



COFFEY ENGINEERING, INC.

# Preliminary Drainage Study

## Erb-Creamer

4285 1/3 Goldfinch Street  
San Diego, CA 92103

APN 444-272-09-00  
PTS 595127

Prepared for:

**Ty Creamer**



**June 14, 2019**

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## **Appendix A – Referenced Plans & Drainage Maps**

- Conceptual Site & Grading Plan
- Drainage Map ‘A’ – Existing Street Drainage Conditions
- Drainage Map ‘B’ – Existing Site Conditions
- Drainage Map ‘C’ – Proposed Site Conditions

## **Appendix B – Calculations/Evaluations**

- Existing and Proposed Flow Characteristics Table A
- Existing and Proposed drainage System Flows Evaluations

## **Appendix C – Reference Tables & Figures (City of San Diego Drainage Design Manual 2017)**

- Figure A-1– Intensity-Duration Design Chart
- Table A-1 Runoff Coefficients for Rational Method

## 1. Existing Conditions

In existing conditions, this site is a 5748 SF vacant lot located in a hillside area and is composed of various shrub, dirt, and other vegetation. In pre-construction conditions, the site contains one drainage basin (Basin 'B'). Basin 'B' generates approximately  $Q(100)=0.30$  cfs that sheet flows downhill in the Southeasterly direction towards an existing vegetated hill open space. Street preconstruction condition are in Drainage Map 'A'.

See Drainage Map 'A' and 'B'.

## 2. Proposed Project

In proposed conditions, this site will contain a single-family residence with new landscape and hardscape features. This project proposes 4,258 SF of impervious area, which includes the roof, driveway, stairs and other proposed on-site hardscape areas. The site is composed one drainage basin (Basins 'C').

This basin is expected to generate  $Q(100)=0.48$  cfs of runoff. We are going to collect in a sump-pump the water generated from Basin 'C' and send it to the street.

See Drainage Map 'C'.

## 3. Purpose and Scope of Report

This report will evaluate the existing street runoff and the proposed site runoff that we are sending to the street and we will verify that no adverse impacts will occur to the street existing storm drain system.

## 4. Method of Calculations

The Rational Method, as defined by *City of San Diego Drainage Design Manual 2017*, will be used to calculate storm water flow rates. Where noted, the following calculations were used to determine flow properties:

### Rainfall Characteristics

$Q = C * I * A$ , where

Q = Flow rate (ft<sup>3</sup>/sec)

C = Runoff coefficient

(Runoff coefficient per *City of San Diego Drainage Design Manual 2017* reproduced in Appendix C. Soil type D determined from the *Soil Hydrologic Groups* map from the County of San Diego Hydrology Manual reproduced in Appendix C also.)

I = Rainfall intensity (in/hr.)

A = Area (acres)

## 5. Results and Conclusions:

In existing conditions the street generates  $Q(100) = 2.29$  cfs. The street runoff is collected on grates 3'X5' inlet located at Barr Ave low point and discharge to the creek with a 15" CMP drain (See Drainage Map 'A'). Based on the calculated post- construction drainage conditions, we are expecting to add (pump) to the street  $Q(100) = 0.367$  cfs generated runoff from the site due to proposed new development.

The capacity of existing 15" CMP @ 32% 19.79 cfs. Therefore no adverse impacts will occur to the street existing storm drain system in post construction conditions.

## 6. Clean Water Act (CWA) Compliance

The proposed project is exempt from permitting under Federal Clean Water Act section 401 or 404 because it does not directly discharge into navigable waters of the United States.

**7. Declaration of Responsible Charge**

I hereby declare that I am the Civil Engineer of work for this project, that I have exercised responsible charge over the design of the project as defined in section 6703 of the business and professions code, and that the design is consistent with current design.

I understand that the check of project drawings and specifications by the City of San Diego is confined to a review only and does not relieve me, as Engineer of Work, of my responsibilities for project design.

PRELIMINARY  
Michael C. Kinnear  
RCE 76785  
Exp. 12-30-18

\_\_\_\_\_ Date



# **Appendix A –Reference Plans Drainage Maps**



**LEGEND**

DESCRIPTION	SYMBOL
PROPERTY LINE	— — — — —
EXISTING CONTOUR	-----
DIRECTION OF FLOW	→ →
SITE BASIN	— (Red)
WATER COURSE DISTANCE	— L-699' — (Green)

BASIN A.1 AREA= 41,607 SF  
 STREETS AREA =13,740 SF  
 SINGLE FAMILY RESIDENCE =27,867 SF  
 $C=(0.90 \times 13,740 + 0.55 \times 27,867) / 41,607 = 0.666$

BASIN 'A.1' -  
 BASIN AREA(A): 0.96 AC.  
 RUNOFF COEFFICIENT(C) (WEIGHTED): 0.666  
 INTENSITY(I)(FIGURE A-1): 3.6  
 100-YR. STORM FLOW: CIA=2.29 CFS.



**BASIN 'A.1'**  
**0.96 Ac.**  
**C=0.666**



SCALE: 1"=50'

ERB-CREAMER  
4285 1/3 Goldfinch Street, CA 92103  
DRAINAGE MAP 'A'  
BARR ST EXISTING CONDITIONS  
 SCALE: 1"=50'



**LEGEND**

DESCRIPTION	SYMBOL
PROPERTY LINE	— — — — —
EXISTING CONTOUR	- - - - -
DIRECTION OF FLOW	→ →
SITE BASIN	— — — — —
WATER COURSE DISTANCE	<b>L-699'</b>

**BASIN 'B.1' –**  
 BASIN AREA(A): 8493 AC.=0.195 Ac  
 RUNOFF COEFFICIENT (C): 0.35  
 INTENSITY(I)(FIGURE A-1): 4.4  
 100-YR. STORM FLOW: CIA=0.30 CFS.

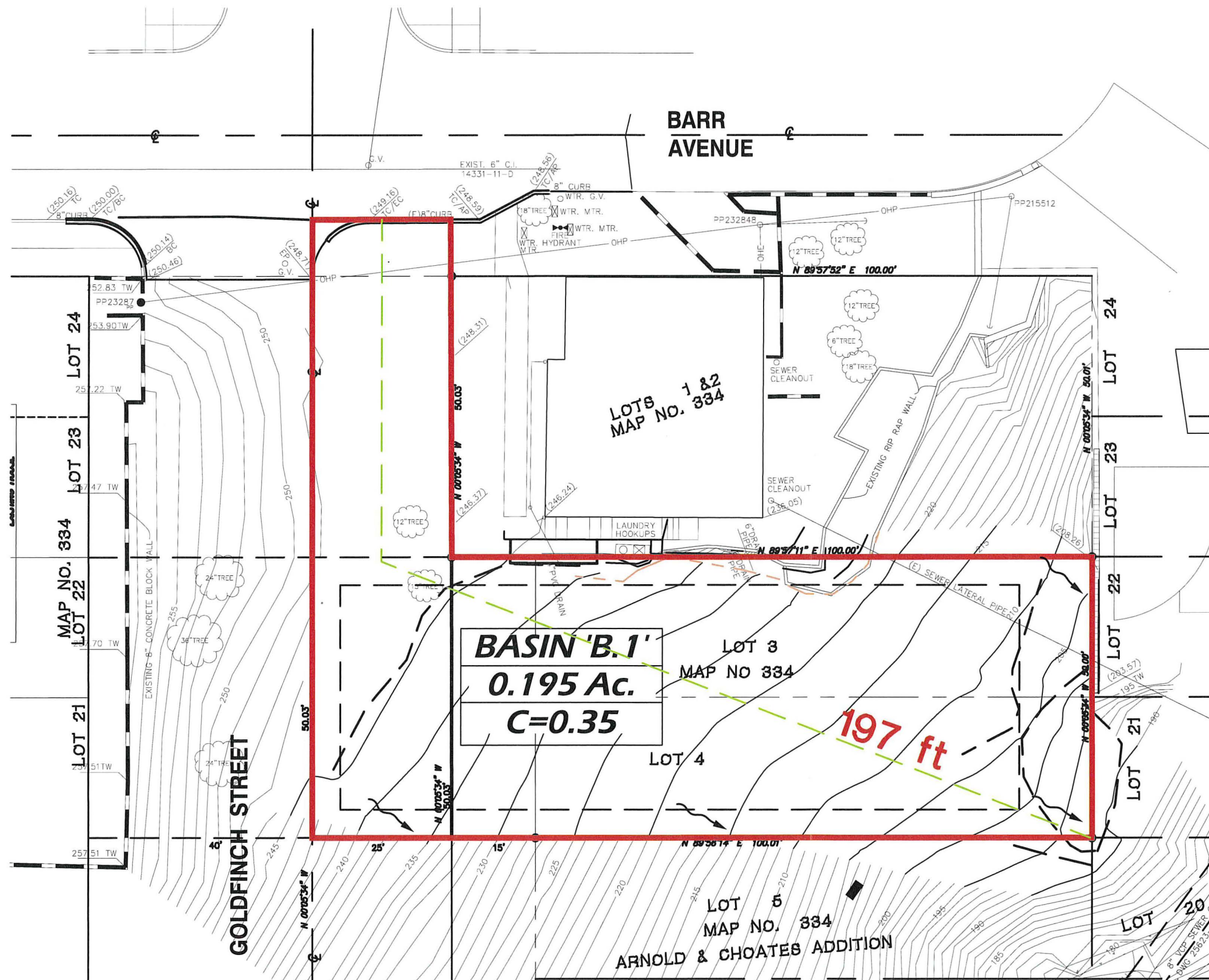


SCALE: 1"=20'

**ERB-CREAMER**

4285 1/3 Goldfinch Street, CA 92103

**DRAINAGE MAP 'B'**  
 SITE-EXISTING CONDITIONS  
 SCALE: 1"=20'

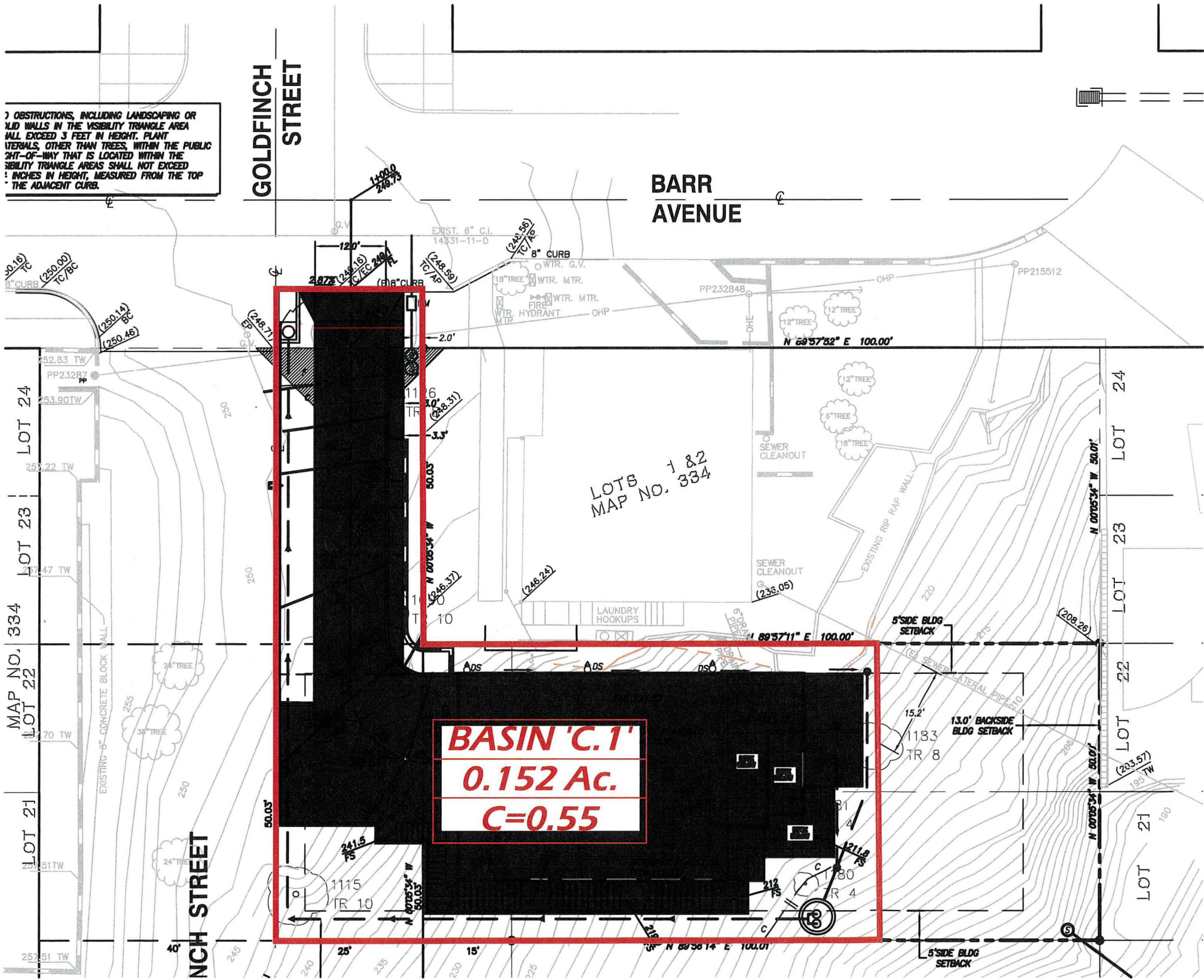




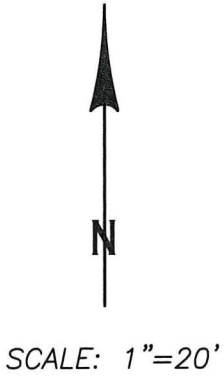
**LEGEND**

DESCRIPTION	SYMBOL
PROPERTY LINE	---
EXISTING CONTOUR	.....
DIRECTION OF FLOW	→ →
SITE BASIN	—
WATER COURSE DISTANCE	<b>L-699'</b>

OBSTRUCTIONS, INCLUDING LANDSCAPING OR OLD WALLS IN THE VISIBILITY TRIANGLE AREA SHALL EXCEED 3 FEET IN HEIGHT. PLANT VERTICALS, OTHER THAN TREES, WITHIN THE PUBLIC SHUT-OFF-WAY THAT IS LOCATED WITHIN THE VISIBILITY TRIANGLE AREAS SHALL NOT EXCEED 12 INCHES IN HEIGHT, MEASURED FROM THE TOP OF THE ADJACENT CURB.



**BASIN 'C.1' -**  
 BASIN AREA(A): 6603 AC.-0.152 Ac  
 RUNOFF COEFFICIENT (C): 0.55  
 INTENSITY(I)(FIGURE A-1): 4.4  
 100-YR. STORM FLOW: CIA=0.367 CFS.



## **Appendix B –Calculation/Evaluations**

**Table A - Time of Concentration Flow Characteristics**

Flow ID	Urban Overland Flow				Pipe Flow			Summary			
	Urban watercourse distance, D <sub>u</sub> (ft)	Watercourse slope, s (%)	Runoff Coefficient, C	Overland Flow Time, T (min)	Pipe Length, L <sub>p</sub> (ft)	Average velocity, V (fps)	Pipe travel time, D <sub>p</sub> (min)	(5 min minimum) Total time-of-concentration, T <sub>c</sub> (min)	Rainfall Intensity, I (in/hr)	Basin Area, A (acres)	Q (cfs)
								PRE-CONSTRUCTION- STREET			
A.1	163	3.00	0.67	6.92	0	6.0	1.50	8.42	3.60	0.955	2.290
								PRE CONSTRUCTION-SITE			
B.1	197	30.00	0.35	0.73	0	0.0	0.00	5.00	4.40	0.195	0.300
								POST-CONSTRUCTION- ON-SITE			
C.1	197	30.00	0.55	4.47	0	0.0	0.00	5.00	4.40	0.152	0.367



ArcGIS Explorer - Coffey Default Map

Feature Layer

Tools Appearance

Information

Coordinates: Degrees-Minutes-Seconds

Distance: Meters, Kilometers

Effects

Receive Signal

Position Information

Target Indicator

Grid

Scalebar

3D Environment

GPS Receiver

Stars Atmospheric Halo

Fog Sun Lighting

Center location

Track location

Go To

Capture Waypoint



EXISTING 15" CMP SD CAPACITY

DATE: 12-04-2018

TIME: 08:27:36

(1) Diameter (inches) ... 15.      (2) Mannings n ..... .024

(3) slope (ft/ft) ..... .3200      (4) Q (cfs) ..... 19.79

(5) depth (ft) ..... 1.25      (6) depth/Diameter ... 1.00

Velocity (fps) ..... 16.12      Velocity Head .... 4.04

Area (Sq. Ft.) ..... 1.23

Critical Depth ..... 1.25      Critical Slope ... 0.3038

Critical Velocity ... 16.13      Froude Number .... N/A

# **Appendix C –Reference Tables & Figures (City of San Diego Drainage Manual 2017)**



APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

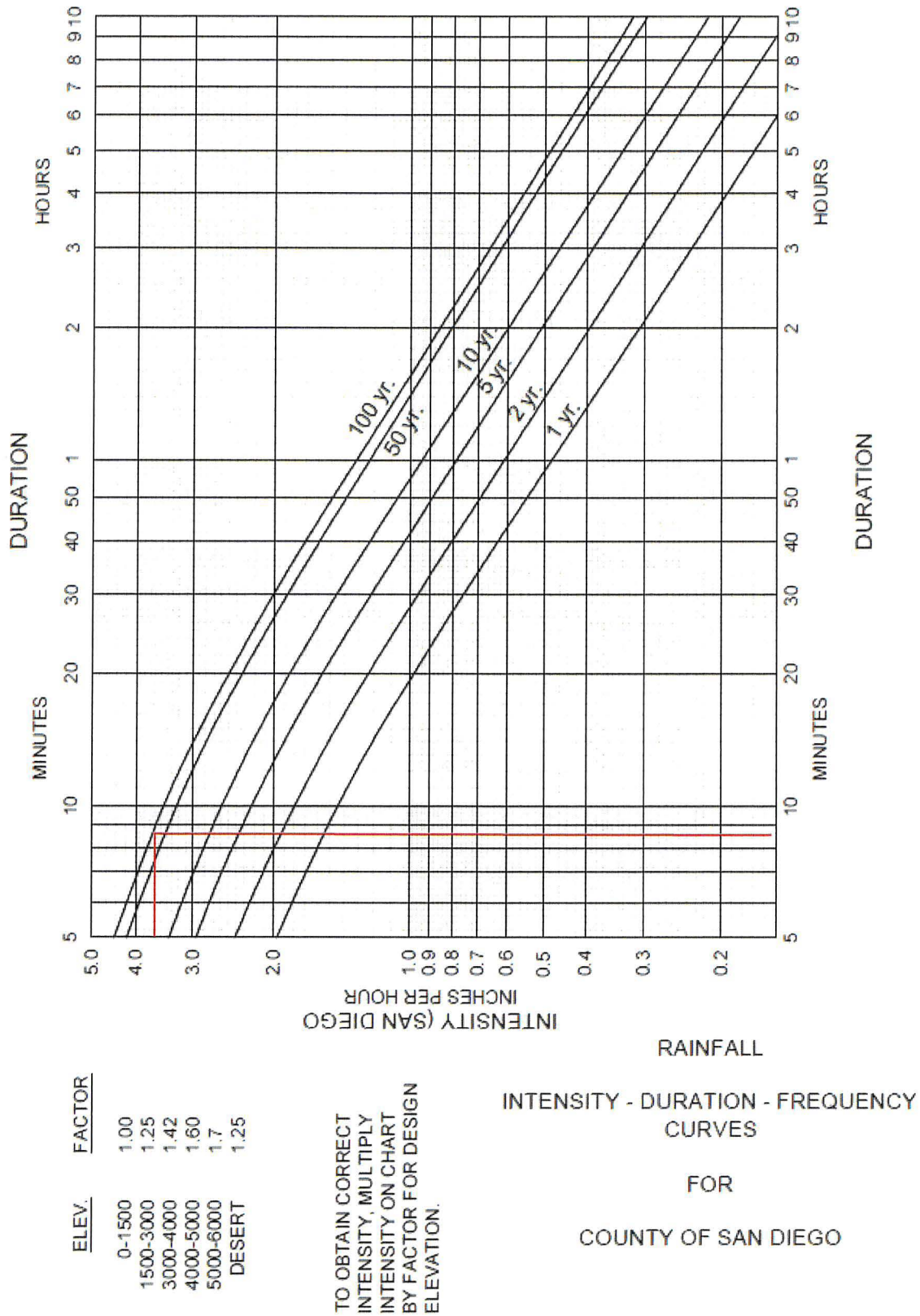


Figure A-1. Intensity-Duration-Frequency Design Chart



Table A-1. Runoff Coefficients for Rational Method

Land Use	Runoff Coefficient (C)
	Soil Type <sup>(1)</sup>
<b>Residential:</b>	
Single Family	0.55
Multi-Units	0.70
Mobile Homes	0.65
Rural (lots greater than 1/2 acre)	0.45
<b>Commercial <sup>(2)</sup></b>	
80% Impervious	0.85
<b>Industrial <sup>(2)</sup></b>	
90% Impervious	0.95

**Note:**

<sup>(1)</sup> Type D soil to be used for all areas.

<sup>(2)</sup> Where actual conditions deviate significantly from the tabulated imperviousness values of 80% or 90%, the values given for coefficient C, may be revised by multiplying 80% or 90% by the ratio of actual imperviousness to the tabulated imperviousness. However, in case shall the final coefficient be less than 0.50. For example: Consider commercial property on D soil.

$$\begin{aligned}
 \text{Actual imperviousness} &= 50\% \\
 \text{Tabulated imperviousness} &= 80\% \\
 \text{Revised C} &= (50/80) \times 0.85 = 0.53
 \end{aligned}$$

The values in Table A-1 are typical for urban areas. However, if the basin contains rural or agricultural land use, parks, golf courses, or other types of nonurban land use that are expected to be permanent, the appropriate value should be selected based upon the soil and cover and approved by the City.

### A.1.3. Rainfall Intensity

The rainfall intensity (I) is the rainfall in inches per hour (in/hr.) for a duration equal to the T<sub>c</sub> for a selected storm frequency. Once a particular storm frequency has been selected for design and a T<sub>c</sub> calculated for the drainage area, the rainfall intensity can be determined from the Intensity-Duration-Frequency Design Chart (Figure A-1).

