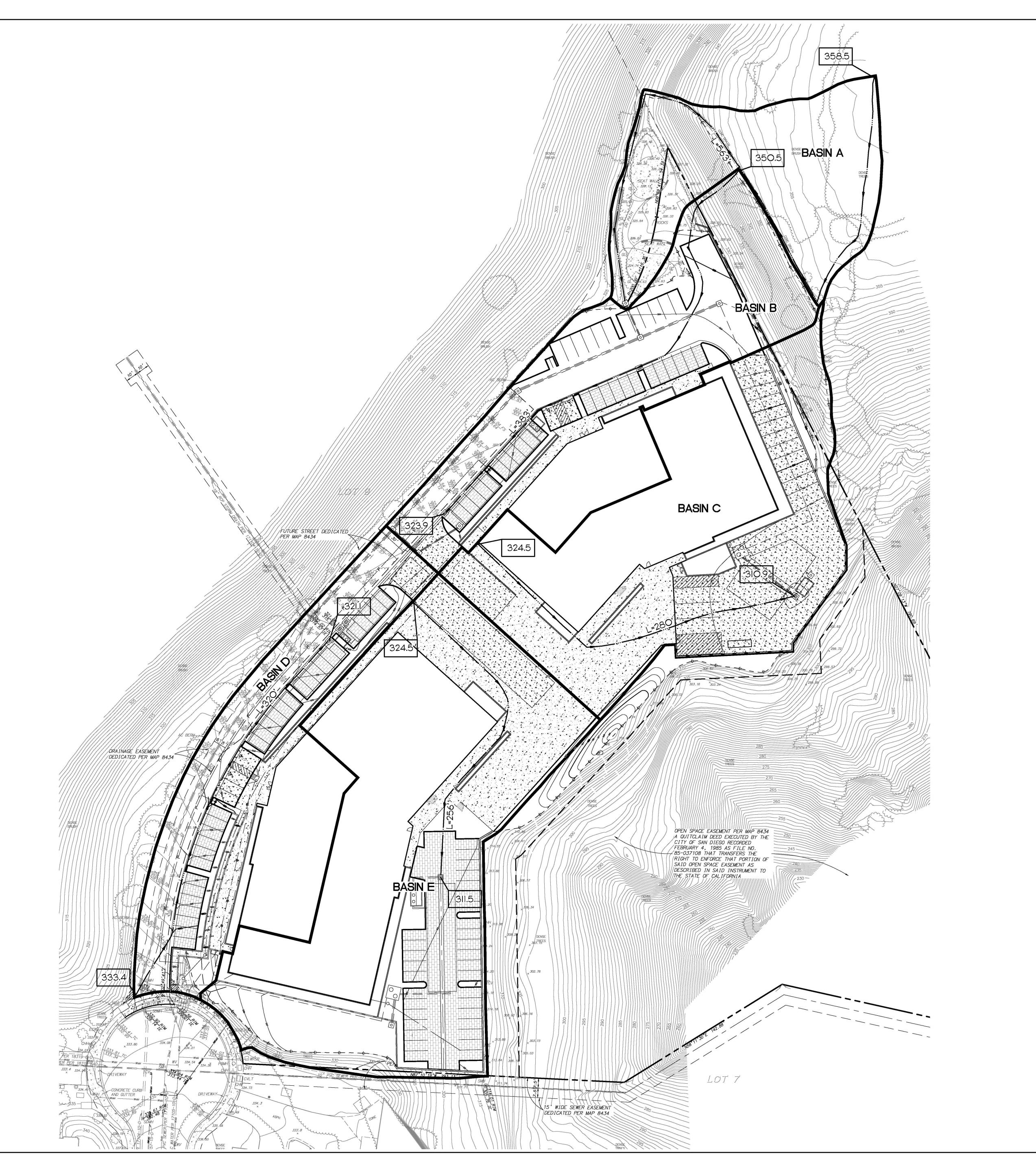
BASIN F - 15,040 SF IMPERVIOUS - 12,915 SF PERVIOUS - 2,125 SF SEMI-PERVIOUS - 0

BASIN G - 12,985 SF IMPERVIOUS - 6,825 SF PERVIOUS - 6,160 SF SEMI-PERVIOUS - 0

BASIN H - 710 SF IMPERVIOUS - 0 PERVIOUS - 710 SF SEMI-PERVIOUS - 0



MESA, CA



PRE	& POS	T DEVELOPA	MENT 100	-YR. P	EAK DISCHA	RGFS
	S	EVELOPMENT	POST-DEVELOPMENT			
BASIN	AREA (ACRE)	TIME OF CONCENTRATION (MIN)	DISCHARGE (CFS)	AREA (ACRE)	TIME OF CONCENTRATION (MIN)	DISCHARGE (CFS)
Α	1.44	15.66	2.02	O.5I	14.07	0.76
В	0.04	1.78	0.09	0.84	8.67	1.88
С	0.81	2.77	3.39	1.02	3.02	4.17
D	0.51	6.56	1.79	0.76	6.17	2.55
E	0.97	3.59	3.71	1.31	7.20	3.09
F	0.35	2.65	1.40			
G	0.30	7.67	O.63			
Н	0.02	3.59	0.04			
TOTAL	4.44		13.07	4.44	==	12.45

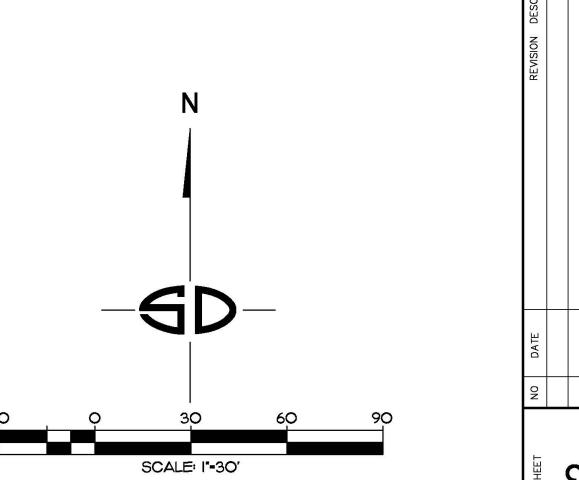
BASIN A - 22,084 SF IMPERVIOUS - 0 PERVIOUS - 22,084 SF SEMI-PERVIOUS - 0

BASIN B - 36,448 SF IMPERVIOUS - 21,364 SF PERVIOUS - 11,056 SF SEMI-PERVIOUS - 4,028 SF

BASIN C - 44,416 SF IMPERVIOUS - 39,016 SF PERVIOUS - 5,400 SF SEMI-PERVIOUS - 0

BASIN D - 33,276 SF IMPERVIOUS - 25,340 SF PERVIOUS - 3,752 SF SEMI-PERVIOUS - 4,184 SF

BASIN E - 57,216 SF IMPERVIOUS - 36,256 SF PERVIOUS - 11,724 SF SEMI-PERVIOUS - 9,236



2

MESA, CA 91942-2910 (619) 697-9234,

REFERENCES

APPENDIX A: RATIO NAL METHO D AND MODIFIED RATIO NAL METHOD

Table A-1. Runoff Coefficients for Rational Method

Land Use	Runoff Coefficient (C) Soil Type ⁽¹⁾			
Residential:				
Single Family	0.55			
Multi-Units	0.70			
Mobile Homes	0.65			
Rural (lots greater than ½ acre)	0.45			
Commercial (2)				
80% Impervious	0.85			
Industrial (2)				
90% Impervious	0.95			

Note:

Actual imperviousness = 35%Tabulated imperviousness (For = 80%Revised C = (35/80) x = 0.37

The values in Table A–1 are typical for urban areas. However, if the basin contains rural or agricultural land use, parks, golf courses, or other types of nonurban land use that are expected to be permanent, the appropriate value should be selected based upon the soil and cover and approved by the City.

Pre-Development

Basin	A	В	С	D	E	F	G	Н
Impervious Area (SF)	28,170	0	34,920	18,155	39,800	12,915	6,825	0
Total Basin Area (SF)	63,000	1,910	35,500	22,085	42,210	15,040	12,985	710
Actual imperviousness (AI) = Imp. Area/Total Area	45%	0%	98%	82%	94%	86%	53%	0%
Tabulated imperviousness =	90%	90%	90%	90%	90%	90%	90%	90%
For Commercial: Revised C = (AI/80) x 0.85, 0.50 Minimum 0.95 Maximum	0.47	0.00	1.04	0.87	1.00	0.91	0.55	0.00
Use	0.50	0.50	0.95	0.87	0.95	0.91	0.55	0.50

 $^{^{\}left(1\right) }$ Type D soil to be used for all areas.

⁽²⁾ Where actual conditions deviate significantly from the tabulated imperviousness values of 80% or 90%, the values given for coefficient C, may be revised by multiplying 80% or 90% by the ratio of actual imperviousness to the tabulated imperviousness. However, in case shall the final coefficient be less than 0.50. For example: Consider commercial property on D soil.

APPENDIX A: RATIO NAL METHO D AND MODIFIED RATIO NAL METHOD

Table A-1. Runoff Coefficients for Rational Method

Land Use	Runoff Coefficient (C) Soil Type ⁽¹⁾			
Residential:				
Single Family	0.55			
Multi-Units	0.70			
Mobile Homes	0.65			
Rural (lots greater than ½ acre)	0.45			
Commercial (2)				
80% Impervious	0.85			
Industrial (2)				
90% Impervious	0.95			

Note:

Actual imperviousness = 35%Tabulated imperviousness (For = 80%Commercial)
Revised C = $(35/80) \times 0.85$ = 0.37

The values in Table A–1 are typical for urban areas. However, if the basin contains rural or agricultural land use, parks, golf courses, or other types of nonurban land use that are expected to be permanent, the appropriate value should be selected based upon the soil and cover and approved by the City.

Post-Development

Basin	A	В	С	D	E
Impervious Area (SF)	0	21,364	39,016	25,340	36,256
Total Basin Area (SF)	22,084	36,448	44,416	33,276	57,216
Actual imperviousness (AI) = Imp. Area/Total Area	0%	59%	88%	76%	63%
Tabulated imperviousness =	90%	90%	90%	90%	90%
For Commercial: Revised C = (AI/80) x 0.85, 0.50 Minimum 0.95 Maximum	0.00	0.62	0.93	0.80	0.67
Use	0.50	0.62	0.93	0.80	0.67

⁽¹⁾ Type D soil to be used for all areas.

⁽²⁾ Where actual conditions deviate significantly from the tabulated imperviousness values of 80% or 90%, the values given for coefficient C, may be revised by multiplying 80% or 90% by the ratio of actual imperviousness to the tabulated imperviousness. However, in case shall the final coefficient be less than 0.50. For example: Consider commercial property on D soil.

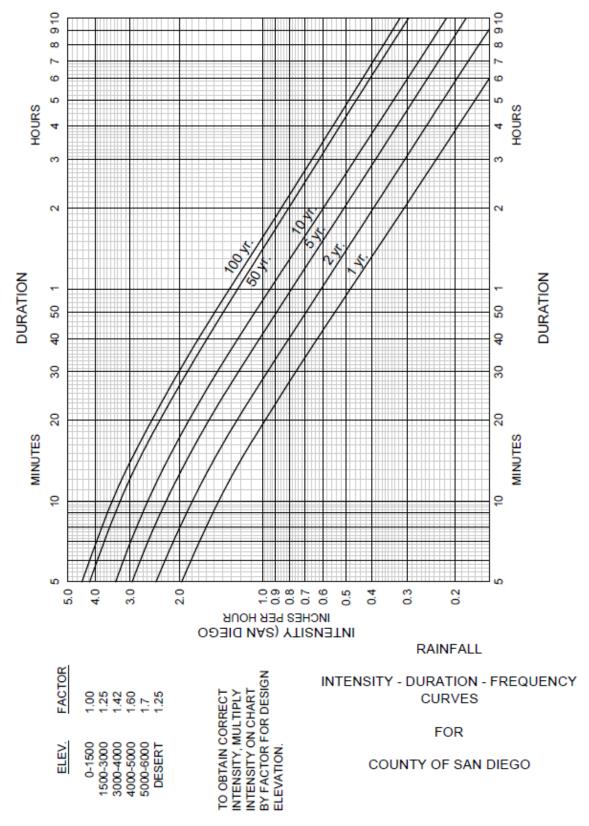


Figure A-1. Intensity-Duration-Frequency Design Chart



