SD CLIMATE ACTION PLAN CONSISTENCY CHECKLIST INTRODUCTION

In December 2015, the City adopted a Climate Action Plan (CAP) that outlines the actions that City will undertake to achieve its proportional share of State greenhouse gas (GHG) emission reductions. The purpose of the Climate Action Plan Consistency Checklist (Checklist) is to, in conjunction with the CAP, provide a streamlined review process for proposed new development projects that are subject to discretionary review and trigger environmental review pursuant to the California Environmental Quality Act (CEQA).¹

Analysis of GHG emissions and potential climate change impacts from new development is required under CEQA. The CAP is a plan for the reduction of GHG emissions in accordance with CEQA Guidelines Section 15183.5. Pursuant to CEQA Guidelines Sections 15064(h)(3), 15130(d), and 15183(b), a project's incremental contribution to a cumulative GHG emissions effect may be determined not to be cumulatively considerable if it complies with the requirements of the CAP.

This Checklist is part of the CAP and contains measures that are required to be implemented on a project-by-project basis to ensure that the specified emissions targets identified in the CAP are achieved. Implementation of these measures would ensure that new development is consistent with the CAP's assumptions for relevant CAP strategies toward achieving the identified GHG reduction targets. Projects that are consistent with the CAP as determined through the use of this Checklist may rely on the CAP for the cumulative impacts analysis of GHG emissions. Projects that are not consistent with the CAP must prepare a comprehensive project-specific analysis of GHG emissions, including quantification of existing and projected GHG emissions and incorporation of the measures in this Checklist to the extent feasible. Cumulative GHG impacts would be significant for any project that is not consistent with the CAP.

The Checklist may be updated to incorporate new GHG reduction techniques or to comply with later amendments to the CAP or local, State, or federal law.

¹ Certain projects seeking ministerial approval may be required to complete the Checklist. For example, projects in a Community Plan Implementation Overlay Zone may be required to use the Checklist to qualify for ministerial level review. See Supplemental Development Regulations in the project's community plan to determine applicability.

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SUBMITTAL APPLICATION

- The Checklist is required only for projects subject to CEQA review.²
- If required, the Checklist must be included in the project submittal package. Application submittal procedures can be found in <u>Chapter 11: Land Development Procedures</u> of the City's Municipal Code.
- The requirements in the Checklist will be included in the project's conditions of approval.
- The applicant must provide an explanation of how the proposed project will implement the requirements described herein to the satisfaction of the Planning Department.

Application Information

Contact Information						
Project No./Name:	Project No. 503848 /UNIVERSITY AVENUE -MIXED USE					
Property Address:	5556-5592,UNIVERSITY AVENUE,SAN DIEGO CA					
Applicant Name/Co.:	MARK GOTTSCHILICH / UNIVERSITY MANOR LLC					
Contact Phone:	858.382.6906 Contact Email: mgreal7@gmail.com					
Was a consultant reta Consultant Name:	ined to complete this checklist? MANJU PAI	● Yes □ No Contact Phone:	If Yes, complete the following 949-608-0245			
Company Name:	STUDIOPI2 INC.	Contact Email:	manju@studiopi2.com			
Project Information	Project Information					
1. What is the size of	the project (acres)?	1.47				
 Identify all applicable proposed land uses: □ Residential (indicate # of single-family units): 						
Residential (indicate # of multi-family units):		63				
🕼 Commercia	l (total square footage):	5,047.70 SF				
🗆 Industrial (t	otal square footage):					
☐ Other (deso 3. Is the project or a p Transit Priority Are	portion of the project located in a	● Yes □ No				
4. Provide a brief des	cription of the project proposed:					

THE PROPOSED PROJECT IS A MIXED USE PROJECT INCLUSIVE OF 63 RESIDENTIAL UNITS, WITH A PROPOSED AREA 55,951.42 SF AND 5,047.70 SF OF COMMERCIAL STREET FRONT, ACCESSIBLE FROM PUBLIC R.O.W.

² Certain projects seeking ministerial approval may be required to complete the Checklist. For example, projects in a Community Plan Implementation Overlay Zone may be required to use the Checklist to qualify for ministerial level review. See Supplemental Development Regulations in the project's community plan to determine applicability.



Step 1: Land Use Consistency

The first step in determining CAP consistency for discretionary development projects is to assess the project's consistency with the growth projections used in the development of the CAP. This section allows the City to determine a project's consistency with the land use assumptions used in the CAP.

Step 1: Land Use Consistency				
Checklist Item (Check the appropriate box and provide explanation and supporting documentation for your answer)	Yes	No		
 A. Is the proposed project consistent with the existing General Plan and Community Plan land use and zoning designations?³, <u>OR</u> B. If the proposed project is not consistent with the existing land use plan and zoning designations, and includes a land use plan and/or zoning designation amendment, would the proposed amendment result in an increased density within a Transit Priority Area (TPA)⁴ and implement CAP Strategy 3 actions, as determined in Step 3 to the satisfaction of the Development Services Department?; <u>OR</u> C. If the proposed project is not consistent with the existing land use plan and zoning designations, does the project include a land use plan and/or zoning designation amendment that would result in an equivalent or less GHG-intensive project when compared to the existing designations? 				

If "**Yes**," proceed to Step 2 of the Checklist. For question B above, complete Step 3. For question C above, provide estimated project emissions under both existing and proposed designation(s) for comparison. Compare the maximum buildout of the existing designation and the maximum buildout of the proposed designation.

If "**No**," in accordance with the City's Significance Determination Thresholds, the project's GHG impact is significant. The project must nonetheless incorporate each of the measures identified in Step 2 to mitigate cumulative GHG emissions impacts unless the decision maker finds that a measure is infeasible in accordance with CEQA Guidelines Section 15091. Proceed and complete Step 2 of the Checklist.

THE PROJECT PROPOSED IS CONSISTENT WITH THE GENERAL PLAN WHICH IDENTIFIES THE SITE AS MULTIPLE USE; ADDITIONALLY THE PROJECT IS CONSISTENT WITH THE MID-CITY COMMUNITIES COMMUNITY PLAN WHICH DESIGNATES THE SITE AS COMMERCIAL MIXED-USE. LASTLY, THE PROJECT IS CONSISTENT WITH THE REQUIREMENTS OF THE CC-5-3 ZONE OF THE CENTRAL URBANIZED PLAN DISTRICT.

³ This question may also be answered in the affirmative if the project is consistent with SANDAG Series 12 growth projections, which were used to determine the CAP projections, as determined by the Planning Department.

⁴ This category applies to all projects that answered in the affirmative to question 3 on the previous page: Is the project or a portion of the project located in a transit priority area.

Step 2: CAP Strategies Consistency

The second step of the CAP consistency review is to review and evaluate a project's consistency with the applicable strategies and actions of the CAP. Step 2 only applies to development projects that involve permits that would require a certificate of occupancy from the Building Official or projects comprised of one and two family dwellings or townhouses as defined in the California Residential Code and their accessory structures.⁵ All other development projects that would not require a certificate of occupancy from the Building Official shall implement Best Management Practices for construction activities as set forth in the <u>Greenbook</u> (for public projects).

Step 2: CAP Strategies Consistency	ý		
Checklist Item (Check the appropriate box and provide explanation for your answer)	Yes	No	N/A
Strategy 1: Energy & Water Efficient Buildings			
1. Cool/Green Roofs.			
• Would the project include roofing materials with a minimum 3-year aged solar reflection and thermal emittance or solar reflection index equal to or greater than the values specified in the voluntary measures under <u>California Green Building</u> <u>Standards Code</u> (Attachment A)?; <u>OR</u>			
 Would the project roof construction have a thermal mass over the roof membrane, including areas of vegetated (green) roofs, weighing at least 25 pounds per square foot as specified in the voluntary measures under <u>California</u> <u>Green Building Standards Code</u>?; <u>OR</u> 			
 Would the project include a combination of the above two options? 			
Check "N/A" only if the project does not include a roof component.			
RESIDENTIAL: THE PROJECT WOULD INCLUDE ROOFING MATERIALS WITH A MINIMUM 3-YEAR AGED SOLAR REFLECTION INDEX EQUAL TO OR GREATER THAN THE VALUES SPECIFIED IN THE VOLUNTRY MEASURES UNDER CALIFORNIA GREEN BUILDING STANDARD CODES. NON-RESIDENTIAL: THE PROJECT WOULD INCLUDE ROOFING			
MATERIAL WITH MINIMUM 3 YEAR AGED SOLAR REFLECTION AND THERMITTANCE OR SOLAR REFLECTION INDEX QUAL TO OR GREATER THAN THE VALUES SPECIFIED IN CALIFORNIA GREEN BUILDING STANDARD CODE.			

⁵ Actions that are not subject to Step 2 would include, for example: 1) discretionary map actions that do not propose specific development, 2) permits allowing wireless communication facilities, 3) special events permits, 4) use permits or other permits that do not result in the expansion or enlargement of a building (e.g., decks, garages, etc.), and 5) non-building infrastructure projects such as roads and pipelines. Because such actions would not result in new occupancy buildings from which GHG emissions reductions could be achieved, the items contained in Step 2 would not be applicable.

2. Plumbing fixtures and fittings		
With respect to plumbing fixtures or fittings provided as part of the project, would those low-flow fixtures/appliances be consistent with each of the following:		
Residential buildings:		
 Kitchen faucets: maximum flow rate not to exceed 1.5 gallons per minute at 60 psi; 		
 Standard dishwashers: 4.25 gallons per cycle; 		
Compact dishwashers: 3.5 gallons per cycle; and		
Clothes washers: water factor of 6 gallons per cubic feet of drum capacity?		
Nonresidential buildings:		
 Plumbing fixtures and fittings that do not exceed the maximum flow rate specified in <u>Table A5.303.2.3.1 (voluntary measures) of the California Green</u> <u>Building Standards Code</u> (See Attachment A); and 		
 Appliances and fixtures for commercial applications that meet the provisions of Section A5.303.3 (voluntary measures) of the California Green Building Standards Code (See Attachment A)? 		
Check "N/A" only if the project does not include any plumbing fixtures or fittings.		
RESIDENTIAL DEVELOPMENT : THE PROJECT WOULD USE LOW -FLOW FIXTURES /APPLIANCES CONSISTENT WITH EACH OF FOLLOWING: KITCHEN FAUCETS: MAX. FLOW RATE NOT TO EXCEED 1.5 GALLONS PER MINUTE AT 60PSI STANDARD DISHWASHER:4.25 GALLONS PER CYCLE COMPACT DISHWASHERS:3.5GALLONS PER CYCLE CLOTHES WASHERS:WATER FACTOR OF 6 GALLONS PER CUBIC FEET OF DRUM CAPACITY		
NON RESIDENTIAL DEVELOPMENT - PLUMBING FIXTURES AND FITTINGS THAT TO DO NOT EXCEED MAXIMUM FLOW RATE SPECIFIED IN TABLE A5.303.2.3.1(VOLUNTARY MEASURES)OF THE CALIFORNIA GREEN BUILDING STANDARD CODE. - APPLIANCES AND FIXTURES FOR COMMERCIAL APPLICATIONS THAT MEET THE PROVISIONS OF SECTION A5.303.3(VOLUNTARY MEASURES)OF THE CALIFORNIA GREEN BUILDING STANDARDS CODE.		

Strategy 3: Bicycling, Walking, Transit & Land Use		
3. Electric Vehicle Charging		
 <u>Multiple-family projects of 17 dwelling units or less</u>: Would 3% of the total parking spaces required, or a minimum of one space, whichever is greater, be provided with a listed cabinet, box or enclosure connected to a conduit linking the parking spaces with the electrical service, in a manner approved by the building and safety official, to allow for the future installation of electric vehicle supply equipment to provide electric vehicle charging stations at such time as it is needed for use by residents? 		
 <u>Multiple-family projects of more than 17 dwelling units</u>: Of the total required listed cabinets, boxes or enclosures, would 50% have the necessary electric vehicle supply equipment installed to provide active electric vehicle charging stations ready for use by residents? 		
 <u>Non-residential projects</u>: Of the total required listed cabinets, boxes or enclosures, would 50% have the necessary electric vehicle supply equipment installed to provide active electric vehicle charging stations ready for use? 		
Check "N/A" only if the project is a single-family project or would not require the provision of listed cabinets, boxes, or enclosures connected to a conduit linking the parking spaces with electrical service, e.g., projects requiring fewer than 10 parking spaces.		
RESIDENTIAL :		
MULTIFAMILY OF MORE THAN 17 DWELLING UNITS - 3% OF PARKING SPACES TOWARD EV SPACES REQUIRED = 3 EV CHARGING SPACES, PROVIDED = 3 EV CHARGING SPACES + ONE ACTIVE ELECTRIC VEHICLE CHARGING STATION		
NON - RESIDENTIAL /COMMERCIAL: REQUIRED = 1EV SPACE PROVIDED = 1 VAN ACCESSIBLE EV SPACE.		
Strategy 3: Bicycling, Walking, Transit & Land Use (Complete this section if project includes non-residential or mixed uses)		
4. Bicycle Parking Spaces		
Would the project provide more short- and long-term bicycle parking spaces than required in the City's Municipal Code (<u>Chapter 14, Article 2, Division 5</u>)? ⁶		
Check "N/A" only if the project is a residential project.		
THE PROJECT PROVIDES TOTAL OF 13 NUMBER OF MORE SHORT-TERM AND LONG-TERM BYCYCLE SPACES THAN REQUIRED PER CODE.		
RESIDENTIAL BICYCLE SPCES REQUIRED = 28 SPACES, PROVIDED = 30 SPACES		
COMMERCIAL - SHORT TERM BICYCLE PARKING SPACES REQUIRED - 0.1/ 1000 SF OR MINIMUM TWO. REQUIRED =2 SPACES , PROVIDED = 8 SPACES.		
LONG TERM BICYCLE PARKING SPACES REQUIRED - 5 % OF TOTAL REQUIRED AUTOMOBILE PARKING OR MINIMUM 1. REQUIRED = 1SPACE, PROVIDED = 6 SPACES.		

⁶ Non-portable bicycle corrals within 600 feet of project frontage can be counted towards the project's bicycle parking requirements.

Number of Tenant Occupants (Employees)	Shower/Changing Facilities Required	Two-Tier (12" X 15" X 72") Personal Effects Lockers Required		
0-10	0	0		
11-50	1 shower stall	2		
51-100	1 shower stall	3		
101-200	1 shower stall	4		
Over 200	1 shower stall plus 1 additional shower stall for each 200 additional tenant-occupants	1 two-tier locker plus 1 two-tier locker for each 50 additional tenant- occupants		
if the project evelopment		occupants or if it does not includ te over 10 tenant occu OPMENT THAT WOULD	e pants	

designated		ise in a TPA, would the project p flow-emitting, fuel-efficient, and with the following table?			
	Number of Required Parking Spaces	Number of Designated Parking Spaces			
	0-9	0			
	10-25	2			
	26-50	4			
	51-75	6			
	76-100	9			
	101-150	11			
	151-200	18			
	201 and over	At least 10% of total			
	luirements.	nicles. See Question 4 for electr		\boxtimes	
Note: Vehic be consider spaces are addition to Check "N/A"	les bearing Clean Air Vehicle red eligible for designated pa to be provided within the ove it.	stickers from expired HOV lane rking spaces. The required desi erall minimum parking requirer ential project, or if it does not inc	gnated parking nent, not in		

Transportation Demand Management Program		
If the project would accommodate over 50 tenant-occupants (employees), would it include a transportation demand management program that would be applicable to existing tenants and future tenants that includes:		
At least one of the following components:		
Parking cash out program		
 Parking management plan that includes charging employees market-rate for single-occupancy vehicle parking and providing reserved, discounted, or free spaces for registered carpools or vanpools 		
 Unbundled parking whereby parking spaces would be leased or sold separately from the rental or purchase fees for the development for the life of the development 		
And at least three of the following components:		
 Commitment to maintaining an employer network in the SANDAG iCommute program and promoting its RideMatcher service to tenants/employees 		
On-site carsharing vehicle(s) or bikesharing		
Flexible or alternative work hours		
Telework program		
Transit, carpool, and vanpool subsidies		
Pre-tax deduction for transit or vanpool fares and bicycle commute costs		
 Access to services that reduce the need to drive, such as cafes, commercial stores, banks, post offices, restaurants, gyms, or childcare, either onsite or within 1,320 feet (1/4 mile) of the structure/use? 		Ø
Check "N/A" only if the project is a residential project or if it would not accommodate over 50 tenant-occupants (employees).		
THE PROJECT WOULD NOT ACCOMMODATE OVER 50 TENANT-OCCUPANTS (EMPLOYEES).		

Step 3: Project CAP Conformance Evaluation (if applicable)

The third step of the CAP consistency review only applies if Step 1 is answered in the affirmative under option B. The purpose of this step is to determine whether a project that is located in a TPA but that includes a land use plan and/or zoning designation amendment is nevertheless consistent with the assumptions in the CAP because it would implement CAP Strategy 3 actions. In general, a project that would result in a reduction in density inside a TPA would not be consistent with Strategy 3.The following questions must each be answered in the affirmative and fully explained.

1. Would the proposed project implement the General Plan's City of Villages strategy in an identified Transit Priority Area (TPA) that will result in an increase in the capacity for transit-supportive residential and/or employment densities?

Considerations for this question:

- Does the proposed land use and zoning designation associated with the project provide capacity for transit-supportive residential densities within the TPA?
- Is the project site suitable to accommodate mixed-use village development, as defined in the General Plan, within the TPA?
- Does the land use and zoning associated with the project increase the capacity for transit-supportive employment intensities within the TPA?
- 2. Would the proposed project implement the General Plan's Mobility Element in Transit Priority Areas to increase the use of transit? Considerations for this question:
 - Does the proposed project support/incorporate identified transit routes and stops/stations?
 - Does the project include transit priority measures?
- 3. Would the proposed project implement pedestrian improvements in Transit Priority Areas to increase walking opportunities? Considerations for this question:
 - Does the proposed project circulation system provide multiple and direct pedestrian connections and accessibility to local activity centers (such as transit stations, schools, shopping centers, and libraries)?
 - Does the proposed project urban design include features for walkability to promote a transit supportive environment?

4. Would the proposed project implement the City of San Diego's Bicycle Master Plan to increase bicycling opportunities? Considerations for this question:

- Does the proposed project circulation system include bicycle improvements consistent with the Bicycle Master Plan?
- Does the overall project circulation system provide a balanced, multimodal, "complete streets" approach to accommodate mobility needs of all users?

5. Would the proposed project incorporate implementation mechanisms that support Transit Oriented Development? <u>Considerations for this question:</u>

- Does the proposed project include new or expanded urban public spaces such as plazas, pocket parks, or urban greens in the TPA?
- Does the land use and zoning associated with the proposed project increase the potential for jobs within the TPA?
- Do the zoning/implementing regulations associated with the proposed project support the efficient use of parking through mechanisms such as: shared parking, parking districts, unbundled parking, reduced parking, paid or time-limited parking, etc.?

6. Would the proposed project implement the Urban Forest Management Plan to increase urban tree canopy coverage?

Considerations for this question:

- Does the proposed project provide at least three different species for the primary, secondary and accent trees in order to accommodate varying parkway widths?
- Does the proposed project include policies or strategies for preserving existing trees?
- Does the proposed project incorporate tree planting that will contribute to the City's 20% urban canopy tree coverage goal?

SD CLIMATE ACTION PLAN CONSISTENCY CHECKLIST ATTACHMENT A

This attachment provides performance standards for applicable Climate Action Pan (CAP) Consistency Checklist measures.

Land Use Type	Roof Slope	Minimum 3-Year Aged Solar Reflectance	Thermal Emittance	Solar Reflective Index
Law Diag Desidential	≤2:12	0.55	0.75	64
Low-Rise Residential	> 2:12	0.20	0.75	16
High-Rise Residential Buildings,	≤2:12	0.55	0.75	64
Hotels and Motels	> 2:12	0.20	0.75	16
Nex Desidential	≤2:12	0.55	0.75	64
Non-Residential	> 2:12	0.20	0.75	16

CALGreen does not include recommended values for low-rise residential buildings with roof slopes of \leq 2:12 for San Diego's climate zones (7 and 10). Therefore, the values for climate zone 15 that covers Imperial County are adapted here.

Solar Reflectance Index (SRI) equal to or greater than the values specified in this table may be used as an alternative to compliance with the aged solar reflectance values and thermal emittance.

able 2 Fixture Flow Rates for Non-Residential Buildings related to Question 2: Plumbing Fixtures and Fittings supporting Strategy 1: Energy & Water Efficient Buildings of the Climate Action Plan				
	Fixture Type	Maximum Flow Rate		
	Showerheads	1.8 gpm @ 80 psi		
	Lavatory Faucets	0.35 gpm @60 psi		
	Kitchen Faucets	1.6 gpm @ 60 psi		
	Wash Fountains	1.6 [rim space(in.)/20 gpm @ 60 psi]		
	Metering Faucets	0.18 gallons/cycle		
Metering	Faucets for Wash Fountains	0.18 [rim space(in.)/20 gpm @ 60 psi]		
Gravit	y Tank-type Water Closets	1.12 gallons/flush		
Flusho	meter Tank Water Closets	1.12 gallons/flush		
Flusho	meter Valve Water Closets	1.12 gallons/flush		
Electromec	nanical Hydraulic Water Closets	1.12 gallons/flush		
	Urinals	0.5 gallons/flush		
Electromec	nanical Hydraulic Water Closets Urinals	1.12 gallons/flush		

Source: Adapted from the <u>California Green Building Standards Code</u> (CALGreen) Tier 1 non-residential voluntary measures shown in Tables A5.303.2.3.1 and A5.106.11.2.2, respectively. See the <u>California Plumbing Code</u> for definitions of each fixture type.

Where complying faucets are unavailable, aerators rated at 0.35 gpm or other means may be used to achieve reduction.

Acronyms:

gpm = gallons per minute psi = pounds per square inch (unit of pressure)

in. = inch

	es and Fixtures for Commercial Applications and Fixtures for Commercial Applications ittings supporting Strategy 1: Energy & V	-			
Appliance/Fixture Type	Standard				
Clothes Washers	Maximum Water Factor (WF) that will reduce the use of water by 10 percent below the California Energy Commissions' WF standards for commercial clothes washers located in Title 20 of the California Code of Regulations.				
Conveyor-type Dishwashers	0.70 maximum gallons per rack (2.6 L) (High-Temperature)	0.62 maximum gallons per rack (4.4 L) (Chemical)			
Door-type Dishwashers	0.95 maximum gallons per rack (3.6 L) (High-Temperature)	1.16 maximum gallons per rack (2.6 L) (Chemical)			
Undercounter-type Dishwashers	0.90 maximum gallons per rack (3.4 L) (High-Temperature)	0.98 maximum gallons per rack (3.7 L) (Chemical)			
Combination Ovens	Consume no more than 10 gallons per hour (3	8 L/h) in the full operational mode.			
Commercial Pre-rinse Spray Valves (manufactured on or after January 1, 2006) Function at equal to or less than 1.6 gallons per minute (0.10 L/s) at 60 psi (414 kPa) and Be capable of cleaning 60 plates in an average time of not more than 30 seconds per plate. Be equipped with an integral automatic shutoff. Operate at static pressure of at least 30 psi (207 kPa) when designed for a flow rate of 1.3 gallons per minute (0.08 L/s) or less.					
Source: Adapted from the <u>California Green Building Standa</u> the <u>California Plumbing Code</u> for definitions of each applia		asures shown in Section A5.303.3. See			
Acronyms: L = liter L/h = liters per hour L/s = liters per second psi = pounds per square inch (unit of pressure) kPa = kilopascal (unit of pressure)					



ELISE ROTHSCHILD DIRECTOR DEPARTMENT OF ENVIRONMENTAL HEALTH LAND AND WATER QUALITY DIVISION P.O. BOX 129261, SAN DIEGO, CA 92112-9261 Phone: (858) 505-6700 or (800) 253-9933 Fax: (858) 514-6583 www.sdcdeh.org

September 8, 2017

Mr. Lutfi Bustami 8051 Main Street Stanton, CA 90680

Dear Mr. Bustami:

VOLUNTARY ASSISTANCE PROGRAM CASE #DEH2017-LSAM-000450 FORMER 2-B RENTALS 5586 UNIVERSITY AVENUE, SAN DIEGO, CA

Staff of the Department of Environmental Health (DEH) reviewed the August 18, 2017 *Removal Action Workplan* (RAW), prepared by Murex Environmental, Inc. (Murex).

According to the RAW, the Site is part of a larger redevelopment project that encompasses a total of three parcels that comprise the range of even-numbered addresses from 5556 to 5592 University Avenue.

Five underground storage tanks (USTs) were present beneath the Site. The tanks were originally used to store Stoddard solvent, a dry cleaning solution. Later, at least one tank was used to store gasoline. The USTs were removed on June 9, 1992.

Based on the results of the soil sampling, Unauthorized Release Case H32242-001 was opened. Liquid free-phase petroleum hydrocarbons were observed in one monitoring well at the Site. Murex oversaw the installation of six soil-vapor probes (SG-1 through SG-6) and two rounds of on-Site analysis of soil-vapor samples in August 2011. An additional vapor sampling event was completed in October 2011.

Following the submittal of the Corrective Action Plan (CAP) in 2013, which recommended remediation by natural attenuation, the case received regulatory closure from DEH on February 24, 2014. Based on the delineated TPH and Stoddard solvent impact in the unsaturated zone, the estimated volume of impacted soil is on the order of 1,500 to 2,000 cubic yards. The case closure document stated that "Property redevelopment for commercial use with demolition of all existing buildings on site is planned. Structure identified as 5586 University Avenue fails for health risk and will remain vacant until it is demolished. Reoccupancy of this structure requires mitigation and/or engineering controls with oversight by the DEH Voluntary Assistance Program". Voluntary Assistance Program Case DEH2017-LSAM-000450 was opened on August 25, 2017.

The current (vacant) buildings at the Site are proposed to be demolished and replaced with a mixed use development, including both residential and commercial buildings. Once the current buildings have been demolished, Murex proposes to remove the residual soil contamination via excavation. It is estimated that an area of approximately 40 by 56 feet will be excavated to a depth of 12 feet below ground surface (bgs). This entails a total of approximately 1,000 cubic yards of soil.

AMY HARBERT ASSISTANT DIRECTOR -2-

Following excavation activities, Murex will perform verification sampling in order to confirm the removal of petroleum-impacted soil from the Site. Soil samples will be collected every 20 feet along the sidewalls at depths of 6 and 12 feet in addition to within the middle of the base of the excavation.

After at least a week, six soil borings will be advanced using direct-push technology within both the areas within and surrounding the excavation of petroleum-impacted soil. Soil vapor probes will be installed within each of these borings at depths of 5 and 15 feet bgs. Following a two-hour equilibration period, each of the soil vapor probes will be purged and then sampled for VOCs by EPA Method TO-15 using an on-Site mobile laboratory.

DEH approves the RAW as proposed with the following modifications:

- 1. Regarding the proposed vapor sampling, please wait a month following probe installation to sample. Conduct three sampling events. The first sampling event should be conducted one month following installation, the second event should be conducted six weeks after installation. and the third sampling event should be conducted eight weeks after installation.
- 2. Regarding public notification, please post the Site with the notice and provide the notice at least a week prior to excavation activities to property owners and occupants of properties adjacent to the Site as well as the City planning group for that area. Please keep a record of public comments and complaints as well as any mitigation actions taken and include the record in the report.

If you have any questions, please call me at (858) 505-6969.

Sincerely,

James Clay Division (July on Department of Environmental Health, ou=County of San Diego, email=james.clay@sdcounty.ca.gov, c=US

Digitally signed by James Clay Date: 2017.09.08 13:09:48 -07'00'

JAMES CLAY, Environmental Health Specialist III Land and Water Quality Division

cc: Mr. Kent Huth, Murex (by email)



DRAINAGE STUDY REPORT

FOR

University Manor, LLC

5556 University Avenue San Diego, CA 92105

April 27, 2018



Prepared For: Mr. Lutfi Bustami

University Manor, LLC 8051 Main Street Stanton, CA 90680

Project No 20150047.01Prepared by:Cindy LimChecked by:John CruikshankP.E. No.C50792

John M. Cruikshank Consultants, Inc. Tel: 310-241-6550 Fax: 310-833-6555 411 N. Harbor Boulevard, Suite 201, San Pedro, CA 90731 www.jmc-2.com

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1. **PROJECT DESCRIPTION**

The purpose of this hydrology and Low Impact Development (LID) report is to establish post construction run-off conditions sizing of the storm water conveyance system and its incorporation into the post construction mitigation measures. The proposed new multi-unit housing and commercial development at 5556-5592 East University Avenue, in the City of San Diego, California.

The proposed new development includes the addition/replacement of approximately 53,143 sf of impervious surfaces consisting of commercial and residential spaces with parking and ADA access along with 12,632 sf of landscaping.

2. PROJECT BACKGROUND

The project site is located at 32°44'54.4"N and 117°04'32.9"W just west of the intersection of University Avenue and Chollas Parkway in the City of San Diego, CA and is located within the Eastern (Mid-City) community plan area. The 1.51-acre project site is currently comprised of commercial and residential structures which will be demolished to build a new multi-unit housing and commercial development. The exiting stormwater runoff is directed towards street.

3. HYDROLOGY STUDY

The approximate area of the project site 1.51 acres. To analyze the runoff response the drainage area will divide into subareas by locating significant points of interest.

- Working from upstream to downstream, assign a number representing each subarea in the drainage system to each point of interest.
- Measure each subarea in the drainage area to determine its size in acres (A).
- Determine the length and effective slope of the flow path in each subarea.
- Identify the soil type for each subarea.
- Determine the runoff coefficient (C) for each subarea base on Table 3-1 of the Hydrology Manual.
- Determine the Intensity for 100-year design storm frequency.

The hydrology map will show the flow path of travel, discharge points, Q and V at all discharge points, area of each basin and drainage nodes.

Comparing the existing flow $Q_{100} = 6.42$ cfs and proposed $Q_{100} = 6.99$ cfs for the impervious area. The BMP infiltration basin of the post development will detain stormwater to 0.75 cfs and release 6.24 cfs into public storm drain which is less

than the existing condition. The BMP infiltration system helps detained stormwater and reduce the runoff to public storm drain. See table 2.

Based on the hydrology and LID study, the site drainage has adequate capacity handle the new flows generated due to the small net increase in the impervious area. The stormwater runoff will be filtered and discharged into an Ecorain Tank system for infiltration; the overflow will discharge into existing public catch basin. The stormwater leaving the project site will be free from sediment and debris before entering the public storm drain.

The new development is not located close to any river or lake environment which involves dredging, filling or other impacts either temporarily or permanently to waters of the US. Therefore, it is not necessary to obtain approval from Regional Water Quality Control Board Under Federal Clean Water Act (CWA) section 401 or 404.

4. DRAINAGE CHARACTERISTICS

The proposed new development will create 53,143 sf of impervious surfaces and is calculated to be 81% of the total construction area (65,775 sf) in the postdevelopment condition therefore requiring the entire site to be treated with structural BMPs. The remaining 19% is allocated as landscape areas. When compared to the pre-development, which has 74% impervious and 26% pervious (undeveloped) area. The post-construction has increased the impervious portion of the site.

In the post development condition, the project will be divided into multiple subareas where stormwater will sheet flow to near-by catch basins then gravity flow through the underground pipe to the infiltration basin (BMP-1) at southeast parking lot of level 1.

The drainage pattern of the existing site drain from North to South toward street then into the public catch basin. The on-site runoff of the new development is capture by roof drain and catch basin via underground pipe to infiltration BMP (Ecorain tank). The overflow from Ecorain tank will then discharge to public catch basin at street. The infiltration BMPs reduce the amount of water leaving the site through existing city storm drains. The only storm water that could potentially leave the site will do so through overflow lines at the infiltration tank which have been sized for the 100-year storm event.

TOTAL AREA	AREA (ACRES)	EXISTING FLOW RATE (Q _E) CFS	PROPOSED FLOW RATE (Q _P) CFS	RUNOFF FACTOR (C)
EXISTING IMPERVIOUS	1.12	6.42	-	0.87
PROPOSED IMPERVIOUS	1.22	-	6.99	0.87
EXISTING PERVIOUS	0.39			
PROPOSED PERVIOUS	0.29			
TOTAL OF PROJECT	1.51			

Table 1. Pre and Post Development Discharge Calculation

RUNOFF AREA	RUNOFF COEFFICIENT	INTENSITY (IN/HR)	AREA (ACRE)	FREQUENCY STORM	RUNOFF (CFS)
EXISTING	0.87	6.59	1.12	100-YR STORM	6.42
PROPOSED	0.87	6.59	1.22	100-YR STORM	6.99
DETAINED					0.57
RELEASED					6.42

5. INFILTRATION BASIN

The methods utilized to determine the potential run-off for a study area is based upon record information that has been collected by both the City of San Diego and the San Diego County Department of Public Works Flood Control Section.

Information for this study has been obtained from The City of San Diego Storm Water Standards Manual (January 2016) for LID analysis and from The San Diego County Hydrology Manual (June 2003).

A review of the soils map shows Hydrologic Soil Group D to be prevalent in the project area based on the San Diego County Hydrology Manual (Appendix A, June 2003).

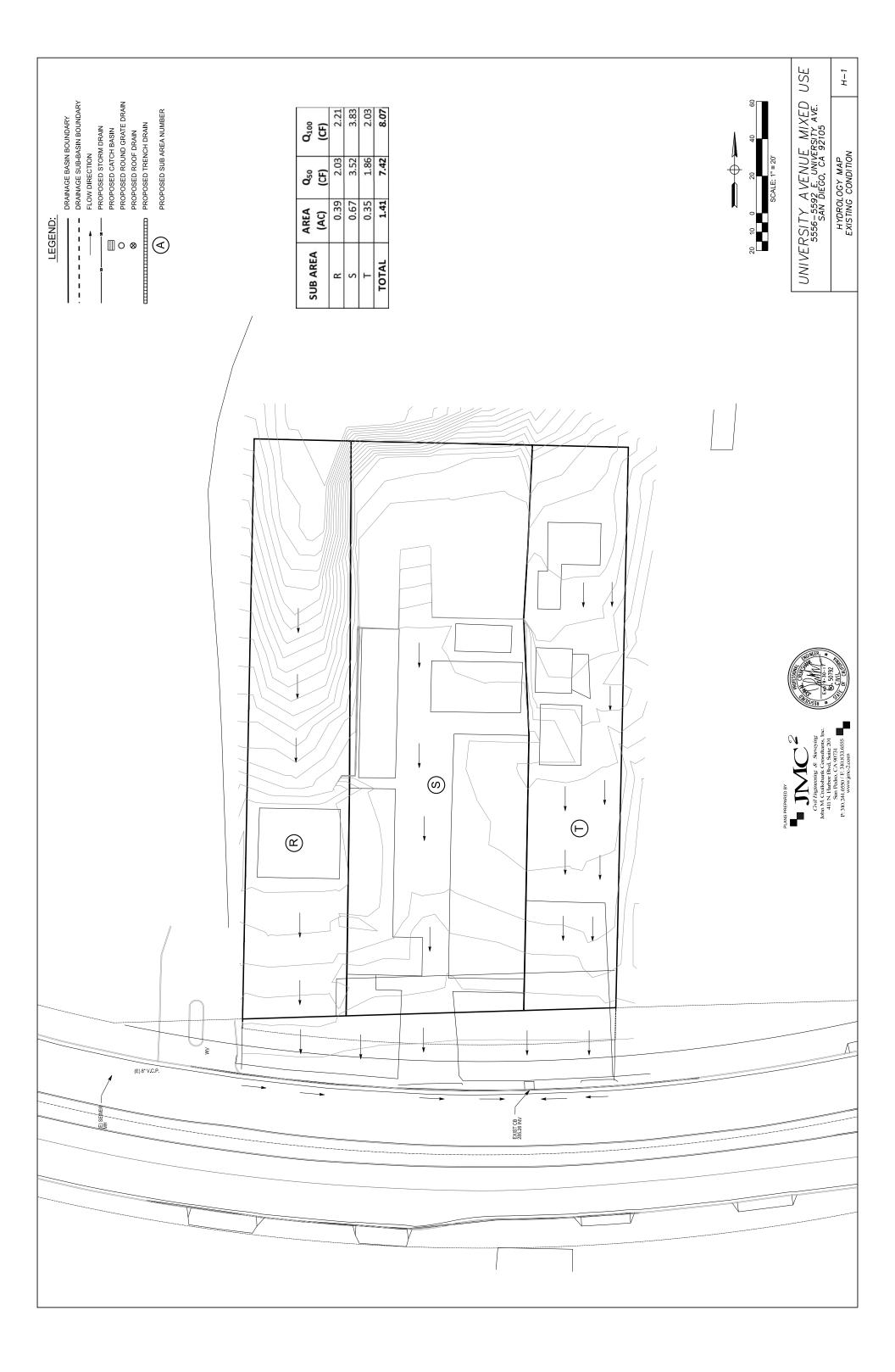
The Stormwater infiltration basin will store runoff into the underground Ecorain Tank until it gradually infiltrates into the soil. The Ecorain tanks are underground modular structures designed to accept and hold rain and runoff water for infiltration into the groundwater table, detention for controlled release. Both the 100-year design storm frequency, and 85th percentile rainfall event were adopted to calculate run-off flow rates for this project.

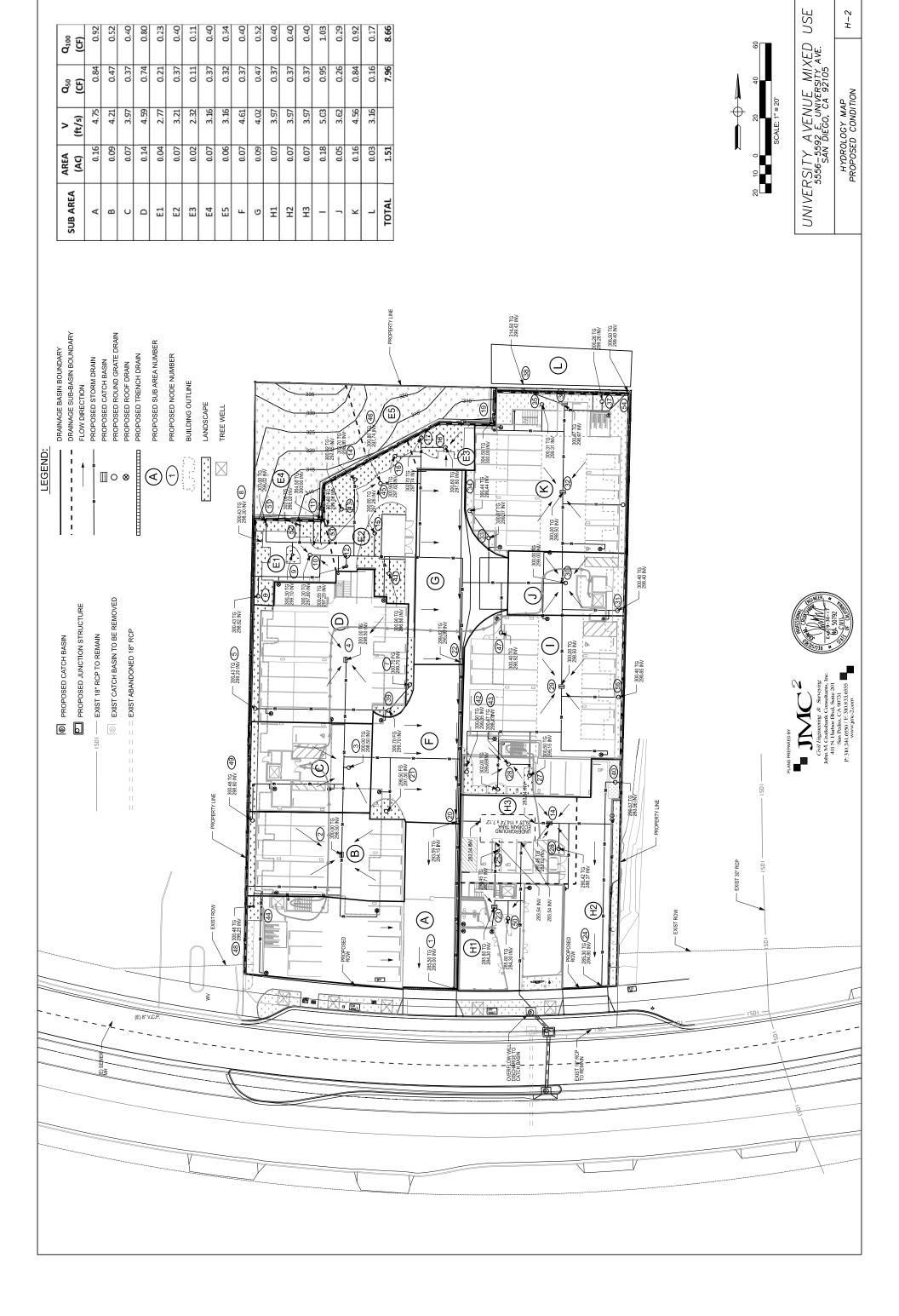
6. APPENDIX

- Vicinity Maps
- San Diego County 100-year 6-hour Isopluvial Map
- Table 3-1: Runoff Coefficients for Urban Areas
- Table 3-2: Max. Overland Flow Length and Initial Time of Concentration
- Figure 3-1: Intensity-Duration Design Chart
- Figure 3-3: Rational Formula Overland Time of Flow Nomograph
- Figure B.1-1: 85th Percentile 24-hour Isopluvial Map
- County of San Diego Hydrology Manual Soil Hydrologic Groups

La	Table 3-1 RUNOFF COEFFICIENTS FOR URBAN AREAS Land Use	Table 3-1 IENTS FOR URBA		Runoff Coefficient "C"	,Ç,	
				Soil	Soil Type	
NRCS Elements	County Elements	% IMPER.	A	в	C	D
Undisturbed Natural Terrain (Natural)	Permanent Open Space	0*	0.20	0.25	0.30	0.35
Low Density Residential (LDR)	Residential, 1.0 DU/A or less	10	0.27	0.32	0.36	0.41
Low Density Residential (LDR)	Residential, 2.0 DU/A or less	20	0.34	0.38	0.42	0.46
Low Density Residential (LDR)	Residential, 2.9 DU/A or less	25	0.38	0.41	0.45	0.49
Medium Density Residential (MDR)	Residential, 4.3 DU/A or less	30	0.41	0.45	0.48	0.52
Medium Density Residential (MDR)	Residential, 7.3 DU/A or less	40	0.48	0.51	0.54	0.57
Medium Density Residential (MDR)	Residential, 10.9 DU/A or less	45	0.52	0.54	0.57	0.60
Medium Density Residential (MDR)	Residential, 14.5 DU/A or less	50	0.55	0.58	0.60	0.63
High Density Residential (HDR)	Residential, 24.0 DU/A or less	65	0.66	0.67	0.69	0.71
High Density Residential (HDR)	Residential, 43.0 DU/A or less	80	0.76	0.77	0.78	0.79
Commercial/Industrial (N. Com)	Neighborhood Commercial	08	0.76	0.77	0.78	0.79
Commercial/Industrial (G. Com)	General Commercial	85	0.80	0.80	0.81	0.82
Commercial/Industrial (O.P. Com)	Office Professional/Commercial	06	0.83	0.84	0.84	0.85
Commercial/Industrial (Limited I.)	Limited Industrial	06	0.83	0.84	0.84	0.85
Commercial/Industrial (General I.)	General Industrial	95	0.87	0.87	0.87	0.87

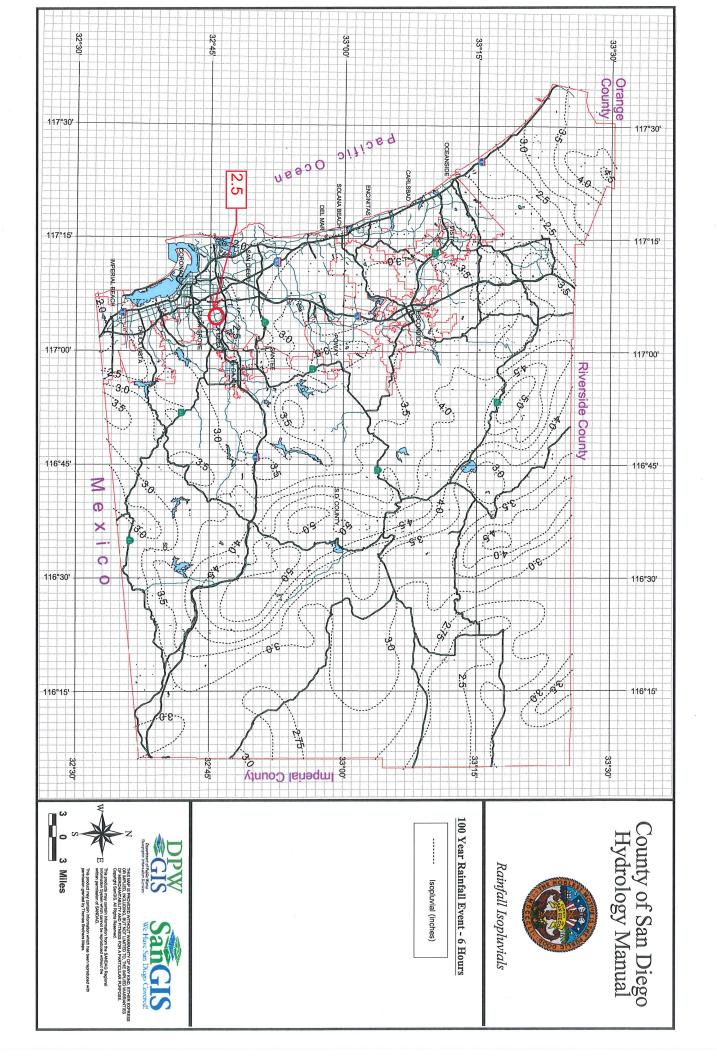
DU/A = dwelling units per acre NRCS = National Resources Conservation Service

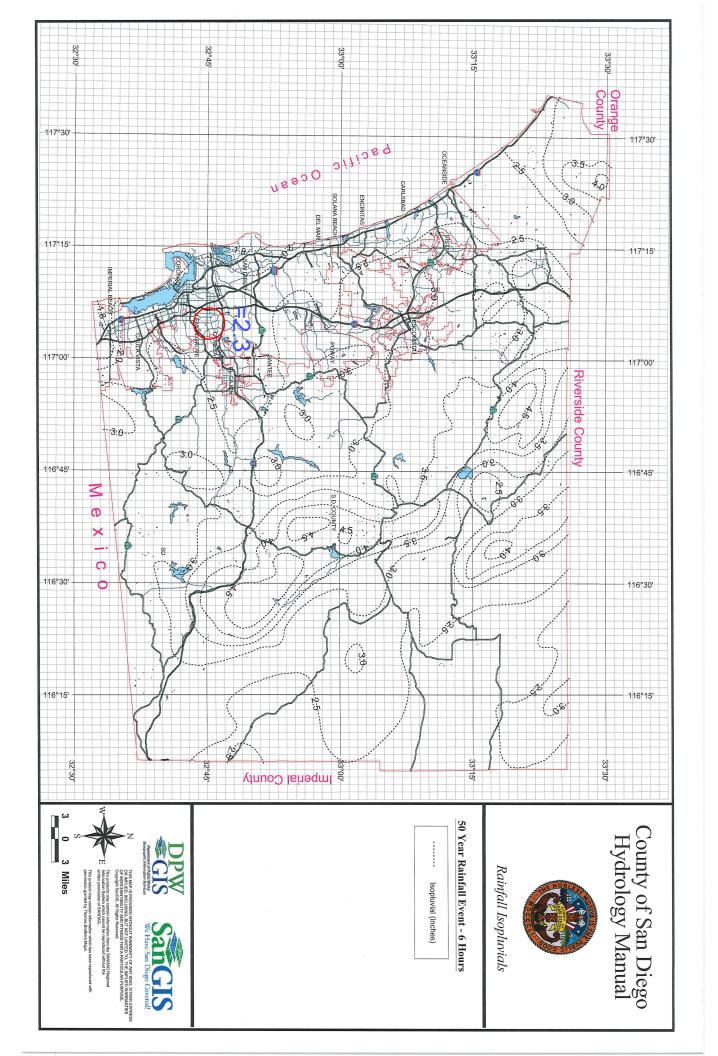


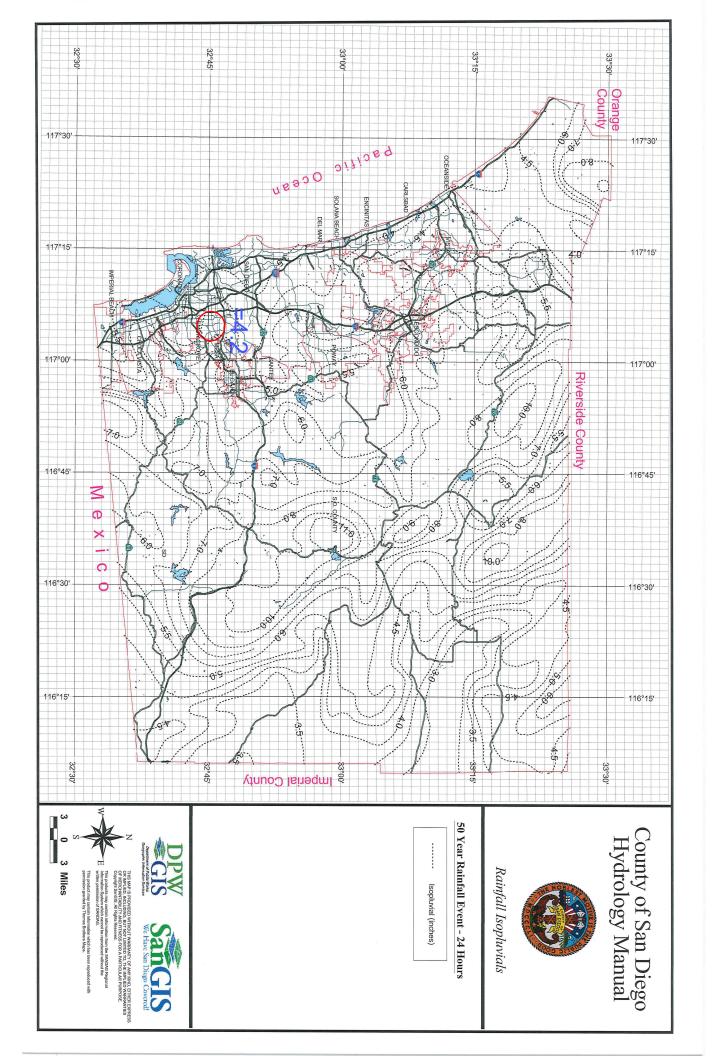


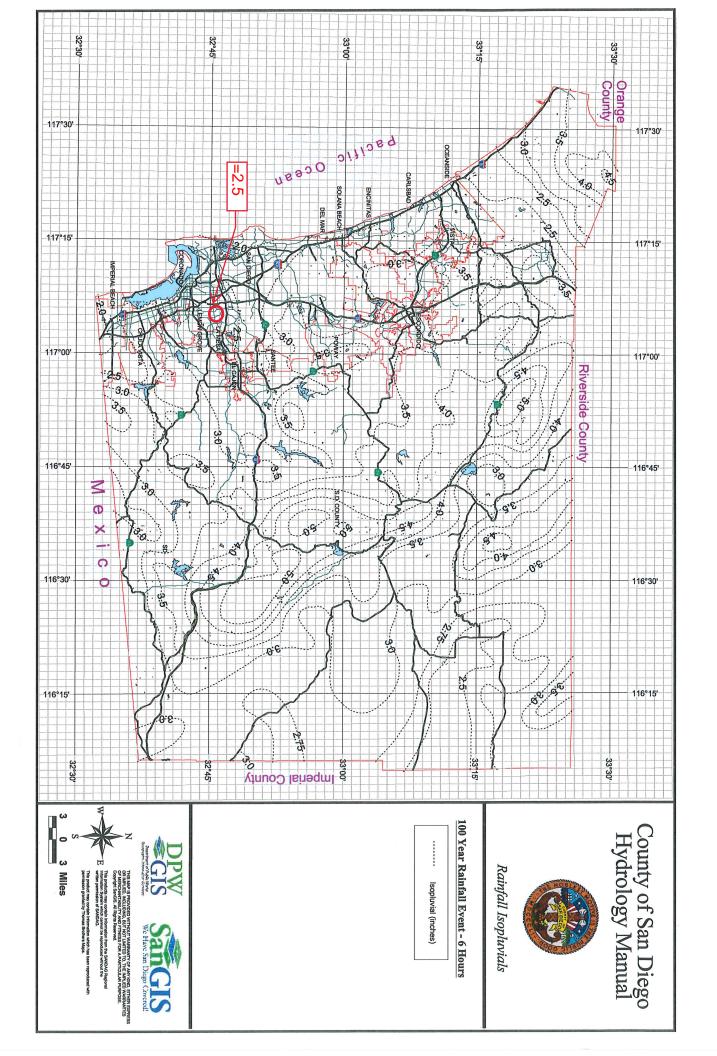
SITE VICINITY MAP

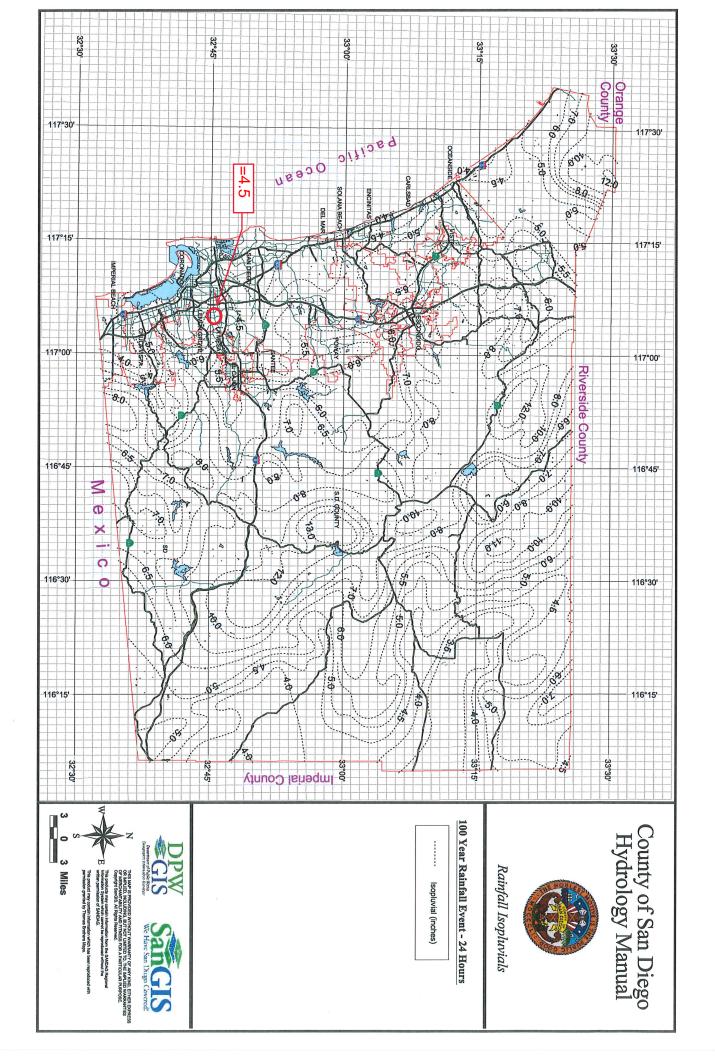












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Note that the Initial Time of Concentration should be reflective of the general land-use at the upstream end of a drainage basin. A single lot with an area of two or less acres does not have a significant effect where the drainage basin area is 20 to 600 acres.

Table 3-2 provides limits of the length (Maximum Length (L_M)) of sheet flow to be used in hydrology studies. Initial T_i values based on average C values for the Land Use Element are also included. These values can be used in planning and design applications as described below. Exceptions may be approved by the Regulating Agency when submitted with a detailed study.

Table 3-2

& INITIAL TIME OF CONCENTRATION (T _i)													
Element*	DU/	.5	5%	1	%	2	%	3	%	59	%	10	%
	Acre	L _M	T _i										
Natural		50	13.2	70	12.5	85	10.9	100	10.3	100	8.7	100	6.9
LDR	1	50	12.2	70	11.5	85	10.0	100	9.5	100	8.0	100	6.4
LDR	2	50	11.3	70	10.5	85	9.2	100	8.8	100	7.4	100	5.8
LDR	2.9	50	10.7	70	10.0	85	8.8	95	8.1	100	7.0	100	5.6
MDR	4.3	50	10.2	70	9.6	80	8.1	95	7.8	100	6.7	100	5.3
MDR	7.3	50	9.2	65	8.4	80	7.4	95	7.0	100	6.0	100	4.8
MDR	10.9	50	8.7	65	7.9	80	6.9	90	6.4	100	5.7	100	4.5
MDR	14.5	50	8.2	65	7.4	80	6.5	90	6.0	100	5.4	100	4.3
HDR	24	50	6.7	65	6.1	75	5.1	90	4.9	95	4.3	100	3.5
HDR	43	50	5.3	65	4.7	75	4.0	85	3.8	95	3.4	100	2.7
N. Com		50	5.3	60	4.5	75	4.0	85	3.8	95	3.4	100	2.7
G. Com		50	4.7	60	4.1	75	3.6	85	3.4	90	2.9	100	2.4
O.P./Com		50	4.2	60	3.7	70	3.1	80	2.9	90	2.6	100	2.2
Limited I.		50	4.2	60	3.7	70	3.1	80	2.9	90	2.6	100	2.2
General I.		50	3.7	60	3.2	70	2.7	80	2.6	90	2.3	100	1.9

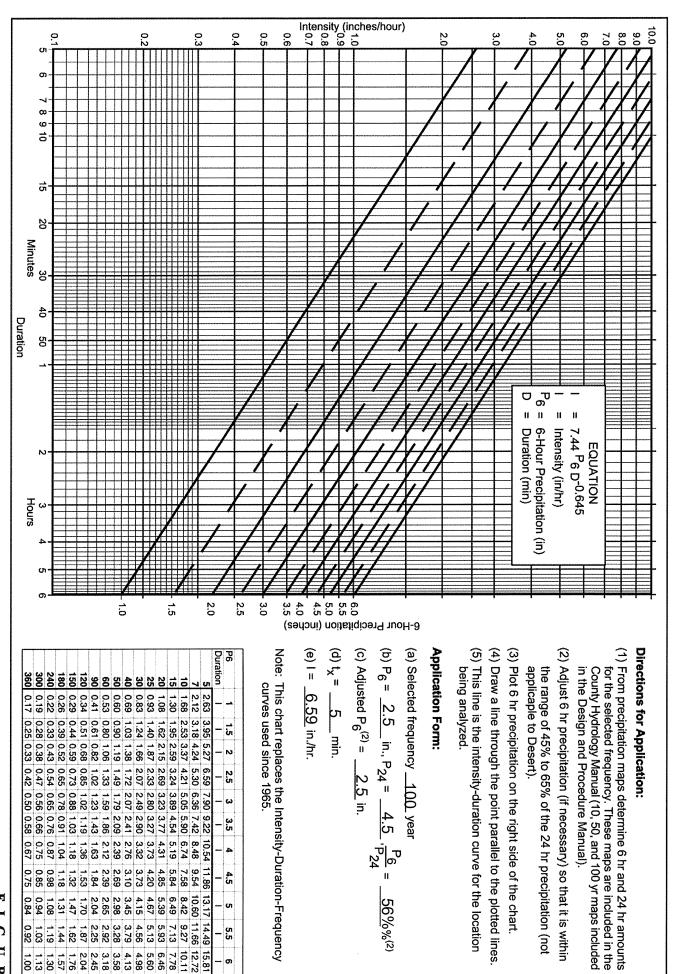
MAXIMUM OVERLAND FLOW LENGTH (L_M) & INITIAL TIME OF CONCENTRATION (T_i)

*See Table 3-1 for more detailed description

Intensity-Duration Design Chart - Template

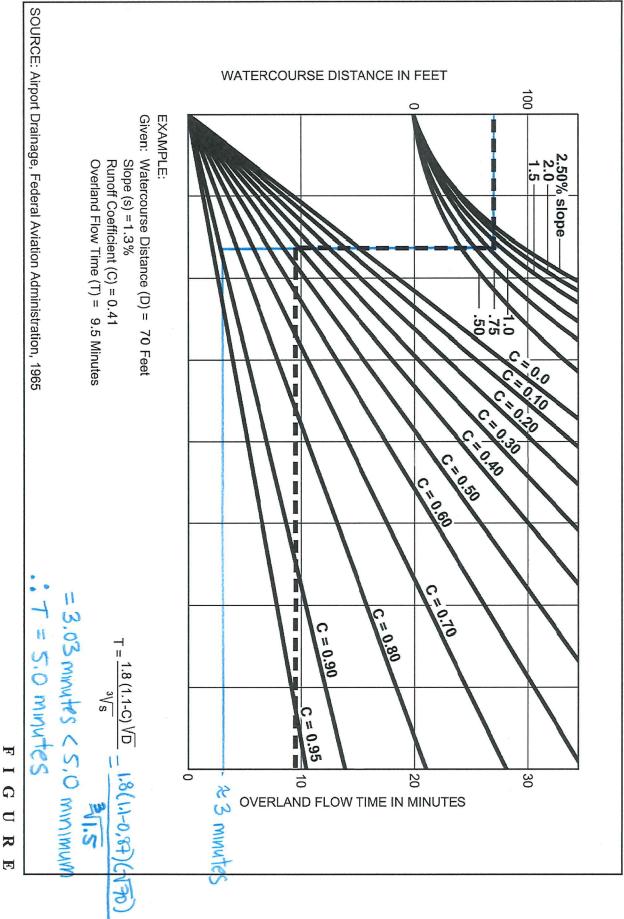
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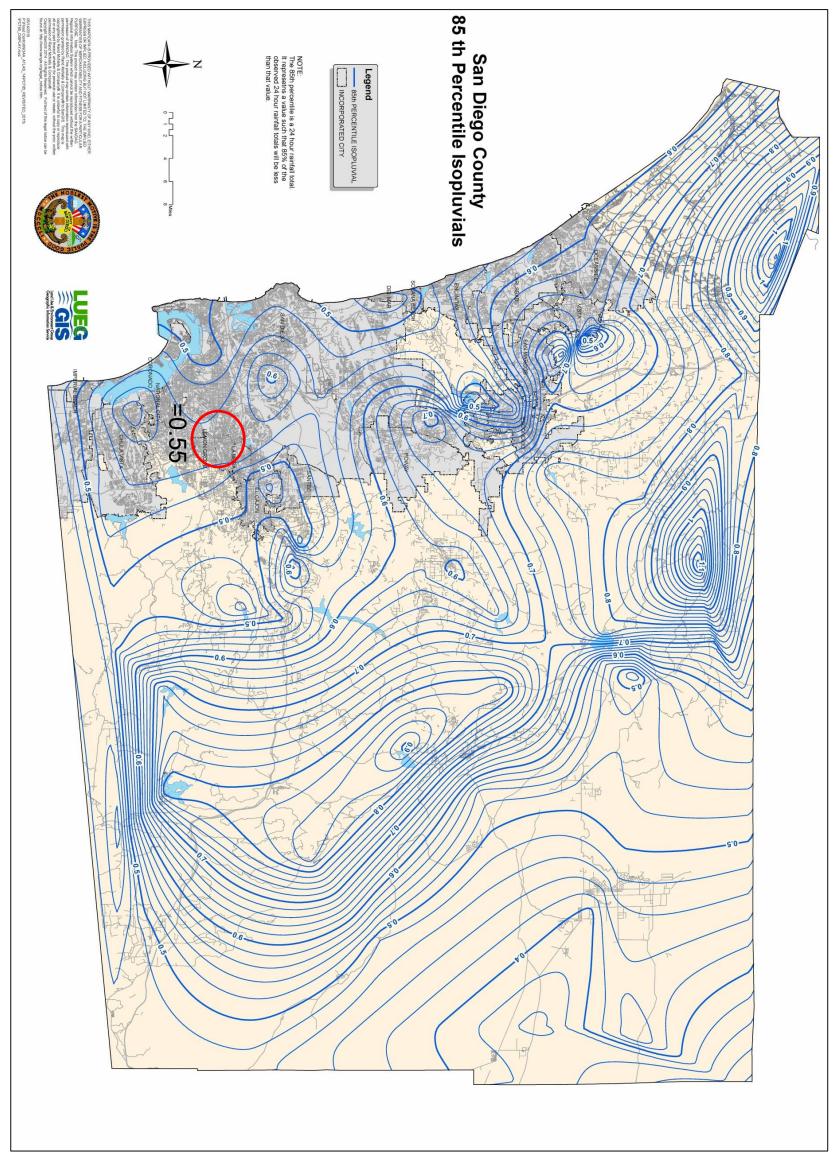
12.72 7.78 10.11 15.81 თ





Rational Formula - Overland Time of Flow Nomograph

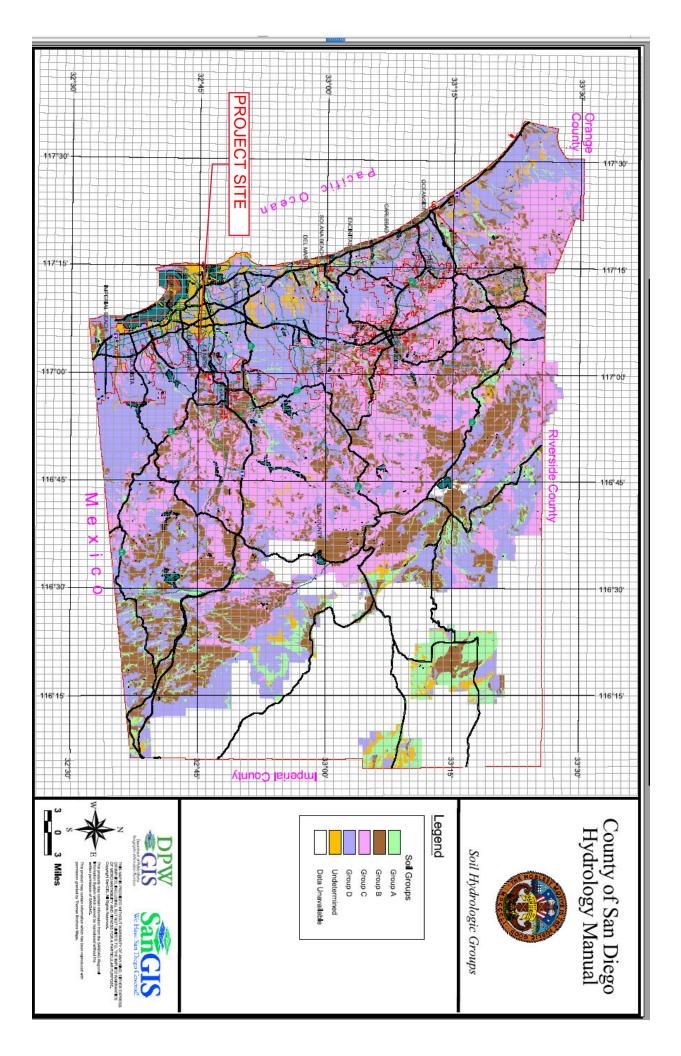






B-5





Fickion Method Manning Formula Solve For Normal Depth Input Data 0.013 Roughness Coefficient 0.0100 Channel Slope 0.02000 Input Data 0.0100 Channel Slope 0.02000 Input Data 0.02000 Regults 0.84 Results 10 Prow Area 0.18 Yoldth 0.28 Rydauk Radius 0.16 Top Width 0.28 Yoldth 0.38 Top Width 0.89 Yoldth 0.89 Top Width 0.89 Yoldth 0.89 Top Width 0.89 Yoldth 0.89 Yoldth 0.89 Yoldth 0.89 Yoldth 8 Yoldth 8 Yoldth 8 Yoldth 0.005 Yoldth 0.005 Yoldth 0.005 Yoldth 10 <		Worksheet fo	r Circular	r Pipe - A
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•	Average End Depth Over Rise		0.00	%
Downstream Velocity Infinity ft/s	Normal Depth Over Rise		27.64	%
	Downstream Velocity		Infinity	ft/s

 Bentley Systems, Inc.
 Haestad Methods Sol External Operate Master V8i (SELECTseries 1)
 [08.11.01.03]

 27 Siemons Company Drive Suite 200 W Watertown, CT 06795 USA +1-203-755-1666
 Page 1 of 2

Upstream Velocity	Infinity	ft/s
Normal Depth	0.28	ft
Critical Depth	0.38	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00571	ft/ft

	Worksheet for	^r Circulaı	r Pipe - B
Project Description			
Friction Method	Manning Formula		
Solve For	Normal Depth		
Input Data			
Roughness Coefficient		0.013	
Channel Slope		0.02000	ft/ft
Diameter		0.50	ft
Discharge		0.47	ft³/s
Results			
Normal Depth		0.28	ft
Flow Area		0.11	ft²
Wetted Perimeter		0.84	ft
Hydraulic Radius		0.13	ft
Top Width		0.50	ft
Critical Depth		0.35	ft
Percent Full		55.4	%
Critical Slope		0.01004	ft/ft
Velocity		4.21	ft/s
Velocity Head		0.28	ft
Specific Energy		0.55	ft
Froude Number		1.57	
Maximum Discharge		0.85	ft³/s
Discharge Full		0.79	ft³/s
Slope Full		0.00702	ft/ft
Flow Type	SuperCritical		
GVF Input Data			
Downstream Depth		0.00	ft
Length		0.00	ft
Number Of Steps		0	
GVF Output Data			
Upstream Depth		0.00	ft
Profile Description			
Profile Headloss		0.00	ft
Average End Depth Over Rise		0.00	%
Normal Depth Over Rise		55.38	%
Downstream Velocity		Infinity	ft/s

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.28	ft
Critical Depth	0.35	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.01004	ft/ft

	Worksheet fo	or Circular	r Pipe - C
Project Description			
Friction Method Solve For	Manning Formula Normal Depth		
Input Data			
Roughness Coefficient Channel Slope Diameter Discharge		0.013 0.02000 0.50 0.37	ft/ft ft
Results			
Normal Depth Flow Area Wetted Perimeter Hydraulic Radius Top Width Critical Depth Percent Full Critical Slope Velocity Velocity Head Specific Energy Froude Number Maximum Discharge Discharge Full Slope Full Flow Type	SuperCritical	0.24 0.09 0.77 0.12 0.50 0.31 48.0 0.00881 3.97 0.25 0.49 1.62 0.85 0.79 0.00435	ft ft % ft/ft ft/s ft ft ft ft ³ /s ft ³ /s
GVF Input Data	SuperChildan		
Downstream Depth Length Number Of Steps		0.00 0.00 0	ft ft
GVF Output Data			
Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise		0.00 0.00	ft %
Normal Depth Over Rise Downstream Velocity		47.99 Infinity	

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.24	ft
Critical Depth	0.31	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00881	ft/ft

	Worksheet fo	r Circulaı	r Pipe - D
Project Description			
Friction Method	Manning Formula		
Solve For	Normal Depth		
Input Data			
Roughness Coefficient		0.013	
Channel Slope		0.02000	ft/ft
Diameter		0.50	ft
Discharge		0.74	ft³/s
Results			
Normal Depth		0.38	ft
Flow Area		0.16	ft²
Wetted Perimeter		1.06	ft
Hydraulic Radius		0.15	ft
Top Width		0.42	ft
Critical Depth		0.43	ft
Percent Full		76.5	%
Critical Slope		0.01603	ft/ft
Velocity		4.59	ft/s
Velocity Head		0.33	ft
Specific Energy		0.71	ft
Froude Number		1.31	
Maximum Discharge		0.85	ft³/s
Discharge Full		0.79	ft³/s
Slope Full		0.01739	ft/ft
Flow Type	SuperCritical		
GVF Input Data			
Downstream Depth		0.00	ft
Length		0.00	ft
Number Of Steps		0	
GVF Output Data			
Upstream Depth		0.00	ft
Profile Description			
Profile Headloss		0.00	ft
Average End Depth Over Rise		0.00	%
Normal Depth Over Rise		76.47	%
Downstream Velocity		Infinity	ft/s

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.38	ft
Critical Depth	0.43	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.01603	ft/ft

	Worksheet fo	r Circular	[·] Pipe - E1
Project Description			
Friction Method Solve For	Manning Formula Normal Depth		
Input Data			
Roughness Coefficient Channel Slope Diameter Discharge		0.013 0.02000 0.50 0.10	ft
Results			
Normal Depth Flow Area Wetted Perimeter Hydraulic Radius Top Width Critical Depth Percent Full Critical Slope Velocity Velocity Head Specific Energy Froude Number Maximum Discharge Discharge Full Slope Full Flow Type	SuperCritical	0.12 0.04 0.51 0.07 0.43 0.16 23.9 0.00707 2.77 0.12 0.24 1.68 0.85 0.79 0.00032	ft ² ft ft ft ft ft % ft/ft ft/s ft ft ft ft ft ft s ft ft s ft ft s ft ft ft
GVF Input Data	ouperonition		
Downstream Depth Length Number Of Steps		0.00 0.00 0	ft ft
GVF Output Data			
Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise		0.00 0.00 0.00	ft ft %
Normal Depth Over Rise Downstream Velocity		23.94 Infinity	%

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.12	ft
Critical Depth	0.16	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00707	ft/ft

	Worksheet fo	r Circular	[·] Pipe - E2
Project Description			
Friction Method Solve For	Manning Formula Normal Depth		
Input Data			
Roughness Coefficient Channel Slope Diameter Discharge		0.013 0.02000 0.50 0.17	ft/ft ft
Results			
Normal Depth Flow Area Wetted Perimeter Hydraulic Radius Top Width Critical Depth Percent Full Critical Slope Velocity Velocity Head Specific Energy Froude Number Maximum Discharge Discharge Full Slope Full Flow Type	SuperCritical	0.16 0.05 0.60 0.09 0.46 0.21 31.5 0.00728 3.21 0.16 0.32 1.68 0.85 0.79 0.00092	ft ² ft ft ft ft ft % ft/ft ft/s ft ft ft ft ft s ft ft s ft ft s ft ft
	SuperChilda		
GVF Input Data Downstream Depth Length Number Of Steps		0.00 0.00 0	
GVF Output Data			
Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise		0.00 0.00 0.00	ft
Normal Depth Over Rise Downstream Velocity		31.47 Infinity	

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.16	ft
Critical Depth	0.21	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00728	ft/ft

	Worksheet for Circu	lar	Pipe - E3
Project Description			
Friction Method Solve For	Manning Formula Normal Depth		
Input Data			
Roughness Coefficient Channel Slope Diameter Discharge	0.02	013 000).40).05	ft/ft ft ft³/s
Results			
Normal Depth Flow Area Wetted Perimeter Hydraulic Radius Top Width Critical Depth Percent Full Critical Slope Velocity Velocity Head Specific Energy Froude Number Maximum Discharge Discharge Full Slope Full Flow Type	(((((((((((((((((((2.32).08).17 .62).47).44	ft ft ² ft ft ft ft % ft/ft ft/s ft ft ft ³ /s ft ³ /s ft/ft
GVF Input Data			
Downstream Depth Length Number Of Steps		0.00 0.00 0	ft ft
GVF Output Data			
Upstream Depth Profile Description Profile Headloss	(0.00	ft
Average End Depth Over Rise Normal Depth Over Rise Downstream Velocity	22	0.00 2.77 nity	% % ft/s

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.09	ft
Critical Depth	0.12	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00761	ft/ft

	Worksheet fo	r Circular	[·] Pipe - E4
Project Description			
Friction Method Solve For	Manning Formula Normal Depth		
Input Data			
Roughness Coefficient Channel Slope Diameter Discharge		0.013 0.02000 0.50 0.16	ft/ft ft ft³/s
Results			
Normal Depth Flow Area Wetted Perimeter Hydraulic Radius Top Width Critical Depth Percent Full Critical Slope Velocity Velocity Head Specific Energy Froude Number Maximum Discharge Discharge Full Slope Full Flow Type	SuperCritical	0.15 0.05 0.58 0.09 0.46 0.20 30.5 0.00724 3.16 0.16 0.31 1.68 0.85 0.79 0.00081	ft/s ft ft ft ³ /s ft ³ /s
GVF Input Data	Superchilical		
Downstream Depth Length Number Of Steps		0.00 0.00 0	ft ft
GVF Output Data			
Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise		0.00 0.00 0.00	ft ft %
Normal Depth Over Rise Downstream Velocity		30.49 Infinity	%

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.15	ft
Critical Depth	0.20	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00724	ft/ft

	Worksheet fo	r Circular	Pipe - E5
Project Description			
Friction Method Solve For	Manning Formula Normal Depth		
Input Data			
Roughness Coefficient Channel Slope Diameter Discharge		0.013 0.02000 0.50 0.16	ft/ft ft ft³/s
Results			
Normal Depth Flow Area Wetted Perimeter Hydraulic Radius Top Width Critical Depth Percent Full Critical Slope Velocity Velocity Head Specific Energy Froude Number Maximum Discharge Discharge Full Slope Full Flow Type	SuperCritical	0.15 0.05 0.58 0.09 0.46 0.20 30.5 0.00724 3.16 0.16 0.31 1.68 0.85 0.79 0.00081	ft ft ² ft ft ft ft ft/ft ft/s ft ft ³ /s ft ³ /s ft/ft
GVF Input Data	Superchica		
Downstream Depth Length Number Of Steps		0.00 0.00 0	ft ft
GVF Output Data			
Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise		0.00 0.00 0.00 30.49	ft ft %
Normal Depth Over Rise Downstream Velocity		Infinity	% ft/s

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.15	ft
Critical Depth	0.20	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00724	ft/ft

	Worksheet fo	or Circula	r Pipe F
Project Description			
Friction Method	Manning Formula		
Solve For	Normal Depth		
Input Data			
Roughness Coefficient		0.013	
Channel Slope		0.02000	ft/ft
Diameter		0.50	ft
Discharge		0.78	ft³/s
Results			
Normal Depth		0.40	ft
Flow Area		0.17	ft²
Wetted Perimeter		1.11	ft
Hydraulic Radius		0.15	ft
Top Width		0.40	ft
Critical Depth		0.44	ft
Percent Full		80.4	%
Critical Slope		0.01738	ft/ft
Velocity		4.61	ft/s
Velocity Head		0.33	ft
Specific Energy		0.73	ft
Froude Number		1.24	
Maximum Discharge		0.85	ft³/s
Discharge Full		0.79	ft³/s
Slope Full		0.01933	ft/ft
Flow Type	SuperCritical		
GVF Input Data			
Downstream Depth		0.00	ft
Length		0.00	ft
Number Of Steps		0	
GVF Output Data			
Upstream Depth		0.00	ft
Profile Description			
Profile Headloss		0.00	ft
Average End Depth Over Rise		0.00	%
Normal Depth Over Rise		80.44	%
Downstream Velocity		Infinity	ft/s

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.40	ft
Critical Depth	0.44	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.01738	ft/ft

	Worksheet fo	or Circula	r Pipe G	
Project Description				
Friction Method Solve For	Manning Formula Normal Depth			
Input Data				
Roughness Coefficient Channel Slope Diameter Discharge		0.013 0.02000 1.00 0.47	ft/ft ft ft³/s	
Results				
Normal Depth Flow Area Wetted Perimeter Hydraulic Radius Top Width Critical Depth Percent Full Critical Slope Velocity Velocity Head Specific Energy Froude Number Maximum Discharge Discharge Full Slope Full Flow Type	SuperCritical	0.21 0.12 0.94 0.12 0.81 0.28 20.6 0.00561 4.02 0.25 0.46 1.86 5.42 5.04 0.00017	ft ft ² ft ft ft ft ft/ft ft/s ft ft ft ³ /s ft ³ /s ft/ft	
GVF Input Data	SuperChildan			
Downstream Depth Length Number Of Steps		0.00 0.00 0	ft ft	
GVF Output Data				
Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise		0.00 0.00 0.00	ft ft %	
Normal Depth Over Rise Downstream Velocity		20.64 Infinity	% % ft/s	

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.21	ft
Critical Depth	0.28	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00561	ft/ft

	Worksheet fo	r Circular	[,] Pipe H1
Project Description			
Friction Method Solve For	Manning Formula Normal Depth		
Input Data			
		0.013	
Roughness Coefficient Channel Slope		0.013	ft/ft
Diameter		0.02000	ft
Discharge		0.37	ft ³ /s
Results			
Normal Depth		0.24	ft
Flow Area		0.24	ft ²
Wetted Perimeter		0.09	ft
Hydraulic Radius		0.12	ft
Top Width		0.50	ft
Critical Depth		0.31	ft
Percent Full		48.0	%
Critical Slope		0.00881	ft/ft
Velocity		3.97	ft/s
Velocity Head		0.25	ft
Specific Energy		0.49	ft
Froude Number		1.62	
Maximum Discharge		0.85	ft³/s
Discharge Full		0.79	ft³/s
Slope Full		0.00435	ft/ft
Flow Type	SuperCritical		
GVF Input Data			
Downstream Depth		0.00	ft
Length		0.00	ft
Number Of Steps		0	
GVF Output Data			
Upstream Depth		0.00	ft
Profile Description			
Profile Headloss		0.00	ft
Average End Depth Over Rise		0.00	%
Normal Depth Over Rise		47.99	%
Downstream Velocity		Infinity	ft/s

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.24	ft
Critical Depth	0.31	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00881	ft/ft

Friction Method Normal DepthManning Formula Normal DepthInput DataReughness Coefficient0.013Channel Slope0.02000 1/fDianeter0.50ItDianeter0.50ResultsResultsNormal Depth0.24Flow Area0.09Prive0.77Rightense Coefficient0.77To Witch Period0.01To Witch Period0.11Prive Area0.09Prive Area0.11Prive Area0.11To Witch Period0.12Rick Area0.11Prive Area0.12Prive Area0.13Prive Area0.14Prive Area0.14 <t< th=""><th></th><th>Worksheet fo</th><th>r Circular</th><th>[,] Pipe H2</th></t<>		Worksheet fo	r Circular	[,] Pipe H2
Solve ForNormal DepthInput DataRoughness Coefficient0.013Channel Slope0.02000Infinitional Slope0.02000Discharge0.03ResultsNormal Depth0.24Pflow Area0.09Wetted Perimeter0.77Rytetted Perimeter0.78Specific Ency0.0881Rytetted Perimeter1.82Percent Full0.80Specific Ency0.49Rytetted Perimeter1.82Rytetted Perimeter1.82Ryt	Project Description			
Regulpmess Coefficient0.013Channel Slope0.02000ftfDianeter0.05ftDischarge0.37ftResults1ftFlow Area0.09f2Wetted Perimeter0.77ftTop Width0.50ftTop Width0.50ftChical Slope0.0081ftChical Slope0.0081ftChical Slope0.0881ftVelocity3.97f/sVelocity3.97f/sVelocity3.97f/sVelocity0.081ftSpecific Energy0.49ftSlope Full0.00435ft/sDischarge Full0.00435ft/sSlope Full0.00435ft/sDownstream Depth0.00ftLength0.00ftNumber Of Steps0ftProfile DescriptionftProfile Headloss0.00ftProfile Headloss0.00ftNormal Depth Over Rise0.00ftNormal Depth Over Rise0.00ft <td>Friction Method Solve For</td> <td>-</td> <td></td> <td></td>	Friction Method Solve For	-		
Regulpmess Coefficient0.013Channel Slope0.02000ftfDianeter0.05ftDischarge0.37ftResults1ftFlow Area0.09f2Wetted Perimeter0.77ftTop Width0.50ftTop Width0.50ftChical Slope0.0081ftChical Slope0.0081ftChical Slope0.0881ftVelocity3.97f/sVelocity3.97f/sVelocity3.97f/sVelocity0.081ftSpecific Energy0.49ftSlope Full0.00435ft/sDischarge Full0.00435ft/sSlope Full0.00435ft/sDownstream Depth0.00ftLength0.00ftNumber Of Steps0ftProfile DescriptionftProfile Headloss0.00ftProfile Headloss0.00ftNormal Depth Over Rise0.00ftNormal Depth Over Rise0.00ft <td>Input Data</td> <td></td> <td></td> <td></td>	Input Data			
Channel Stope 0.02000 ft/ft Diameter 0.50 ft Discharge 0.37 ft//s Results 1 Normal Depth 0.24 ft Flow Area 0.09 ft* Wetted Perimeter 0.77 ft Hydraulic Radius 0.12 ft Top Width 0.50 ft Chical Depth 0.11 ft Percent Full 48.0 % Chical Stope 0.00881 ft/ft Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 Specific Energy 0.49 ft Frouge SuperCritical 0.79 ft/s Discharge Full 0.00435 ft/ft Stope Full 0.00435 ft/ft Discharge Full 0.00435 ft Number Of Steps 0 ft Porolie Description ft ft			0.013	
Diameter0.50ftDischarge0.37ftResults0.09ftKew Area0.09ftWetted Perimeter0.77ftWorkana0.12ftHydrauic Radius0.12ftTop Width0.50ftOrtical Depth0.31ftPercent Full48.0%Critical Slope0.00881ft/ftVelocity Head0.25ftSpecific Energy0.49ftFroude Number0.52ftSlope Full0.79ft/sDischarge Full0.79ft/sSlope Full0.79ft/sStope Full0.79ft/sDischarge Full0.00435ft/ftDownstream Depth0.00ftLength0.00ftLength0.00ftProfile Description16Profile Description17Profile Description16Profile Headloss0.00ftNumal Depth Over Rise0.00ftNormal Depth Over Rise0.00ftNormal Depth Over Rise0.00ftProfile Description16Profile Headloss0.00ftNormal Depth Over Rise0.00ftNormal Depth Over Rise<	-		0.02000	ft/ft
Results Nomal Depth 0.24 ft Flow Area 0.09 ft* Wetted Perimeter 0.77 ft Top Width 0.50 ft Top Width 0.50 ft Critical Depth 0.112 ft Percent Full 48.0 % Critical Slope 0.00881 ft/ft Velocity Head 0.25 ft Specific Energy 0.49 ft Flow Area 0.25 ft Specific Energy 0.49 ft/ft Specific Energy 0.49 ft/ft Flow Type Super Critical ft/ft Slope Full 0.00435 ft/ft Flow Type Super Critical ft/ft Slope Full 0.000 ft Length 0.00 ft Number Of Steps 0 ft Profile Description Frolife Headloss 0.00 Profile Headloss 0.00 ft <tr td=""> Karag</tr>	Diameter		0.50	
Normal Depth 0.24 ft Flow Area 0.09 ft² Wetted Perimeter 0.77 ft Hydraulic Radius 0.12 ft Top Width 0.50 ft Critical Depth 0.31 ft Percent Full 48.0 % Critical Slope 0.00881 ft/ft Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 Fouge Full 0.0435 ft/fs Discharge Full 0.79 ft/s Slope Full 0.00435 ft/f Flow Type SuperCritical ft Edwardter Energy 0.00 ft Length 0.00 ft Number Of Steps 0 ft Core Cutput Data ft ft Profile Description ft ft Profile Headloss 0.00 ft Average End Depth Over Rise 47.99 % <td>Discharge</td> <td></td> <td>0.37</td> <td>ft³/s</td>	Discharge		0.37	ft³/s
Flow Area 0.09 ft Wetted Perimeter 0.77 ft Hydraulic Radius 0.12 ft Top Width 0.50 ft Critical Depth 0.31 ft Percent Full 48.0 % Critical Slope 0.00881 ft/ft Velocity 3.97 f/s Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 r Foude Number 1.62 r Slope Full 0.00438 ft/ft Slope Full 0.00438 ft/ft Discharge Full 0.00438 ft/ft Slope Tupt Data 0.00 ft Length 0.00 ft Number Of Steps 0 ft OVEr Upt Data ft ft Profile Description ft ft Profile Description ft ft Profile Description ft ft	Results			
Weted Perimeter 0.77 ft Hydraulic Radius 0.12 ft Top Width 0.50 ft Critical Depth 0.31 ft Percent Full 48.0 % Critical Slope 0.00881 ft/ft Velocity 3.97 ft/s Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 Velocity Maximum Discharge 0.85 ft/s Discharge Full 0.00435 ft/ft Stope Full 0.00435 ft/ft Prouge Number 1.62 Velocity Stope Full 0.00435 ft/ft Stope Full 0.00435 ft/ft Stope Full 0.00435 ft/ft Stope Full 0.00435 ft/ft Stope Full 0.00435 ft Stope Full 0.00 ft Length 0.00 ft Number Of Steps 0 ft<	Normal Depth		0.24	ft
Hydraulic Radius 0.12 f Top Width 0.50 f Top Width 0.31 ft Percent Full 48.0 % Critical Stope 0.00881 fvft Velocity 3.97 fvs Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 Discharge Full 0.79 ft/s Discharge Full 0.00435 ft/ft Flow Type SuperCritical ft/ft Stope Full 0.00435 ft/ft Flow Type SuperCritical ft/ft Stope Full 0.00435 ft/ft Flow Type SuperCritical ft/ft Stope Full 0.00435 ft/ft Flow Type SuperCritical ft Stope Full 0.00435 ft/ft Flow Type SuperCritical ft Stope Full 0.00435 ft Number Of Steps 0.00 ft Profie Description st ft	Flow Area		0.09	ft²
Top Width 0.50 ft Critical Depth 0.31 ft Percent Full 48.0 % Critical Slope 0.00881 ft/ft Velocity 3.97 ft/s Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 Maximum Discharge 0.85 ft/s Discharge Full 0.79 ft/s Stope Full 0.00435 ft/ft Flow Type SuperCritical ft Stope Full 0.00 ft Provent Detta 0.00 ft Cotype SuperCritical ft ft Stope Full 0.00 ft Downstream Depth 0.00 ft Number Of Steps 0 ft Profile Description ft ft Profile Description ft ft Profile Headloss 0.00 ft Average End Depth Over Rise 0.00	Wetted Perimeter		0.77	ft
Critical Depth 0.31 ft Percent Full 48.0 % Critical Slope 0.00881 ft/ft Velocity 3.97 ft/s Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 Froude Number 1.62 Maximum Discharge 0.85 ft/s Discharge Full 0.79 ft/s Stope Full 0.00435 ft/ft Flow Type SuperCritical thft Pownstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 ft Profile Description ft ft Profile Description 0.00 ft Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 ft Normal Depth Over Rise 0.00 %	Hydraulic Radius		0.12	ft
Percent Full 48.0 % Critical Slope 0.00811 ft/ft Velocity 3.97 ft/s Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 Maximum Discharge 0.85 ft?/s Discharge Full 0.79 ft?/s Slope Full 0.00435 fuft Flow Type SuperCritical ft Bownstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 ft Profile Description ft Profile Description ft Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 %	Top Width		0.50	ft
Critical Slope 0.00881 fr/f Velocity 3.97 fr/s Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 Maximum Discharge 0.85 ft%s Discharge Full 0.79 ft%s Slope Full 0.00435 ft/f Flow Type SuperCritical ft Bownstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 ft Profile Description ft Profile Description Profile Description 0.00 ft Numal Depth Over Rise 0.00 ft	Critical Depth		0.31	ft
Velocity 3.97 ft/s Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 maximum Discharge Maximum Discharge 0.85 ft*/s Discharge Full 0.79 ft*/s Slope Full 0.0435 ft/ft Flow Type SuperCritical ft Bownstream Depth 0.00 CVF Input Data ft Length 0.00 ft Number Of Steps 0 ft GVF Output Data Upstream Depth 0.00 ft Profile Description ft ft Profile Description gt ft Average End Depth Over Rise 0.00 ft Normal Depth Over Rise 47.99 %	Percent Full		48.0	%
Velocity Head 0.25 ft Specific Energy 0.49 ft Froude Number 1.62 Maximum Discharge 0.85 ft/s Discharge Full 0.79 ft/s Slope Full 0.00435 ft/ft Flow Type SuperCritical ft/ft Bownstream Depth 0.00 Length 0.00 ft Number Of Steps 0 ft GVF Output Data Upstream Depth 0.00 ft Profile Description ft ft Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 ft	Critical Slope		0.00881	ft/ft
Specific Energy 0.49 ft Froude Number 1.62 Maximum Discharge 0.85 ft*/s Discharge Full 0.79 ft*/s Slope Full 0.00435 ft/ft Flow Type SuperCritical GVF Input Data Downstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 ft GVF Output Data Profile Description 0.00 ft Profile Description 0.00 ft Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 ft Normal Depth Over Rise 47.99 %	Velocity		3.97	ft/s
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Slope Full0.00435ft/ftFlow TypeSuperCriticalGVF Input DataDownstream Depth0.00ftLength0.00ftNumber Of Steps0ftGVF Output DataUpstream Depth0.00ftProfile DescriptionProfile Headloss0.00ftAverage End Depth Over Rise0.00ftNormal Depth Over Rise0.00ft	Maximum Discharge		0.85	ft³/s
Flow Type SuperCritical GVF Input Data 0.00 ft Downstream Depth 0.00 ft Length 0.00 ft Number Of Steps 0 ft GVF Output Data 0 ft Profile Depth 0.00 ft Profile Description ft Profile Headloss 0.00 ft Average End Depth Over Rise 0.00 ft Normal Depth Over Rise 47.99 %	Discharge Full		0.79	ft³/s
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•	Average End Depth Over Rise		0.00	%
Downstream Velocity Infinity ft/s	Normal Depth Over Rise		47.99	%
,	Downstream Velocity		Infinity	ft/s

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.24	ft
Critical Depth	0.31	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00881	ft/ft

	Worksheet for	Circular	гре пз
Project Description			
Friction Method	Manning Formula		
Solve For	Normal Depth		
Input Data			
		0.040	
Roughness Coefficient		0.013	
Channel Slope		0.02000	ft/ft
Diameter		0.50	ft
Discharge		0.37	ft³/s
Results			
Normal Depth		0.24	ft
Flow Area		0.09	ft²
Wetted Perimeter		0.77	ft
Hydraulic Radius		0.12	ft
Top Width		0.50	ft
Critical Depth		0.31	ft
Percent Full		48.0	%
Critical Slope		0.00881	ft/ft
Velocity		3.97	ft/s
Velocity Head		0.25	ft
Specific Energy		0.49	ft
Froude Number		1.62	
Maximum Discharge		0.85	ft³/s
Discharge Full		0.79	ft³/s
Slope Full		0.00435	ft/ft
Flow Type	SuperCritical		
GVF Input Data			
Downstream Depth		0.00	ft
Length			ft
Number Of Steps		0	
GVF Output Data			
Upstream Depth		0.00	ft
Profile Description			
Profile Headloss		0.00	ft
Average End Depth Over Rise		0.00	%
Normal Depth Over Rise		47.99	%
Downstream Velocity		Infinity	ft/s

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.24	ft
Critical Depth	0.31	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00881	ft/ft

	Worksheet f	or Circula	ar Pipe I
Project Description			
Friction Method	Manning Formula		
Solve For	Normal Depth		
Input Data			
Roughness Coefficient		0.013	
Channel Slope		0.02000	ft/ft
Diameter		0.67	ft
Discharge		0.95	ft³/s
Results			
Normal Depth		0.35	ft
Flow Area		0.19	ft²
Wetted Perimeter		1.09	ft
Hydraulic Radius		0.17	ft
Top Width		0.67	ft
Critical Depth		0.46	ft
Percent Full		52.8	%
Critical Slope		0.00895	ft/ft
Velocity		5.03	ft/s
Velocity Head		0.39	ft
Specific Energy		0.75	ft
Froude Number		1.67	
Maximum Discharge		1.86	ft³/s
Discharge Full		1.73	ft³/s
Slope Full		0.00602	ft/ft
Flow Type	SuperCritical		
GVF Input Data			
Downstream Depth		0.00	ft
Length		0.00	ft
Number Of Steps		0	
GVF Output Data			
Upstream Depth		0.00	ft
Profile Description			
Profile Headloss		0.00	ft
Average End Depth Over Rise		0.00	%
Normal Depth Over Rise		52.84	%
Downstream Velocity		Infinity	ft/s

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.35	ft
Critical Depth	0.46	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00895	ft/ft

	Worksheet	for Circula	r Pipe J		
Project Description					
Friction Method Solve For	Manning Formula Normal Depth				
Input Data					
Roughness Coefficient Channel Slope Diameter Discharge		0.013 0.02000 0.50 0.26	ft/ft ft ft³/s		
Results					
Normal Depth Flow Area Wetted Perimeter Hydraulic Radius Top Width Critical Depth Percent Full Critical Slope Velocity Velocity Head Specific Energy Froude Number Maximum Discharge Discharge Full Slope Full Flow Type	SuperCritical	0.20 0.07 0.68 0.11 0.49 0.26 39.4 0.00783 3.62 0.20 0.40 1.67 0.85 0.79 0.00215	ft ft ² ft ft ft ft ft/ft ft/s ft ft ft ³ /s ft ³ /s ft/ft		
GVF Input Data	Supercifical				
Downstream Depth Length Number Of Steps		0.00 0.00 0	ft ft		
GVF Output Data					
Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise		0.00 0.00 0.00	ft ft %		
Normal Depth Over Rise Downstream Velocity		39.35 Infinity	% ft/s		

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.20	ft
Critical Depth	0.26	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00783	ft/ft

	Worksheet f	or Circula	r Pipe K	
Project Description				
Friction Method Solve For	Manning Formula Normal Depth			
Input Data				
Roughness Coefficient Channel Slope Diameter Discharge		0.013 0.02000 0.50 0.84	ft/ft ft ft³/s	
Results				
Normal Depth Flow Area Wetted Perimeter Hydraulic Radius Top Width Critical Depth Percent Full Critical Slope Velocity Velocity Head Specific Energy Froude Number Maximum Discharge Discharge Full Slope Full Flow Type	SuperCritical	0.44 0.18 1.23 0.15 0.32 0.45 88.7 0.01968 4.56 0.32 0.77 1.06 0.85 0.79 0.02241	ft ft ² ft ft ft ft ft/ft ft/s ft ft ft ³ /s ft/ft	
GVF Input Data	Supercritical			
Downstream Depth Length Number Of Steps		0.00 0.00 0	ft ft	
GVF Output Data				
Upstream Depth Profile Description Profile Headloss Average End Depth Over Rise		0.00 0.00 0.00	ft ft %	
Normal Depth Over Rise Downstream Velocity		88.69 Infinity	% % ft/s	

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.44	ft
Critical Depth	0.45	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.01968	ft/ft

	Worksheet fo	or Circula	r Pipe L	
Project Description				
Friction Method	Manning Formula			
Solve For	Normal Depth			
Input Data				
Roughness Coefficient		0.013		
Channel Slope		0.02000	ft/ft	
Diameter		0.50	ft	
Discharge		0.16	ft³/s	
Results				
Normal Depth		0.15	ft	
Flow Area		0.05	ft²	
Wetted Perimeter		0.58	ft	
Hydraulic Radius		0.09	ft	
Top Width		0.46	ft	
Critical Depth		0.20	ft	
Percent Full		30.5	%	
Critical Slope		0.00724	ft/ft	
Velocity		3.16	ft/s	
Velocity Head		0.16	ft	
Specific Energy		0.31	ft	
Froude Number		1.68		
Maximum Discharge		0.85	ft³/s	
Discharge Full		0.79	ft³/s	
Slope Full		0.00081	ft/ft	
Flow Type	SuperCritical			
GVF Input Data				
Downstream Depth		0.00	ft	
Length		0.00	ft	
Number Of Steps		0		
GVF Output Data				
Upstream Depth		0.00	ft	
Profile Description				
Profile Headloss		0.00	ft	
Average End Depth Over Rise		0.00	%	
Normal Depth Over Rise		30.49	%	
Downstream Velocity		Infinity	ft/s	

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Upstream Velocity	Infinity	ft/s
Normal Depth	0.15	ft
Critical Depth	0.20	ft
Channel Slope	0.02000	ft/ft
Critical Slope	0.00724	ft/ft



GEOTECHNICAL EVALUATION UNIVERSITY AVENUE MIXED USE RESIDENTIAL/COMMERCIAL SAN DIEGO, CALIFORNIA

PREPARED FOR:

University Manor 8051 Main Street Stanton, California 90680

PREPARED BY:

Ninyo & Moore Geotechnical and Environmental Sciences Consultants 5710 Ruffin Road San Diego, California 92123

> May 11, 2017 Project No. 108335001

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May 11, 2017 Project No. 108335001

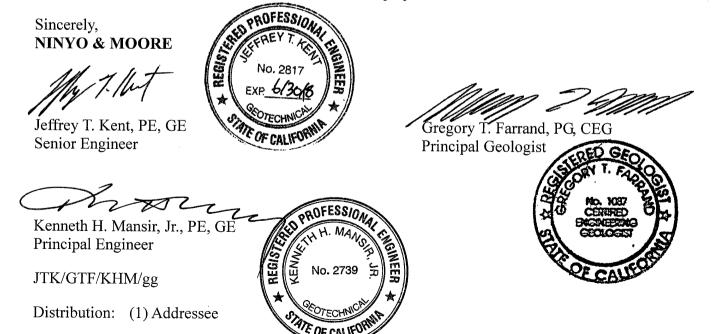
Mr. Lutfi Bustami University Manor 8051 Main Street Stanton, California 90680

Subject: Geotechnical Evaluation University Avenue Mixed Use Residential/Commercial San Diego, California

Dear Mr. Bustami:

In accordance with your authorization, we have performed a geotechnical evaluation for the proposed mixed use residential and commercial development to be constructed at 5556 to 5592 University Avenue in San Diego, California. Our evaluation was performed in accordance with our proposal dated January 13, 2017. This report presents our findings, conclusions, and geotechnical recommendations for the project.

We appreciate the opportunity to be of service on this project.



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- Appendix A Boring Logs
- Appendix B Infiltration Testing
- Appendix C Laboratory Testing

1. INTRODUCTION

In accordance with your authorization, we have performed a geotechnical evaluation for the proposed mixed use residential and commercial development to be constructed at 5556 to 5592 University Avenue in San Diego, California (Figure 1). Our evaluation was performed in accordance with our proposal dated January 13, 2017. This report presents our findings, conclusions, and geotechnical recommendations for the project. The objectives of this study were to assess the prevailing soil conditions at the site, evaluate the engineering properties of the soils encountered, and provide recommendations relative to the geotechnical aspects of the proposed project. This geotechnical evaluation included the review of a previous site geotechnical report (PEC, 2015), field exploration, infiltration testing, laboratory testing of representative samples, and engineering analyses of the data obtained.

2. SCOPE OF SERVICES

Our scope of services for this evaluation included the following:

- Review of readily available published and in-house geotechnical literature, topographic maps, geologic maps, fault maps, stereoscopic aerial photographs, and the previous site geotechnical report (PEC, 2015).
- Review of preliminary and conceptual site plans prepared by the project architect.
- Acquisition of a boring permit from the County of San Diego Department of Environmental Health (DEH).
- Performance of a field reconnaissance to observe site conditions and to locate and mark the exploratory boring locations.
- Performance of a site reconnaissance to the geologic features exposed in the slopes along the northern and western boundaries of the site.
- Notification of Underground Service Alert (USA) to clear boring locations for the potential presence of underground utilities.
- Performance of a subsurface exploration consisting of the drilling, logging, and sampling of four exploratory borings. Bulk and in-place soil samples were obtained at selected intervals from within the borings. The collected samples were transported to our in-house geotechnical laboratory for analysis.

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- Performance of infiltration testing within two borings.
- Performance of geotechnical laboratory testing on representative samples to evaluate soil characteristics and design parameters.
- Compilation and analysis of the data obtained from our background review, subsurface exploration, and laboratory testing.
- Preparation of this report presenting our findings, conclusions, and recommendations regarding the geotechnical design and construction aspects of the project.

3. SITE AND PROJECT DESCRIPTION

The site for the proposed mixed use development is located on the north side of University Avenue approximately a ¹/₄ mile east of its intersection with 54th Street in San Diego, California (Figure 1). The rectangular site consists of three parcels located at 5556 through 5592 University Avenue. The site is bounded by University Avenue to the south, a private driveway to the west, open space to the east, and landscaping associated with the adjacent private development to the north. The site coordinates are approximately 32.7481°N latitude and -117.0758°W longitude. Existing improvements includes several abandoned buildings and various concrete and asphalt concrete (AC) pavements. Topographically, the site generally has a downward gradient of approximately 6 percent to the south. In addition to this gradient, there are existing ascending cut slopes up to approximately 30 feet high along the northern and northwestern boundaries of the site. These slopes have inclinations that approach being near vertical and have experienced some erosion. Elevations at the site range from approximately 286 feet above mean sea level (MSL) at the south end of the site near University Avenue to approximately 340 feet above MSL at the north end of the site at the top of the south facing slope.

Based on our review of conceptual plans provided by the project architect, we understand the project will involve the construction of four buildings. Two buildings are proposed for residential usage and will be up to four stories tall. The other two buildings are proposed for commercial use and will be on the order of two stories tall. To accommodate grade changes, several retaining walls will be constructed at the site, with some of the retaining walls being incorporated into the buildings. Grade changes at the site are anticipated to generate significant

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amounts of export soil. Additionally, the steep slopes along the north and northwestern boundaries will be regraded to flatter inclinations and retaining walls will be constructed in front of them. Further improvements are anticipated to include concrete and AC pavements, concrete flatwork, infiltration devices, underground utilities, and landscaping.

4. SUBSURFACE EXPLORATION

Our subsurface exploration was conducted on March 23 and 24, 2017, and consisted of the drilling, logging, and sampling of four small-diameter exploratory borings (B-1, B-2, IT-1, and IT-2). Borings B-1 and B-2 were drilled using a limited access drill rig equipped with hollow stem augers to depths up to approximately 31 feet. Borings IT-1 and IT-2 were manually excavated up to depths of approximately 8 feet and were used for infiltration testing. Bulk and in-place soil samples were obtained from the borings at selected intervals. The samples were then transported to our in-house geotechnical laboratory for testing. The approximate locations of the exploratory borings are shown on Figures 2A and 2B. Boring logs are included in Appendix A.

5. INFILTRATION TESTING

On March 23, 2017, two exploratory borings (IT-1 and IT-2) were manually excavated to evaluate the infiltration characteristics of the site. The borings were manually excavated to depths of up to approximately 3-1/3 feet and 8 feet. During excavation, the borings were logged and sampled by Ninyo & Moore personnel. Logs of the borings are presented in Appendix A. Following excavation, infiltration tests were performed in the borings. The infiltration tests were performed in general accordance with County of San Diego BMP Design Manual (2016). Approximately 2 inches of gravel was placed on the bottom of each prepared boring. Then, a 2-inch-diameter, perforated PVC pipe was installed in the boring and the annulus was then backfilled with pea gravel. As part of the test procedure, a presoak was performed to represent adverse conditions for infiltration. The presoak consisted of maintaining an approximately 1 foot column of water in each boring for approximately 4 hours. The water level was then allowed to drop overnight. Infiltration testing was performed on March 24, 2017 in the presoaked borings. The borings were filled with approximately 6 inches of water and measurements of the water depth were generally recorded every 30 minutes until consistent measurements were obtained. The borings were refilled as needed to maintain the water level until the infiltration rate stabilized.

Infiltration rates were then calculated using the Porchet method. Adjusted infiltration test results ranged between <0.01 and 1.08 inches per hour (in/hr). Infiltration test measurements and calculations are included are included in Appendix B, and the results are summarized in Table 1 below.

Infiltration Test (depth)	Soil Description at Test Depth (Geologic Unit)	Infiltration Rate in/hr
IT-1 (8.2 feet)	T-1 (8.2 feet) Silty SANDSTONE (Mission Valley Formation)	
IT-2 (3.3 feet)	Sandy CLAY (Fill)	< 0.01

 Table 1 – Infiltration Test Results Summary

6. LABORATORY TESTING

Geotechnical laboratory testing was performed on representative soil samples to evaluate in-situ moisture content and dry density, shear strength, expansion index, soil corrosivity, and R-value. The results of the in-situ moisture content and dry density tests are presented on the boring logs in Appendix A. The results of the other laboratory tests are presented in Appendix C.

7. GEOLOGY AND SUBSURFACE CONDITIONS

The following sections provide information regarding the geologic conditions relative to the project site.

7.1. Regional Geologic Conditions

The project area is situated in the coastal foothill section of the Peninsular Ranges Geomorphic Province. This geomorphic province encompasses an area that extends approximately 900 miles from the Transverse Ranges and the Los Angeles Basin south to the southern tip of Baja California (Norris and Webb, 1990; Harden, 2004). The province varies in width from approximately 30 to 100 miles. In general, the province consists of rugged mountains underlain by Jurassic metavolcanic and metasedimentary rocks, and Cretaceous igneous rocks of the southern Cali-



fornia batholith. The portion of the province in western San Diego County that includes the project area consists generally of uplifted and dissected coastal plain underlain by Tertiary-age sedimentary rocks (Figure 3).

The Peninsular Ranges Province is traversed by a group of sub-parallel faults and fault zones trending roughly northwest. Several of these faults are considered active faults. The Elsinore, San Jacinto, and San Andreas faults are active fault systems located northeast of the project area and the Coronado Bank, San Diego Trough, San Clemente, and Rose Canyon faults are active faults located west of the project area (Figure 4). Major tectonic activity associated with these and other faults within this regional tectonic framework consists primarily of right-lateral, strike-slip movement. The Rose Canyon Fault Zone, the nearest active fault system, has been mapped approximately 6 miles west of the project site (Figure 4). Additionally, the La Nacion Fault system has been mapped approximately 900 feet west of the site per the City of San Diego Safety Element (2008). However, the La Nacion Fault is not considered an active fault by the State of California.

7.2. Subsurface Conditions

The geology of the site vicinity is shown on Figure 3. Geologic units encountered during our background review and subsurface exploration included fill and materials of the Mission valley Formation (Kennedy and Tan, 2008). Generalized descriptions of the earth units encountered during our field reconnaissance and subsurface exploration are provided in the subsequent sections. Additional descriptions of the subsurface units are provided on the boring logs in Appendix A. Geologic cross sections for the project are presented on Figures 5 and 6.

7.2.1. Fill

Fill materials were encountered in our exploratory borings IT-1 and IT-2 at the ground surface or underlying the pavements and extending to depths up to approximately 7 feet. As encountered, these materials generally consisted of brown, moist, stiff to hard, sandy clay with gravel and cobbles. The previous geotechnical report (PEC, 2015) encountered fill materials to depths of approximately 1 to 3 feet that generally consisted of

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brown, loose to medium dense, dry to moist, sandy silt with gravel. Deeper fills may be present at the site. Documentation of the placement and compaction of existing fill was not available for our review.

Laboratory testing presented in Appendix C indicates that the existing fill soils possess a medium to high potential for expansion and are considered corrosive to ferrous metals.

7.2.2. Mission Valley Formation

Materials mapped as the Mission Valley Formation (Kennedy and Tan, 2008) were encountered at the surface, beneath pavements, or underlying the fill materials in our borings B-1, B-2, and IT-1. As encountered, the materials generally consisted of varying shades of brown and gray, moist, moderately to strongly cemented, silty sandstone and well indurated, sandy siltstone. The previous geotechnical report (PEC, 2015) also encountered materials mapped as the Mission Valley Formation either at the ground surface or underlying the fill materials. These materials generally consisted of gray, damp to moist, sandstone.

During our site reconnaissance, we performed geologic mapping of the exposed cut slopes along the northern and western boundaries of the project site. Based on the exposures, it was observed that the bedding within the Mission Valley Formation was dipping approximately 2 to 4 degrees to the south.

7.3. Groundwater

Groundwater was not encountered in our explorations at the time of drilling. Based on review of monitoring well data in the site vicinity using the Geotracker website (2017), groundwater has been encountered at elevations ranging from approximately 276 to 280 feet above MSL (Morgan & Associates, 2008 and Murex Environmental, 2011). Specifically, the groundwater elevation was measured at approximately 280 feet above MSL at the northeastern portion of the site and approximately 276 feet above MSL at the southwestern portion of the site. Fluctuations in the groundwater level may occur due to variations in ground surface

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topography, subsurface geologic conditions and structure, rainfall, irrigation, and other factors not evident at the time of our subsurface evaluation. Additionally, perched water conditions may be encountered in such areas as existing utility trenches.

8. GEOLOGIC HAZARDS

In general, hazards associated with faulting and seismic activity include strong ground motion, ground surface rupture, and liquefaction. These considerations and other potential geologic hazards such as tsunamis and landsliding are discussed in the following sections.

8.1. Faulting and Seismicity

Based on our review of the referenced geologic map (Kennedy and Tan, 2008) and stereoscopic aerial photographs, as well as on our geologic field mapping, the subject site is not underlain by known active or potentially active faults (i.e., faults that exhibit evidence of ground displacement in the last 11,000 years and 2,000,000 years, respectively). Like the majority of southern California, the site is located in a seismically active area and the potential for strong ground motion is considered significant during the design life of the proposed structures. The nearest known active fault, the Rose Canyon fault is located approximately 6 miles west of the site. Additionally, the La Nacion Fault system has been mapped approximately 900 feet west of the site per the City of San Diego Safety Element (2008). However, the La Nacion Fault is not considered an active fault by the State of California.

8.1.1. Strong Ground Motion

The 2016 California Building Code (CBC) specifies that the Risk-Targeted, Maximum Considered Earthquake (MCE_R) ground motion response accelerations be used to evaluate seismic loads for design of buildings and other structures. The MCE_R ground motion response accelerations are based on the spectral response accelerations for 5 percent damping in the direction of maximum horizontal response and incorporate a target risk for structural collapse equivalent to 1 percent in 50 years with deterministic limits for near-source effects. The horizontal peak ground acceleration (PGA) that

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corresponds to the MCE_R for the site was calculated as 0.39g using the United States Geological Survey (USGS, 2017) seismic design tool (web-based).

The 2016 CBC specifies that the potential for liquefaction and soil strength loss be evaluated, where applicable, for the Maximum Considered Earthquake Geometric Mean (MCE_G) peak ground acceleration with adjustment for site class effects in accordance with the American Society of Civil Engineers (ASCE) 7-10 Standard. The MCE_G peak ground acceleration is based on the geometric mean peak ground acceleration with a 2 percent probability of exceedance in 50 years. The MCE_G peak ground acceleration with adjustment for site class effects (PGA_M) was calculated as 0.393g using the USGS (USGS, 2017) seismic design tool that yielded a mapped MCE_G peak ground acceleration of 0.389g for the site and a site coefficient (F_{PGA}) of 1.011 for Site Class C.

8.1.2. Ground Surface Rupture

Ground surface rupture due to active faulting is not considered likely in the project area due to the absence of any known active faults underlying the site. However, lurching or cracking of the ground surface as a result of nearby seismic events is possible.

8.1.3. Liquefaction and Seismically Induced Settlement

Liquefaction of cohesionless soils can be caused by strong vibratory motion due to earthquakes. Research and historical data indicate that loose granular soils and nonplastic silts that are saturated by a relatively shallow groundwater table are susceptible to liquefaction. Based on the relatively dense nature of the underlying earth materials, it is our opinion that the potential for liquefaction and seismically induced settlement to occur at the site is not a design consideration.

8.2. Tsunamis

Tsunamis are long wavelength seismic sea waves (long compared to the ocean depth) generated by sudden movements of the ocean bottom during submarine earthquakes, landslides, or volcanic activity. Seiches are similar oscillating waves on inland or enclosed bodies of

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water. Based on the inland location and elevation of the site, the potential for a tsunami or seiche to affect the site is not a design consideration.

8.3. Flood Hazards

Based on review of a Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map (FIRM) (FEMA, 2012), the site is mapped in an area labeled as "Other Areas: Zone X: Areas determined to be outside the 0.2% annual chance floodplain" that means the site is lying outside of mapped 100- and 500-year flood zones. Based on this review, the potential for flood-ing of the site is considered low.

8.4. Landsliding

Based on our review of referenced geologic maps, literature, topographic maps, and stereoscopic aerial photographs, no landslides or indications of deep-seated landsliding are known to underlie the project site. Although the bedding of the Mission Valley Formation does possess a slight inclination of approximately 2 to 4 degrees to the south, the potential for significant large-scale slope instability of the south facing slope and the site is not a design consideration.

8.5. City of San Diego Seismic Safety Study

As part of our evaluation, we have reviewed the 2008 City of San Diego Seismic Safety Study (Figure 7). The site is located in Category 53 (considered an area of "Level or sloping terrain, unfavorable geologic structure, low to moderate risk", as indicated on Figure 7.

Additionally, the La Nacion Fault system has been mapped approximately 900 feet west of the site per the City of San Diego Safety Element (2008). However, the La Nacion Fault is not considered an active fault by the State of California.

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9. CONCLUSIONS

Based on our review of the referenced background data, subsurface exploration, and laboratory testing, it is our opinion that construction of the proposed improvements is feasible from a geotechnical standpoint provided the recommendations presented in this report are incorporated into the design and construction of the project. In general, the following conclusions were made:

- The project site is underlain by fill and materials of the Mission Valley Formation.
- The existing fills are considered unsuitable for support of structures and compacted fills in their current condition. Recommendations for remedial grading of the existing fill materials are presented in the following sections.
- Competent materials of the Mission Valley Formation are considered suitable for support of new structures and compacted fills.
- Soils derived from the existing on-site fills are anticipated to generate clayey soils that possess a medium to high potential for expansion. These clayey soils possess a medium to very high potential for expansion are not suitable for reuse in compacted fills within the building pads, behind retaining walls, or beneath concrete sidewalks. Due to the anticipated export of site soils, the expansive soils should be selectively graded and removed from the site.
- Soils derived from excavations into the Mission Valley Formation are generally considered suitable for reuse as compacted fill.
- Difficult drilling conditions were encountered during the excavation of our borings. Due to the potential presence of strongly cemented zones or concretions within the Mission Valley Formation, the contractor should anticipate encountering difficult excavating or drilling conditions that may require heavy ripping or coring.
- Groundwater was not encountered in borings at the time of drilling. Based on review of monitoring well data in the site vicinity using the Geotracker website (2017), groundwater has been encountered at elevations ranging from approximately 276 to 280 feet above MSL (Morgan & Associates, 2008 and Murex Environmental, 2011).
- The active Rose Canyon fault is located approximately 6 miles west of the site. Accordingly, the potential for relatively strong seismic ground motions should be considered in the project design.
- Based on the results of our soil corrosivity tests presented in Appendix C, we consider the site soils to be corrosive, specifically to ferrous metals.



10. RECOMMENDATIONS

Based on our understanding of the project, the following recommendations are provided for the proposed design and construction of the new mixed-use buildings and the associated site improvements. The proposed site improvements should be constructed in accordance with the re-requirements of the applicable governing agencies.

10.1. Earthwork

In general, earthwork should be performed in accordance with the recommendations presented in this report. Ninyo & Moore should be contacted for questions regarding the recommendations or guidelines presented herein.

10.1.1. Site Preparation

Site preparation should begin with the removal of existing foundations, other structures and improvements, vegetation, utility lines, asphalt, concrete, and other deleterious debris from areas to be graded. Tree stumps and roots should be removed to such a depth that organic material is generally not present. Clearing and grubbing should extend to the outside of the proposed excavation and fill areas. The debris and unsuitable material generated during clearing and grubbing should be removed from areas to be graded and disposed of at a legal dumpsite away from the project area.

10.1.2. Remedial Grading of Existing Fills

We recommend that the existing fills at the site, if not removed during grading, be overexcavated down to competent materials of the Mission Valley Formation. Fill depths at the site varied from approximately 0 feet at borings B-1 and B-2 up to approximately 7 feet within boring IT-1. However, deeper fills may be present at the site. The extent and depths of removals and overexcavations should be evaluated by the geotechnical consultant's representative in the field based on the materials exposed. The resultant overexcavation surface should be scarified to a depth of approximately 8 inches, moisture conditioned and recompacted to a relative compaction of 90 percent as evaluated by the ASTM International (ASTM) Test Method D 1557 prior to placing new fill.

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10.1.3. Temporary Excavations

For temporary excavations, we recommend that the following Occupational Safety and Health Administration (OSHA) soil classifications be used:

Fill	Type C
Mission Valley Formation	Type A

Upon making the excavations, the soil classifications and excavation performance should be evaluated in the field by the geotechnical consultant in accordance with the OSHA regulations. Temporary excavations should be constructed in accordance with OSHA recommendations. For trench or other excavations, OSHA requirements regarding personnel safety should be met using appropriate shoring (including trench boxes) or by laying back the slopes to no steeper than 1½:1 (horizontal to vertical) in fill and ¾:1 in the Mission Valley Formation. Temporary excavations that encounter seepage may be shored or stabilized by placing sandbags or gravel along the base of the seepage zone. Excavations encountering seepage should be evaluated on a case-by-case basis. On-site safety of personnel is the responsibility of the contractor.

10.1.4. Excavation Characteristics

The results of our field exploration program indicate that the project site, as presently proposed, is underlain by fill and materials of the Mission Valley Formation. As noted earlier, very difficult drilling conditions were encountered during the drilling of our borings. Excavations that extend into the Mission Valley Formation should anticipate difficult excavating conditions due to concretions, cobbles, conglomerate, or strongly cemented zones that may require heavy ripping or coring.

10.1.5. Materials for Fill and Backfill

Materials for fill may be obtained from on-site excavations or may be import materials. Fill soils should possess an organic content of less than approximately 3 percent by volume (or 1 percent by weight). In general, fill material should not contain rocks or lumps over approximately 3 inches in diameter, and not more than approximately 30 percent larger than ³/₄ inch.

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Soils derived from the existing site fills are anticipated to generate clayey soils that possess a medium to high potential for expansion and should be exported, where feasible. Clayey soils that possess a medium to very high potential for expansion are not suitable for reuse in compacted fills within the building pads, behind retaining walls, or beneath concrete sidewalks. Soils derived from excavations into the Mission Valley Formation are generally considered suitable for reuse as compacted fill.

Imported fill materials, if needed, should generally be granular soils with very low to low expansion potential (i.e., an expansion index of 50 or less as evaluated by ASTM D 4829). Imported fill material should also be tested for corrosive potential and exhibit an resistivity value greater than 1,000 ohm-centimeters, a chloride content of less than 500 parts per million (ppm), a sulfate content of less than 1,000 ppm and pH greater than 5.5. The contractor should be responsible for the uniformity of import material brought to the site. Imported fill material should not contain rocks or lumps over approximately 3 inches in diameter, and not more than approximately 30 percent larger than 3⁄4 inch. We recommend that materials proposed for use as import fills be evaluated from a contractor's stockpile rather than in place materials.

10.1.6. Reuse of AC and Concrete Materials in Fill

We understand that there is consideration to reusing AC and concrete materials generated by on-site demolition within the engineered fills. AC and concrete materials to be reused in engineered fills should not have painted, stained, or coated surfaces, contain rebar or other metal reinforcement, vegetation, or other debris. The concrete and asphalt should be crushed to sizes of 3 inches or less. Crushed AC and concrete materials to be reused, should be stockpiled and blended with soil prior to placement. The mixture of crushed AC and concrete materials with soils should be blended and processed to meet the requirements of the "Materials for Fill" section of this report. Placement and compaction of these blended materials should be performed in accordance with the "Compacted Fill" section of this report. Additionally, the crushed AC and concrete materials should not be used beneath buildings. Furthermore, the crushed AC and concrete materials should not be used within 5 feet of finish grades for slope faces, beneath permeable pavements, or as retaining wall backfill. The use of these materials in vegetated areas should be done at the discretion of the landscape architect.

10.1.7. Compacted Fill

Prior to placement of compacted fill, the contractor should request an evaluation of the exposed ground surface by Ninyo & Moore. The evaluation of compaction by the geotechnical consultant should not be considered to preclude any requirements for observation or approval by governing agencies. It is the contractor's responsibility to notify this office and the appropriate governing agency when project areas are ready for observation, and to provide reasonable time for that review.

Fill materials should be moisture conditioned to generally above the laboratory optimum moisture content prior to placement. The optimum moisture content will vary with material type and other factors. Moisture conditioning of fill soils should be generally consistent within the soil mass.

Prior to placement of additional compacted fill material following a delay in the grading operations, the exposed surface of previously compacted fill should be prepared to receive fill. Preparation may include scarification, moisture conditioning, and recompaction.

Compacted fill should be placed in horizontal lifts of approximately 8 inches in loose thickness. Prior to compaction, each lift should be watered or dried as needed to achieve a moisture content generally above the laboratory optimum, mixed, and then compacted by mechanical methods, to a relative compaction of 90 percent as evaluated by ASTM D 1557. The upper 12 inches of subgrade soils beneath vehicular pavements should be compacted to a relative compaction of 95 percent as evaluated by ASTM D 1557. The aggregate base materials beneath vehicular pavements should also be compacted to a relative compaction of 95 percent as evaluated by ASTM D 1557. Successive lifts should be treated in a like manner until the desired finished grades are achieved.

10.1.8. Slopes

We anticipate that new cut and fill slopes will be constructed for the project. Unless otherwise recommended by our offices and approved by the regulating agencies, permanent cut and fill slopes should not be steeper than 2:1 (horizontal to vertical). Buildings, structures, and improvements should be set back from the top of slopes in accordance with the 2016 CBC. We recommend buildings and structures be set back 20 feet or more from the top of slopes.

Compaction of the face of fill slopes should be performed by backrolling at intervals of 4 feet or less in vertical slope height, or as dictated by the capability of the available equipment, whichever is less. Fill slopes should be overbuilt and cut back to finish grades. The placement, moisture conditioning, and compaction of fill slope materials should be done in accordance with the recommendations presented herein.

Site runoff should not be permitted to flow over the tops of slopes. Positive drainage should be established away from the top of slopes. This may be accomplished by utilizing brow ditches placed at the top of slopes to divert surface runoff away from the slope face where drainage devices are not otherwise available.

The on-site soils are susceptible to erosion. The project plans and specifications should contain design features and construction requirements to mitigate erosion of soils or contain a maintenance program to redress erosion features as they develop on a periodic basis.

10.1.9. Pipe Bedding and Modulus of Soil Reaction (*E'*)

It is our recommendation that the new pipeline (pipes), where constructed in open excavations, be supported on 6 or more inches of granular bedding material. Granular pipe bedding should be provided to distribute vertical loads around the pipe. Bedding material and compaction requirements should be in accordance with this report. Pipe bedding typically consists of graded aggregate with a coefficient of uniformity of three or more. The pipe bedding should conform to the specifications presented for pipe zone backfill materials.



Pipe bedding and pipe zone backfill should have a Sand Equivalent of 30 or more, and be placed around the sides and the crown of the pipe. In addition, the pipe zone backfill should extend 1 foot or more above the crown of the pipe. If open-graded gravel is used as pipe zone backfill, we recommend that the pipe bedding and pipe zone materials be wrapped in a non-woven geotextile fabric.

The modulus of soil reaction (E') is used to characterize the stiffness of soil backfill placed at the sides of buried flexible pipes for the purpose of evaluating deflection caused by the weight of the backfill over the pipe (Hartley and Duncan, 1987). A soil reaction modulus of 1,200 pounds per square inch (psi) may be used for design provided that granular bedding material is placed adjacent to the pipe, as recommended in this report.

10.1.10. Utility Trench Zone Backfill

Trench zone backfill should generally be free of rocks or hard lumps of material in excess of 3 inches in diameter. Rocks or hard lumps larger than about 3 inches in diameter should be broken into smaller pieces or should be removed from the site. On-site trench excavations may generate cobbles larger than 3 inches in diameter. Oversize materials should be separated from material to be used as trench backfill. Moisture conditioning (including drying and/or mixing) of existing on-site materials is anticipated if reused as trench backfill. Additionally, soils that possess a medium to high potential for expansion should not be used in trench zone backfill beneath buildings, beneath concrete sidewalks, or within retaining wall backfills.

10.2. Seismic Design Considerations

Design of the proposed improvements should be performed in accordance with the requirements of governing jurisdictions and applicable building codes. Table 2 presents the seismic design parameters for the sites in accordance with the CBC (2016) guidelines and adjusted MCE_R spectral response acceleration parameters (USGS, 2017).

Factors	Values
Site Class	С
Site Coefficient, F _a	1.018
Site Coefficient, F _v	1.436
Mapped Spectral Response Acceleration at 0.2-second Period, S _s	0.955g
Mapped Spectral Response Acceleration at 1.0-second Period, S ₁	0.364g
Spectral Response Acceleration at 0.2-second Period Adjusted for Site Class, S_{MS}	0.972g
Spectral Response Acceleration at 1.0-second Period Adjusted for Site Class, S _{M1}	0.523g
Design Spectral Response Acceleration at 0.2-second Period, S _{DS}	0.648g
Design Spectral Response Acceleration at 1.0-second Period, S _{D1}	0.349g

Table 2 – 2016 California Building Code Seismic Design Criteria

10.3. Foundations

The proposed residential and commercial buildings may be supported on shallow, spread, or continuous footings bearing entirely on competent materials of the Mission Valley Formation. Foundations should be designed in accordance with structural considerations and the following recommendations. In addition, requirements of the appropriate governing jurisdictions and applicable building codes should be considered in the design of the structures.

10.3.1. Shallow Foundations

Shallow, spread, or continuous footings bearing entirely on competent materials of the Mission Valley Formation may be designed using an allowable bearing capacity of 4,000 pounds per square foot (psf) based on the embedment depths described below. These allowable bearing capacities may be increased by one-third when considering loads of short duration such as wind or seismic forces. Shallow, spread, or continuous footings should be embedded 36 inches below finished building pad subgrade elevation and should have a width of 24 inches or more. From a geotechnical standpoint, footings should be reinforced with three No. 5 reinforcing bars at the top and bottom. The footing reinforcing should be designed by the project structural engineer.

If required by the topography of the site or due to fill thickness, portions of the building foundations may need to be deepened to bear on the Mission Valley Formation. As an alternative method to stepping down and deepening the footings, the deepened portions of the foundation excavations more than 36 inches below finished pad subgrade elevation

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may be backfilled with controlled low-strength material (CLSM) to the bottom elevation of the concrete footing. For this alternative, footings may bear on a controlled low strength material (CLSM) backfill with a compressive strength of 150 pounds per square inch (psi) according to "Greenbook," Section 201-6 specifications. CLSM backfill should extend down to Normal Heights Mudstone.

10.3.2. Shallow Foundation Lateral Earth Pressures

For resistance of footings to lateral loads, we recommend an allowable passive pressure of 400 psf per foot of depth be used with a value of up to 4,000 psf. This value assumes that the ground is horizontal for a distance of 10 feet, or three times the height generating the passive pressure, whichever is greater. We recommend that the upper 1 foot of soil not protected by pavement or a concrete slab be neglected when calculating passive resistance.

For frictional resistance to lateral loads, we recommend a coefficient of friction of 0.35 be used between soil and concrete. The passive resistance values may be increased by one-third when considering loads of short duration such as wind or seismic forces.

10.3.3. Static Settlement

We estimate that the proposed apartment building, designed and constructed as recommended herein, will undergo total settlements of less than approximately $\frac{3}{4}$ inch. Differential settlement on the order of $\frac{1}{2}$ inch over a horizontal span of 40 feet should be expected.

10.4. Interior Building Slabs-on-Grade

We recommend that conventional, interior building slab-on-grade floors, underlain by compacted fill materials of generally very low to low expansion potential, be 5 inches in thickness and be reinforced with No. 4 reinforcing bars spaced 12 inches on center each way. The reinforcing bars should be placed near the middle of the slab height. As a means to help reduce shrinkage cracks, we recommend that the slabs be provided with crack control joints at intervals of approximately 12 feet each way. The slab reinforcement and expansion joint spacing should be designed by the project structural engineer.

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If moisture sensitive floor coverings are to be used, we recommend that slabs be underlain by a vapor retarder and capillary break system consisting of a 10-mil polyethylene (or equivalent) membrane placed over 4 inches of medium to coarse, clean sand or pea gravel.

10.5. Retaining Walls

We understand that several retaining walls will be used as part of the project. Retaining walls will be constructed along the north, west, and east property lines along with various other site retaining walls. Additionally, some site retaining walls will be incorporated into the contraction of the buildings. For this project, recommendations for various types of retaining walls, including standard (concrete and masonry), soldier beam and lagging, and segmental block (geogrid) walls have been considered. The following sections provide recommendations for the various types of retaining walls considered.

10.5.1. Standard Retaining Walls

For the purpose of this report, cast-in-place and masonry retaining walls supported on shallow, continuous foundations are considered to be standard retaining walls. Due to site constraints, standard retaining walls are not considered suitable for use along the northern and western property lines.

Standard retaining walls may be supported on a continuous footing wholly bearing on competent materials of the Mission Valley Formation. Allowable bearing capacities of 4,000 psf may be used for the design of retaining wall foundations. The allowable bearing capacity may be increased by one-third when considering loads of short duration, such as wind or seismic forces. Note, for standard retaining walls that are incorporated or tied into the proposed materials, we recommend that the wall foundations bear on competent materials of the Mission Valley Formation, similar to those recommended for building foundations and not on compacted fills.

For the design of a yielding standard retaining wall that is not restrained against movement by rigid corners or structural connections, lateral pressures are presented on Figure 8. Restrained walls (non-yielding) may be designed for lateral pressures presented on Figure 9. These pressures assume low-expansive backfill and free draining conditions. Measures should be taken to reduce the potential for build-up of moisture behind the retaining walls. A drain should be provided behind the retaining wall as shown on Figure 10. The drain should be connected to an appropriate outlet.

10.5.2. Permanent Soldier Beam and Lagging Walls

Due to space constraints, we recommend that the retaining walls to be constructed on the northern and western property lines consist of permanent soldier beam and lagging walls. For the design of a permanent soldier pile wall, the wall may consist of steel Hpiles installed in drilled holes with a reinforced shotcrete façade or lagging placed between the soldier piles. The design and construction of the permanent soldier pile wall should be in accordance with the following recommendations.

The construction sequence of the wall should be such that the soldier piles are installed prior to the start of the partial removal of the slope. We recommend that the steel H-piles be placed within pre-drilled holes. Once the steel H-piles are placed within the pre-drilled holes, we recommend that concrete be placed within the embedment zone. The annular space above the concrete embedment may be filled with an excavatable slurry. Once the concrete and slurry are allowed to cure, excavation of the existing slope in front of the shafts may begin. Thus, the drilled shafts will provide temporary stability of cuts during construction and will act as shoring elements.

The permanent soldier pile wall should be designed for a static safety factor of 1.5 and the lateral deformation of the ground surface should be controlled by structural design in order to reduce the potential for damage to the adjacent structures. The wall should be designed to support surcharge loads of any adjacent structures (if present), in addition to the earth pressures exerted by the retained soils as presented on Figure 11. The key design parameters and considerations for the soldier piles are as follows:

- The drilled shafts should be 24 inches or more in diameter. The embedment depths and spacing of the shafts should be evaluated by the project structural engineer based on the estimated total service (dead and live) and lateral loads. The shafts should be installed at a center-to-center spacing of three diameters or more.
- The drilled shafts should be designed to resist lateral earth pressures linearly increasing at the rates presented on Figure 11. In calculating the total lateral load acting on a given shaft, the spacing between adjacent shafts should be considered as the wall span length. For example, for a shaft center-to-center spacing of 8 feet, a wall span length of 8 feet should be considered in design. The drilled shafts should be designed assuming a free-head condition (i.e., unrestrained at the top), and the allowable head deflection should not exceed ¹/₄ inch.
- A passive earth pressure linearly increasing at the rate presented on Figure 11 may be used to evaluate the lateral resistance for drilled shafts. For evaluation of lateral deflection of the drilled shaft, a subgrade modulus of 350 pounds per cubic inch (pci) may be considered.
- The soldier piles may be designed with a soil arching three times the diameter of the pile itself.
- We recommend concrete be placed by tremie method to mitigate the potential for aggregate and cement segregation during concrete placement.
- Pre-drilling operations should be observed by Ninyo & Moore to confirm the embedment depths designed by the project structural engineer. The drilled holes should be cleared of loose soil and/or construction debris prior to placing concrete. Steel H-piles and concrete should be placed the same day the holes are drilled.

The wall details should be included in the project plans. The project plans should be signed and stamped by a professional engineer registered in the state of California. Ninyo & Moore should be given the opportunity to review the project plans to check for compliance with design and construction recommendations presented herein.

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10.5.3. Segmental Block Retaining Walls

Due to site constraints, segmental block retaining walls are not considered suitable for use along the northern and western property lines. Segmental block retaining walls consist of stackable concrete facing blocks combined with geogrid reinforcing elements embedded in the soil backfill.

Segmental block retaining walls may be designed using the soil parameters presented in Table 3 below, provided the backfill (reinforced soil) materials are generally granular, with 100 percent passing the 1-inch sieve, no more that 35 percent passing the No. 200 sieve, possess an expansion index less than 50, and have a plasticity index (PI) of 20 or less. The following parameters are based on the assumption that materials derived from on-site excavations in the Mission Valley Formation will be reused as reinforced soil behind the segmental block retaining wall. The existing on-site fill materials that are clayey and possess a medium to high potential for expansion are not suitable for reuse as reinforced soil backfill.

Wall Zone	Cohesion (psf)	Friction Angle (deg.)	Unit Weight (psf)
Reinforced Soil	0	30	120
Retained Soil	200	30	120
Foundation Soil	200	30	120

Table 3 – Segmental Retaining Wall Design Parameters

10.6. Exterior Pedestrian Concrete Flatwork

Exterior concrete flatwork (sidewalks) should be 4 inches in thickness and should be reinforced with No. 3 reinforcing bars placed at 18 inches on-center both ways. This assumes that the sidewalks are underlain by materials that possess a very low to low expansion index. No vapor retarder is needed for exterior concrete flatwork. To reduce the potential manifestation of distress to exterior concrete flatwork due to movement of the underlying soil, we recommend that such flatwork be installed with crack-control joints at appropriate spacing as designed by the project engineer. The subgrade soils should be scarified to a depth of



8 inches, moisture conditioned to generally above the laboratory optimum moisture content, and compacted to a relative compaction of 90 percent as evaluated by ASTM D 1557. Positive drainage should be established and maintained adjacent to flatwork.

10.7. Flexible Vehicular Pavements

Our laboratory testing indicated the site soils have R-values of 5 and 8. Accordingly, we have used a design R-value of 5 and Traffic Indices (TI) of 5 through 7 for the basis of preliminary design of flexible pavements for the project. However, actual pavement recommendations should be based on R-value tests performed on bulk samples of the soils exposed at the finished subgrade elevations during grading operations. We recommend that the geotechnical consultant re-evaluate the pavement design at the time of construction. The recommended preliminary flexible pavement sections for on-site areas should be as presented in Table 4. Off site pavements should be constructed in accordance with the City of San Diego guidelines.

Traffic Index (Pavement Usage)	Design R-Value	Asphalt Concrete (in)	Class 2 Aggregate Base (in)
5 (Parking Stalls)	5	3	10
6 (Drive Aisles)	5	4	12
7 (Fire Lanes and Delivery Routes)	5	4	16

 Table 4 – Recommended Preliminary Flexible Pavement Sections

These values assume traffic indices of seven or less for site pavements. In addition, we recommend that the upper 12 inches of the subgrade and aggregate base materials be compacted to a relative compaction of 95 percent relative density as evaluated by the current version of ASTM D 1557. The AC materials should be compacted to a relative compaction of 95 percent as evaluated by the materials Hveem density. If traffic loads are different from those assumed, the pavement design should be re-evaluated.

10.8. Rigid Concrete Pavements

For the design of rigid concrete pavements, we have used a design R-value of 5 and TIs of 5 through 8. However, actual pavement recommendations should be based on R-value tests performed on bulk samples of the soils exposed at the finished subgrade elevations during grading operations. We recommend that the geotechnical consultant re-evaluate the pavement design at the time of construction. The recommended preliminary rigid pavement sections for on-site areas should be as presented in Table 5. Off site pavements should be constructed in accordance with the City of San Diego guidelines.

Traffic Index (Pavement Usage)	Design R-Value	Portland Cement Concrete (in)	Class 2 Aggregate Base (in)
5 (Parking Stalls)	5	5	4
6 (Drive Aisles)	5	6	6
7 (Fire Lanes and Delivery Routes)	5	7	6
8 (Trash Enclosures)	5	8	6

 Table 5 – Recommended Preliminary Rigid Pavement Sections

10.9. Pervious Pavements

Although specifics have not been provided to our office, we understand that the project may include the design and construction of pervious pavements. In general, pervious pavements consist of a permeable layer of AC or concrete underlain by a rock reservoir layer. From a geotechnical standpoint, we recommend that the rock reservoir layer be 14 inches or more thick. Furthermore, we recommend that the rock reservoir layer consist of open-graded gravel that meets the gradation limits for one of the following:

- Size Number 2 and 3 materials per ASTM C33;
- Size Number 2, 24, and 3 materials per American Association of State Highway and Transportation Officials (AASHTO) M43;

- Caltrans Class 4 permeable;
- Or an approved equivalent.

Furthermore, we recommend that the pervious AC and/or concrete pavements be designed and constructed in accordance with the following recommendation sections and the Caltrans Pervious Pavement Design Guidance manual (2014b).

10.9.1. Pervious AC Pavements

In the event pervious AC pavements are used, we recommend that pervious AC pavement sections consist of a non-structural wearing course consisting of a Caltrans opengraded friction course (OGFC) underlain by a Caltrans asphalt treated permeable base (ATPB) layer. For the design of the pervious AC section we have used a design R-value of 5. This design R-value, along with estimated TI values of 5 through 7, are the basis of our pervious AC pavement design. The recommended pervious AC pavement sections are presented in Table 6.

Traffic Index (Pavement Usage)	Design R-Value	OGFC Thickness (in)	ATPB Thickness (in)	Rock Reservoir Thickness ¹ (in)		
5 (Parking Stalls)	5	2	4	14 or more		
6 (Drive Aisles)	5	2	41⁄2	16 or more		
7 (Fire Lanes and Delivery Routes)	5	2	5	18 or more		
Notes: ATPB = Caltrans Asphalt-Treated Permeable Base OGFC = Caltrans Open-Graded Friction Course ¹ Minimum recommended thickness. Thickness of reservoir should be evaluated by the project civil engineer based on capacity demands.						

 Table 6 – Recommended Pervious Asphalt Concrete Pavement Sections

10.9.2. Pervious Concrete Pavements

We recommend that pervious concrete pavement sections consist of pervious concrete underlain by a Caltrans ATPB layer. The purpose of the ATPB layer is to act as a choker course between the pervious concrete and the rock reservoir. For the design of the per-



vious concrete sections we have used a design R-value of 5. This design R-value, along with estimated TI values of 5 and 7 is the basis of our pervious concrete pavement design. The recommended pervious concrete pavement sections are presented in Table 7.

Traffic Index (Pavement Usage)	Design R-Value	Pervious Concrete Thickness (in)	ATPB Thickness (in)	Rock Reservoir Thickness ¹ (in)		
5 (Parking Stalls)	5	6	3	14 or more		
6 (Drive Aisles)	5	7	3	16 or more		
7 (Fire Lanes and Delivery Routes)	5	8	3	18 or more		
Notes: ATPB = Caltrans Asphalted Permeable Base						

Table 7 – Recommended Pervious Concrete Pavement Sections

¹Minimum recommended thickness. Thickness of reservoir should be evaluated by the project civil engineer based on capacity demands.

10.9.3. Construction Considerations

The pervious pavements should generally be constructed on relatively undisturbed and uncompacted native subgrade materials. However, in the event that the design finish surface elevations are such that the reservoir layer is underlain by fill, the project civil engineer should anticipate and design for a reduced infiltration rate. Construction traffic and equipment should not disturb the exposed subgrade conditions once excavated.

Subsequent to excavation to subgrade elevation, the reservoir rock should be placed and spread over the relatively undisturbed and uncompacted subgrade materials. Low-pressure construction equipment should be used to lightly compact the rock reservoir materials in 6 to 12 inch lifts. Following the installation of the rock reservoir, the ATPB, OGFC, and/or pervious concrete materials may be placed. The pervious pavements should be constructed by an experienced and qualified specialty contractor.

Additionally, we recommend that the pervious pavements be separated from the adjacent conventional pavements. The separation may consist of a modified curb in accordance with Section 2.4 of the Caltrans guidelines (2014b) or a non-woven geofabric (i.e., Mirafi 140N or an approved equivalent).

Pervious pavements may be subject to reduced performance due to the accumulation of debris and sediment if not maintained. In order to provide continued performance of the pervious pavement system, we recommend that a maintenance plan be prepared, adopted, and performed on a routine basis. The Caltrans guidelines (2014b) may be referenced and used for additional recommendations and/or specific maintenance considerations.

10.10. Corrosive Soils

Laboratory testing to evaluate pH, electrical resistivity, soluble sulfate and chloride contents was performed on a representative sample of the near-surface soils. The pH and electrical resistivity tests were performed in accordance with California Test (CT) Method 643. Soluble sulfate and chloride content tests were performed in accordance with CT Methods 417 and 422, respective-ly. The results of the corrosivity tests are summarized below and presented in Appendix C.

The results of the corrosivity testing indicated soil pH values of approximately 7.1 to 8, electrical resistivities on the order of 290 to 710 ohm-cm, sulfate contents of approximately 0.032 to 0.070 percent (i.e., 320 to 700 parts per million [ppm]), and chloride contents of about 90 to 1,590 ppm for the tested samples. Based on the Caltrans (2015) criteria, ACI 318, and our experience with similar soils, the tested soils would be classified as corrosive due to the electrical resisitivity being less than 1,000 ohm-cm and the chloride contents in excess of 500 ppm. A corrosive soil is defined as having an electrical resistivity of less than 1,000 ohm-cm, more than 500 ppm of chlorides, more than 0.1 percent sulfates and/or a pH less than 5.5. Based on the results of our laboratory testing, the site soils would be corrosive to ferrous metals. We recommend that a corrosion engineer be consulted for the project.

10.11. Concrete

Concrete in contact with soil or water that contains high concentrations of soluble sulfates can be subject to chemical deterioration. Laboratory testing indicated sulfate contents of the samples tested of 0.032 to 0.070 percent by weight. Based on ACI 318, the potential for sulfate attack is negligible for water-soluble sulfate contents in soil ranging from 0.00 to 0.10 percent by weight. Thus, the sulfate exposure to concrete from near-surface site soils is considered negligible. However, we recommend that the use of Type II, V, or II/V cement be considered for concrete in contact with soil on the project due to potential for variable soil conditions at the site.

10.12. Site Drainage

Surface drainage should be provided to convey water away from structures and off pavement surfaces. Surface water should not be permitted to drain toward the structures or to pond adjacent to footings or on paved areas. Positive drainage is defined as a slope of 2 percent or more over a distance of 5 feet or greater away from the structures. Roof gutters should be installed on structures. Downspouts should discharge to controlled drainage systems away from structures, pavements, and flatwork.

10.13. Infiltration Devices

Although specifics have not been provided to our office, we anticipate that the project may include the construction of pervious pavements, bio-retention swales, and/or other infiltration devices. We recommend that the site design include the use of pavement edge drains and cutoff curbs to reduce the potential for lateral migration of water from the infiltration devices. We also recommend that infiltration devices be set back approximately 20 feet from future structures. Gravel backfill should generally be fully wrapped with a non-woven filter fabric (such as Mirafi 140N), to reduce the potential for fines to migrate to the voids in the gravel.

10.14. Pre-Construction Meeting

We recommend that a pre-construction meeting be held prior to commencement of grading. The owner or his representative, the agency representatives, the architect, the civil engineer, Ninyo & Moore, and the contractor should be in attendance to discuss the plans, the project, and the proposed construction schedule.

10.15. Plan Review and Construction Observation

The conclusions and recommendations presented in this report are based on analysis of observed conditions in widely spaced exploratory excavations. If conditions are found to vary from those described in this report, Ninyo & Moore should be notified, and additional recommendations will be provided upon request. Ninyo & Moore should review the final project drawings and specifications prior to the commencement of construction. Ninyo & Moore should perform the needed observation and testing services during construction operations.

The recommendations provided in this report are based on the assumption that Ninyo & Moore will provide geotechnical observation and testing services during construction. In the event that it is decided not to utilize the services of Ninyo & Moore during construction, we request that the selected consultant provide the owner with a letter (with a copy to Ninyo & Moore) indicating that they fully understand Ninyo & Moore's recommendations, and that they are in full agreement with the design parameters and recommendations contained in this report. Construction of proposed improvements should be performed by qualified subcontractors utilizing appropriate techniques and construction materials.

11. LIMITATIONS

The field evaluation, laboratory testing, and geotechnical analyses presented in this geotechnical report have been conducted in general accordance with current practice and the standard of care exercised by geotechnical consultants performing similar tasks in the project area. No warranty, expressed or implied, is made regarding the conclusions, recommendations, and opinions presented in this report. There is no evaluation detailed enough to reveal every subsurface condition.

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Variations may exist and conditions not observed or described in this report may be encountered during construction. Uncertainties relative to subsurface conditions can be reduced through additional subsurface exploration. Additional subsurface evaluation will be performed upon request. Please also note that our evaluation was limited to assessment of the geotechnical aspects of the project, and did not include evaluation of structural issues, environmental concerns, or the presence of hazardous materials.

This document is intended to be used only in its entirety. No portion of the document, by itself, is designed to completely represent any aspect of the project described herein. Ninyo & Moore should be contacted if the reader requires additional information or has questions regarding the content, interpretations presented, or completeness of this document.

This report is intended for design purposes only. It does not provide sufficient data to prepare an accurate bid by contractors. It is suggested that the bidders and their geotechnical consultant perform an independent evaluation of the subsurface conditions in the project areas. The independent evaluations may include, but not be limited to, review of other geotechnical reports prepared for the adjacent areas, site reconnaissance, and additional exploration and laboratory testing.

Our conclusions, recommendations, and opinions are based on an analysis of the observed site conditions. If geotechnical conditions different from those described in this report are encountered, our office should be notified and additional recommendations, if warranted, will be provided upon request. It should be understood that the conditions of a site could change with time as a result of natural processes or the activities of man at the subject site or nearby sites. In addition, changes to the applicable laws, regulations, codes, and standards of practice may occur due to government action or the broadening of knowledge. The findings of this report may, therefore, be invalidated over time, in part or in whole, by changes over which Ninyo & Moore has no control.

This report is intended exclusively for use by the client. Any use or reuse of the findings, conclusions, and/or recommendations of this report by parties other than the client is undertaken at said parties' sole risk.

Ninyo « Moore

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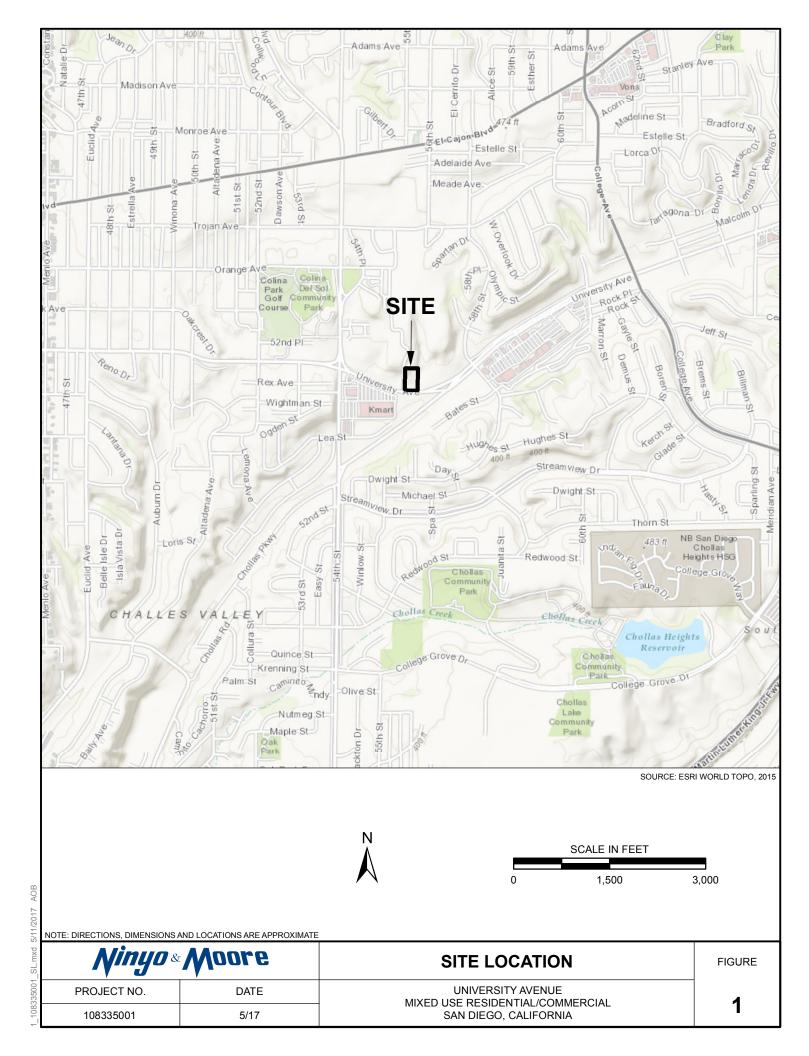
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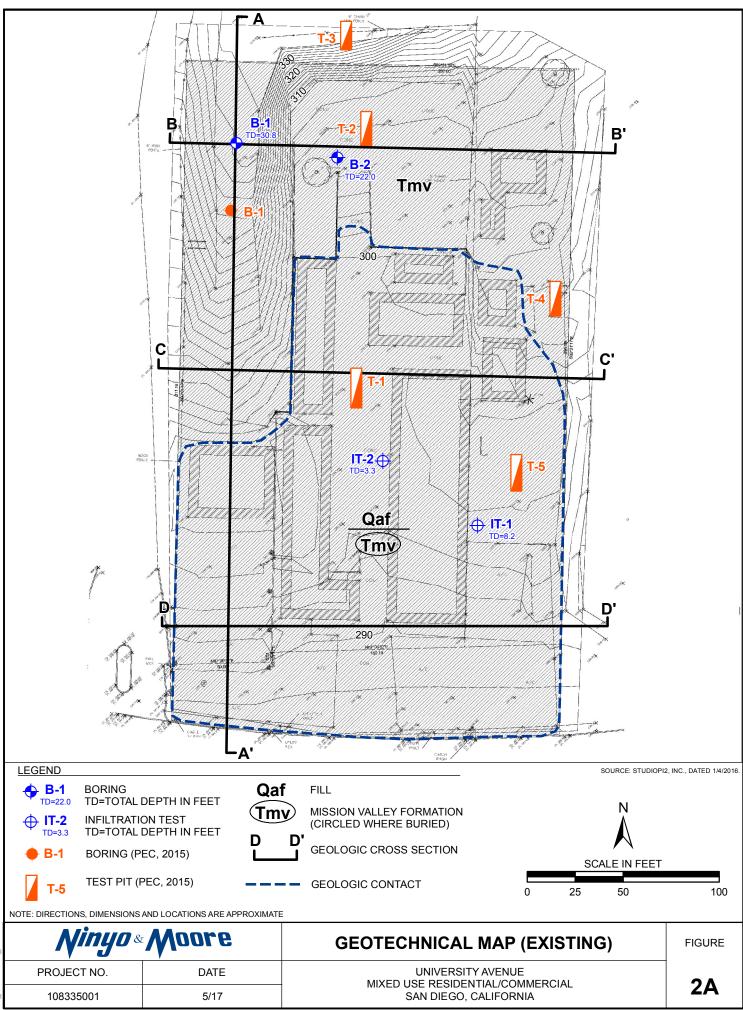
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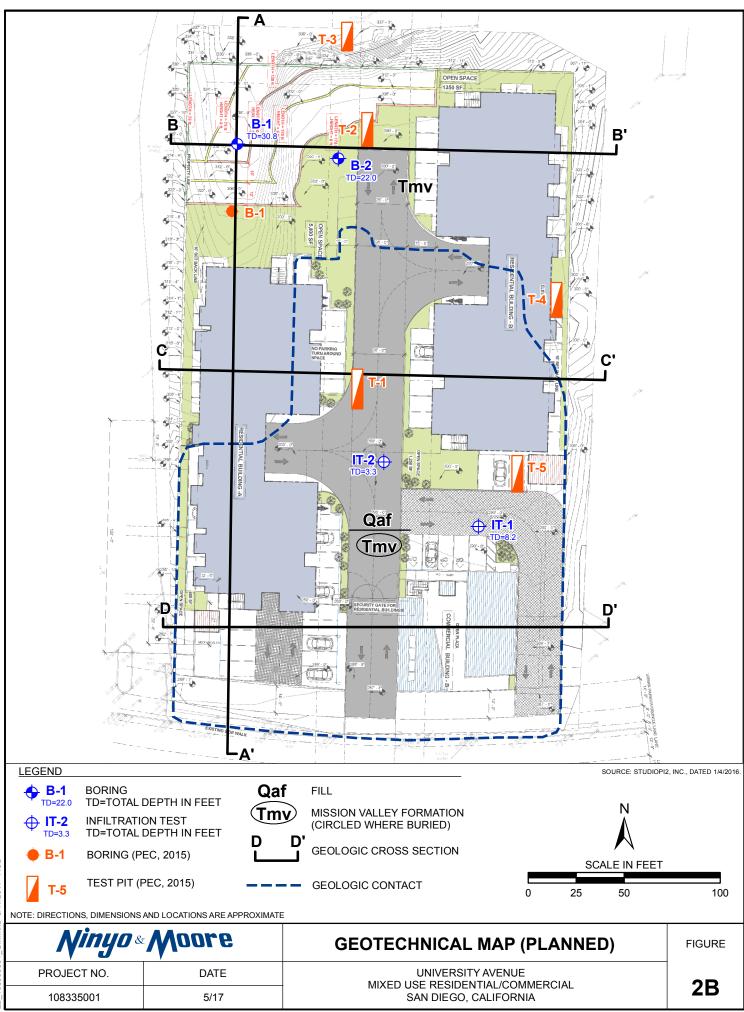
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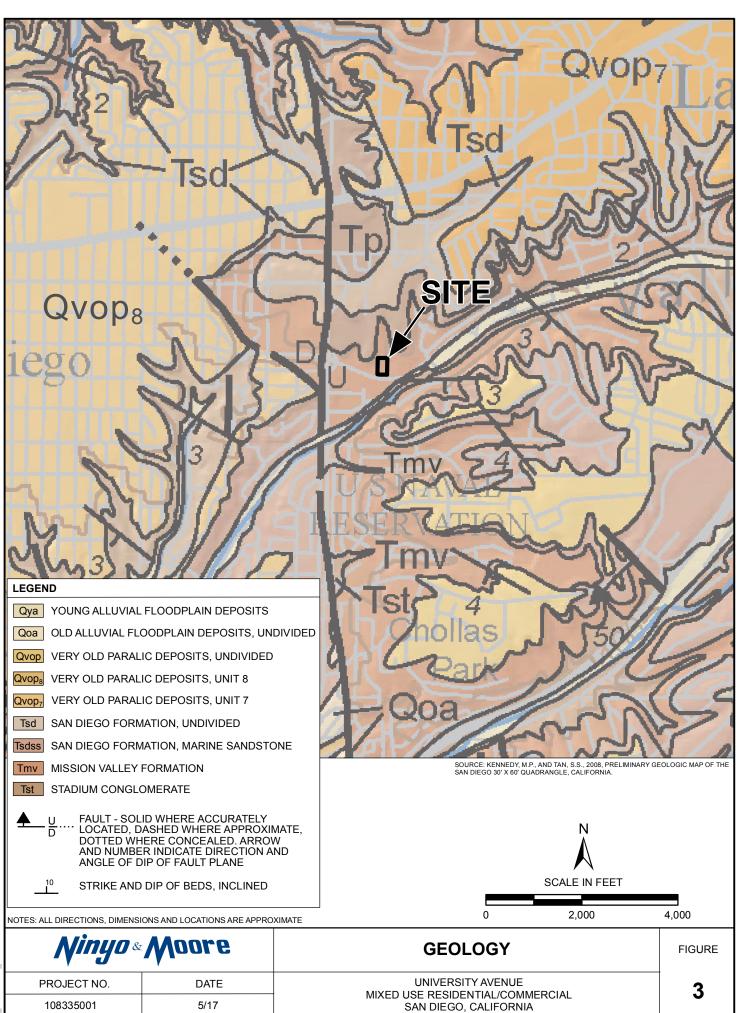
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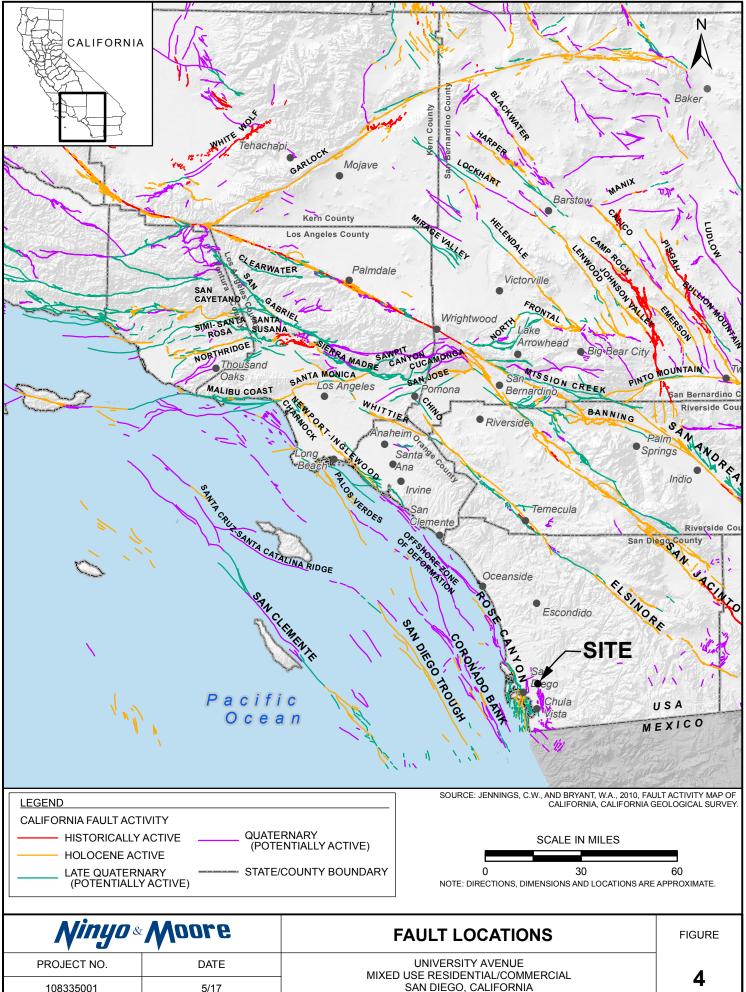


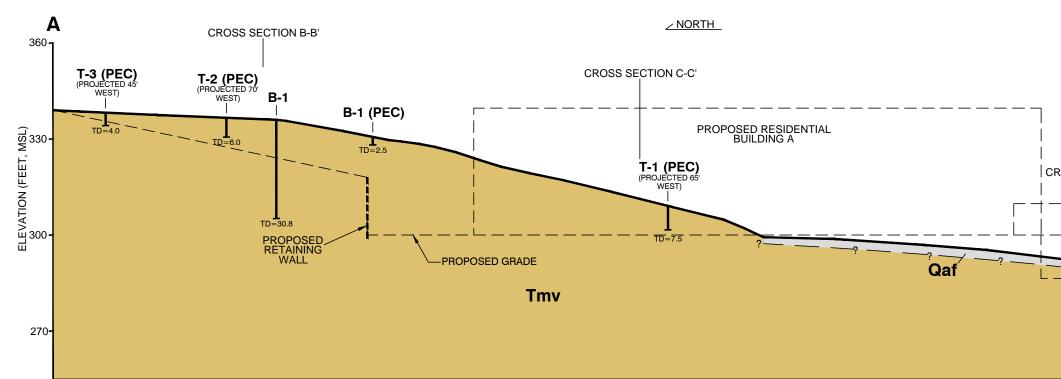


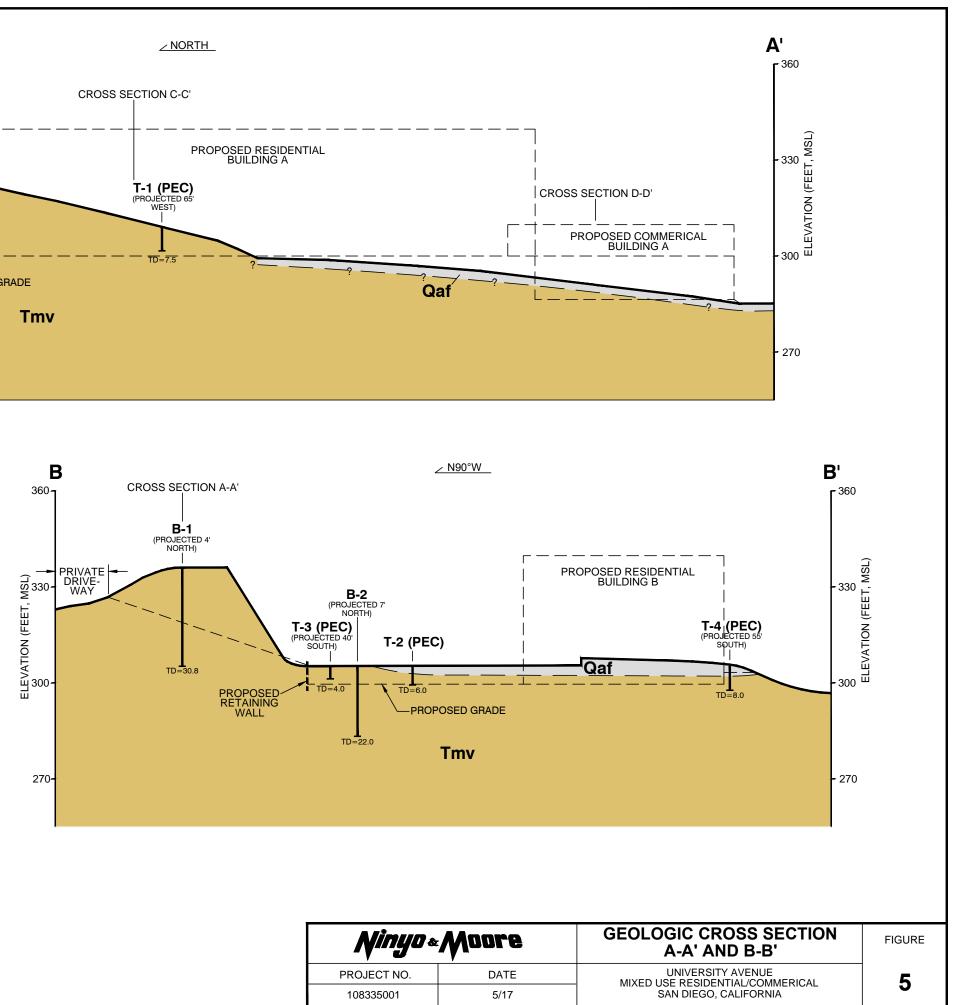
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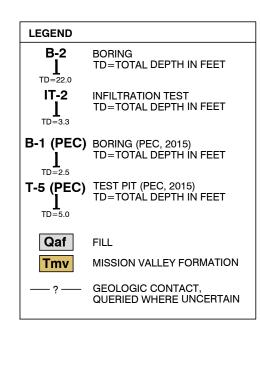


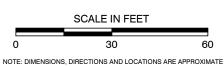




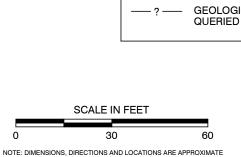


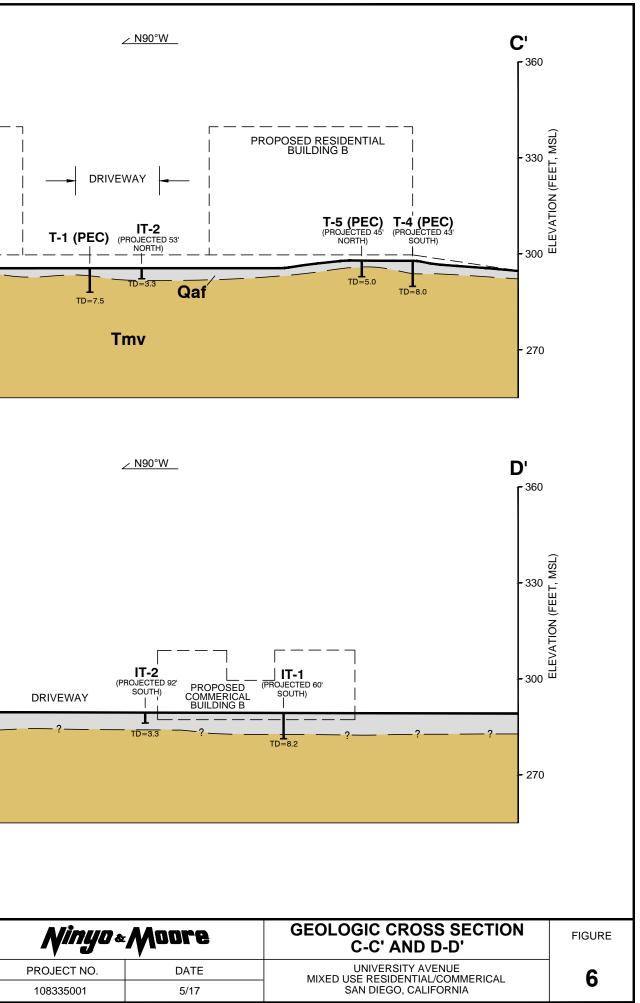
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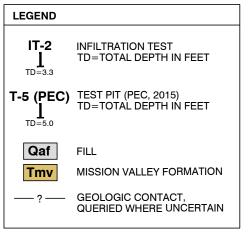


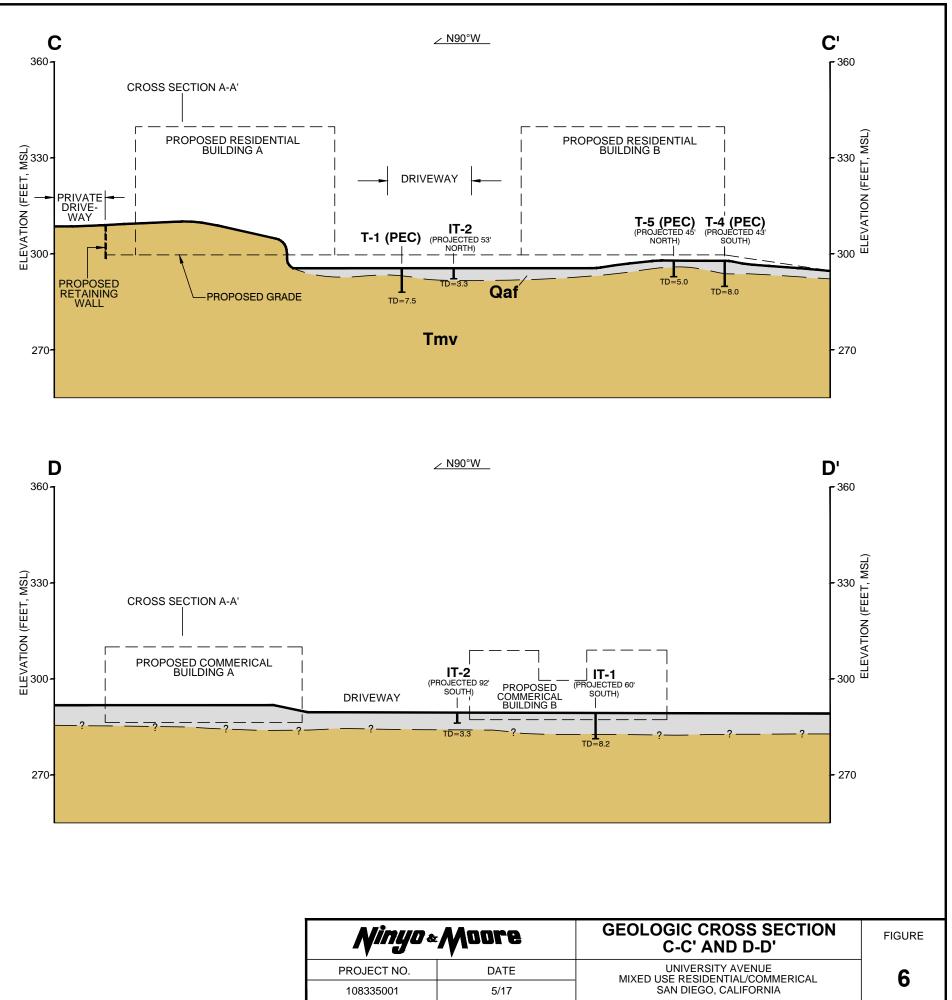


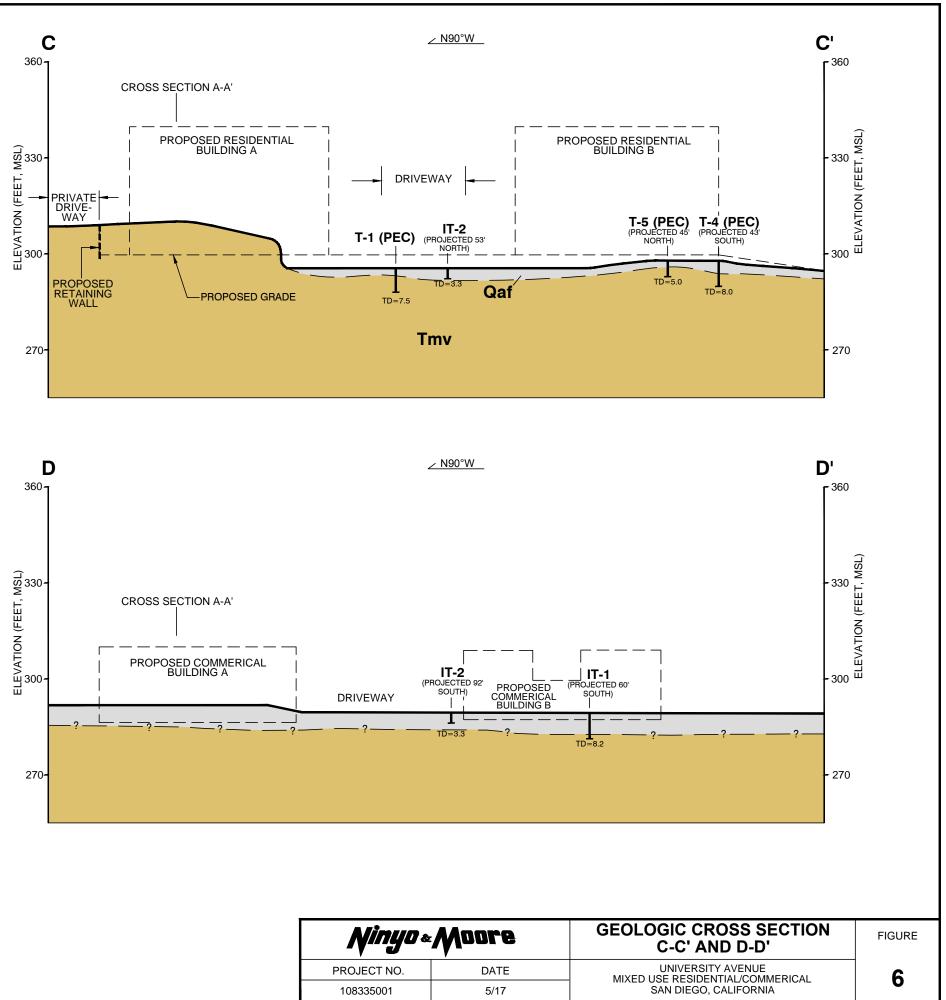


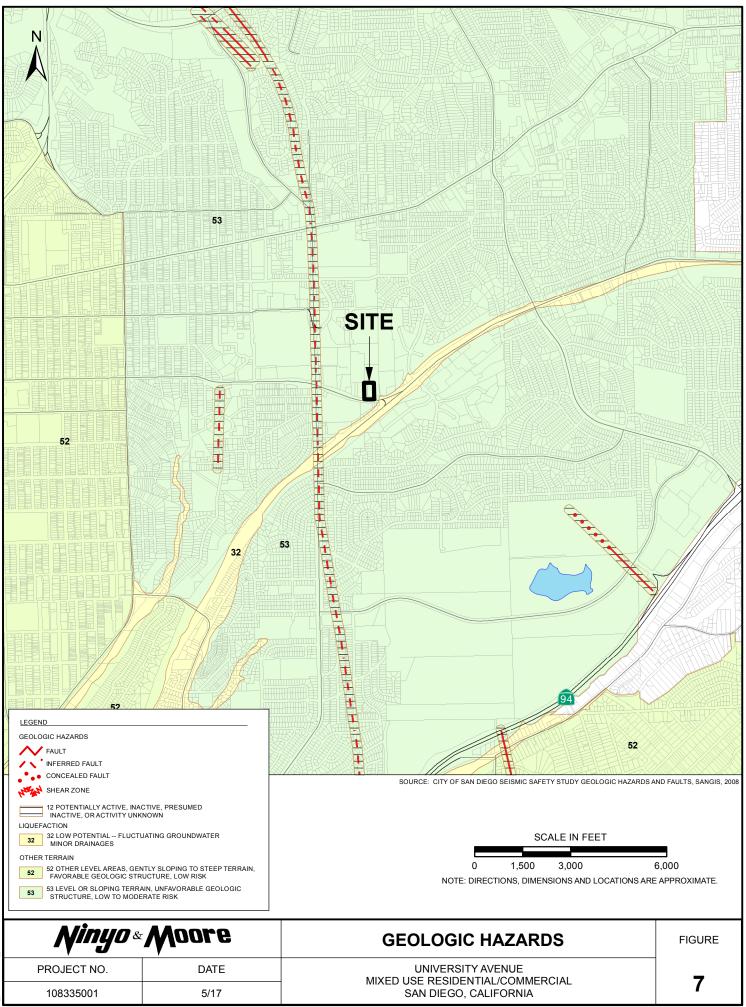




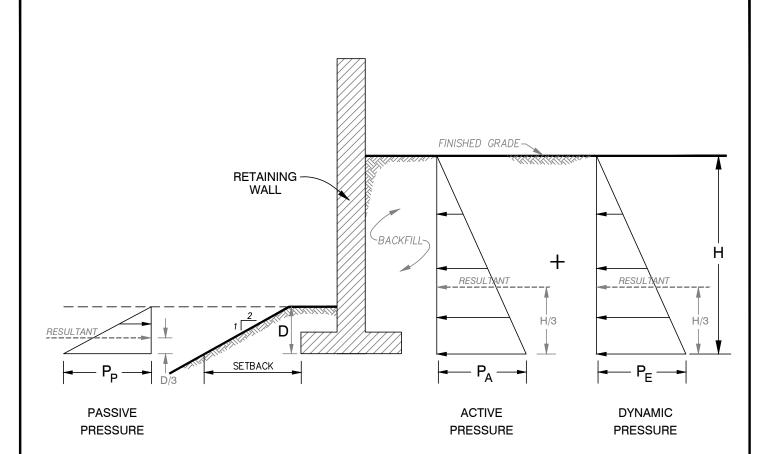








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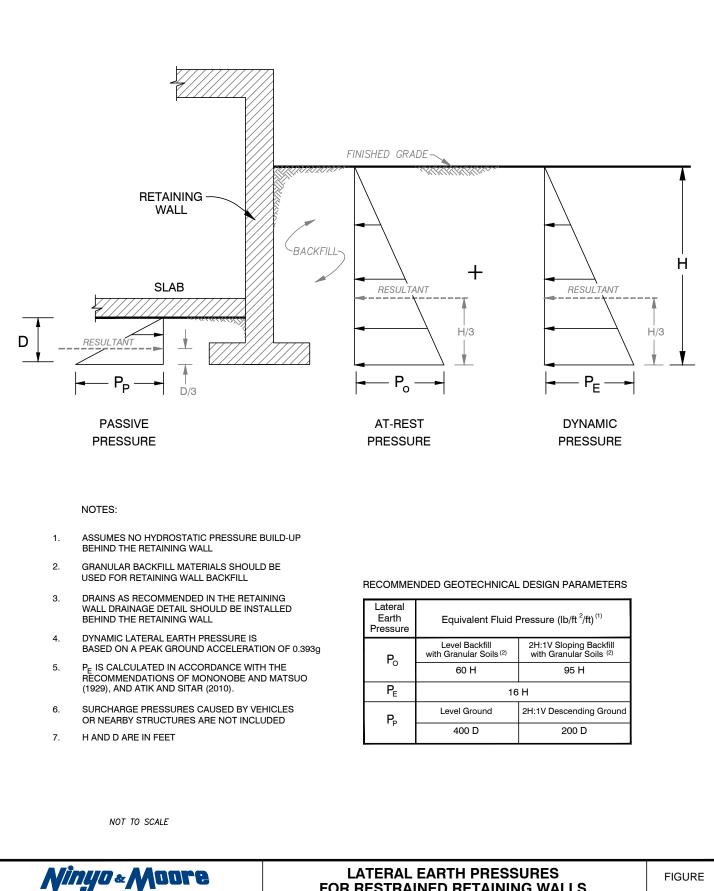
- 1. ASSUMES NO HYDROSTATIC PRESSURE BUILD-UP BEHIND THE RETAINING WALL
- 2. GRANULAR BACKFILL MATERIALS SHOULD BE USED FOR RETAINING WALL BACKFILL
- 3. DRAINS AS RECOMMENDED IN THE RETAINING WALL DRAINAGE DETAIL SHOULD BE INSTALLED BEHIND THE RETAINING WALL
- 4. DYNAMIC LATERAL EARTH PRESSURE IS BASED ON A PEAK GROUND ACCELERATION OF 0.393g
- 5. P_E IS CALCULATED IN ACCORDANCE WITH THE RECOMMENDATIONS OF MONONOBE AND MATSUO (1929), AND ATIK AND SITAR (2010).
- 6. SURCHARGE PRESSURES CAUSED BY VEHICLES OR NEARBY STRUCTURES ARE NOT INCLUDED
- 7. H AND D ARE IN FEET
- 8. SETBACK SHOULD BE IN ACCORDANCE WITH THE CBC

RECOMMENDED GEOTECHNICAL DESIGN PARAMETERS

Lateral Earth Pressure	Equivalent Fluid Pressure (lb/ft ²/ft) (1)				
P _A	Level Backfill with Granular Soils ⁽²⁾	2H:1V Sloping Backfill with Granular Soils ⁽²⁾			
• 4	40 H	65 H			
P _E	16	5 H			
P₀	Level Ground	2H:1V Descending Ground			
	400 D	200 D			

NOT	то	SCALE

Ninyo	Maare	LATERAL EARTH PRESSURES FOR YIELDING RETAINING WALLS	FIGURE
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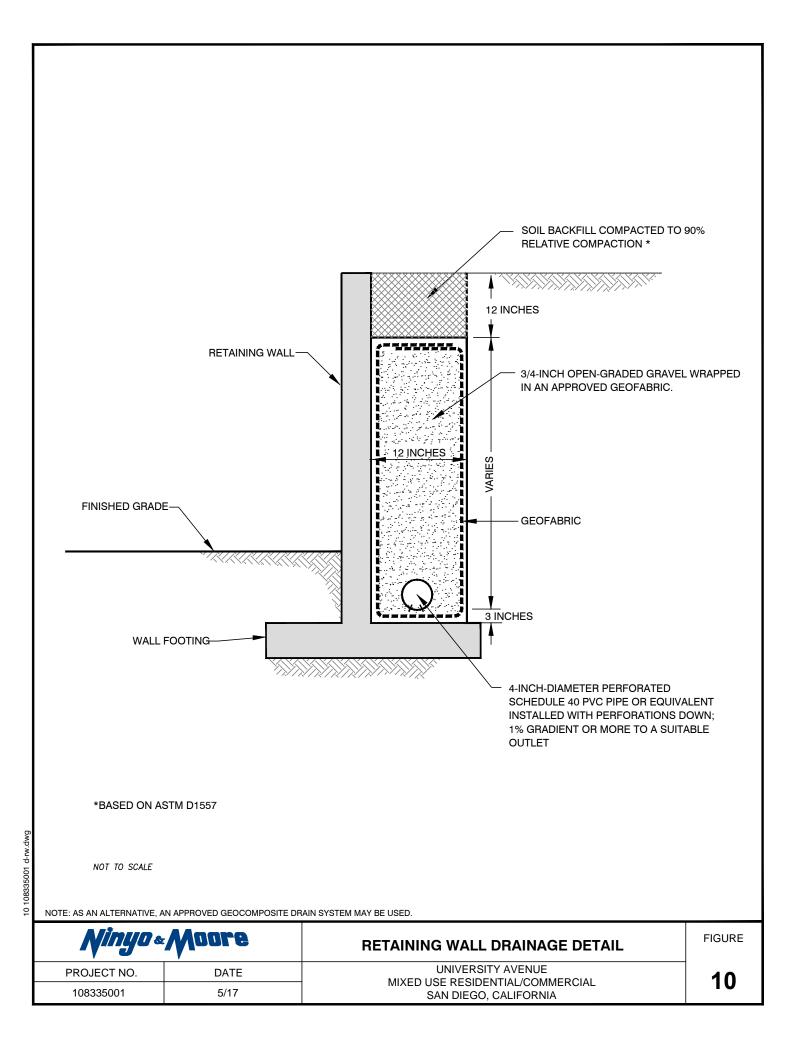
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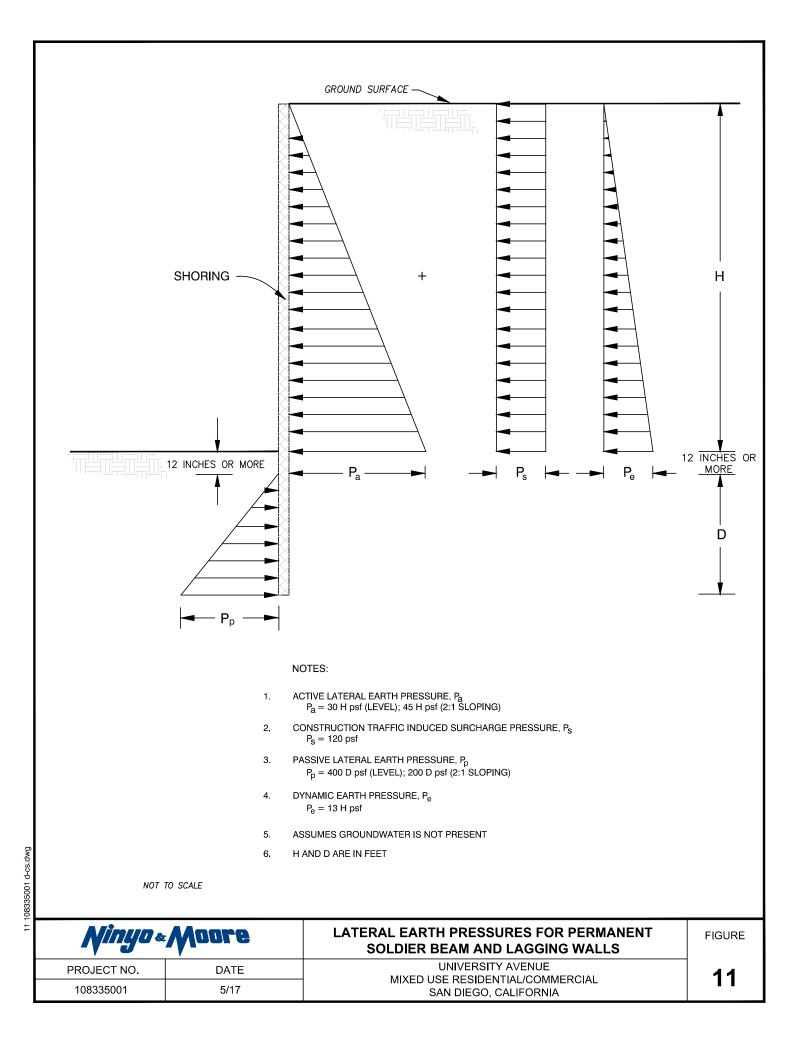
UNIVERSITY AVENUE MIXED USE RESIDENTIAL/COMMERCIAL SAN DIEGO, CALIFORNIA

FOR RESTRAINED RETAINING WALLS

FIGURE

9





APPENDIX A

BORING LOGS

Field Procedure for the Collection of Disturbed Samples

Disturbed soil samples were obtained in the field using the following method.

Bulk Samples

Bulk samples of representative earth materials were obtained from the exploratory borings. The samples were bagged and transported to the laboratory for testing.

Field Procedure for the Collection of Relatively Undisturbed Samples

Relatively undisturbed soil samples were obtained in the field using the following method.

The Modified Split-Barrel Drive Sampler

The sampler, with an external diameter of 3.0 inches, was lined with 1-inch long, thin brass rings with inside diameters of approximately 2.4 inches. The sample barrel was driven into the ground with the weight of a hammer of the drill rig in general accordance with ASTM D 3550. The driving weight was permitted to fall freely. The approximate length of the fall, the weight of the hammer, and the number of blows per foot of driving are presented on the boring logs as an index to the relative resistance of the materials sampled. The samples were removed from the sample barrel in the brass rings, sealed, and transported to the laboratory for testing.

			1		1				
DEPTH (feet) Bulk SAMPLES Driven BLOWS/FOOT	MOISTURE (%)	DRY DENSITY (PCF)	SYMBOL	CLASSIFICATION U.S.C.S.	BORING LOG EXPLANATION SHEET				
0					Bulk sample.				
					Modified split-barrel drive sampler. 2-inch inner diameter split-barrel drive sampler. No recovery with modified split-barrel drive sampler, or 2-inch inner diameter split-barrel drive sampler. Sample retained by others. Standard Penetration Test (SPT). No recovery with a SPT. Shelby tube sample. Distance pushed in inches/length of sample recovered in inches. No recovery with Shelby tube sampler. Continuous Push Sample. Seepage. Groundwater encountered during drilling.				
	Ŧ				Groundwater measured after drilling.				
				SM	MAJOR MATERIAL TYPE (SOIL): Solid line denotes unit change. Dashed line denotes material change. Attitudes: Strike/Dip b: Bedding c: Contact j: Joint f: Fracture F: Fault cs: Clay Seam s: Shear bss: Basal Slide Surface sf: Shear Fracture sz: Shear Zone sbs: Shear Bedding Surface The total depth line is a solid line that is drawn at the bottom of the boring.				
20									
		1	<u>· </u>		BORING LOG				
	\overline{n}		&	Mn	BORING LOG Explanation of Boring Log Symbols PROJECT NO. DATE FIGURE				
∥ ″▼″″	7			V 1 -	PROJECT NO. DATE FIGURE				
II *				,					

		SIFICATION	СН	ART PER A	STM D 2488			GRAI	N SIZE	
DD				SECON	DARY DIVISIONS	DESC		SIEVE	GRAIN	APPROXIMATE
FN				OUP SYMBOL	GROUP NAME	DEOC		SIZE	SIZE	SIZE
		CLEAN GRAVEL		GW	well-graded GRAVEL	В	oulders	> 12"	> 12"	Larger than basketball-sized
		less than 5% fines		GP	poorly graded GRAVEL					
	GRAVEL			GW-GM	well-graded GRAVEL with silt	С	obbles	3 - 12"	3 - 12"	Fist-sized to basketball-sized
	more than 50% of	GRAVEL with DUAL		GP-GM	poorly graded GRAVEL with silt					Thumb-sized to
	coarse fraction	CLASSIFICATIONS 5% to 12% fines		GW-GC	well-graded GRAVEL with clay		Coarse	3/4 - 3"	3/4 - 3"	fist-sized to
	retained on No. 4 sieve			GP-GC	poorly graded GRAVEL with clay	Grave			0.40.0.75"	Pea-sized to
004805		GRAVEL with		GM	silty GRAVEL		Fine	#4 - 3/4"	0.19 - 0.75"	thumb-sized
COARSE- GRAINED		FINES more than		GC	clayey GRAVEL		Coarse	#10 - #4	0.079 - 0.19"	Rock-salt-sized to
SOILS more than		12% fines		GC-GM	silty, clayey GRAVEL			#10 #4	0.075 0.15	pea-sized
50% retained on No. 200	SAND 50% or more of coarse fraction	CLEAN SAND		SW	well-graded SAND	Sand	Sand Medium	#40 - #10	0.017 - 0.079"	Sugar-sized to rock-salt-sized
sieve		less than 5% fines		SP	poorly graded SAND					TOCK-Sait-Sizeu
		SAND with DUAL CLASSIFICATIONS 5% to 12% fines SAND with FINES more than 12% fines		SW-SM	well-graded SAND with silt		Fine	#200 - #40	0.0029 - 0.017"	Flour-sized to sugar-sized
			DUAL ASSIFICATIONS	SP-SM	poorly graded SAND with silt					
				SW-SC	well-graded SAND with clay		Fines	Passing #200	Passing #200 < 0.0029"	Flour-sized and smaller
	passes No. 4 sieve			SP-SC	poorly graded SAND with clay					
				SM	silty SAND	PLASTICITY CHART				
				SC	clayey SAND					
				SC-SM	silty, clayey SAND		70			
				CL	lean CLAY		60			
	SILT and	INORGANIC		ML	SILT	A (P	50		CH or OF	
	CLAY liquid limit			CL-ML	silty CLAY	NDE	40			
FINE- GRAINED	less than 50%	ORGANIC		OL (PI > 4)	organic CLAY	Τ	30			
SOILS				OL (PI < 4)	organic SILT	STICITY INDEX (PI),	20	CL or C		MH or OH
50% or more passes		INORGANIC		СН	fat CLAY	PLAS				
No. 200 sieve	SILT and CLAY				elastic SILT					
	liquid limit 50% or more	ORGANIC		OH (plots on or above "A"-line)	organic CLAY		°0 10	20 30 40	50 60 70	
		-		OH (plots below "A"-line)	organic SILT			LIQUID	LIMIT (LL), %	1
	Highly 0	Organic Soils		PT	Peat					

APPARENT DENSITY - COARSE-GRAINED SOIL

		AUTOMATIC TRIP HAMMER				
APPARENT DENSITY	SPT (blows/foot)	MODIFIED SPLIT BARREL (blows/foot)	SPT (blows/foot)	MODIFIED SPLIT BARREL (blows/foot)		
Very Loose	≤4	≤ 8	<u>≤</u> 3	≤ 5		
Loose	5 - 10	9 - 21	4 - 7	6 - 14		
Medium Dense	11 - 30	22 - 63	8 - 20	15 - 42		
Dense	31 - 50	64 - 105	21 - 33	43 - 70		
Very Dense	> 50	> 105	> 33	> 70		

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CONSISTENCY - FINE-GRAINED SOIL

	SPOOLING CA	ABLE OR CATHEAD	AUTOMATIC TRIP HAMMER			
CONSIS- TENCY	SPT (blows/foot)	MODIFIED SPLIT BARREL (blows/foot)	SPT (blows/foot)	MODIFIED SPLIT BARREL (blows/foot)		
Very Soft	< 2	< 3	< 1	< 2		
Soft	2 - 4	3 - 5	1 - 3	2 - 3		
Firm	5 - 8	6 - 10	4 - 5	4 - 6		
Stiff	9 - 15	11 - 20	6 - 10	7 - 13		
Very Stiff	16 - 30	21 - 39	11 - 20	14 - 26		
Hard	> 30	> 39	> 20	> 26		

USCS METHOD OF SOIL CLASSIFICATION

Explanation of USCS Method of Soil Classification DATE

PROJECT NO.

FIGURE

DEPTH (feet)	SAMPLES	BLOWS/FOOT	MOISTURE (%)	DRY DENSITY (PCF)	SYMBOL	CLASSIFICATION U.S.C.S.	DATE DRILLED GROUND ELEVATION METHOD OF DRILL	ON <u>337</u>	' ± (MSL)		G NO SHEET _ (Limited Acce	1		1
DEP'	Bulk Driven	BLOW	LSIOM	DRY DEN	SΥ	CLASSI U.S	DRIVE WEIGHT	GSW	140 lbs. (Cathead)	GSW			30" GTF	
									DESCRIPTION/I				011	
0							MISSION VALLEY Light brown to gray, SANDSTONE,	<u>FORM</u> , moist, a	<u>ATION:</u> strongly cement	ed, silty fi	ine- to medi	um-grai	ined	
-		88/9"	11.0	110.3			Gray.							
		81/9"	12.1	111.4			Iron-oxide staining.							
-		94/10"	9.9	111.0			Medium- to coarse-g	grained.						
20 -		85/9"	13.5	110.8			Fine- to medium-gra	iined; m	icaceous.					
-		88/9"	17.3	109.8			Fine-grained.							
30-		98/10"	18.4	106.2			Iron-oxide staining. Total Depth = 30.8 for	eet Gro	undwater not en	countered	l during dril	ling		
-							Backfilled with approx $3/24/17$.						after dril	lling on
-							<u>Note:</u> Groundwater, level due to seasonal the report.							
-							The ground elevation of published maps ar not sufficiently accur	nd other	documents revi	ewed for	the purpose	s of this	evaluat	
40		•)	1	I		<u> </u>			BORI	NG LOG			
		MŽ	D_{i}	10 2	Se		ore	Ν	1IXED USED RESIDE	UNIVERS	SITY AVENUE	N DIEGO,	CALIFOR	NIA
		V	J					11	OJECT NO. 08335001	DA1 5/1			FIGURE A-1	

	SAMPLES	L	(9	CF)		Z			
DEPTH (feet)	S/	BLOWS/FOOT	URE (%)	DRY DENSITY (PCF)	SYMBOL	CLASSIFICATION U.S.C.S.	GROUND ELEVATION <u>304' ± (MSL)</u> METHOD OF DRILLING 8" Diameter Hollow Stem A		
DEPT	Bulk Driven	BLOW	MOISTURE	Y DEN	SYI	LASSI U.S	DRIVE WEIGHT 140 lbs. (Cathead)	DROP	<u> </u>
				DR		C	SAMPLED BY <u>GSW</u> LOGGED BY <u>GS</u> DESCRIPTION/INTER		GTF
0							PORTLAND CEMENT CONCRETE: Approximately 4.25 inches thick.		
							MISSION VALLEY FORMATION: Brown to gray, moist, moderately cemented, silty SANDSTONE.	y, fine- to medium-gr	ained
		89/9"					Micaceous.		
							Brown to gray, moist, well indurated, sandy SIL	TSTONE; micaceous	- — — — — — — — — — — — — — — — — — — —
10 -		50/6"	14.5	109.3					
		50/6"	14.6	112.5					
20 -		60/6"	15.1	114.2					
							Total Depth = 22 feet. Groundwater not encountered during drilling. Backfilled with approximately 7 cubic feet of ber 3/24/17.	entonite grout shortly	after drilling on
							<u>Note:</u> Groundwater, though not encountered at the level due to seasonal variations in precipitation a the report.		
30 -							The ground elevation shown above is an estimati of published maps and other documents reviewed not sufficiently accurate for preparing construction	d for the purposes of	this evaluation. It is
	$\left \right $						not surrecentry accurate for preparing construction	on olus and design de	cuments.
	$\left \right $								
	$\left \right $								
40									
					0			ORING LOG	
		\	IJ		×		MIXED USED RESIDENTIA PROJECT NO.	AL/COMMERCIAL, SAN DIE	FIGURE
11		,				,	108335001	5/17	A-2

DEPTH (feet) Bulk SAMPLES	BLOWS/FOOT	MOISTURE (%)	DRY DENSITY (PCF)	SYMBOL	CLASSIFICATION U.S.C.S.	METHOD OF DRILLII DRIVE WEIGHT SAMPLED BYGS	N <u>296' ± (MSL)</u> NG <u>Manual</u> N/A W LOGGED BY _	DROP		_ OF	
						FILL: Brown, moist, very sti MISSION VALLEY H Brown to gray, moist, Total Depth = 8.2 feet Groundwater not enco Backfilled on 3/24/17. <u>Note:</u> Groundwater, th level due to seasonal v the report. The ground elevation and not sufficiently accura	FORMATION: weakly cemented, silty untered during drilling ough not encountered variations in precipitati shown above is an esti	y SANDSTONE. g. at the time of drillin on and several othe mation only. It is ba ewed for the purpos	ng, may r factors ased on ses of th ign doc	v rise to a s as discu our interj is evalua	issed in pretations
	V Í	Ŋ	 0 -	St	Ma	ore	MIXED USED RESIDE PROJECT NO. 108335001	UNIVERSITY AVENU ENTIAL/COMMERCIAL, S DATE 5/17		O, CALIFOR FIGURE A-3	

DEPTH (feet) Bulk SAMPLES	BLOWS/FOOT	MOISTURE (%)	DRY DENSITY (PCF)	SYMBOL	D CLASSIFICATION U.S.C.S.	GROUND ELEVATION METHOD OF DRILL DRIVE WEIGHT SAMPLED BYG PORTLAND CEME Approximately 3.5 in	ON <u>295'</u> .ING <u>Ma</u> .sw .NT COI	nval N/A LOGGED BY DESCRIPTION/II NCRETE:	GSW		1	_ OF	
						FILL: Brown, moist, very st inches in diameter. Total Depth = 3.3 fee Groundwater not ence Backfilled and patche <u>Note:</u> Groundwater, t level due to seasonal the report. The ground elevation of published maps an not sufficiently accur	et. (Refu countere ed with though 1 variatic a shown ad other	usal) d during drilling concrete on 3/2- not encountered ons in precipitati above is an esti documents revi	g. 4/17. at the tin on and se mation of ewed for	ne of drilling everal other f nly. It is base the purposes	, may factor ed on s of th	orrise to a s as discu our inter is evalua	higher ussed in pretations
	M	<i>nu</i>	10 -	Se		ore	N	IIXED USED RESIDE	UNIVER	NG LOG SITY AVENUE IMERCIAL, SAI	N DIEG	O, CALIFOI	RNIA
	V	J						OJECT NO.	DA ⁻ 5/1			FIGURE	

APPENDIX B

INFILTRATION TESTING

University Avenue, Mixed Use Residential/Commercial San Diego, California

Appendix B Project No. 108335001

Test Date:	3/24	4/2017				Infiltration	n Test No.:	IT-1
Test Hole Di	ameter, D	(inches):	6.0			Excavation De	epth (feet):	8.20
Test perform	ed and rec	orded by:	GSW			Pipe Lei	ngth (feet):	8.20
t ₁	d ₁ (feet)	t ₂	d ₂ (feet)	Δt (min)	ΔH (feet)	Percolation Rate (min/in)	H _{avg} (feet)	Infiltration Rate (in/hr)
2:00 PM	7.00	2:25 PM	7.25	25	0.25	8	1.08	0.75
2:25 PM	7.00	2:50 PM	7.30	25	0.30	7	1.05	0.92
2:50 PM	7.00	3:00 PM	7.15	10	0.15	6	1.13	1.08
3:00 PM	7.00	3:10 PM	7.20	10	0.20	4	1.10	1.47
3:10 PM	7.00	3:20 PM	7.15	10	0.15	6	1.13	1.08
3:20 PM	7.00	3:30 PM	7.15	10	0.15	6	1.13	1.08
3:30 PM	7.00	3:40 PM	7.15	10	0.15	6	1.13	1.08
3:40 PM	7.00	3:50 PM	7.15	10	0.15	6	1.13	1.08
Test Date:								
rest Date.	3/24	4/2017				Infiltration	n Test No.:	IT-2
Test Hole Di	ameter, D	(inches):	6.0			Excavation De	epth (feet):	3.30
	ameter, D	(inches):	6.0 GSW			Excavation De		3.30
Test Hole Di	ameter, D	(inches):		Δt (min)	ΔH (feet)	Excavation De	epth (feet):	3.30
Test Hole Di Test perform	ameter, D ed and reco d ₁	(inches): orded by:	GSW			Excavation Do Pipe Ler Percolation Rate	epth (feet): ngth (feet): H _{avg}	3.30 3.30 Infiltration Rate
Test Hole Di Test perform t ₁	ameter, D ed and reco d ₁ (feet)	(inches): orded by:	GSW d ₂ (feet)	(min)	(feet)	Excavation De Pipe Ler Percolation Rate (min/in)	epth (feet): ngth (feet): H _{avg} (feet)	3.30 3.30 Infiltration Rate (in/hr)
Test Hole Di Test perform t ₁ 11:25 AM	ameter, D ed and reco d ₁ (feet) 2.20	(inches): orded by: t ₂ 11:50 AM	GSW d ₂ (feet) 2.20	(min) 25	(feet) 0.00	Excavation Do Pipe Ler Percolation Rate (min/in)	epth (feet): ngth (feet): H _{avg} (feet) 1.10	3.30 3.30 Infiltration Rate (in/hr) <0.01
Test Hole Di Test perform t ₁ 11:25 AM 11:50 AM	ameter, D ed and reco d ₁ (feet) 2.20 2.20	(inches): orded by: t ₂ <u>11:50 AM</u> <u>12:15 PM</u>	GSW d ₂ (feet) 2.20 2.20	(min) 25 25	(feet) 0.00 0.00	Excavation Do Pipe Ler Percolation Rate (min/in) 	epth (feet): ngth (feet): H _{avg} (feet) 1.10 1.10	3.30 3.30 Infiltration Rate (in/hr) <0.01 <0.01
Test Hole Di Test perform t ₁ <u>11:25 AM</u> <u>11:50 AM</u> <u>12:15 PM</u>	ameter, D ed and reco d ₁ (feet) 2.20 2.20 2.20	(inches): orded by: t ₂ <u>11:50 AM</u> <u>12:15 PM</u> <u>12:45 PM</u>	GSW d ₂ (feet) 2.20 2.20 2.20	(min) 25 25 30	(feet) 0.00 0.00 0.00	Excavation Do Pipe Ler Percolation Rate (min/in) 	H _{avg} (feet): 1.10 1.10 1.10	3.30 3.30 Infiltration Rate (in/hr) <0.01 <0.01 <0.01
Test Hole Di Test perform t ₁ 11:25 AM 11:50 AM 12:15 PM 12:45 PM	ameter, D ed and reco d ₁ (feet) 2.20 2.20 2.20 2.20	(inches): orded by: t ₂ 11:50 AM 12:15 PM 12:45 PM 1:15 PM	GSW d ₂ (feet) 2.20 2.20 2.20 2.20	(min) 25 25 30 30	(feet) 0.00 0.00 0.00 0.00	Excavation De Pipe Ler Percolation Rate (min/in) 	epth (feet): ngth (feet): H _{avg} (feet) 1.10 1.10 1.10 1.10 1.10	3.30 3.30 Infiltration Rate (in/hr) <0.01 <0.01 <0.01 <0.01
Test Hole Di Test perform t ₁ 11:25 AM 11:50 AM 12:15 PM 12:45 PM 1:15 PM	ameter, D ed and reco d ₁ (feet) 2.20 2.20 2.20 2.20 2.20 2.20	(inches): orded by: t ₂ 11:50 AM 12:15 PM 12:45 PM 1:15 PM 1:45 PM	GSW d ₂ (feet) 2.20 2.20 2.20 2.20 2.20	(min) 25 25 30 30 30	(feet) 0.00 0.00 0.00 0.00 0.00	Excavation De Pipe Ler Percolation Rate (min/in) 	H _{avg} (feet): H _{1.10} 1.10 1.10 1.10 1.10 1.10	3.30 3.30 Infiltration Rate (in/hr) <0.01 <0.01 <0.01 <0.01 <0.01

Notes:

 t_1 = initial time when filling or refilling is completed

 d_1 = initial depth to water in hole at t_1

- $t_2 =$ final time when incremental water level reading is taken
- d_2 = final depth to water in hole at t_2
- Δt = change in time between initial and final water level readings
- ΔH = change in depth to water or change in height of water column (i.e., d₂ d₁)
- $H_0 =$ Initial height of water column

in/hr = inches per hour

Percolation Rate to Infiltration Rate Conversion¹

$$I_t = \frac{\Delta H \times 60 \times r}{\Delta t \left(r + 2H_{avg} \right)}$$

 I_t = tested infiltration rate, inches/hour

- ΔH = change in head over the time interval, inches
- $\Delta t = time interval, minutes$
- r = effective radius of test hole

 H_{avg} = average head over the time interval, inches

¹ Based on the "Porchet Method" as presented in:

Riverside County Flood Control, 2011, Design Handbook for Low Impact Development Best Management Practices: dated September.

APPENDIX C

LABORATORY TESTING

Classification

Soils were visually and texturally classified in accordance with the Unified Soil Classification System (USCS) in general accordance with ASTM D 2488. Soil classifications are indicated on the logs of the exploratory borings in Appendix A.

In-Place Moisture and Density Tests

The moisture content and dry density of relatively undisturbed samples obtained from the exploratory borings were evaluated in general accordance with ASTM D 2937. The test results are presented on the logs of the exploratory borings in Appendix A.

Direct Shear Tests

Direct shear tests were performed on relatively undisturbed samples in general accordance with ASTM D 3080 to evaluate the shear strength characteristics of the selected materials. The samples were inundated during shearing to represent adverse field conditions. The results are shown on Figures C-1 through C-3.

Expansion Index Tests

The expansion indexes of selected materials were evaluated in general accordance with ASTM D 4829. The specimens were molded under a specified compactive energy at approximately 50 percent saturation. The prepared 1-inch thick by 4-inch diameter specimens were loaded with a surcharge of 144 psf and were inundated with tap water. Readings of volumetric swell were made for a period of 24 hours. The results of the tests are presented on Figure C-4.

Soil Corrosivity Tests

Soil pH and resistivity tests were performed on representative samples in general accordance with CT 643. The chloride contents of the samples were evaluated in general accordance with CT 422. The sulfate contents of the samples were evaluated in general accordance with CT 417. The test results are presented on Figure C-5.

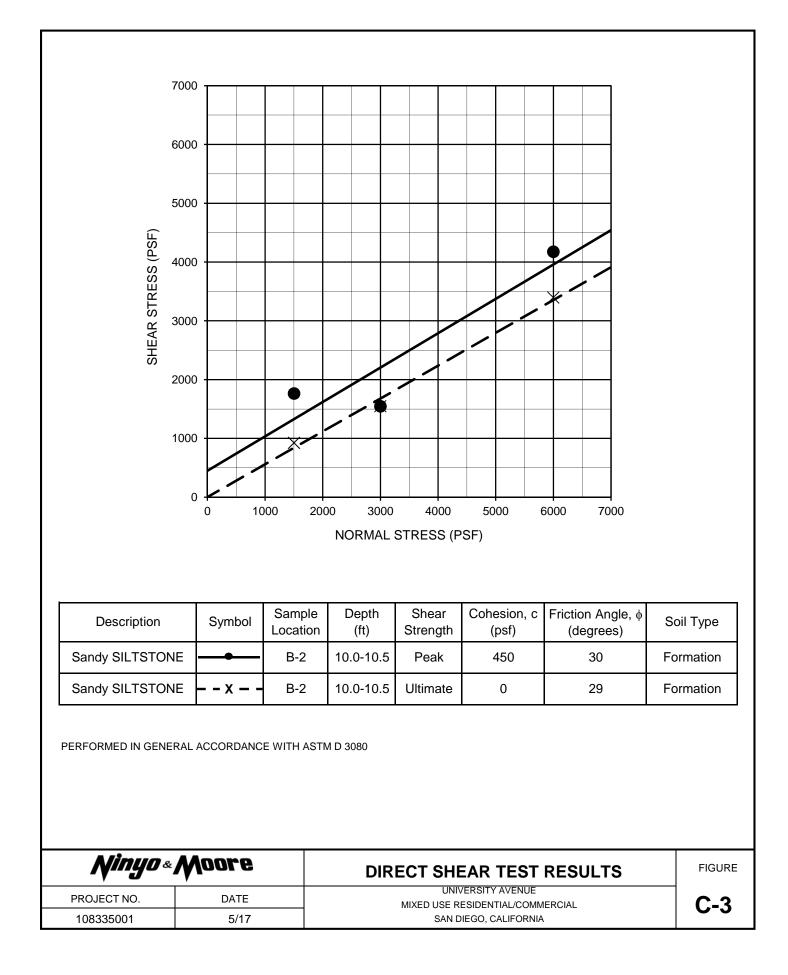
R-Value

The resistance value, or R-value, for site soils was evaluated in general accordance with CT 301. The samples were prepared and evaluated for exudation pressure and expansion pressure. The equilibrium R-value is reported as the lesser or more conservative of the two calculated results. The test results are shown on Figures C-6 through C-8.

	7000							
	6000							
<u> </u>	5000							
ESS (PSF)	4000						*	
SHEAR STRESS (PSF)	3000							
Ϋ́	2000							
	1000							
		0 10	000 200		4000		6000 7000	
				NORMAL	STRESS (P	51)		
Description		Symbol	Sample Location	Depth (ft)	Shear Shear	Cohesion, c (psf)	Friction Angle, φ (degrees)	Soil Type
Description Silty SANDSTC		Symbol		Depth	Shear	Cohesion, c		
-	DNE	Symbol ————————————————————————————————————	Location B-1	Depth (ft)	Shear Strength	Cohesion, c (psf)	(degrees)	Formation
Silty SANDSTC	DNE DNE	• X	Location B-1 B-1	Depth (ft) 15.0-15.8 15.0-15.8	Shear Strength Peak	Cohesion, c (psf) 640	(degrees) 41	Formation
Silty SANDSTC	DNE DNE	X	Location B-1 B-1	Depth (ft) 15.0-15.8 15.0-15.8	Shear Strength Peak Ultimate	Cohesion, c (psf) 640	(degrees) 41 32	Soil Type Formation Formation

	7000							
	6000							
	5000							
ESS (PSF)	4000							
SHEAR STRESS (PSF)	3000							
S	2000							
	1000							
	0	0 10	00 200	00 3000	4000	5000	6000 7000	
	0	0 10	<u> </u>		4000 STRESS (F		6000 7000	
Description		0 10 Symbol	Sample Location	NORMAL : Depth			Friction Angle, φ	Soil Type
Description Silty SANDSTO			Sample	NORMAL	STRESS (F Shear	PSF)		Soil Type Formation
	DNE		Sample Location B-1	NORMAL	STRESS (F Shear Strength	PSF) Cohesion, c (psf)	Friction Angle, φ (degrees)	Formation
Silty SANDSTO	DNE	Symbol ————————————————————————————————————	Sample Location B-1 B-1	NORMAL Depth (ft) 30.0-30.8 30.0-30.8	STRESS (F Shear Strength Peak	PSF) Cohesion, c (psf) 100	Friction Angle, φ (degrees) 33	
Silty SANDSTO	DNE DNE	Symbol	Sample Location B-1 B-1	NORMAL Depth (ft) 30.0-30.8 30.0-30.8	STRESS (F Shear Strength Peak Ultimate	PSF) Cohesion, c (psf) 100 100	Friction Angle, φ (degrees) 33	Formation

Γ



SAMPLE LOCATION	SAMPLE DEPTH (FT)	INITIAL MOISTURE (%)	COMPACTED DRY DENSITY (PCF)	FINAL MOISTURE (%)	VOLUMETRIC SWELL (IN)	EXPANSION INDEX	POTENTIAL EXPANSION
IT-1	0.0-7.2	12.5	99.0	28.3	0.118	116	High
IT-2	0.3-3.3	10.0	110.8	19.0	0.052	53	Medium
	I GENERAL A	CCORDANCE WIT	н 🗆	UBC STANDAR	D 18 A	STM D 4829	
Ninu	<i>10</i> « A A	Dore	EVDA				FIGU
, -	<i>10</i> « Ma		EXPA		DEX TEST R	ESULTS	FIGU
Ving ROJECT NO. 08335001	,	DOFCE DATE 5/17	EXPA	UNIVERS MIXED USE RESID	DEX TEST R ITY AVENUE ENTIAL/COMMERCIAL D, CALIFORNIA		FIGU C-4

SAMPLE LOCATION	SAMPLE DEPTH (FT)	pH ¹	RESISTIVITY ¹ (Ohm-cm)	SULFATE ((ppm)	CONTENT ² (%)	CHLORIDE CONTENT ³ (ppm)
B-2	0.4-5.0	7.2	290	700	0.070	1,590
IT-1	0.0-7.2	7.1	300	640	0.064	780
IT-2	0.3-3.3	8.0	710	320	0.032	90

¹ PERFORMED IN GENERAL ACCORDANCE WITH CALIFORNIA TEST METHOD 643

² PERFORMED IN GENERAL ACCORDANCE WITH CALIFORNIA TEST METHOD 417

³ PERFORMED IN GENERAL ACCORDANCE WITH CALIFORNIA TEST METHOD 422

Ninyo «	Woore	CORROSIVITY TEST RESULTS	FIGURE
PROJECT NO.	DATE	UNIVERSITY AVENUE MIXED USE RESIDENTIAL/COMMERCIAL	C-5
108335001	5/17	SAN DIEGO, CALIFORNIA	C-J

SAMPLE LOCATION	SAMPLE DEPTH (FT)	SOIL TYPE	R-VALUE
B-2	0.4-5.0	Sandy CLAYSTONE [excavates as Sandy CLAY (CL)]	8
IT-1	0.0-7.2	Sandy CLAY (CL)	5
RFORMED IN GENERAL ACCORD	ANCE WITH ASTM D 2844/CT 301		

Ninyo «	Moore	R-VALUE TEST RESULTS	FIGURE
PROJECT NO.	DATE		C-6
108335001	5/17	MIXED USE RESIDENTIAL/COMMERCIAL SAN DIEGO, CALIFORNIA	

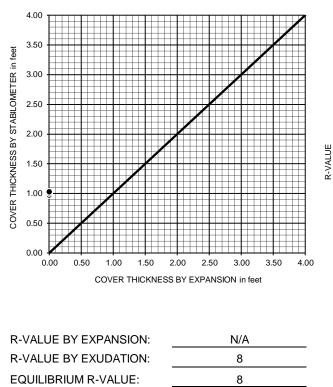


R-VALUE TEST RESULTS

5710 Ruffin	Road, Sa	n Diego,	Čalifornia 92123	

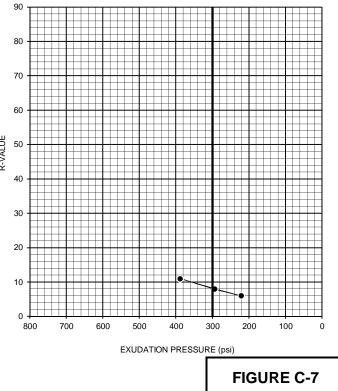
PROJECT NAME:	UNIVERSITY AVENUE	PROJECT NUMBER	108335001
	MIXED USE RESIDENTIAL/COMMERCIAL	DATE:	5/17
	SAN DIEGO, CALIFORNIA	TECHNICIAN:	DV
SAMPLE DESCRIPTION:	Sandy CLAYSTONE (excavates as Sandy CLAY	SAMPLE LOCATION:	B-2 @ 0.4-5.0

TEST SPECIMEN	а	b	С
MOISTURE AT COMPACTION %	18.3	18.9	19.5
HEIGHT OF SAMPLE, Inches	2.49	2.50	2.50
DRY DENSITY, pcf	111.3	109.4	108.9
COMPACTOR AIR PRESSURE, psi	50	50 50 50	
EXUDATION PRESSURE, psi	388	294	221
EXPANSION, Inches x 10exp-4	0	0	0
STABILITY Ph 2,000 lbs (160 psi)	129	135	140
TURNS DISPLACEMENT	5.03	5.10	5.15
R-VALUE UNCORRECTED	11	8	6
R-VALUE CORRECTED	11	8	6
R-VALUE BY EXUDATION	8		
DESIGN CALCULATION DATA	а	b	С
GRAVEL EQUIVALENT NEEDED ft.	1.42	1.47	1.50
TRAFFIC INDEX	5.0		
STABILOMETER THICKNESS, ft.	0.98	1.01	1.03
EXPANSION PRESSURE THICKNESS, ft.	0.00	0.00	0.00



EXPANSION PRESSURE CHART

EXUDATION PRESSURE CHART



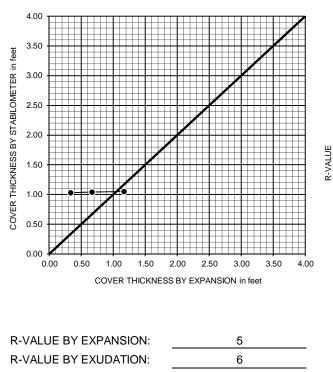


R-VALUE TEST RESULTS

5710 Ruffin Road, San Diego, California 92123

PROJECT NAME:	UNIVERSITY AVENUE	PROJECT NUMBER	108335001
	MIXED USE RESIDENTIAL/COMMERCIAL	DATE:	5/17
	SAN DIEGO, CALIFORNIA	TECHNICIAN:	DV
SAMPLE DESCRIPTION:	Sandy CLAY	SAMPLE LOCATION:	IT-1 @ 0.0-7.2

TEST SPECIMEN	а	b	с
MOISTURE AT COMPACTION %	16.4	17.0	17.6
HEIGHT OF SAMPLE, Inches	2.50	2.50	2.50
DRY DENSITY, pcf	108.8	107.4	106.5
COMPACTOR AIR PRESSURE, psi	50	50	50
EXUDATION PRESSURE, psi	395	301	205
EXPANSION, Inches x 10exp-4	10	20	35
STABILITY Ph 2,000 lbs (160 psi)	137	140	144
TURNS DISPLACEMENT	5.50	5.62	5.71
R-VALUE UNCORRECTED	7	6	5
R-VALUE CORRECTED	7 6		5
R-VALUE BY EXUDATION	6		
DESIGN CALCULATION DATA	а	b	с
GRAVEL EQUIVALENT NEEDED ft.	1.49	1.50	1.52
TRAFFIC INDEX	5.0		
STABILOMETER THICKNESS, ft.	1.03	1.04	1.05
EXPANSION PRESSURE THICKNESS, ft.	0.33	0.67	1.17

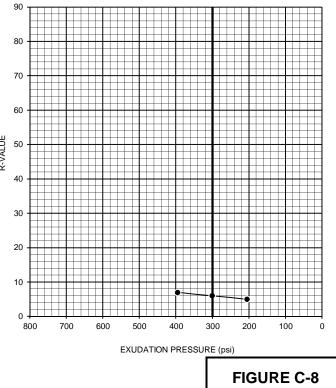


5

EQUILIBRIUM R-VALUE:

EXPANSION PRESSURE CHART

EXUDATION PRESSURE CHART





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info@rinconconsultants.com www.rinconconsultants.com

September 29, 2017 Project No: 17-04840

Mark Gottschlich USS Cal Builders 8051 Main Street Santa Ana, CA 90680

Subject: University Manor Mixed-Use Project, 5556-5592 University Avenue, City and County of San Diego, California

Dear Mr. Gottschlich:

Rincon Consultants, Inc. (Rincon) was contracted to provide Cultural Resources services for the University Manor Mixed-Used Project (project), located at 5556-5592 University Avenue in the City and County of San Diego, California. Cultural Resources services were provided in support of the project's Preliminary Review by the City of San Diego Planning Department, and were prepared in accordance with applicable local guidelines, including the City of San Diego Development Services' Potential Historical Resource Review Informational Bulletin 580, and additional plan review comments by the City of San Diego in the project Issues Report, provided to Rincon by the applicant.

The purpose of this memorandum is to document the results of the tasks performed by Rincon, which included:

- A site visit, photographic documentation
- Architectural descriptions
- Building Permit and Assessor Building Records Research
- City Directory Research and Occupant History
- Notice of Completion letter

Project Description

Rincon understands that the project applicant proposes to construct a mixed-use development consisting of 63 residential units in two four-story buildings, and two commercial buildings fronting University Avenue that would include eating and drinking establishments and shops totaling approximately 62,117 square feet on the 1.55-acre site. Development of the proposed project would include demolition of the three properties containing a total of nine buildings.



Methodology

Archival Research

On September 21, 2017, Rincon Cultural Resources Specialist Breana Campbell, M.A., RPA, conducted historical building research for the existing structures located on the project site. The following sources and research repositories were examined to establish known historical land uses and the locations of research materials pertinent to the subject property:

- San Diego History Center (SDHC)
- City of San Diego Building and Safety Department
- County of San Diego Office of the Assessor
- Sanborn Fire Insurance Maps, EDR Inc.

Agency/Archive	Sources Reviewed	Results/ Notes
San Diego History Center	City Directories	The SDHC City Directories span from 1892 to 1984; the addresses included in this project are listed in the directories from 1950 through 1984. These directory pages were photocopied and are included in Appendix A.
San Diego History Center, EDR Inc.	Sanborn Maps	Ms. Campbell reviewed Sanborn maps available at the SDHC. The SDHC does not include Sanborn maps printed after 1940, and the project site was not depicted on the 1940 map or any previous editions. Subsequently, a review was made of Sanborn maps available online through ProQuest. The project site was not depicted in the later 1956 map.
San Diego History Center	Historic Photos	Ms. Campbell reviewed the historical photo archives for City Heights, University Avenue, and local businesses; no historic photos of the project site were found.
City of San Diego Building and Safety Department	Building Permits	Building permits were reviewed at the City of San Diego Building Department; microfilm records for 5580 University Avenue were not found and the staff suggested it may have been misfiled. Building permits records were copied and are included in Appendix B.
County of San Diego Office of the Assessor	Building Records	Ms. Campbell obtained the Assessor's Building Records for each of the addresses associated with the project site. These records are included in Appendix C.
County of San Diego Office of the Assessor	Master Property Records	A review of the Master Property Records was completed at the County Assessor's Office, and a copy of the records is included in Appendix D.
County of San Diego Office of the Assessor	Notice of Completion	Notice of Completions could not be located for any of the subject properties. Per the City of San Diego's Development Services Department, a note stating such is included in Appendix E.

Table 1 Research Results

Field Visit and Constraints

A site visit was made on September 22, 2017 to conduct the photographic documentation of all of the buildings on the project site. Digital photographs were taken of every building elevation that was clearly visible and safely accessible. Physical access to some portions of the property was constrained by metal fencing, overgrown vegetation and active beehives. Notes were taken to document buildings'



architectural style, materials, condition, and visible alterations to inform the architectural descriptions in this memo. A site map with a photograph key is included in Appendix F and digital photos are provided on an accompanying CD.

Results

The project site consists of three parcels, generally bounded by University Avenue to the south, a private driveway on the west, a vacant lot on the east, and undeveloped hilly property to the north. Within the subject property are nine buildings. Multiple addresses have been associated with the buildings on the property over the decades and, due to the deteriorated nature of the structures, most of the buildings did not display an address. Each building was assigned a reference number to provide a clear understanding of the developmental history for each property and building (Table 2).

Property APN	Bld. Ref. No(s).	Property Address(es)	Const. Date	Photo Reference Nos.
472-410-05	No. 1	5556 University Avenue	1951	1
472-410-12	No. 2	5570 University Avenue	1952	2
u	No. 3	5582 University Avenue	1953	3
и	No. 4	5582 University Avenue	Between 1953-1960	4
u	No. 5	5582 University Avenue	Between 1953-1960	5
u	No. 6	5586 University Avenue	1954	6
472-410-13	No. 7	5590 University Avenue	1947	7
u	No. 8	5590 University Avenue	Between 1947-53	8
u	No. 9	5592 University Avenue	1951	9

Table 2 Property APNs and Buildings

APN 472-410-05/ 5556 University Avenue

This parcel contains one building accessed by a sloped driveway from University Avenue:

Building No. 1, 5556 University Avenue

Building Description

Building No. 1, 5556 University Avenue

This is a one-story building, originally constructed in 1951, that has been heavily altered. It has a rectangular plan and is approximately 1,828 sq. ft. in size. It has a side-gabled roof clad with composite shingles; a portion of which is covered with tarps. The building exterior is clad with rough stucco, and patchwork is evident. The primary entry on the south façade is multi-paned, and appears to be sliding or French doors. The east elevation once had a carport or shed attached, most of which has been removed. There is shed attached to the north (rear) elevation. It is made of wood, has a nearly flat roof, and is walled on two sides. Two window openings have been closed for HVAC and another window has been



covered with plyboard. The building is separated from the street by a paved parking lot, and is accessed by a long driveway from University Avenue. There is a temporary shade canopy in front of the building.



Photograph 1: Building No. 1, south elevation, view to north.

Construction History

This chronology provides a summary of the property's development and alterations that have been made over time.

Bld. No.	Date	Description of Work Performed	Source	Owner on Record
1	1951	Permit for construction of dwelling	Assessor's Building Record	Unknown
1	1960	Plumbing Work	Building Permits	Art Cuiffo
1	1973	Notice of Occupancy Approval for a one- story wood-frame residence to be used as an office for used car sales.	Building Permits	Unknown
1	1979	Electrical work done on the building	Building Permits	John Romeo
1	1980	Electrical work done on the building	Building Permits	William L. Baker

Table 3 Construction History for APN 472-410-05

Occupant History

This chronology provides a summary of the property's occupancy over time.



Bld. No. / Address	Date	Occupant Name	Owner on Record	Source
Bldg. 1, 5556 University Avenue	1951-1954	Schoenhoff Saml	Shoenhoff, Sam/Martha	City Directory
Bld. No. / Address	Date	Occupant Name	Owner on Record	Source
Bldg. 1, 5556 University Avenue	1955-1956	Young Robt L	Ciuffo, Arthur/Sadie	City Directory; Master Property Records
u	1958	Hughs Wm	Likely Arthur Ciuffo	u
u	1959	No return	Likely Arthur Ciuffo	u
u	1960	Martinez Bennie	Art Cuiffo	City Directory
u	1961-1964	Vacant	Unknown	u
u	1965	Martinez Bennie	Unknown	u
u	1966-1972	Gomez Mary M Mrs.	Unknown	u
u	1973-1974	Vacant	Unknown	u
u	1975	Romeo John	Likely John Romeo	u
u	1976-1979	Romeo John; Automart	Likely John Romeo	u
u	1980	Sportscar Emporium	William L. Baker	u
u	1984	Two Guys Auto Sales	Unknown	u
u	2017	Quality Auto Body	University Ave Manor LLC	Visual observation; client info

Table 4 Occupant History for APN 472-410-05

APN 472-410-12, 5570-5586 University Avenue

This parcel contains five buildings, all accessed by a long sloped driveway from University Avenue:

- Building No. 2, 5570 University Avenue
- Building No. 3, 5582 University Avenue
- Building No. 4, 5582 University Avenue
- Building No. 5, 5582 University Avenue
- Building No. 6, 5586 University Avenue

Building Descriptions

Building No. 2, 5570 University Avenue

This is a one-story building, originally constructed in 1952, that has been heavily altered. It has an L-shaped plan and is approximately 2,787 sq. ft. in size. The longer north-south portion of the building is set at a higher elevation and therefore is taller than the east-west portion. It is accessed via a set of concrete steps that are abutted by a concrete block retaining wall/planter. The building is covered with



flat roofs that have exposed rafter tails. In some areas the fascia boards are missing and the roof in general is deteriorated. Most of the exterior cladding of the building has been removed, leaving exposed studs and barrier paper. Windows and doors have been covered with plyboard or removed. The south elevation contains a single solid entry door, and the east elevation contains a single wooden, six-panel door.



Photograph 2: Building No. 2, east elevation, view to northwest.

Building No. 3, 5582 University Avenue

This is a one-story building, originally constructed in 1953, that has been heavily altered. It is approximately 1,584 sq. ft. in size, has a long, rectangular plan, and a flat roof. A portion of the building at the north (rear) elevation is higher than the remainder of the building and has a large sliding door hung on a metal rail. The building is in such dilapidated condition to be considered a ruin. It displays various types of deteriorated wood board siding, and has portions of walls missing. The majority of the doors and windows are missing; however, two roll-up garage doors remain on the east elevation. No address was detected anywhere on the building.



Photograph 3: Building No. 3, east elevation, view to northwest.

Building No. 4, 5582 University Avenue

This is a one-story ancillary building constructed between 1953 and 1960. It has a rectangular plan, and is approximately 437 sq. ft. in size. It has a gabled roof covered with corrugated metal panels. Its primary entry is on the west elevation and is composed of large metal double doors. Metal venting structures are affixed to the roof of the building. No fenestration was noted. The north and east elevations are not clearly visible due to the building's proximity to walls and other structures. No address was detected anywhere on the building.



Photograph 4: Building No. 4, west elevation, view to east.



Building No. 5, 5582 University Avenue

This is a one-story building, originally constructed between 1953 and 1960, and has been heavily altered. It has a rectangular plan and is approximately 1,318 sq. ft. in size. The building is of concrete or masonry construction, and has a flat roof. The north and south elevations have large sliding doors hung on metal rails. Other doors are missing and a portion of the wall on the west elevation is also missing. No fenestration was noted. No address was detected anywhere on the building.



Photograph 5: Building No. 5, west elevation, view to east.

Building No. 6, 5586 University Avenue

This is a two-story building with a rectangular plan. Originally constructed in 1954, it has been heavily altered. Square footage is unknown; however, the building's ground level footprint is 6,340 sq. ft. The exterior cladding has mostly been removed, exposing concrete block construction. The primary entry is located at the southwest corner under a small flat porch roof. The majority of the windows and doors have been covered or are missing. A non-structural triangular projection pierces through the façade near the southwest corner. There is an addition against the north elevation composed of vented metal panels supported on metal posts, which creates a parking space or storage area below. The addition has a sloped roof that appears to be made of corrugated metal panels.



Photograph 6: Building No. 6, south elevation, view to north.

Construction History

This chronology provides a summary of the property's development and alterations that have been made over time.

Bld. No.	Date	Description of Work Performed	Source	Owner on Record
Unk.	1954	Int. well modification	Assessor's Building Records	Master Cleaners
2	Illegible	Electrical work	Building permits	Spinali
2	1952	Construct apartments and store	Assessor's Building Records	Unknown
2	1952	Convert to storage	Assessor's Building Records	Unknown
2	1954	Construct boiler room and garage at 5570 University Ave	Building permits	Dominic Spinali
2-6	1968	Repair fire damage on buildings at 5570- 5586 University Avenue	Building permits	D. Spinali and D. Strazzulla
3	1953	Construct storage and boiler room (listed year built at 5582 University Ave)	Assessor's Building Records	Unknown
6	1954	Construct store & apt.	Assessor's Building Records	Spinali Men's Wear
6	1954	Construct retaining wall & roofed area	Building permits	Dominic Spinali

Table 5Construction History for APN 472-410-12



Occupant History

This chronology provides a summary of the property's occupancy over time.

Table 6Occupant History for APN 472-410-12

Bld. No. / Address	Date	Occupant Name	Owner on Record	Source
Bldg. 2, 5570 University Avenue	1953-1955	Master Cleaners	Dominic Spinali	City Directory
u	1956	Taste & Tell Restaurant	Unknown	и
u	1958-1959	Dick's Hide-A-Way Restaurant	Unknown	u
u	1960-1962	Millie's Place Tavern	Unknown	u
u	1963-1965	Club Detroiter Tavern	Unknown	u
Bld. No. / Address	Date	Occupant Name	Owner on Record	Source
Bldg. 2, 5570 University Avenue	1966-1967	Vacant	Unknown	u
<i>u</i>	1968-1970	Master Cleaners	D. Spinali and D. Strazzulla	u
u	1971	Tuxedo Rentals	Unknown	"
u	1972	Master Cleaners: Tuxedo Rentals	Unknown	u
	1973	Master Cleaners; Campus Tuxedos	Unknown	u
	1974-1975	Campus Tuxedos	Unknown	u
	1976-1979	University Tuxedo Shop	Unknown	u
	1980-1984	Tuxedo Junction	Unknown	"
Bldg. 3, 5582 University Avenue	1953-1954	Elgier Geo J	Unknown	"
	1955	Vacant	Unknown	u
	1956	Hogg Harvey	Unknown	u
	1958	Vacant	Unknown	u
	1959	Comb Richd	Unknown	u
"	1960	Gutierrez Bertha Mrs.	Unknown	u
	1961-1964	Not listed	Unknown	u



<i>u</i>	1965	Vacant	Unknown	u
<i>u</i>	After 1965	Address stopped being used	-	u
Bldg. 6, 5586 University Avenue	1956-1971	Master Cleaners	Dominic Spinali; Master Cleaners	и
u	1972	Casa di Spinali Men's Wear; Spinali Auto Sales	Unknown	u
u	1973	Casa di Spinali Men's Wear	Unknown	u
	1974	Vacant	Unknown	"
"	1975-1980	Humphrey Appliance; John's TV & Appliances	Unknown	u
u	1984	Humphrey Appliance	Unknown	u



APN 472-410-13, 5590-5592 University Avenue

This parcel contains three buildings, all accessed by a long sloped driveway from University Avenue:

- Building No. 7, 5590 University Avenue
- Building No. 8, 5590 University Avenue
- Building No. 9, 5592 University Avenue

Building No. 7, 5590 University Avenue

This is a one-story residential building constructed in 1947. It has a rectangular plan, and is approximately 720 sq. ft. in size. It has a side-gabled roof with slightly overhanging eaves and exposed rafter tails. The building is clad with painted stucco. The primary entry on the east façade is composed of a single, wood, paneled entry door covered by a sloped-roof porch supported by slender square posts. It is accessed via a set of concrete steps with a wooden railing. Fenestration includes multi-pane vinyl sliding windows, and single-pane wooden double-hung windows. Patchwork on the south elevation indicates that the three-window openings have likely been modified, and patchwork is noted around a wall vent on the east elevation. Windows on the north and west elevations are covered with plyboard. No address was detected on the façade.



Photograph 7: Building No. 7, south elevation, view to north.

Building 8, 5590 University Avenue

This is a one-story garage building constructed between 1947 and 1953 between Buildings 7 and 9. It has a rectangular plan, is approximately 465 sq. ft. in size, and is clad with painted stucco. It has a side-gabled roof with overhanging eaves and exposed rafter tails. Much of the roof has been covered with tarps. The primary entrance (garage door opening) on the east elevation has been covered with plywood.



Photograph 8: Building No. 8, east elevation, view to northwest.

Building 9, 5592 University Avenue

This is a one-story residential building originally constructed in 1951. The building has an irregular plan, and is approximately 913 sq. ft. in size. The northern portion of the building has a gabled roof covered with rolled roofing material; the roof on the southern portion is not clearly visible because it is covered with tarps. The building is clad with asbestos shingles. The eastern end of the building has a breezeway under the gabled roof. There is a brick chimney, partially covered with a tarp, on the north elevation. Windows are not clearly visible due to foliage, and some are covered with boards or security grilles. Visibility overall was limited due to dense foliage surrounding the building.





Photograph 9: Building No. 9, south elevation, view to north.

Construction History

This chronology provides a summary of the property's development and alterations that have been made over time.

Bld. No.	Date	Description of Work Performed	Source	Owner on Record
7	1949	Construction permit for dwelling	Assessor's Building Records	Unknown
7/9	1958	Construct single poster panel at 5590/5592 University Avenue	Building permits	Cordtz Div. Pacific Outdoor Adv. Co.
9	1951	Construction permit for dwelling	Assessor's Building Records	Unknown
9	1959	Construct den and bath attached to residence	Building Permits	Wm. C. Hubrich

Table 7 Construction History for APN 472-410-13

Occupant History

This chronology provides a summary of the property's occupancy over time.

Table 8Occupant History for APN 472-410-13

Bld. No. / Address	Date	Occupant Name	Owner on Record	Source
Bldg. 7, 5590 University Avenue	1950	Vacant	Unknown	City Directory
u	1952	Keller G S	Unknown	u
u	1953-1971	Shurig Carl	Unknown	u
u	1972-1975	Shurig Clara F Mrs.	Unknown	"
u	1976	Testa Mildred	Unknown	"
u	1977-1979	No return	Unknown	u
u	1980	Vacant	Unknown	u
u	1984	Vacant	Unknown	u
5591 University Avenue	1952-1955	Markov Louis Htg & Sht Mtl	Unknown	u
"	1956-1973	Markov Louis Sheet Metal Wks	Unknown	"
"	1974	Markov Louis Sheet Metal & Heating	Unknown	u
"	1975-1979	Baker Electricians	Unknown	u



Bldg. 9, 5592 University Avenue	1952-1979	Hubrich W C	Wm. C. Hubrich	u
Bld. No. / Address	Date	Occupant Name	Owner on Record	Source
u	1980	Not listed	-	"
u	1984	Vacant	Unknown	u

Should you have any questions or comments regarding the information provided in this memo, please do not hesitate to contact me at 805-644-4455, or szgurrola@rinconconsultants.com.

Sincerely, Rincon Consultants, Inc.

non fundie Shade

Susan Zamudio-Gurrola Architectural Historian

Appendix A

City Directories

UNIVERSITY AV-Contd 4730 Patterson H C restr 4734 Foster D B Mrs variety store 4736 Holtschlag R T restr 4741 Southern Venetian Blind Co 4744 Dob's & Jim's Serv gas sta 6304AHoxsey J M 5324AIngraham G E Mrs (6382 Angelo Julio 6384AYoung F B bldg con. 6384AJohnson M T Mrs (6362ARichards C P signs ' 6370AHanley Genè real e AStoney E M Mrs re Balando blud artafabo sta sin s Serve as
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Winona av intersects
4951 Philbrook G I
4953 Hilderbrand G A
4954 ΔHilderbrand G R
Philbrook G I auto repr
4957 ΔSkillman B D
4961 ΔLynn LeRoy
4963 Frank C W
4968 ΔThompson H H gro
4978 ΔThomas J P furn
4977 Kritzer D E confy
4979 Vacant
4981 Vacant
4983 ΔEmbleton H A real est
ΔSandsted M S Mrs seauty
shop shop 50th intersects 5045ΔHarrington T M Mrs 5045ΔHarrington T M Mrs 5047 Hall W H 5049ΔWakeman E J © 5051 McQueen H R 5053 Kuhns C F 5055 Smallwood L C © 5057 Reilly M J 5067ΔLowry J E 5055ΔHattabough C A © 52d intersects 5225ΔTurner H C bildg contr FORREST **De FRATE** 5225ATurner H C blog contr 54th intersects 5423 Hanes A L restr 5429 Hanes A L @ 5430 Crestview Trailer Park Clare C H Diehl W W ARuegg R J 5447AEdwards J R pntr 5450AWestfall John @ 5451AMcGrath C W garden sup **SINCE 1920** Complete AUTO REPAIR SERVICE 5451ΔMcGrath C W garden
sup
5467ΔUniversity Showcase & Fixture Co
5467ΔUniversity Showcase &
5467ΔUniversity E @
5469 Ziegler J E @
5485 Magoffin G A plmbr
5485ΔMacKee A H geni contr
5586ΔO & R Nursery
5586ΔShear W H @
5602 Ingram Laura J
6196ΔC B M Co Inc bldg mtls ΔCosgrave C A bldg contr
Cartagena dr intersects All Makes All Models MOTOR REBUILDING AND 428ABroderic) 430ALemke A REBORING BRAKE AC os grave C A bidg contr Cartagena dr intersects 6206 ΔPage R E Θ 6210 ΔKester R R Θ 6214 ΔPerry J C Θ 6212 ΔNester J R Θ 6222 ΔJones H D Θ 6226 ΔNester F L Θ 6230 ΔSchneider F L Θ 6230 ΔSchneider F L Θ 6230 ΔAschneider F L Θ 6238 ΔHrastich W C 6242 ΔMathel Benj Θ Mathel L B 6244 ΔMoe Jas restr 62444 Vacant 62444 Vacant 62444 Vacant 62444 Vacant 62444 Vacant 62444 Vacant 62440 Source C M ras sta Bonillo dr begins 718ACheney] SERVICE 811 Jeter E 1 823 Donlevy Go 909AGifford 911 Gifford 912AFriedm 932ABaker TOWING Mechanical Service 24-Hours 1005ABart 1015ASag(1019 Ker 1020AH) 831 2nd Ave. Main 1021AF 1036AF 1029AF 1439

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1950

MELIX SUPPLY, INC. On Yellow Paper Immediately Following **F.** 9-9394 Buyers DISTRIBUTORS OF PABCO PRODUCTS FOR DEALER **JLTS** 1952 2228 KETTNER BLVD. NEAREST YOU 4344 Standard Stations R 9594 Fairmount av intersects UNIVERSITY AV—Contd 5447 Edwards J R pntr T 9234 5450 Valley Grove Courts Westfall John @ R 7702 5451 Vacant 5467 University Show Case & Fix-ture Co T 1-2513 5485 Magoffin G A Quality Pimbr T 1-2530 5493 West and Party Pimbr 4686 Bon's Market R 9754 Williams D D Stores 4687 Dorral Arth E real est T 9686 4350 Vacant
4351 Haizlip's Fairmount av intersects
4351 Haizlip's Fairmount Pharm T 2121
4353 University Av Hdw R 6275
4354 Fox Crest Theatre T 8404
4357 Brittain's Anne Plastic Shop R 9892
4359 Pick L A jwir T 9211
4361 Quality Shoe Shop shoe repr
4362-64 King Television Co T 6407
4366 Fairmount Chrs R 0045
4367 New Rec Cafe R 9347
4368 Red G Mason Co real est R 5540
4379 Haley's Pay-Less gro R 9680
Hamilton Max R 0070
Allbright's Barber Shop R 0070
Allbright G O Guide 4687 Dorval Arta 2 4689 Dave's Cafe 4693 Cafe Hacienda T 7786 47th intersects 47th arta R 7400 4350 Vacant 538 ۲ 4693 Cafe Hactenda T 7786 4701 Cooley's Hardware R 7400 4703 Euclid Beauty Parlor R 4592 4705 Euclid Card Room 4711 Log Cabin Cafe liquors R 9520 Denure J V, R 9520 4714 Euclid Pastry Shop R 8460 4715 Merit Shoe Shop Hargis J C 4716 Gruenwald E A, R 0688 Styer D C Vaughn Mattie Mrs © T 1-5159 Wadeigh Laurel Mrs 4717 Kingdom Hall of Jehovah's Witnesses 4718 Shannon's Five Ten & Twenty-Five Cent Store T 6306 4725 Vacant 7462 5 T1-2530 5493 McKee A H genl contr T1-6653 5494 Mel's Service T 1-6174 5506 Wilson Floor Co T 6513 5515 Vacant 5515 I & S Sales Inc real est T S184 Schlehuber Elwin W Inc bldg contrs T S184 ive Serv itersects dry 3655 Schlehuber Elwin W Ine bldg contrs T 8184 5556 Schoenhoff Saml © 5590 Keller G S 5591 Markov Louis Htg & Sht Mtl T 3-1353 5592 Hubrich W C @ 5595 Edmiston J J, T 8442 5999 Metal Masters, T 5411 6080 Star Clnrs T 5336 6082 Village Furn, T 5253 6084 Vacant 6086 Nuttitional Food Some 1-4274 3 ials R 0070 Allbright G 0 4380 Golden Eagle Serv Sta R 9781 4383 Coo Coo Club T 2788 4387 Fairmount Bakery 4391 Jerry's Certified Stores liquors R 3817 44th intersects Laisu R 9751 R 2187 D.CIINS Alphabetical T 6306 4725 Vacant 4726 Masters Cleaners R 9196 Lowery Mabel C Mrs 4727 Thry's Place Houors R 9773 4730 Paddock Cafe Houors R 9774 4734 Vacant 4734 Valnut Cafe 4741 Southern Venetian Blind Co T 6712 4744 Bob's & Jim's Service gas sta T 8268 4745 Parker's Pharmacy R 4288 R 0119 1-5541 日本は見 4391 Jerry's Certified Stores liquors R 3317
44th intersects
4404 Lehmkuhl Dick Shell Serv gas sta R 9745
4411 Flinn F E, R 3688
4417 Pressley P C
4419 Shourds B A jr T 5545
4425 Glenn's Cycle Co R 3373
4431 Mustain I E
4433 Kellner David
4437 Priebe L W, R 0922
4439 Fairmount Shop
4444 Bank of America
4455 Sambo Inn R 9648
4465 Jacks Richfield Serv R 9541
4457 Better Upholstery Studio T 1-7727
4479 Bob & Joe's Auto Repair R 9765
Stein R J
4488 Harry's Automotive Serv B 5645 1-5541 fineral R 4610 Salta M Eusiness 0 60S6 Nutritional Food Serv gro T 1-4344 Saliti S undwich 60SS Neuenswander Real Estate Calling Contract T 1-2122 6090 House of Helene beauty shop T 2209 0 9556 0 6098 Owen's Richfield Serv gas R 9467 6098 Oven's Richfield Serv gas R 9467
6104 Rolando Service gas sta R 9601
6118 C Coast Market T 1-1727
6129 Jimbo's Inn liquors R 9715
6134-50 Rancho Market gro R 7272
6129 Jimbo's Inn liquors R 9715
6134-50 Rancho Market gro R 7272
6120 Mission Hills Bky T 5140
6172 Modern Drug Co R 4523
6175 Roberts' Five & Ten
6179 Ladies Teen & Tot Shop R 5225
6130 Family Liquor R 1846
6181 Shamblen's Mildred Studio of Beauty T 0873
6138 Butler's Barber Shop
6134 Esquire Cleaners T 0564
6187 Gene's Cafe R 9709
6189 C As Nors & Key Shop R 9778
6199 Inage Shop The T 6136
6191 Barger Appliance Co T 1-5725 Studio Tile mosaic work T 4043
6192 Village Launderette
6195 C B M Co (vard) List 19 4745 Parker's Pharmacy R 4288 Euclid av intersects 4749 Mans Ben Serv Sta Mans Benj 617 Lucid av intersects 4749 Mans Ben Serv Sta Mans Ben Serv Sta A750 Bay City Liquor House R 2336 4750 Bay City Liquor House R 2336 4755 Silverado Ball Room R 0137 Joy Dance Studio 4756 Summers Complete Food Mar-ket gro R 2715 4757 Tower The restr R 9484 4771 Beck His A real est T 8152 4804 Davisson W G, R 2947 4809 Dierlam Calvin 4813 Schmidtke H A © T 1-7945 4815 Corban Paul 4818 Middleswart E B © T 1-4877 4819 Hudson Ed Nursery R 6491 4834 Miller G E © T 2317 4836 Miller G E © T 2317 4836 Miller G E T at est R 9224 Estrella av intersects 4925 Gibson Auto Repair T 1-3420 Winona av intersects 4951 Philbrook G I 4953 McPherson Raymond 4954 Georges Auto Repair R 1478 Hilderbrand Texaco Service R 1473 4957 Nesselrode P L 49572 Lansaw Cath Mrs © T 6893 9 S оге **Names** THUR R 3508 ection tessen 3 7000 Store j4 ersects re i22 112 4485 Harry's Automotive Serv B 5645 Hertsch H W J O. 4489 Bob & Joe's Service Station B 9765 i. etter Ð 11/1 10/1 6733 - stand . gro 45th intersects Rayons Woolens 4502 Two J's restr R 1513 4503 Myrts Friendly Corner R 9677 4511 U-Do Self Serv Lndry R 8968 Morantini Jos 45110 Lovelady Georgia Mrs 4520 Dorsey Grace Beauty Shop R 6626 Marcell Barber Shop T 5181 clo Silks 4490 12 4033 6192 Village Launderette 6195 C B M Co (yard) 6196 C B M Co Inc bldg sups T 1-6655 C B M Co bldg materials T 1-6655 • 1.112.000 334 10.97 tore Cosgrove C A real est T 1-6655 Cartagena dr intersects
 R
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 Marcell Barber Shop T 7181

 4521 Cook L C Furn Store R 0143

 4526 Falk J E

 4526 Falk J E

 4528 Kelley V L, T 1-2449

 4531 Coopers Shade & Linoleum Co

 4532 Fane G W, T 1-5633

 4534 Page W H, R 1810

 4535 Kirk's Serv gas sta R 9222

 Chamoune av intersets

 4555 Kirk's Nerv Gas Sta C 923
 0 6205 Pottingers Texaco Serv T 3-1678 R 1475 4957 Nesselrode P L 4957 J. Lansaw Cath Mrs © T 6593 4961 Sogge Dwane J 4963 Hueston J K 4968 Thompson's Market T 1-1430 4969 Vacant 4979 Stonecrafters of San Diego cut stone contrs T 1-2325 4971 Lo-Heet Stainless Steel Co-house furng gds R 4264 4953 Studio Girl Hollywood cosmetics B 5666 $\begin{array}{c} \begin{array}{c} \begin{array}{c} 1 & 3-1016\\ \hline 0&206 & Vacant\\ \hline 0&210 & Kester R R @ T 7430\\ \hline 0&214 & Vacant\\ \hline 0&215 & Wagner's Motor Serv auto repr\\ T 9274\\ \hline 0&215 & Wagner's Motor Serv auto repr\\ \hline 0&219 & Muster J R @\\ \hline 0&219 & Thurston's Welding Shop\\ \hline 0&212 & Jones H D @ T 1-3962\\ \hline 0&222 & Jones H D @ T 1-3962\\ \hline 0&226 & Rose R E @ T 4209\\ \hline 0&236 & Hernandez A M Mrs @ R 4891\\ \hline 0&236 & Hernandez A M Mrs @ R 4891\\ \hline 0&236 & Hernandez A M Mrs @ R 4891\\ \hline 0&236 & Hernandez A M Mrs @ R 4891\\ \hline 0&236 & Hernandez A M Mrs @ R 4891\\ \hline 0&236 & Hernandez A M Mrs @ R 1500\\ \hline 0&2142 & Your Home Workshop Inc\\ R 4211\\ \hline 0&246 & Club Rolando liquors R 9215\\ \hline \end{array}$ 6206 Vacant 30th ST. 3986 1151163 NORTH WE: N PE IS INST Chamoune av intersets 4555 Kirkman Oil Co R 9293 San Diego U-Drive Truck Rental Co T 1-7455 4560 B & B Auto Serv reprs R 6343 4578 Ryan A H @ R 2620 4576b Henrickson Elmer 4576b Henrickson Elmer 4576b Junger F A @ R 4446 4581 Munger F A @ R 4446 4581 Stewart Cammie Mrs @ R 5019 4592 Merius Serv R 5195 GIVE A LEASE PARK Studio Girl Hollywood cosmetics E 5666 50th intersects
Harrington T M Mrs © R 2279 Murphy J G
Fox L A
Smith Anna M, T 0249 Dickerson J W © T 1-6391
Flores F D, T 5951
Feilly M J
Lowry J E, R 4508
Eckholm C W
Hattabough C A © T 1-2898 52d intersects
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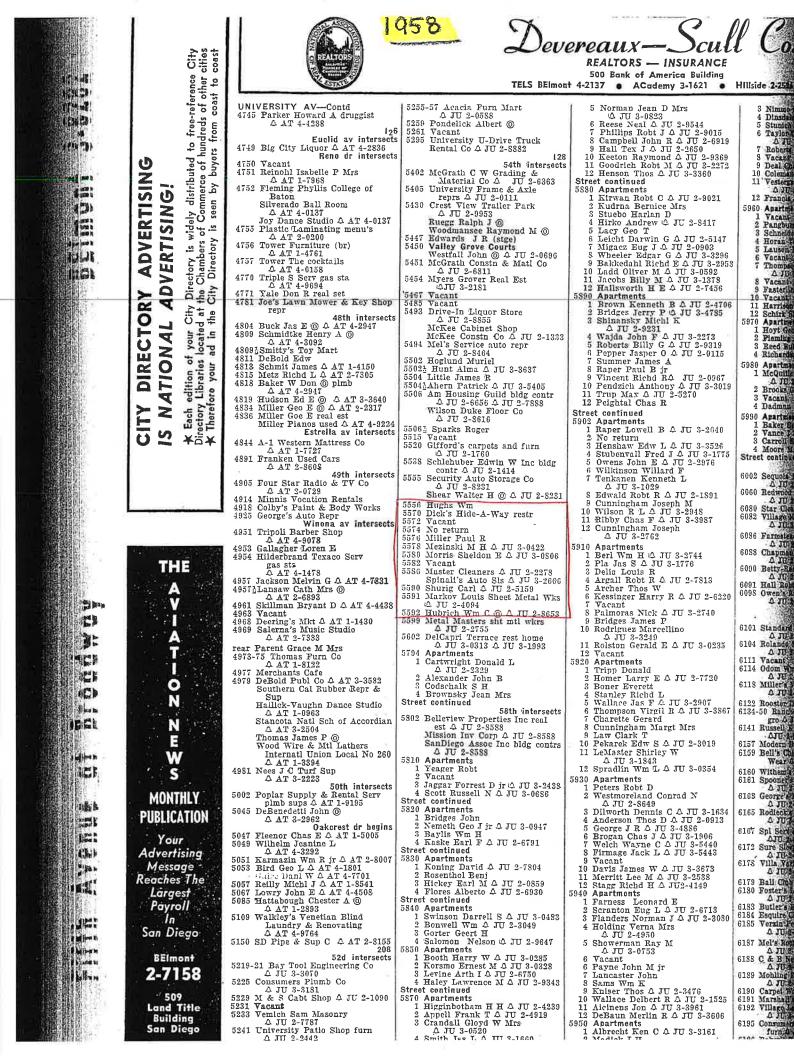
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Harold Signal Der 4-9639 d'A druggist (26 Eudild av intersects Ke Vault Co AT 4-0628 Reno dr Intersects Aduge House Cooper's Window A AT 2-8764 12209.11 OdB A AT 4-0137 onture br 61.35 cooktails AT 1-9222 Chamoune al (cont) TISI est Street continued A AT 4-9293 TI Acet 48th Intersects A AT 4-2947 AT 4-2620 bry A @ allaer B 🔘 ursery. A AT 4-6494 A AT 2-2317 creal est Entrella av Intersects cipie L Mrs 49th Intersects inding & Htg Co Body Works IC reprs RED. TABAT AV. Intersects teal est AAT.1-3360 Copair Construction Service gas 1478 (1) Hrè 💿 5572 Vacant W. 5576 Vacant 5578 Vacant A AT 1-1430 Studio. 5580 Vacant alville Co 5582 Vacant a bkpg serv conditioner Dance Studio oducts Co Softener Co Toal est Soth Intersects Rental Serv ne A @ Small Leo LO futereit dr begins AT 1-5268 2 Cercone Dani 3 Stone Robt R 4 Clark Bille are ATC 1-8541 Street continued

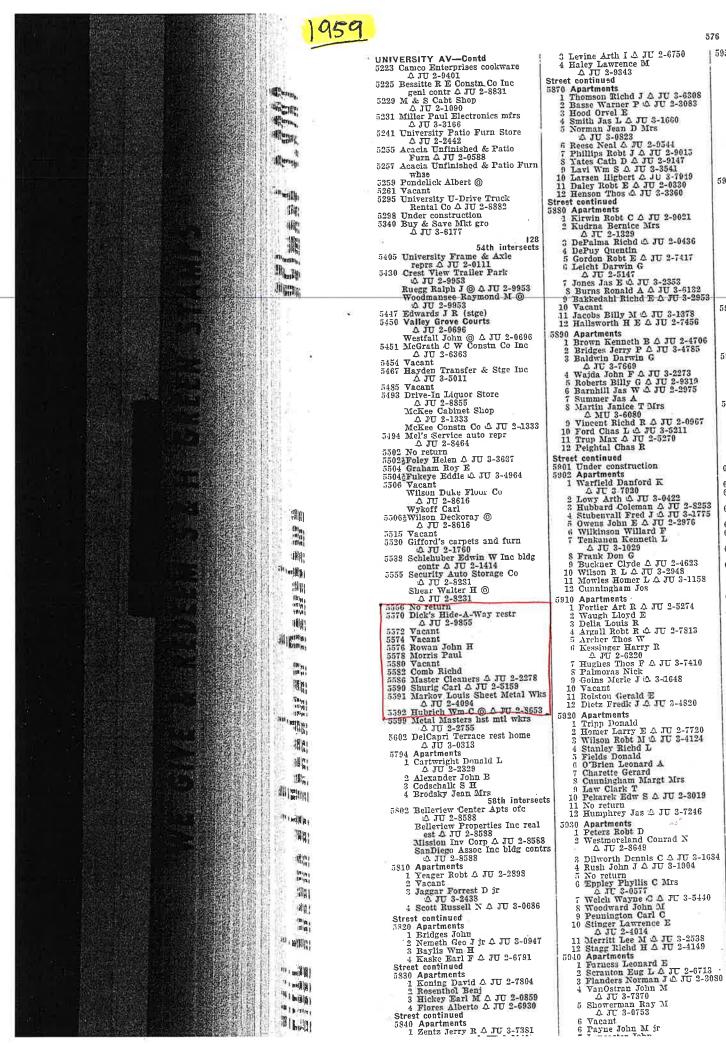
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5 Mason Arlene Mrs & JU Higginbotham Louise Salgado Joseph P & JU 2-6125 6 Payne Lucille Mrs 7 Luce Carl P & JU 2-4980 8 Barnes Noble D 9 Vacant Campbell Edw J & JU 2-3549 10 Runyon Robt L A JU 2 11 Lepore Louis R jr △ JU 2-3203 8 Campbell John R & JU 2-6919 12 Laurent Theo F A JU Street continued 5950 Apartments 1 Divers Omar F jr △ JU 2-7518 2 Wilson Horace E 3 Chlpman Virginia R Toomey Robt J & JU 2-5209 △ JU 2-6852 Vaughan I Worth & JU 5 Bowen Kenneth E Revell Alvin Y & JU 2-0877 6 Hardin Wilbur G △ JU Hirko Andrew A JU 2-8417 7 No return Leicht Darwin G & JU 2-5147 8 Riley June R Mrs Walsh Dennis G & JU 2-2168 9 Hudson Wm L 10 Wren John G 11 Vestergaard Julius L A J 2-3126 12 Francis Joseph S Street continued 5960 Apartments 1 Meininger Joseph A JU Irvine Robt S A JU 2-6274 2 Sall Barry J A JU 2-6-Burnham Edw A A JU 2-0930 3 Homier Randall E Hook Leonard R A JU 2-8586 A JU 2-4093 4 Kergard Jacob K 6 Pepper Jasper O △ JU 2-0015 A JU 2-3273 Melanson Joseph E & JU 2-6051 Greer Jas A & JU 2-492 Henry Audrey G & JU 2-6872 6 Lay Robt C Taylor Loy E & JU 2-4160 7 Cox Sharle Mrs 4 JU 2-8 Vacaut 9 McDonald Stella Mrs △ JU 2-3326 10 Bashluski H M △ JU 2-6746 11 Sayle Clifford T Gump Joseph R & JU 2-5619 12 Snowman E A Brittain Wm J A JU 2-5018 Dempster Lloyd R A JU 2-4156 Street continued 5970 Apartments 1 Cooper Edwin L A JU 2 0 Zackery Walter B A JU 2-5423 7 Mellen Kathleen J A JU 2-8216 2 Kinney Thos H 3 Fellows Fred J A JU 2 4 Vacant Street continued 5980 Apartments 10 English Harley E A JU 2-4673 11 Smith Alf E A JU 2-5419 1 McOullien 0 G 2 Surber Jeff J △ JU 2-1-3 Huston Margt Mrs 4 Dadmun Sarah E △ JU 2-8741 Street continued 5990 Apartments Queen Edwin C Wragg Burton A A JU 2-3647 Barrington Harold N Szymczak Robet C & JU 2-0138 Carroll Kenneth C & JU 2-7791 △ JU 2-0602 3 Finley Reece E 7 Noble Doris T Mrs A JU 2-1967 4 Robinette Nell Mrs A JU 2-8817 10 Declerco Maurice A JU 2-5070 Street continued 11 Joyce Claude C A JU 2-8605 6060 Redwood Baptist Church 6080 Star Clurs & JU 2-2688 6082 Village Furn A JU 2-552 6084 San D Water Softener Quinte Roger L A JU 2-6033 A JU 2-5757 6086 Nutritional Food Serv gr 4 Wright Coley T 5 Minizak Walter 6 Keller Clarence S jr 7 Coppinger James W 4 JU 2-1209 A JU 2-6233 6088 Earley & Wyman ins ∆ JU 2-8030 Whitmore F R real est A JU 2-6061 6090 House of Helenc Beauty 10 Pekarek Edw S & JU 2-3019 ↓ JU 2-2191 6098 Owen's Richfield Service sta 4 JU 2-9808 1955

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NEUN	4716 Apts—Contd 3 Strickland Luther J	5259 Pondelick Albert @	4 Vacant	5940 Apts-Col 10 Wallace I 11 Vacant
SIGN	△ AT 1-7905 4 McCallister Richd V	A JU 2-0588 5261 Miller LeRoy CA JU 3-1571 5295 University U-Drive Truck	5850 Apartments 1 Stern Carl A JU 2-7497 2 Korsmo Ernest M	12 Laurent : 5950 Apartmen
CO.	AAT 2-5775 Street continued	Rental Co 4 JU 2-8882	3 Rathje Marvin J 🔘	1 Dibble M 2 Vacant
LUMINAD	4717 Kingdom Hall of Jehovah's Witnesses	54th intersects		3 Scarpati 4 Vaughan
CORP.	4718 Shannon's Five Ten & Twenty- Five Cent Store A AT 2-6306	Material Co 4 JU 2-6363	5870 Apartments	5 Mason Ju 6 Hardin V 7 Wood Flo
	4725 Kings Uphol A AT 2-8768 4726 Vacant	reprs 4 JU 2-0111	1 Higginbotham Louise	7 Wood Flo S Bendixor
452 8th Ave.	4727 Tiny's Place tavern \Q AT 4-9773	5423 Chauncey's Dining Room	2 Salgado Jos P Δ JU 2-6125 3 Judis Billy F @ Δ JU 2-6392 4 Judis Wm F	9 Deal Cha
BElmont	4730 Paddock Cafe The tavern	5429 Wakefield Harry E Δ JU 2-5801 5430 Crest View Trailer Park	4 Norman M E 5 Jordan Ausvell & JU 2-3236	10 Victor D 11 Vesterga A JU 1
3-7194	4734 Vacant 4736 Three A's Cafe restr	A JU 2-9953 Ruege Ralph J @	7 Potts W Gene A JU 2-3166 8 Campbell John R A JU 2-6919	12 Francis 5960 Apartme
	4737 Vacant 4739 Rene Shop Δ ΔT 3-2160	Woodmansee Raymond M @ Breeding Chester Δ JU 2-5856 Chiles Miles B Δ JU 2-2294	9 Hall Tex J A JU 2-0849 10 Hansen Hans C A JU 2-3782	1 Eshlemat 2 Pangbur
SIGN	4741 Blshop's Vacuum Sls	Dover Howard A JU 2-3274 Elliott Jos H A HU 2-1177	11 Finley Jas R A JU 2-2843 12 Meindl Oswald J	3 Fields W 4 Vacant
REPAIRS	4743 Leisure Time Shop The A AT 1-5710	Gordon Chas A JU 2-1549 Kobush Russell J A JU 2-3356	5880 Apartments 1 Toomey Robt J A JU 2-5209	5 Greet Ja 6 Lay Rob
RENTALS	4744 Cowan & Son Signal Serv 4745 Parker Howard A druggist	Milton Howard T 4 JU 2-1409 Pool Eug B	2 Kudrna Bernice Mr A JU 2-1329	7 Cox Sha McGover
REPAINTING		26 Porter Esther M A JU 3-1171	3 Levin Stanley △ JU 2-4625 4 Derry Thos A △ JU 2-5190	S Cook Pat 9 McCullor
ERECTIONS	Euclid av intersec 4749 Vacant	Shuford Hazel Mrs	5 Hirko Andrew A JU 2-8417 6 Leicht Darwin G A JU 2-5147	10 Welsch I 11 Sayle Cl
LETTERING	4750 Bay City Liquor House	ts 0 JU 2-7918 Snyder Arth 0 JU 2-6438 Street continued	7 Shrand Linda E Mrs 8 Leonhardt Fredk M	12 Douglass 5970 Apartmo
TRUCKS	4751 Vacant	5447 Edwards Jos R	A JU 2-2586 9 Kozina Martin J	1 Vacant 2 Kinney 7
OFFICES	7452 Silverado Ball Room A AT 4-0137	5450 Valley Grove Courts Westfall John @ △ JU 2-0696 5451 McGrath Construction & Ma-	10 Parr John W 11 Boungiorno Salvatore	3 Howic H 4 Mott Hat
WALLS	Joy Dance Studio \triangle AT 4-0137 4756 Tower Furniture (br)	terial Co A JU 2-6811 5467 University Showcase & Fixture	12 Vacant 5890 Apartments	5980 Apartme 1 McQuille
	AT 1-4761 4757 Tower Cafe cocktails	Co A JU 2-4626 5485 Magoffin G A Quality Pimbr	1 Meagher Lawrence	2 Nethers 3 Dugan A
GOLDLEAF ETC.	 △ AT 4-0158 477.1 Bech Iris A real est △ AT 2-8152 ✓ Verleel Helen Mrs real est 48th interset 48th interset 48th interset 	A TU 2-8080	2 Irvine Robt S & JU 2-6274 3 Shinansky Michl K	4 Dadmun A JU
ETC.	A AT 2-8152 Vericel Helen Mrs real est	5493 Drive-In Liquor A JU 2-8855 McKee Cabinet Shop	4 Bonjorno Tony 5 Werba Robt G Δ JU 2-4600	5990 Apartme 1 Wise Ger
ò	48th intersec 4804 Buck Jas E @ A AT 4-2947	1 9494 MELS GOLVICE TOTO TODI	6 Pepper Jasper O \triangle JU 2-0015 7 Melanson Jos E \triangle JU 2-6051	2 Vance Jr 3 Finley R
	4809 Hammond Olin 4811 Felde Richd	40 JU 2-8464 5502 Vacant	S Holland Glenn A JU 2-6872	4 Moore J Street continu
	4813 Schmidtke Henry A @ 4815 King Walter C A AT 2-0790	5502hClegg Edwin 5504 Paradise Godeen	9 Slms Eug C 10 Pendzich Anthony 11 Sauer C Wm	6002 Sequola
	4818 Middleswart Elmer B ⊚ ☆ AT 1-4877	55042Crocker Jas 5506 Am Housing Gulld Δ JU 2-6656 Wilson Floor Co Δ JU 2-8616	11 Sauer C Wm 12 Reinke Kenneth A JU 2-2299 5902 Apartments	6060 Redwood
	4819 Hudson Ed E Nursery	Wilson Duke	1 Nelissen Fred A JU 2-8345 2 Abaire Archie L	6080 Star Cin 6082 Village J
	Hudson Ed E @ 4834 Miller Geo E @ A AT 2-2317	Wilson Deckoray @	3 Vacant 4 Owens Otis E △ JU 2-3192	6084 Ellison's
VELLE		A JU 2-0877	5 Owens John E 6 Najors Carl N A JU 2-6686	foods
KELLE	Estrella av intersed 4844 'A-1 Western Mattress Co		7 Cono Danl Δ JU 2-SS14 S Samson Frank R Δ JU 2-0650	6088 Vacant 6090 House of
CLEANERS	A AT 1-7727 4891 Marshall Gennie L Mrs	5538 Schlehuber Edwin W Inc bldg contr Δ JU 2-1414	9 Gantz Forrest F Δ JU 2-7589 10 Graham Chas R Δ JU 2-7087	A JU 6098 Owen's I
AND	A AT 1-6508 49th intersed	ts Shear Walter H @ A JI 2-S231 Shear Walter H @ A JI 2-S231	11 Underwood Glenn H	sta A
LAUNDRY	4905 McGrath Plumbing & Htg Co	5556 Young Boht L / LL Z-488h		6101 Standar
LAUNVET	4918 Colby's Paint & Body Works 4925 George's Auto Repr	5570 Taste & Tell restr A JU 2-9754 5572 Vacant	2 Wade Oscar F A JU 2-8735 2 Roat Robt A A MI 2-8167	6104 Rolando
-	4951 Logsdon Louise E		4 Anderson W Carlisle 5 Szymczak Robt C A JU 2-0138	A JU 6111 East Gat A JU
	4953 Schallhorn Wm F 4954 Hilderbrand Texaco Service ga	5578 Vacant 5580 Stratton C Mrs	6 Kessinger Harry 0 7 No return	6114 Odom W 4 JU
Serving	sta A AT 4-1478 4957 Lansaw Cath Mrs @	5586 Mexter Cleaners A JU 2-2278	8 Palmares Helen Mrs 9 Heath Wm D	6118 Miller's
San Diego		5590 Shurig Carl A JU 2-5159 5591 Markov Louis Sheet Metal	10 Rodriguez Marcelind 11 Joyce Claude C A JU 2-8605	6122 Rooster
Since	4961 Simpson Harry E pntr	Wks & JU 2-4094 5592 Hubrich Wm C @ A JU 2-8653 5595 University Ave Welding &	12 Vacant 5920 Apartments	6141 Russell
1917	AT 4-68718 4963 Vacant	Refrigeration	1 Leinbaughs Ernest W 2 Barrowclough Crispine	Sup D 6143 Vacant
1211	4968 Rutherford's Mkt gro	Baker Winfield S ©	3 Gere Fay R @ 4 Yost Stanley	6160 Withem'
•	Δ AT 1-1430 4969 Salerno Music Studio tchrs Δ AT 2-7333	5599 Metal Masters mtl wks	5 Breeson Robt C \triangle JU 3-1346 6 Daniels Wm E	6172 Modern 6177, Ball The
N A ¹	4973 Thomas Furn Co ↔ AT 1-8122 4975 Lee's Music Co ↔ AT 1-8122	1 1 Cartwright Donald L	7 LeBaron Richd D 8 Partlow Jos A	6178 Villa Vi
Rug Cleaning		Δ JU 2-2329 2 Whitfield Robt W Δ JU 2-6936	9 Fowler Maurice E 10 Pekarek Edw S A JU 2-3019	61S0 Foster's
Pillow-	Hallick-Vaughn Dance Studio 4981 Buckner Mfg Co Inc	4 Brownsky Jean Mis	11 LeMaster Shirley W	6183 Butler's 6184 Esquire-
Renovating	A AT 3-2223 4985 Vacant	58th intersect 5802 Belleview Center Aparments	5930 Apartments 1 Vacant	6185 Verzin I A JU
Dry Cleaning	5002 Poplar Supply & Rental Serv	Belleview Properties Inc real es		6187 Mel's re 6188 C & B 1
Laundry	jlmb sups △ AT 1-9195 5045 Vacant	A JU 2-8588 Mission Inv Corp A JU 2-8588	4 Vacant	6189 Vacant
j	5047 Vacant Oakcrest dr beg	ins San Diego Assoc Inc blug contr	6 Bray Gail K 7 Born Bruce	
	5049 Maddox Jos F 5051 Navak Jos A A AT 4-1484	5S10 Apartments 1 Sofrank Julius A A JU 2-6074	S Vacant	6191 Marshal 6192 Village 6195 Consum- furn 4
PHONE	5053 Galloway David J A AT 2-833 5055 Kesler Herman J A AT 3-217	5 3 Sides Harry A	9 Vacant 10 Fuller Eslie Mrs	6197 Vacant
BElmoni	5057 Relly Michl J & AT 1-8541 5067 Lowry John E A AT 4-4508	4 Stills Louis 5820 Apartments	11 Chamberlin Gordon L 12 Stagg Richd H A JU 2-4149	6198 Karzen A JU-
3-7452		1 Eide Henry J A JU 2-1753 2 Nemeth Geo J jr	5940 Apartments 1 Farness Leonard E	619ShVacant
	5109 Vacant	3 Sutherland Paul R Δ FU 2-373 4 Guerin P C	1 3 Elanders Norman J	6201 Joe's Co
KETTNER	5150 SD Pipe & Sup Co A AT 2-81 52d interse	cts] Tappan Wm 4 JU 2-2911	A JU 2-3080 4 Mabry Herman R 5 Mason Arlon Mrs A 1U 2-3098	6205 Auto Se
and	5225 Vemlch Sam mason contr A JUI 2-7787	2 Sawicki Paul P 3 Vacant	5 Mason Atlene Mrs & JU 2-3098 6 Payne John M jr 7 Brunello Edw W	6206 Linschel 6210 Valade
		1 Mianov Alborto 0 bit 9-6920	7 Brunelle Edw W	DT A JU
GRAPE ST	5. 5229 M & S Cabt Shop A JU 2-105 5241 University Patho Shop play-	5840 Apartments I Bruce Chas V A JU 2-6253	8 Vacant 9 DeGuide Jas J	6211 Vacant



3 Nimme 4 4 Dinstand 5 Stunich 6 Taylon 0 A Th 7 Roberta 9 Deal Con 10 Colonal 11 Vestoria 12 Francis 950 Apprendix 5960 Apartmin 1 Vacant 2 Pangburn 3 Schneides 4 Horan 5 Lauson 1 6 Vacant 7 Thompson 7 Thompson A 710 5-8 Vacant 9 Fasterlin 10 Vacant 11 Harrison 12 Schirt Sm 5970 Apartner 1 Hoyt Ge 2 Piemlin 3 Reed Built 3 Reed Bi 4 Richard 2 Alcourtes 5980 Apartmet 1 McQuillas A JU 1 2 Brooks 00 3 Vacani, 4 Dadman 5990 Apariman 1 Baker 6 2 Vance J 3 Carroll 5 4 Moore Ma Street continue 6002 Sequela 1 Δ , JU 6060 Redwood Δ JU 6050 Star Clea 6082 Village H Δ JU 2002 6086 Farmates AJUS ATT 6101 Standard CIUI 6111 Vacant 6114 Odom W A JU 1 6118 Miller a A JUIZ 6122 Roostor 6134-50 Rand gro & 1 6141 Russell 1 AJU 1 6157 Modern In 6159 Bell's C Wear 6160 Withem 6165 RodiecE A JUT 6167 Spl Serve ATU 6172 Sure Silve A TU-6178 Villa Yat A JU 6179 Ball Crob 6130 Foster's b A JU 28 6183 Butler's 1 6184 Esquire C 6185 Versin Pe A JUN Δ JU.2 6190 Carpei W 6191 Marshall 6192 Village J Δ JU.2 6195 Cons furn,



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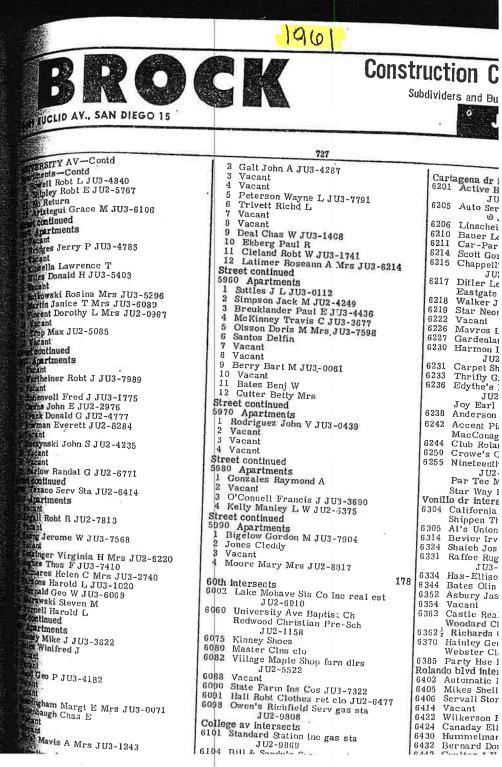
5950 Apartments 1 Albrecht Ken C 2 Shaver Glenn A 3 Reed Hubert D Nechesnoff And Smith Keith Trivett Richd F Vacant 67 7 Vacant S Falanga Shirle: A JU 3-6577 9 Deal Chas W 4 10 Armstrong Rob A JU 2-8134 11 Vestergaard Ju A JU 2-3126 12 Francis Jas S. Δ JU 2-3126 12 Brancis Jns S. 5960 Apartments 1 Miller Gary L 2 Pangburn Don Δ JU 2-2510 3 Schneider Edw 4 Horan Thos J 5 Lauson Lloyd 6 Peters Kenneti Δ JU 3-2160 7 Provost Willie 8 Griffin Clark J 9 Jones Donald . 10 No return 11 Bartlett Jos C 12 Wikinson Joh Δ JU 3-6785 5970 Apartments 1 Hoyt Geo Δ JJ 2 Adams Anthon 3 Humphrey Ru 4 Ekberg Paul J Δ JU 2-330' 2 Brooks Glen (3 Hicks Oliver 4 Dadmun Sara 4 Dadmun Sara 5990 Apartments 1 Vacant 6060 Redwood Bap \triangle JU 2-115 6080 Star Cleaners 6082 Village Mapk \triangle JU 2-552 1. JU 2-352 6086 Farmstead Fc Δ JU 2-62: 6088 Chapman's B 6090 Betty-Rae Be Δ JU 2-16' 6091 Hall Robt Cli Δ JU 2-64' (000 Ω στω)2 Bichf A JU 2-76 6114 D & M Engl electronics electronics 6118 Central Fed Assn A JU 6122 Rooster Inn A JU 2-98 6134-50 Gordon's A JU 2-34 6141 Russell E S Sup A JU 6141 Russell E S Sup & JU 6157 Modern Dru: 6159 Bell's Child Wear & J 6160 Withem's E: 6161 Sportsman's A JU 2-25 4. iT 2-2; 6163 George's Ha Δ JL 2-2; 6165 Rodieck's P Δ JU 2-4; 6167 Spl Serv Su Δ JU 2-4; 6172 Sure Sleep Δ JU 2-6; 6175 Villa Variet Δ JU 2-7; 6179 Bali Club T 6179 Bali Club r 6180 Fred's Liqu \triangle JU 2-2 6183 Butler's Ba 6184 Esquire Cle Δ JU 2-t 6189 Mohling F Δ JU 2-(6190 Carpet Woi 6191 Marshall's 6192 Village Lai Δ JU 3-(6901 Active Hea

	100			Δ AT 1-5429	i ÷
	800	10/nD	4743	Leisure Time Shop The hobbles	5210 Camco Enterprises
	18LJ	1100	4744	Sperry Bros Automotive Serv	△ JU 2-9401
			4745	gas sta & AT 4-9772 Parker Pharm & AT 4-4288 [26]	5221 Vacant 5223 Am Libr of Sacred A JU 2-7460
ŀ		XC	4749	Euclid av intersects Big City Liquor & AT 4-2836	5225 Bessitte R E Constr genl contr A JU
t.		2		Reno dr intersects	5229 M & S Cabt Shop
	aw) Ht	10	4752	Reinohl Idabelle P Mrs	5231 Miller Paul Electro
	ing a			Baton Plastic Laminating Serv	5241 Univ Patlo Furn St
	H)	1.0		menus A AT 2-0200 Lucille's Dance Studio	5255 Acacia Unfinished Furn Δ JU 2-058
j,		141	1.00	A AT 4-0137 Silverado Ball Room	5257 Acacia Unfinished Furn whse
		3		A AT 4-0137 Tower Furniture	5259 Pondelick Albert @ 5261 Vacant
		9	1757	AT 1-4761 Tower The cocktails	5295 University U-Drive
		9		A AT 4-0158 Cafagua's Flying A Serv gas sta	Rental Co A JU 2 5296 Johnson's Texaco S sta A JU 3-6678
		5		A AT 4-5909 Dumont Constn Co contrs	5340 Buy & Save Mit gr
		8		A AT 2-2167 Beck Iris A real est	-∆ JU 3-6177 541
		17 (2	1,01	AT 3-4727 48th Intersects	5405 University Frame &
			4801	Capri Homes bldg contrs Å AT 4-1173	5405 University Frame & reprs Δ JU 2-011 5420 McGrath C W Const mal Co Δ JU 2-05
		2	4804	Buck Jas E @ A AT 4-2947	rial Co & JU 2-63 5130 Crest Vlew Trailer & JU 2-9953
			4809 48093	Kaiser Walter G Universal Sewing Mach Co	Ruegg Ralph J (0) 4
		S		A AT 2-2158 Vacant	Woodmansee Raymo
		al and a second s	4S15	Wolf Peter A AT 1-8082 Rooney Hazel G Mrs	5450 Valley Grove Courts Westfall John @ A
	11			Baker Plmb & Heating	5451 Vacant
	4		4819	Pilley Real Estate Δ AT 1-7709 Dilley Con W. C. AT 1 0020	5454 Vacant 5467 Hayden Transfer & 5467-85 Drapery Corner of
			4830	Pilley Geo W () A AT 1-0039 Enright Douglas E	JU 2-5608 5493 McKee Cabinet Shop
	-1			A AT 3-2789 Johnson Elliott L A AT 3-3414	ے JU 2-1333 McKee Constn Co
		040		Miller Geo E (AT 2-2317 Miller Geo E real est	5494 Mel's Service auto r
	1940	110 - 110 -		A AT 4-9224 Miller Piano's used	5502 Lauer Jos W 55023Provan Caroline J 4
	al			△ AT 4-9224 Estrella av intersects	5504 Wilson Mark F 55041Hullinger Gary H
	#1	8 a 1		A-1 Western Mattress Co A AT 1-7727	5506 Carl Club tavern Δ Wilson Duke Floor (
		÷		Dean's Photo Serv	△ JU 2-8616 Wykoff Carl
		3.4	4891	Franken Used Cars A AT 2-8608	55061Wilson Deckoray ()
		1	4905	49th Intersects Four Star Radio & TV Co	5515 U F A Towing & St 5520 Gifford's furn A JU
			4914	△ AT 2-0729 Minnis Vacation Bentals	5538 Schlehuber Edwin W contr △ JU 2-1414
		a.	4918	trailers A AT 1-3158 Colby's Paint & Body Works Lucky Boy of SanDiego restr	5555 Security Auto Stora
		2 I		4 A1 3-4020	Shear Walter H ()
	5	1	4925	George's Garage auto repr A AT 4-6992	5336 Martinez Bennie 5370 Millie's Place tavern Δ JU 2-9855
	E.	1	4951	Winona av Intersects Tripoli Barber Shop	5572 Sherry Frank 5574 Joyner Jerry
		-	4953	△ AT 4-9078 Mickey's Beauty Salon	5576 Carey Thos
			4954	A AT 1-7138 Hilderbrand Texaco Serv gas	5578 Samara Manuel A 5580 Marshall Chas L
		5 2		sta A AT 4-1478 Beta Phi Sigma McEleonor Tenia B	5582 Gutierrez Bertha Mr 5586 Master Cleaners Δ J 5590 Shurig Carl Δ JU 2-
	41	<u>.</u>	4961 4963	McElvaney Tenic B Maja Multilith Printing Serv	5590 Shurig Carl 4 JU 2- 5591 Markov Louis Sheet Wks A JU 2-4094
	31	3	1000	A CY 7-3003 Smootz Franklin P A CY 7-3003 Desring's MDt Λ AT 1-1430	5592 Hubrich Wm C 🛞 🕰
	9127		4969	Deering's Mkt \triangle AT 1-1430 Salerno's Music Studio	5397 Wright & Assocs of
į.	싺	8	4973	Δ AT 2-7333 Thos' Furn Co Δ AT 1-8122	A JU 2-2755 5602 DelCapri Terrace res
		8. 4	4977	Thomas Jas P () \triangle AT 4-1524 Wood Wire & Mtl Lathers	A JU 3-1993 5791 Apartments
		-		Internatl Union Local No 260 Nolan Arth B	1 Cartwright Donald I
52		к:	4981	Bacon Piano Studio mus tchr Buy & Sell Post newspaper	2 Alexander John D
				AT 4-9275 50th intersects	3 Edgar Clifford L
	220	25	5002	Poplar Supply plmb sups	4 Brodsky Jean Mrs 58t
	131	*	5045	Δ AT 1-9195 Turner Virginin L bkpg serv Δ AT 1-6453	5802 Belleview Center Ap Belleview Properties
		(4)	5047	Wofford Michl A AT 1-6277	est \triangle JU 2-8588 Mission Inv Corp \triangle
			504 9	AT 4-3292	SanDiego Assoc Inc
				Karmazin Wm R jr A AT 2-8007	5810 Apartments 1 Yeager Robt △ JU 2
	A		5055	Higgins Arnold R & AT 3-3631 Tolun Edith	2 Vacant 3 Ladd Oliver M Δ JU
		2	$5057 \\ 5067$	Reilly Michl J & AT 1-8541 Medina Estanisalo N	4 Scott Russell N A JI 5820 Apartments
			5085	Hattabough Chester A ⊚ △ AT 1-2S93	1 Vacant 2 Nometh Geo J jr △ J
17.					

208 52d intersects	2 Vacant 3 Kirk Jack R A JU 2-7951	2 Fletcher Lawrence 1 3 Rutledge David L
Enterprises cookware J 2-9401	4 Flores Alberto A JU 2-6930 Street continued	4 Przybylski John D ζ 5 Farness Dennis N Δ
br of Sacred Records	5833 Mayfair Mkt gro	6 Eppley Phyllis C Mr
J 2-7460 e R E Constn Co Inc	5840 Apartments 1 Vollebregt Frans & JU 2-4601	7 Williams Robt C A 8 Vacant
contr A JU 2-8831 Cabt Shop	2 Bonwell Wm A JU 2-3049 3 Gorter Geert H A JU 3-4834	9 Pennington Carl CJ
J 2-1090 Paul Electronics mfrs	4 Vacant 5850 Apartments	10 Stringer Lawrence 1 11 Toomey Cornelius E Merritt Lee M A JU
J 3-3166 atio Furn Store	1 Grenville John B A JU 3-4692 2 Armstrong Leo D	12 Stagg Richd H & JU 12 Huchinson Edw J
J 2-2442	Δ JU 2-7023 3 Levine Arth I Δ JU 2-6750	Street continued
Unfinished & Patio	4 Bakst Aaron A JU 2-0310 5S70 Apartments	5933 University Lanes In
Unfinished & Patio	1 Galloway Frank	5940 Apartments 1 Farness Leonard E
ick Albert @	2 Holbein Robt E 3 Hood Arvel E A JU 3-7636	↑ JU 3-2590 2 Rogers W H ↓ JU 8-
sity U-Drive Truck al Co A JU 2-8882	4 DeLuca Emilie B Mrs	3 Flanders Norman J 4 VanOstran John M
n's Texaco Serv gas JU 3-6678	5 Gilbert Kenneth B ↔ JU 2-5704 6 Hinojosa Rudy	△ JU 3-7370
Save MLt gro 3-6177	7 D'Agostino Anthony C 8 Vacant	5 Showerman Roy M 2 6 Strahan Jas
128	9 McCoy Robt E 10 Cavanaugh Crescent P	7 Lancaster John Δ J 8 Bergh Wm E Δ JU 2
54th intersects sity Frame & Axle	Δ JU 3-7367 11 Daley Robt E Φ JU 2-0330	9 Vacant 10 Armstrong Robt B
△ JU 2-0111 h C W Constn Mate-	12 Kramer Chas 5880 Apartments	10 Armstrong Robt R Δ JU 2-8134 11 McKenny Robt 12 Russell Rex D Δ JU
Co & JU 2-6363 Ylew Trailer Park 7 2-9953	1 Jonson Betty D Mrs 2 Kudrna Bernice Mrs	12 Russell Rex D A JU 5950 Apartments
Ralph J @ 4 JU 2-9953	△ JU 2-1329	1 Bishop Jas W A JU 2 Galt John A A JU 3-
ansee Raymond M @ 2-9953	3 DePalma Richd A JU 2-0436 4 Beddoe Eliot L	3 Delia Anthony Μ Δ. 4 Vacant
s J R (stge) Grove Courts	5 Gordon Robt E & JU 2-7417 6 Mitchem Hampton R	5 Peterson Wayne L
1 John ⊚ △ JU 2-0696	△ JU 3-7283 7 Vacant	6 Trivett Richd L A J
Transfer & Stge Inc	8 Vacant 9 Howell Robt L △ JU 3-4840	7 Eccker Jack W A JU S Falanga Shirley A M
2-5608	10 Greogrick Wm 11 Ferguson Ralph C	Δ JU 3-6577 9 Deal Chas W Δ JU 3
Cabinet Shop	A JU 3-7994 12 Vacant	10 Coleman Claude T A 11 Vacant
2-1333 Constn Co 🗅 JU 2-1333	Street continued 5890 Apartments	12 Francis Jos S A JU :
ervice auto repr 2-8464	1 Vacant 2 Bridges Jerry P △ JU 3-4785	5960 Apartments 1 Suttles J L A JU 3-0
los W Caroline J & JU 3-3637	3 Moses John J 4 Wajda John F A JU 3-2273	2 Vacant 3 Roegler Marie H Mrs
Mark F or Gary H	5 Stiles Donald H A JU 3-5403 6 Weber Otto W A JU 2-5337	4 McKinney Travis C
ub tavern A JU 2-9800 Duke Floor Co	7 Vacant	5 Olsson Doris M Δ JU
2-8616 Carl	A JU 3-6080	6 Krawczyk Frank R
Deckoray (0) 2-8616	9 Vincent Richd R & JU 2-0967 10 Carmichael Fred B	7 McDaniel Steven B]
Towing & Stge s furn A JU 2-1760	△ JU 3-6025 11 Trup Max △ JU 2-5085	8 Griffin Clark R Δ π 9 Vacant
ber Edwin W Inc bldg △ JU 2-1414	12 Peightal Chas R 5902 Apartments	10 Stanford Donald E j
y Auto Storage Co 2-S231	1 Warfield Danford K	11 Okey Walter L A JU
Valter H (0)	2 Wescott Dan H 3 Vacant	12 Synja John & JU 3-7 5970 Apartments
2-8231 z Bennie Place tavern	4 Stubenvoll Fred J	1 Rodriguez John V 2 Adams Anthony
2-9855	5 Owens John E & JU 2-2976	3 Humphrey Ruth E M △ JU 3-1511
Frank Jerry	6 Wilkinson Willard F 7 Owens Riley L	4 Ekberg Paul R jr
Manuel A	8 Frank Don G 9 Watson Robt D A JU 2-4275	5980 Apartments 1 Vacant
ll Chas L ez Bertha Mrs	10 Haygood David C	2 Gonzales Tony A E 4 3 Vacant
Cleaners & JU 2-2278 Carl & JU 2-5159	11 Motheral Chas W Δ JU 3-5550	4 Vacant 5990 Apartments
Louis Sheet Metal JU 2-4094	12 Bremerman Harold R	1 Bigelow Gordon M
Wm C (A JU 2-S653 & Assocs of c sups	5910 Apartments 1 Yacant	2 Vacant 3 O'Connell Francis J
2-1530 fasters sht mtl wks	2 Vacant 3 Young Jerome W	A JU 3-3690
2-2755 A Terrace rest home	Δ JU 3-7568 4 Argall Robt R Δ JU 2-7813	4 Moore Mary Mrs A JU 2-8817
3-1993 ents	5 Archer Thos W 6 Kessinger Harry R	601
ght Donald L 2-2329	Δ JU 2-6220 7 Hughes Thos F Δ JU 3-7410	6002 Townsend L O Bealt
ler John D 2-7060	S Palmares Helen C Mrs	6060 University Ave Bapti Redwood Christian P
Clifford L 3-6648	9 Elsner Gerald R 10 Fernald Geo W A JU 3-6069	△ JU 2-1158 6075 Kinney Shoes
Jean Mrs 58th intersects	11 Vacant 12 Glimore Richd V	6080 Master Cins Clo
w Center Apts ofc w Properties Inc real	5920 Apartments 1 Vacant	60S2 Village Maple Shop 1
JU 2-8588 Inv Corp 4 JU 2-8588	2 Homer Larry E A JU 2-7220 3 Bush Melvin R	6088 Chapman's Barber Sl 6090 State Farm Ins Cos
2-8588	4 Meyers Audrey Mrs A JU 2-9195	△ JU 3-7322 6091 Hall Robt Clothes ret
ents Robt A JU 2-2898	5 Wood Geo P A JU 3-4182	A JU 2-6477 6098 Owen's Richfield Ser
liver M A JU 3-0592	6 O'Brien Leonard A	A JU 2-9808 College a
ussell N A JU 3-0686 ents	S Cunningham Margt E Mrs	6101 Standard Station Inc
Geo J jr △ JU 3-0947	9 Vacant 10 Bonafede Jos P 🛆 JU 3-7681	6104 Rolando Serv gas sta

	726
ontd	4749 Big City Liquor AT4-2836
C optom A T4-3892	Reno dr intersects
niv Highland br)	4751 Reinohl Idabelle P Mrs ATI-0150
1 ts	4752 Fleming Phyllis College of Baton
er Mkt AT4-9463	ATI-0437 4755 Plastic Laminating Serv menus
7 Bar AT1-0904	AT2-0200 4756 Lucille's Dance Studio AT4-0137
(br ofc) AT1-5541	4756 Lucille's Dance Studio AT4-0137 McCann Furn AT3-6146
eld Serv Sta AT4-9641	Silverado Ball Room AT4-0137
) Pit AT1-0444	4757 Tower The cocktails AT4-0158
e Meats AT2-8181	4770 Flying A Club Serv AT3-6025
rn AT4-9592	4771 Vacant
- aban	4781 Beck Iris A real est AT3-4727
<pre>shop)</pre>	48th intersects
otive Serv repr	4801 Capri Homes bldg contrs AT4-1173
5	4804 Bakers Plmbg AT4-2947
e dlr AT2-2303	Baker Don
	4809 Dugger J D 4809 $\frac{1}{2}$ Brite Jack Telev repr AT4-8200
In restr AT4-1513	4811 Gyton Max
lub tavern AT4-9483	4813 Duimstra Betty AT1-5147
ng & Distrg Co 6	4815 McAmes Leslie C
M Mrs AT4-6876	4818 Baker Albert R AT3-4213
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arber Shop AT2-7131	4830 Vacant
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AT4-0148	4834 Miller Geo E © AT2-2317 4836 Miller Geo E real est AT4-9224
a Court	Miller Piano's used AT4-9224
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ana AT4-8435	AT1-7727
	4849 Dean's Photo Serv AT1-6676
S Mrs	4891 Franken Used Cars AT2-8608
ban E	49th intersects 4905 Four Star Radio & TV Co AT2-0729
ermo	4914 Minnis Vacation Rentals trailers
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s sta AT4-6666	4920 Lucky Boy of SanDiego restr
ntal Co Inc AT4-9292	AT3-4020
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3	4953 Mickey's Beauty Salon AT1-7138
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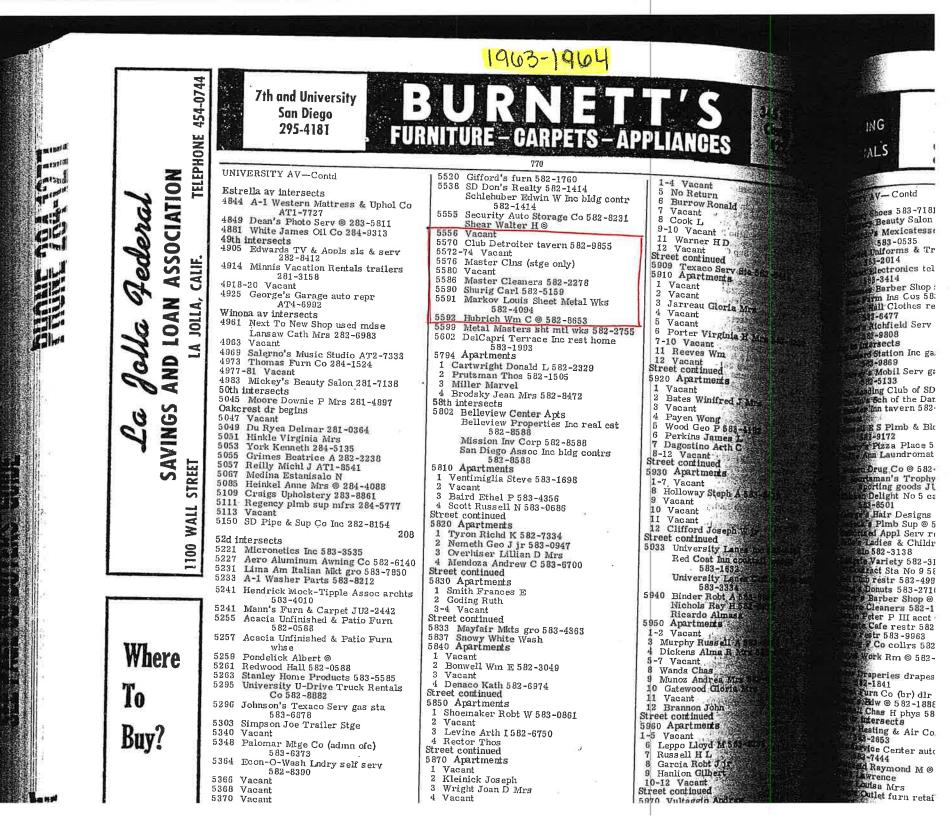
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		5450D VACANT 5450e Vacant	42 NIESEN THOS M 286-2392 44 SHANER FRANK N 286-7068	583-3953 5889 CINERAMA 7 5890 APARTMENTS
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		5590 SHURIG CARL 582-5159 5591 MARKOV LOUIS SHEET METAL WORKS	2 GODING RUTH MRS	12 WHEELER STREET CON
	ža,	582-4094 592 HUBRICH VM C 0 582-8653 5599 COLLEGE SECRETARIAL SERVICE PUB	3 TATE WELCA MRS 582-8269 4 SHARKEY DOROTHY	5920 APARTHENTS
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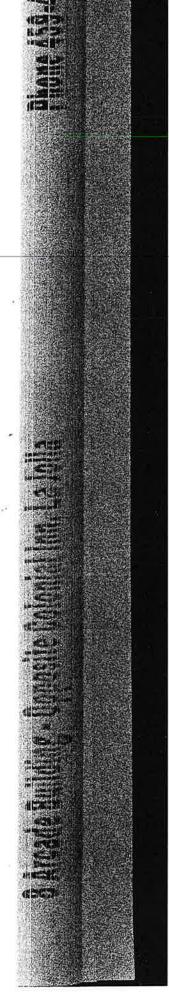
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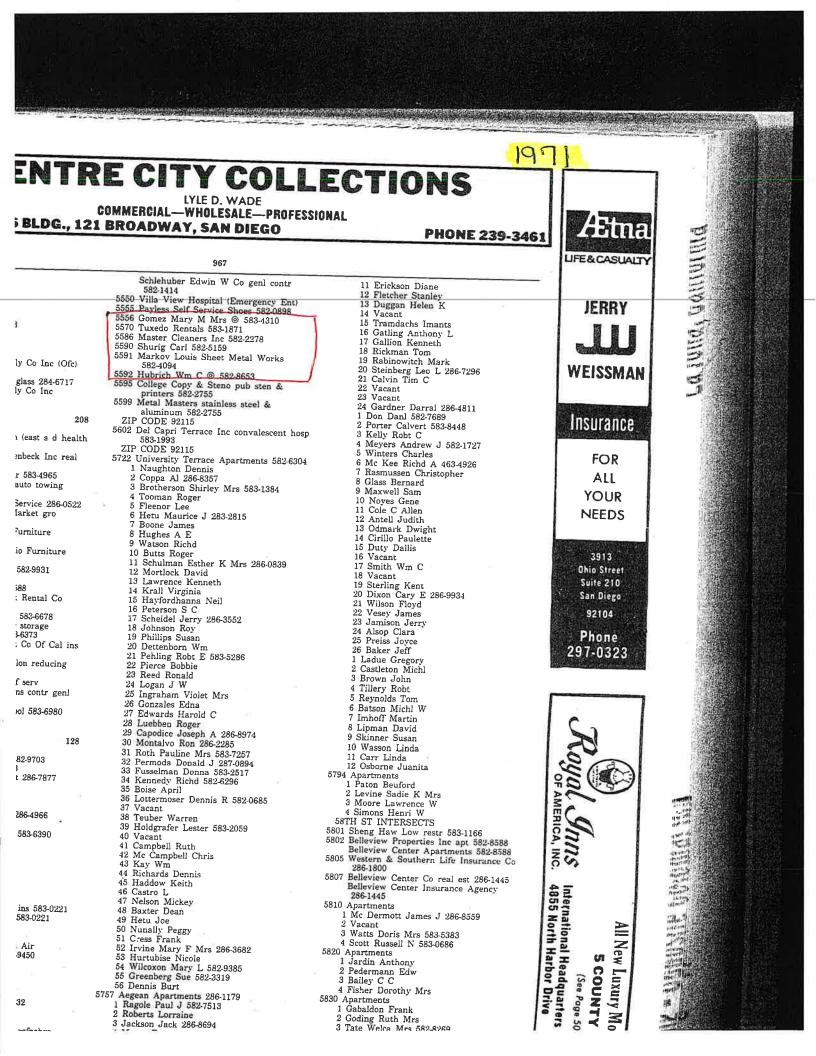
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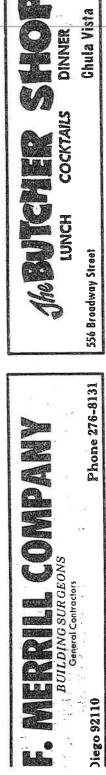
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4689 Anthony's Pizza House 283-8211	5045 Moore Wilson K 281-4897 5047 Haarberg Kent E 282-2391	5430
4693 Hacienda Danceland 282-2530 47TH ST INTERSECTS	5049 Johnson Sue A	
4701 Kirby's Silver Mirror Beauty Salon	5051 Gale Henry G 281-1783	
294-4592 4702 San Diego Electric contr 283-7031	5053 Cullins Kath 5055 Kenny Kath	
4703 Harrison's Card Room 282-2559	5057 Reilly Michl J 281-8541	
4705 Westin Bart Realty 583-7231	5085 Heinkel Anne Mrs © 5109 Vacant	
4711 Sundown Club tavern 284-9320 4714 Vacant	5111 Vacant	5450
4715 J & J Appliances washer & dryer	5113 Antofte Studios stained glass 284-6717	
sls & serv 281-7626 4716 Apartments	5150 San Diego Pipe & Supply Co Inc	
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4730 Paddock Cafe The 284-9359	Walker Hydraulic Jack Service 286-0522	
4734 Synthetic Trips electro- psychedelicart 282-1445	5231 Lima American-Italian Market	545
4736 H & M Cafe	583-7850 5241 George's Bill House Of Furniture	545(546(
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trucking 280-5454 Jerry's Auto Radio & Speedometer	5263 Fancy That Massage 287-5771	
Service radio repr 283-9680	Willey's Gym health conditioning	550
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4804 Baker's Plumbing Inc contr 284-2947 Baker W Don 284-2947	5382 Centracom Computer School 583-69	80 558
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4809 ½ Hayward's T V Rentals 283-1731 4811 Leon Mary A Mrs 281-2181	gas sta 582-9703	
4813 Hope Roy	5405 K-Mart dept store 286-9733	559
4815 Powell Geraldine F Mrs	5415 Cal Bazar Inc gro & meats ret K-Mart Foods gro-meats-ret	559
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4834 Miller Geo E @ 282-2317	Van De Kamp Bakeries Inc	555
4836 Spra-Rite Appliance Refinishing	5422 Villa Verde Apartments 267-4565	559
284-2288 4844 A-1 Western Inc mattress mfrs	1 Vacant (apts 1 Thru 7) 8 Goode Lucy K Mrs	
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4905 George's Garage 284-6992 4918 Professional Tire Co 284-2123	16 Phelan Patricia Mrs 287-5286 17 Vacant (apts 17 Thru 24)	ia -
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4925 George's Garage 284-6992	25 Loizeaux Inez Mrs 287-5915 26 Vacant	• • •
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4969 Salerno's Music Studio mus tchr 282-7333	29 Vacant 30 Hensell Beverly	
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4973 Mission Carpet Distributors fl	32 Vacant 33 Wilson Charles 287-5441	
coverings 283-7109 4977 San Diego Lather's Trust Funds	33 Wilson Charles 287-5441 34 Vacant	
281-7729	35 Switala Bonnie Mrs 582-1275	
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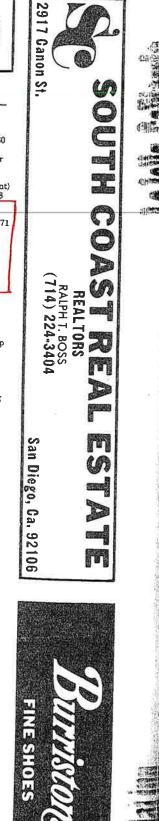
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urance	Local No 28 284-6151	
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11	4979 Hoff Eula School Of Dancing	
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-	4979a Thomas Furniture Co 283-7109	
283-5901	Vacant	
282-7438	4979b Young Paul B & Assocaites Inc	
2082	artificial grass 284-0511	
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	4983 Mickey's Beauty Salon 281-7138	
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	2*Weise Shirley G Mrs 281-4632	
	3*Lovelace Maurice	
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4-4592	7 Vacant	
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34-9320	11 * Gamboa John	
	12*Thompson Alice B Mrs 282-0620	
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ie used	5049 ★ Mitchell Karen 281-1783	
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5 * Porter Carroll 6*Davelos Mary Mrs 7 * Secoy Debbie 583-5558 8 Goode Lucy K Mrs 287-4807 9 Schrieber Diana Mrs 286-7601 10 Ornelas Albert D 287-4491 11 Cerwinski Robt 289-3972 12 # Goodwell Charles 5424 Apartments 13 Green H Lee 582-7047 14 * Sproul Scott J 287-8696 15 * Robinson Paul 16 * Armijo Richard 17 * Anderson Bruce 18 Davy Geo L 286-7659 19★Valletta Robert J 20 * Negrete John 287-7265 21 * Dudley James 22 Kyandal Scott C 582-2544 23 * Burkhart Winton 582-0796 24*Boath Ann 286-4397 5426 Apartments 25 Loizeaux Inez Mrs 287-5915 26 * Harrahill Jesse D 582-1150 27*Goodlatte Peter R 287-4749 28 * Bills Jerri 29 * Young Ronald 30 * Logan Ward 583-9068 31 Johnston Jo A 286-7220 32 * Gardner Kathryn A 287-0275 33 * Tindell Anthony 34 * Little Cindy 35 Escalante Carmen Mrs 286-3454 36 * Arguello Eddie A 286-2046 37 * Orter June M 287-6167 38 Boyer Tim 286-4145 39★Knapp Melvin C 40 Maffel John J 287-2404 41 * Jones Wak 42 Milford Richard 43 * Johnson Paul W 286-9309 44 ★ Hansen Susan 45*Gnadt Paul H 46 Chavis Norman 583-8447 47 * Rudd James C 286-3138 48 Vacant 49 Fox Margt A 583-4634 50 Goldin Robt S 583-1771 5428 Apartments 51 Kler Michael 287-7096 52★Kerr Geraldine M Mrs 53 * Martin Nancy 54 Milliman Diane 582-6478 55 Neuman Carol A Mrs 583-8215 56★Rosenthal Esther M Mrs 287-4967 57 t Donnell La Salle 287-8763 58 # James Tim 59 No Return 60 * Hight Gordon R 286-6919 61 Kane John A 286-4563 62*Bravile Josephine 63 Sinclair Jean 64 * Hull David W 287-7256 65 # Reddell Marty 286-8352

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TRSITY AY-Contd Worrell Robt L 582-3367 facant No Return Vandenberg Gernld D 582-6934 No Return Vacant Banks Q E Sweetser Ethel T 287-4367 Cohen Michl I 287-8675 Estrada M Mejia L D 287-8549 Vicant Storck W G 582-1195 Leung M K 287-5368 Piccolo A Mason Allan M Mosby Regie R 287-0580 Vacant Green John S 287-0697 Greene Charles M 582-9242 Reid Charles E 583-8467 Anderson James R 583-9585 Brackett J C. Saunders Russell P 286-1993 Legere Ruby Brown Robert Kallas Cath A Mrs 287-2679 Caskey W Ruble Peter H 582-7042 Morgan E M 287-4928 Osborne Junnita 582-5389 Mesa Glass & Shower Door Co 12 2393 Curimenta No Return Misdoza Harriett Mrs 286-3557 1 No Return Dida Paul 582-5245 Haw Low restr 583-1166 Wew Properties Inc apts 262.6588 Center Apartments fram & Southern Life Insurance 286-1800 view Center Co real est wa Allen G Insurance Agency ments Dermott James J 286-8559 cant Sni artments athly Lydin M Mrs 286-2112 innann Eduardo 582-8899 ther Dorothy L Mrs sholtz Irven M 287-7218 Welca Mrs 582-8269 Way Bohhy I 582 porce

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3 Wm > Lon on Brent or Ronald 287-4550 es James 287-4550 es James 287-9075 Lucy K Mrs 287-4807 ber Diana Mrs 286-7601 Charles D 287-0449 inski Robt 289-3972 Masto Nancy enta unna Robt in Gail 180n Paul urson Carolyn E 287-6168 nberger Janet usend Susan tta Robert J rist James D 286-2980 m Robt r Jack n John ιt ente ux Inez Mrs 287-5915 rhelyi Glenn R 583-3978 ley John Jeraldine ski Gregory D 583-2928 horn Marianne 287-9947 tum pie Kelly A 582-2131 nte Carmen Mrs 286-3454 June M 287-6167 Donnell 286-4145 1y Allison 287-5794 John J 287-2404 pson Lois n Beverly J 287-6457 d Richd Joseph Norman 583-8447 1 Thos R tt Patricia argt A 583-4634 Robt S 583-1771 ıts lichael 287-7096 eraldine M Mrs Iartin 286-9538 and Robt 287-4476 indra ma Howard Kent M 286-2921 : Brand um Josephine ty John ple Mary H 286-0571 Vancy 286-4602 e Peter 583-3644 Vickye 287-9597

80*Sulcer Edw 81 Vacant 82*Dowl David 83 Willoughby Katherine Mrs 84*Maisei Jeffrey 85*Corvin F 86 Vacant 86 Vacant 5450 Apartments 1*Westfall Frank J 2*Tull Maria C 3 Ledingham Jean H Mrs 286-4966 4 Bell Lyman G 287-0774 5 Vacant 5 vacant 6*Jenkins Marvin 5450a House Of Curl 582-5601 5450b Webster Don Agency genl ins 583-0221 Farmers Insurance Group 583-0221 Webster Donald E real est 583-0221 5450c Arrow Hart Inc electrical equip mfrs 287-1300 5450d Vacant 5466 Poodle Haven The dog grooming 286-0942 54661/2 Fox Photo (Br) photog finishing 582-1392 56% J.95%
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5530 Colina Veterinary Hospital 286-3360
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5556*Romeo John 287-7546
5570 Campus Tuxedos rentals 583-1871
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q 52 Irvine Mary F Mrs 53 Matthews James F 582-0300 54*Mark R 286-1298 55 Chapman Clara Mrs 287-3069 56 Spahn Eliz Mrs 583-4864 bo Spann Enz MIF 305-4884 5757 Aegean Apartments 286-2917 1 Ross Albert 582-0043 2*Parks Edw 3*Fair Harold 4*Childers John 5 Iliffe Joseph C 6 Pisalyaput Piyapote 287-1730 o Fraslyaput Flyapote 287-1730 7*Ullmann Charles A 287-4029 8 Murren Mark 9 Fox Robt L 583-4328 10*French S L 11*Smith Patricia G 287-6818 12*Walters Robt E 287-7611 13 Calkins James 14*Gomez Dolores M 582-0751 15*Chacon K 16*Bakowski Zygncint 287-5039 17 Fritsen Leonard 583-2335 18 Waters Norma Mrs 19 Rickman Thos 20 Rinzler B 21 Curry John T 287-6389 22 Vocati 22 Vacant 23 Vacant 24*Unger Kathy E 286-7567 1*Bellon Linda 2*Lockhard Luther M 286-2736 3*Kalman James 286-7073 4 Vacant 5*Hancock Y 6 Worrell Robt L 582-3367 7 Vacant 8*Johnson Linda 9*Bullion Fredk 10*Vin Coli R J 11#Shorter Tyrone 12*Riley G 13 Benks Q E 14 Sweetser Ethel T 287-4367 15 Cohen Michl I 287-8675 16 Estrada M 17*Slezak Karen J 18*Kelly Jennifer 19 Vacant 20*Kelley R E 21*Murillo Edw V 583-1137 22*Polk L 23*Saifer Sean 582-5372 24 Vacant 25 Green John S 287-0697 26*Wetstone M 1*Short S 2*Gassaway R H 3 Vacant 4 Vacant 5*Levin David 6*Blais Philip L 287-7854 7 Kallas Cath A Mrs 287-2579 Vacant 9 Welles Cal 10 Morgan E M 287-4928 11 Osborne Juanita 582-5389 Rear La Mesa Glass & Shower Door Co 582-2393 5794 Apartments 1 Vacant 2 Mendoza Harriett Mrs 286-3557 3 Vacant 4*Newton Merlin 58TH ST INTERSECTS 5801 Sheng Haw Low restr 583-1166 5802 Belleview Properties Inc apts 582-8588 Belleview Center Apartments 582-8588 5805 Western & Southern Life Insurance Co 286-1800 5807 Belleview Center Co real est developers 286-1445 Benson Allen G Insurance Agency 286-1445 5810 Apartments 1 Mc Dermott James J 286-8559 $\overline{2}$ Vacant 3*Tormo Manuel 4*Hall Patricia Apartments 1 Leighty Lydia M Mrs 286-2112 2 Pidermann Eduardo 582-8899 а Vacant 4 Fisher Dorothy L Mrs 5830 Apartments 1 Gabaldon Frank 2 Bucholtz Irven M 287-7218 3 Tate Welca Mrs 582-8269 4 Vacant

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4950 Spotless Cleaners clo 284-2928 Boyles Coin-Op self serv laundromat Rug Doctor Rents carpet clars 284-2928 4961 Certified Janitor & Chem Sup sls

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- 2 No Return 3*Wingo Laura Mrs 282-5439 4*Simons Alvin J 5*Pray Lisa Mrs 563-1295 6*Nelson Bernard 7*Colio Dolores Mrs 283-1988 8*Poole Louis S 280-6888 9*Shannon Dick 10*Kotlan John 281-4181 11*Shumway Maudie Mrs 281-5300 12*Pulley Frances Mrs 282-4147 5045 Moore Wilson K 281-4897 5047*Capps Dora Mrs 5049*Micasio Jose 282-9611 5051+White Fred 5053*Agle John 283-1144 5055 Kenny Kathy 282-3296 5057 Reilly Michl J 281-8541 5085 Hienkel Janice @ 281-9826 5109 Stone's Furniture Stripping & Refinishing furn repr 280-4834 5111 P T S Electronics telev sups whol 280-7070 5150 San Diego Pipe & Supply Co Inc 282-8154 52D ST INTERSECTS

5202 County Dept Pub Health (east s d health center) 582-6433

5211 S G S Real Estate Co Inc 583-6051 5219 Ace New & Used Furniture 287-6811

8 Goode Lucy K Mrs 287-4807 9 Schrieber Diana Mrs 286-7601 10 Bay Charles D 287-0449 11 Cerwinski Robt 289-3972 12 Del Masto Nancy 5424 Apartments 13+Hisington Howard 286-8977 14*Antonelli Andy J 582-0294 15 Robinson Paul 287-1594 16*Garrett Sam 287-7624 17*Morrison Greta Mrs 18*O'Connor Richd M 287-1727 19 Valletta Robert J 20 Gilchrist James D 286-2980 21 Lemon Roht 22*Harris Gordon 286-9141 23*Partovich Joel M 286-4096 24*Frantz David S 286-8140 5426 Apartments 25 Loizeaux Inez Mrs 287-5915 26*Manganiello G L 583-2559 27*Kotthoff Jeanette Mrs 583-3925 28 Bills Geraldine Mrs 29 Vacant 30 Hartshorn Marianne 287-9947 31+Blotner Bernard 583-0890 32*Edwards Roland E 286-7220 33*Gendioff Rupert J 582-2341 34*Hanson Omar 35 Escalante Carmen Mrs 286-3454 36*Richardson Henry 37 Orter June M 287-6167 38*Eisele Patk 583-0098 39 Ramsay Allison 287-5794 40 Maffei John J 287-2404 41*Bates Herman 42 Hassan Beverly J 287-6457 43*Labowitz Diane Mrs 286-9127 44 Weeks Joseph 45*Hess Beatrice Mrs 286-4467 46 Vacant 47*Gastelum Hector Mrs 582-4861 48 Bonnett Patricia 287-8013 49 Fox Margt A 583-4634 50 Goldin Robt S 583-1771 **5428** Apartments 51 Kler Michael 287-7096 52*Carver John M 287-9714 53*Kalman James 286-7073 54+Gifford Chris 55 Gall Sandra C 582-3273 56 Vacant 57*Brienzo Barbara 583-1947 58*Nicholson Geoffrey F 583-5390 59 Tackett Ronald B 287-6153 60*Trupiano Joseph 582-5014 61*Bennett Louise Mrs 286-4629 62*Riley Jerry D 287-3616 63 Moriarty John J III 287-9409 64*Geus Harold F 287-0256 65 Dries Nancy 286-4602 66 Eldridge Peter 583-3644

7*Lehman Jenet Mrs 286-1069

5520 M & G Offset Lithographer prntrs 583-9591 5530 Colina Veterinary Hospital 5538 M & G Oilset Lithographer (Ofc) 583-9591 5555 Pay-Less Self Service Show 582-0898 5556 Automart used cars 287764 Romeo John 287-7548 5570 University Tuxedo Shop pros 286-3030 5571 Gap The clo ret 286-2271 5586 Humphrey Appliance reference als 286-8501 John's T V & Appl televre & serv 582-7878 5590*Testa Mildred E 583-1604 5591 Baker Electricians contra 200 5592 Hubrich Wm C @ 5820110 55921/2 * Cooper Ralph 287-2193 5595 College Copy & Steno pilon printers 582-2755 5599 Metal Masters stainless 504 aluminum 582-2755 ZIP CODE 92115 5602 Del Capri Terrace conval 583-1993 Horton Cordia Mrs CHOLLAS PARKWAY INTERIO 5722 University Terrace Apartment 582-6304 1 Raquet Robt 287-1046 2 Phomin Income Tax & Boot Service 582-7844 2 Phomin Isaac I 582.784 3 Brotherson Shirley Mri du 4 Hynes Louise 287-5891 5 Day Corwin 6 Hetu Maurice J 283-2816 7 Boone James A 583-2678 8 No Return 9 Baker E M 287-4361 10 Jackson Annie 286.877 11+Jones J L 583-0356 12+Crockett Mary Mrs 2614 13 Kruger Betty M Mrs 200 19*Wiebe Warren 287-5519 15*Johnson Claire Mr 16*Hathaway Steven 25630 1"*Rosenberg Bryan D. 18 Stone Olga Mrs 583 2045 19 Nelson Roe J 582-1472 20+Marks Barry 287-3750 2. Pehling Robt E 583-525 22 Weingarten Sam 287-84 23 Eisenberg Henry 582-554 24 Logan James W 58243 25 Ingraham Violet M. Mar 26 Gonzales Edna ' 27 Edwards Harold C ballo 28*Winget Gerald 29 Silberzweig Thelma Mr. 30*Weintraub Jacob 583 31 Roth Pauline Mrs 683

HARTER KOPECKYB

5*Parks Jerome

CONTRACTOR selion Linda Jost Melody Mrs 583-4576 Hole David Godrum Pearl Mrs Whitt Benj Eddie 286-1404 Guder Amos Satellanos Rudy J 583-2356 Ringgold Ernest 286-8233 Smith Ron 287-4073 Mason Harriett Mrs Milling Robt Firkson Mabel Mrs 287-8393 Content Ethel T 287-4367 Morrison Peter Secak Karen J 583-5953 Wander Linden Ruth 287-8282 Kelley R E Firmer Grace Mrs 583-4116 Allen Jamo 287-1039 Kawamoto Tony Adams Alvin Ferce Luella A Mrs 583-0962 Rooper Glenn 286-4402 Sorter Tyrone 287-1517 Hetcherson Opal Mrs Arialyaput Plyapote 287-1730 Alyman Larry Alexan Merlin 287-7891 Astant Morton Lee 286-1172 Ford Leonard Stand Bobby Morgan E M 287-4928 Goome Juanita 582-5389 tint inments oren Rose Mrs Indoza Hermina Mrs 286-3557 Torday Jess ra Helen Mrs **INTERSECTS** Haw Low restr 583-1166 Froperties Inc apts Genter Apartments Copers 286-1445 Allen G Insurance Agency King restr 583-9915 Dermott James J 286-8559 Autheson Steven ard Arth M 582-4401 Patricia enta thly Lydia M Mrs 286-2112

6*Searles Timothy 7*Hooper Estelle Mrs 8 Northway Larry P 583-5427 9 Ruale Wm J 10*Cowen Harry C 11*Crews A Wayne 583-6125 12 Simpson Claude 287-9197 5883 International Hair & Beauty 582-7171 5885 Hair Place The mens-womens hair styling 286-8404 5887 Fitting Room The women's clo 287-9160 5889 Cinerama Theatre 583-6201 5890 Apartments 1*Burruss Rudy L 286-7043 2*Cheek Robt W 286-8515 3*Barkley Toby 4*Ohlin Myrtle Mrs 5*Watts Rodney A 583-5216 6 Vacant 7*Jefferson Myrtle Mrs 8*Potter Fred 287-8357 9 Moss Mildred Mrs 287-1412 10*Baxter Lorenz 11*Zimmer Kathleen Mrs 583-2463 12*Connor John 5902 Apartments 1*Nguyen Van Mai 583-7757 2 Gardner Ronald M 3*Sizemore Louis 287-6587 4 Brisk Lydia E Mrs 286-9276 5*Malone Orabelle Mrs 6+Williams Eug C 286-9517 7*Maxwell Marvin D 287-8264 8*Ellingsen Ethel Mrs 9 Manuel Bette 10 Molumby Ronald D 11 Keats Ronald 12*Condon Patk 5909 Owen & Son Texaco Service 583-9040 5910 Apartments 1*Morales Arth 286-3718 2*Marusic Herta Mrs 286-8852 3 Mc Donald Merlin 582-3458 4*Marulli Deanna Mrs 287-3295 5*Paul Rachel J Mrs 583-8687 6 Rice Patricia D 7*Jackson Homer 8 Mc Intosh Andrew 9*Gonzales Eusebio Jr 286-2216 10 Maddox Alden D 11+Fitzpatrick Henry 583-9086 12 Knipfer Michl A 5920 Apartments 1*Parker Jack 287-7458 2*Lopez Eddie 582-8592 3*Lindsey Oscar 4 Moore Barbara J Mrs 5*Strattard John E 287-7832 6*Malzinger Warren 7*Mac Kenzie Louise Mrs 8#Golman Eddie

1*Polk Derek A 2 Orong Victor 1 3*Nutter David 4*Black Jerald 5*Lawrence Tern 6*Freed Danl 28 7*Johnson Sherr 8+Fisher Bernari 9*Hudson Aaron 10 Fox Earl 11+Davis Harold 12*Peterson Dar 5970 Apartments 1+Williams Gary 2*Burch Bonnie 3+Perkins Wm 4+Simmons Want 5980 Apartments 1*Pearson Burt 2 Sparlin Michl I 3 Bell Ronald D 4 Alvarez Luis G 5985 College Car Was 5990 Apartments I Fernandez Gen 2*Villalobos Robt 3 Terry Janice 28 4*Gomez Rachel 60TH ST INTERSE 6000 College Beauty § 6002 Vacant 6035 College Plaza Of ROOMS 1 Kriston Jhnara 286-3444 3 Shelter Island E property mgmi 3 Emerald Investi investments 28 5 Mohling F Co c 7 Vacant 8 Steckman Jane 9 Vacant 10 Vacant 11 Vacant 12 Dupree Ray Pt 286-1560 12 Dupree Ray Ri 14 World Wide Hu exams 583-714 16 College Plaza I Investments 5 17 Armstrong Rea 19 Vacant 21 Vacant 24 Parson Bishop collection ager 26 Twogood Richd financial consu 27 Vacant 28 National Pet Pi dog tags mfr ! 29 Vacant

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0201 LUNRRIGE LUD TEAGLD 2828 5259 Vacant iphy 5263 Vacant 5265 Gemini Adult Books No 2 • & Air 5295 Vacant 5296 Goerge's Garage 284-5992 80-6173 5303 University Wood Yard fire 283-1500 282-8081 5348 San Diego Community G District 583-6422 itr 284-2947 S D Community Colleges D Educ (Admn Ofcs) 2807610 5350 5354 Alcala Travel travel agenci 5360 Transamerica Title Insuran 723 583-7221 5362 Weight Watchers health Cleaners 286-0120 283-0447 5364 Ad-Visor The weekly adv 81-6741 286-9800 5366 Vacant 5368 Kirby Co Of Pacific Court clna 583-8070 Co Inc ogs 281-7727 5372 Vacant 5380 Vacant tes Inc -1046 5382 Vacant 1-5811 54TH ST INTERSECTS 5401 Ed's Self Service Shell S 851 287-5221 5405 K-Mart dept store 286-9734 7737 711 5415 K-Mart (Stge) 5422 Villa Verde Apartments 234 3 282-4161 1★Daniels B 1112 2*Crenshaw L 3+O'Rourke Lawrence W 201 ' sta 4*Gotch K 5 Lipson Betty 583-8226 tog finishers 6*Stein Irving 582-6455 7*Abdel-Messeh E 8 Goode Lucy K Mrs 9*Solley L 287-3717 281-4020 10 Newman David 582-9774 5 restr 11 Ehrlich Theo 583-3108 12*Press Sam 582-7428 -2928 5424 Apartments 13 Smith Billie J 286-8481 laundromat 14#Leadbitter C clars 15*Ramirez Danl Jr 28845 16*Landesman Louis 287.854 1 Sup sla 17*Pich D 18#Fullre Ruby E 5834627 19 Valletta Robert J lus tchr 20 Jacobs Harry G 287-9181 21*Marlbrough Kenny E а 22 Palumbo Mary 582-9616 es sis & 23 No Return 24*Royal Ronald L 287-220 t Funds 5426 Apartments 25 Loizeaux Inez Mrs 287-561 Shopmn & 26#Rietz Steven J 287-3900 **⊢615**1 27 Kotthoff Jeanette Mr 50 hers Intl 28 Bills Geraldine Mrs 54 29*Chennell Irene 583 3780 83-7109 30*Grassel J 286-1196 31*Hoffman Ernst K 287-5 pes 282-9487 141 32*Kaufman Louis 287.065 81-7138 33 Harrer Robt 287-8277 34*Jacobsen Fredk J 5829 35 No Return 36*Davis J 787-4277 37 Orter June M 287-6161 38*Ogren Herbert W Jr 200 30-6697 39*Burnett Wm A 583-6876 4645 11494 SORRENTO VALLE SUITE J SAN DIEGO, CALIF. STEVE (714) 755-4

Ernst K 287-6764 H Mrs Mary E 583-7944 Robt F James 286-7073 Re Clemmin R 286-9064 David W 286-8165 Geo L 287-9514 Michl L mneth R 287-9081 m Jean H Mrs 286-4966 S 287-0774 Jos L 583-4383 seph C Curl 582-5601 Limousine renting & 583-0221 inds electrical equip mfrs firm The dog grooming rage 286-1252 he self serv gas sta The 286-9754 Marie Harris E 287-2256 tavern 583-0080 Ine Massage 583-0880 Grace E **Offset** Lithographers Inc 27816 Continuary Hospital 286-3360 Offset Lithographers Inc 39591 mink Self Service Shoes used cars 287.7546 John 287-7546 Tuxedo Shop rentals do ret 286-2271 Appliance refgr-freezer Appl telev-radios sls 82-7878 ctricians contrs 287-8760 **∀m C** 582-0110 bopy & Steno pub sten & astera stainless steel & 6 582-2755 2115 Terrace convalescent hosp ordia Mrs ARKWAY INTERSECTS Terrace Apartments M J 582-7930 on Shirley J Mrs 583-1384 Mary 286-1985 Harry 582-5410 Aurice J 283-2815

45 Conry Kathryn A Mrs 287-1862 46*Butzke L 47 Pedermann Alina 582-0523 48 Rollefson Jerry G 583-8636 49 Hetu Joe 50*Fuks Israel 287-0478 51*Jackson Jack 287-0221 52 Irvine Mary F Mrs 53 Matthews James F 582-0300 54 Mark R 286-1298 55*Fletcher Craig C 287-2268 56 Spahn Eliz Mrs 583-4864 5754 Apartments 1*Green 2*Sontag 3*May Ronald T 583-8262 5757 Aegean Apartments 286-2917 1*Cornwell Dale T 286-0393 2 Argow T W 3 Blais Philip L 287-7854 4 Van Brunt Godwin A Jr 286-1438 5 Iliffe Joseph C 286-2917 6 Marquez Lupe 7*Johnson L 8+Ezell Eva 9*Crawford Richd 287-1824 10 Fieden Howard 287-0259 11 Mc Kain Martha 12*Jones P 13 Calkins John 14*Marcus K 15*Kallas G M 583-6808 16 Bakowski Zygncint 287-5039 17 French Stanley L 287-1692 18 Waters Norma Mrs 19 Rickman Tom 20 Rinzler Betty 21 Morris L M 286-7822 22*Reynolds P 23+Van Waldick David 287-3343 24*Mortazvi B 582-7918 1 Westbrook R A 2 Krumholz E C 286-8177 3 Flakes D S 4*Eckert Kevin W 287-6705 5 Hancock L E 6*Worrell L 7*Norman Myrtle M 8*Provost Jerry E 582-1368 9 King Mary A 286-3394 10*Bass Michl J 286-7528 11*Atwood Lillian M 287-8003 12 Acosta S R 13∗Padilla E 14 Sweetser Ethel T 287-4367 15*Lamb D S 16*King A 17*Adler Saml 582-1209 18*Delgado Jose L 286-0122 19 Vander Linden Ruth 287-8282 20*Butcheck Larry 287-3604 21*Sharp S W 22 Allen Jamo 287-4039 23*Smith Mike 24*Caywood Ernest L 583-9330 25+Lagerguist J 26 Austin Gene A 1*Scherer Douglas F 287-1189 2 Gassaway Roy H 3*Fahey Pat W 287-3042 4*Wang John C 287-8475 5*Cooper Chas H 582-8609 6*Sanchez A M 286-2398 7*White Ferris G 582-6327 8*Green B 583-7505 9 No Return 10 Morgan E M 287-4928

3 Stevenson Lou V 287-5964 4*Wagner John 58-2490 5841 Universal Television And Sterco Repair 287-2750 5843 Commercial Credit Corp collateral loans 583-3953 5845 Vacant 5850 Apartments 1 Camps Juan 2 Echeverria Orlando J 286-4028 3 Diaz Hilda Mrs 4 Marin Otilio 582-9316 5853 Vacant 5855 Take-A-Break restr 286-0748 5865 University Avenue Florist 582-8300 5870 Apartments 1 No Return 2 Cole Walter J 287-6048 3*Miller Thos 4 Mc Intire Kenneth 5*Pham Linh Van 583-7733 6★Williams Jodi V 7 Eccles Michl D 287-6627 8*Shober Michl R 583-8298 9 Richline Stewart W 10 Phillips Robt E 287-2553 11 Wayne Alf R 12 Brown Thos A 562-6784 5871 Sav-On Drug Store 583-6961 5877 Bank Of Am Natl Tr & Savs Assn (Ofc) 294-2185 5880 Apartments 1 Stanley Sandy 2 Vacant 3*Patterson Cassanora 287-2840 4 Vacant 5+Brandy Bertha L 286-3664 6+Avre Chas 7*Rosemeyer Dorothy R 286-4872 8 Northway Larry P 583-3427 9*Marshall Jan 287-4516 10+Simpson Jean 11*Tewes Diosdada R 12*Byrne Tom 5883 International Hair & Beauty 582-7171 5885 Fuzzy Richards mens-womens hair styling 286-8404 5887 Fitting Room The women's clo 287-9160 5889 Cinerama Theatre 583-6201 5890 Apartments 1 Burruss Rudy L 286-7043 2 Vacant 3*Johnson Joan S 286-2526 4 Smart Juanita 286-1533 5*Coon John 6*Oates Saml W 582-0965 1*Ajou M 287-9366 8 No Return 9 Moss Mildred Mrs 286-8049 10*Everett Grace 11 Zimmer Kathleen Mrs 12*Weatherford Damon 853-8631 5902 Apartments 1 Vacant 2*Welsck Bruce 3 Street G M 286-7991 4 Brisk Michael A 286-9276 5*Lawson Joann 6*Nguyen Tin Dahl 287-5911 7 Vacant 8*Fugate Donald R 583-4892 9*Chanthavisouk Tien Chi 582-1465 10*Watkins Doris 583-2718 11*Beebe Mike J 12*Isaacs Elsworth J Jr 582-4553

11 Bailey Issiah 583-0905 12 Reyes Linda R 582-8467 5933 University Lanes Inc bowling alley 583-5444 Leisure Time Catering 583-1715 University Lanes Coffee Shop 583-5444 Nashville Country cocktail lounge 583-6670 5940 Apartments 1*Davis V 2*Chelton Ernest B 3*Liu Theresa 4*Quinn Mike 5*Haberstroh Phil J 6 Vacant 7 Vacant **4902 MARKET** 8*Tsai Saml W 583-9144 9*Vogle Don M 287-4653 10 Marsili Mike 583-6330 11 Jordan Jane 583-6956 12 Valez Sherry 5950 Apartments I*Etal Chuck 2 Baughn Kath A 582-7509 3 Mc Duff Marsha Mrs 287-6198 4 Vacant 5+Huizenga Donna J 582-0873 6 Vacant 7*Rodriguez Tony 286-0158 8 Gilmore Geo 582-6423 ŝ 9 Salsedo Valerie 10 Brown M L 11 Kidd Peggy A 12 Hearn Olivia 286-9991 5960 Apartments 1*Frye Chas M 286-9188 2*Taylor Laruth 3*Norwood Will H 286-9831 4 Vacant 5*Avila Sylvia 6*Jones Zachary B 7*Medina Raul P 8*Bradley Eunice M 9*Bourque Bob 287-4309 10*Ngo Van Thi 582-0070 11*Angel Diane L 265-1661 12+Kuhl L 583-0087 5970 Apartments 1 La Fountain Jo Anne 583-6224 2 Carbaugh Roger 286-2379 3*Buccella Anne M 4 Hipple Geo C 287-6026 5980 Apartments 1*Mills Clifford V 2 Thompson E Juanita 582-2869 3*Proffer Bruce 4 Alvarez Luis G 286-1216 5985 College Car Wash 583-8760 5990 Apartments 1 Conatser Judith K 287-6236 2+Watson Thelma V 583-5944 3*Wilson Chas D 583-8170 4*Thaung Mye 582-9432 60TH ST INTERSECTS 6000 Madam Ruth Spiritual Psychic Palm Reader 232-0351 6002 Vacant 6035 College Plaza Office Building ROOMS 62-072 1 Kriston Jhnara marriage counselor 286-3444 3 Shelter Island Financial Inc property mgmnt 287-1872 3 Emerald Investment Co real est investments 287-1875

5 Mohling F Co collns med 582-0757

SIKUCTURAL . Carlos Fernandez, OHNAMENTAL . President - Jim Nieto, WELDING

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281-7729 /4 SHERID/ | DIEGO, C, . (714) 28(Tile Marble & Terrazzo Shopmn & Fnshrs Local No 28 284-6151 Fishirs Local NO 28 284-6151
Wood Wire & Metal Lathers Intl Union Local 260 281-6654
4979 Bob's Carpet Service layers 282-2041
4979a Thomas Furniture Co 283-7109
4979b Fashion Carpets & Drapes 282-9487
4981 Finch Cookie Co baked gds distrs 280-2042 4334 : SAN D TEL. (1978 280-2042 4983 Mickey's Beauty Salon 281-7138 50TH ST INTERSECTS 5027 Apartments 1*Boone Leon 2*Dunn Edw J 3*Ladd Thelma Mrs 4 Simons Alvin J 5*Hall Paul 6 Nelson Bernard 7 Colio Dolores Mrs 283-1988 8 No Return 8 No Return 9*Houle Benj 10*Gaines Melvin R 11*Harper Douglas N 12 Pulley Frances Mrs 282.4147 5045 Moore Wilson K 281-4897 5047 Capps Dora Mrs 5049*Martin Keith E 283.9570 5051*Hall Heidi S Mrs 283.4531 5053*Alberts Gary 283.1187 j Helm Kenneth 284.8056 5057 Reilly Michl J 281-8541 5085 Hienkel Janice © 281-9826 5109 P T S Electronics (Work Shop) 5111 P T S Electronics telev sups whol 280-7070 5150 San Diego Pipe & Supply Co Inc 282-8154 282-8134 52D ST INTERSECTS 5202 County Dept Pub Health (east s d health center) 582-6433 5211 S G S Real Estate Co Inc 583-6050 5219 J M Electric contr 583-4021 First С. At Briggs, Anuren, ... Property Management Comes Action Plumbing sup 583-2442 5221 Mexicali Restaurant 287-8470 5227 Antofte Studios stained glass 231-0534 BROADWAY 286-2892 5229 Geo's Custom Jewelry mfrs 583-0413 5231 Milano Restaurant & Italian Delicatessen 583-7850 5241 George's Bill House Of Furniture BROAI 582-1616 5255 Acacia Unfinished & Patio Furniture 582-1616 5257 Pudggie's Pub tavern 583-9925 TEL. 5259 Vacant 625 5263 Vacant 5265 Southern California Books 583-9746 Southern California Books 305-9740 Transitainer truck rentals 287-4860 Bus That Goes In Circles Inc The tour buses 583-6393 University Wood Yard firewood 5295 5303 282-8081 5348 San Diego Community College District 583-6422 5350 S D Community Colleges Dist Adult Educ (Admn Ofcs) 280-7610 5354 Alcala Travel travel agency 287-1530 5360 Transamerica Title Insurance Co 583-7221 5362 Weight Watchers health salon 286-0120 5364 Ad-Visor The weekly adv publ 286-9800 5366 Vacant 5368 Kirby Co Of Pacific Coast vacuum clns 583-8070 5372 Vacant 54TH ST INTERSECTS 5401 Ed's Self Service Shell gas sta 287-5221 281-3221 5405 K.Mart dept store 286-9733 5415 K.Mart (Stge) 5422 Villa Verde Apartments 286-7220 1 Guintini James 286-8836 2 Stoner John E 3 Vacant 3 Vacant 4 Stein Kath Mrs 287-8182 5*Lipson Betty 583-8226 6*Knutson James R 582-7643 7*Horton John A 583-8075 8 Goode Lucy K Mrs 287-4807 9*Finch Shirley A 10*Newman David 582-9274 11*Ebid: Theo 582-3108 11*Ehrlich Theo 583-3108 Vacant

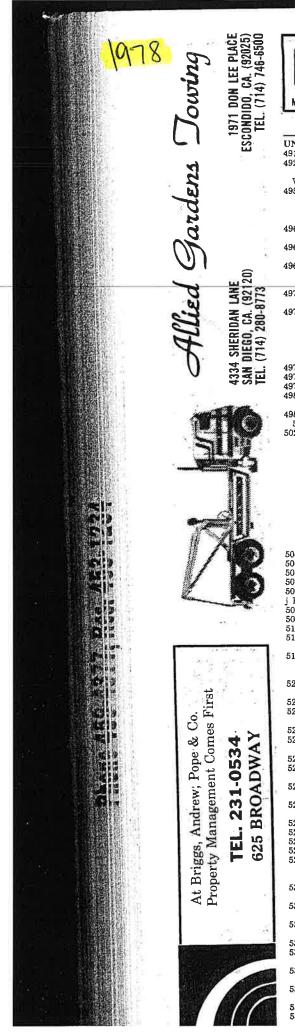
29 Vacant 30*Nicholson Geoffrey 583-5390 31*Shure Irving 287-7328 32*Goldman Leonora 33*Harrer Robt 287-8277 33*Harrer Robt 287-8277 34*Clayton Phillip C 35 Escalante Carmen Mrs 286-3454 36*Burkes James C 37 Orter June M 287-6167 38 Vacant 39 No Return 40*Yip Chas Y 287-4509 41*Torrison John R 287-8478 42 Hassan Beverly J 287-6457 43*Baker P D 286-7795 44*Torrison Leonard L 583-2371 45*Wilson Pamela A 287-7541 46 No Return 46 No Return 47*Linsk Steven 583-7258 48*Woolery K S 287-9788 49 Vacant 50 Goldin Robt S 583-1771 5428 Apartments 51 Kler Michael 287-7096 52 Vacant 53★Geiger Debbie A 582-8326 54★Schoff P J 287-5895 55★Brady Robt P 286-4629 56 No Return 57 Brienzo Barbara 583-1947 58±U'Ren Stuart C 582-1976 59±Walton Wip 286-4183 60±Blitstein R 582-6785 60 × Blitstein K 582-6785 61 Vacant 62 Riley Jerry D 287-3616 63 Moriarty John J III 287-9409 64 Geus Harold F 287-0256 65 Dries Nancy 286-4602 66 Eldridge Peter 583-3644 67*Kalman Helene B Mrs 286-9900 68*Rapay Elmer D 287-6079 5430 Apartments 69 Maloney James D 70*Gardiner Emory 287-3640 71*Bulgin R G 72 No Return 73 Vocant 74*Shap Linda C 78 Hoffmann Ernst K 287-6764 76 Bates Lodi Mrs 77 *Phillips Lisa 583-6772 78*Mulroy Mary E 583-7944 79 Sexual Freedom League 582-5143 80*Berg Michl D 582-1979 81 Thompson Mary Mrs 496 7000 80*Berg Michi D 562-1975 81 Thompson Mary Mrs 286-7933 82*Kalman James 286-7073 83 No Return 84*Parks David W 286-8165 85 Vacant 86*Rubin Michl L 5450 Apartments 1 Parks Kenneth R 287-9081 Vacant 3 Ledingham Jean H Mrs 286-4966 4 Bell Ruth S 287-0774 5★Mokhtarian Farnaz 6★Barta Joseph C 5450a House Of Curl 582-5301 5450b Webster Don Agency genl ins 583-0221 583-0221 Farmers Insurance Group 583-0221 5450c Arrow Hart Inc electrical equip mfrs 287-1300 5450d Vacant 5466 Poodle Haven The dog grooming 286-0942 5494 Nik's Garage 286-1252 5498 Pit Stop The self serv gas sta 286-9754 Pop Place The 286-9754 FOP Flace the 286-9754 5502 Thomas Charles R 583-3809 5502½ *Briscoe James 5504*Clark Harry A 5504^{*} Turner Harry E 287-2256 5506 Cloud Nine tavern 583-0800 5506a Cloud Nine Massage 583-0880 5506½ * Marks Grace E 5508 Vacant
 5508 Vacant
 5520 M & G Offset Lithographers Inc prntrs 582-7816
 5530 Colina Veterinary Hospital 286-3360
 5538 M & G Offset Lithographers Inc (Ofc) 583-9591 Rear*Schlehuber Edward W 5555 Pay-Less Self Service Shoes 582-0898

5556 Automart used cars 287-7546

583-1993 Horton Cordia Mrs CHOLLAS PARKWAY INTERSECTS 5722 University Transaction 31 4 51 61 5722 University Terrace Apartments 71 582-6304 81 *Flahide Richd A 287-4274 9₩ 10 2 Vacant 2 Vacant 3 Brotherson Shirley Mrs 583-1384 4*Howlett Susan D 286-1928 5*Dussman Harry 582-5410 6 Hetu Maurice J 283-2815 7 Boone James A 583-2673 8 No Return 9*Mc Mikle Ernest C 287-1935 10 Vacant 11 Rear V 5794 · A 1. 2 3 10 Vacant 11 Jones J L 583-0356 12 Crockett Mary Mrs 287-9920 13 Kruger Betty M Mrs 286-9366 58TE 5801 S 5802 B 5805 W 14*Matthews Albert 14 Matthews Albert 15 Vacant 16 Krauth Robert 286-4826 17 Rosenberg Bryan D 18 Kidd Bruce S 287-6490 19 Vacant 5807 B B 5810 A 20 No Return 21 Pehling Robt E 22*Abell Harry M 583-6733 23 Eisenberg Henry 582-5361 2+1 3**±**1 24 No Return 25 Vacant 4*****] 5820 Aj 25 Vacant 26 Gonzales Edna 27★De Water Susan 28 Winget Gerald 29 Silberzweig Thelmc. Mrs 582-5339 30★Dupont Barbara 287-2911 31 Roth Pauline Mrs 583-7257 32 Zegart Joseph 583-7192 33 Nemeroff Max 287-3912 34 Ortega Alicia Mrs 11 2 . P 3#5 4 F 5830 Ar L X 1 2 B 3 T 4 L 34 Ortega Alicia Mrs 35 Snouffer Lydia Mrs 5833 ME Me Va 36 Vacant 37 Ten Broek Howard 582-4471 38*Sherbondy Mark A R 5837 Va 39 No Return 40 Vacant 5840 Ap 1*E 2 M 41 Hopkins Robt 42*Rainne Larry W 287-7466 3 St 4 Vi 43*Williams Stanley 286-2434 43 williams Starley 286-2434 44 Vacant 45 Conry Clare A Mrs 287-1862 46*Sukramule Choosak 582-1455 47*Pedermann Alina 582-0523 5841 Un Re 5843 Cor 48*Rollefson Jerry G 583-8636 49 Hetu Joe 108 5845 Dis 5850 Api 1*C: 43 netu Joe 50 Vacant 51 Vacant 52 Irvine Mary F Mrs 53 Matthews James F 582-0300 54 Mark R 286-1298 $2 E_c$ 3 Di 4 Mi 54 Mark R 286-1298 55 Chapman Clara Mrs 287-3069 56 Spahn Eliz Mrs 583-4864 5757 Aegean Apartments 286-2917 1★Clarke C A 286-7877 2★Argew T W 3 Blais Philip L 287-7854 4 Van Brunt Godwin A Jr 286-1438 5 Iliffe Joseph C 286-2917 6★Marquez Linda 7★Mitchell Karen 8★Jones P L 5853 For de 5855 Dor 5865 Uni 5870 Apa 1 No 2 Co. 3 Va 4 Mc 5 Pa 6 No 8*Jones P L 7*Ec 9×Juhi J A 10 Fieden Howard 287-0259 11★Mc Kain Martha 12★Lamb Phyllis A 286-4048 13★Calkins John 8 Bit 9+Ri-10*****P 11+1 12**#**B 13*Celkins John 14*Chung Daniel 15*Nelson Arth D 583-6808 16 Bakowski Zygncint 287-5039 17 French Stanley L 287-1692 18 Waters Norma Mrs 5871 Payl 5877 Bani (Of 5880 Apa: 1*Ste 19 *Rickman Tom 20 *Rinzler Betty 21 *Morris L M 286-7822 2+Srr 3 Miz 4*An 5*Va 22★Parsons N J 23★Chandler R M 6 Vac 24*Raciti Frank 582-7918 1*Westbrook R A 2*Krumholz E C 286-8177 7*Th 8 No: 9+Ru 3★Flakes D S 10 Vε 11*****Fl 4*Lee Terry D 286-2923 5*Hancock L E 6 Vacant 7 Gruder Amos 12 Sii 5883 Inter Gruder Amos 582 8 Vacant 5885 Fuzz styli 5887 Fittir

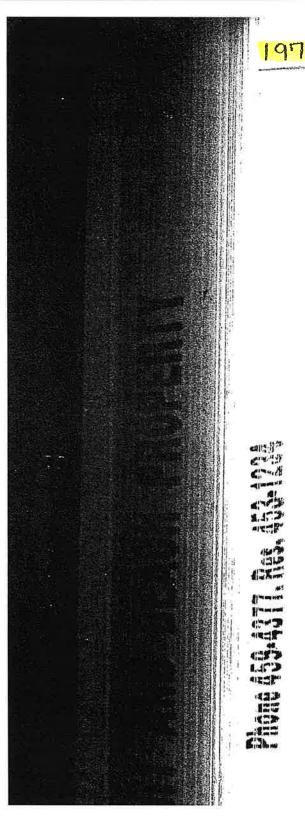
9★King Mary A 286-3394 10★Manaus C T 11+Shorter Tyrone 287-1517

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Hereiter H	IOME FEDEL	
MAIN OFFICE: 701 Broadway, San Diego	AND LOAN ASSOCIA	TION OF SAN DIEGO
		788
UNIVERSITY AV—Contd	5424 Apartments	Romeo John 287-7546
4919 Seven-Eleven Food Store 281-4020	13*Smith Billie J 286-8481	5570 University Tuxedo Shop
4920 San Ysidro Taco Shop No 5 restr	14 Vacant	286.3030
283-5636	15±Zywanoski Robt J 287-4756	5571 Gap The clo ret 286-227
WINONA AV INTERSECTS	16 Garrett Sam 287-7624	5586 Humphrey Appliance ref
4950 Spotless Cleaners clo 284-2928	17★Hagenbauth Daniel 582-2826	sls 286-8501
Boyles Coin-Op self serv laundromat	18★Penaloza Carlos J 286-8401	John's T V & Appl telev
Rug Doctor Rents carpet clnrs	19 Valletta Robert J	& serv 582-7878
284-2928	20★Jacobs Harry G 287-9187	5590 No Return
4961 Certified Janitor & Chem Sup sls	21★Nix D M 286-8642	5591 Baker Electricians contre
280-3603 4961a Community Service Center For The Disabled social serv 283-5901	22*Palumbo Mary 582-9616 23 No Return 24 Vacant	5592 Hubrich Wm C © 582-0 5592½ + O'Donnell Jim
4969 Salerno's Music Studio mus tchr 282-7333	5426 Apartments 25 Loizeaux Inez Mrs 287-5915	5595 College Copy & Steno p printers 582-2755 5599 Metal Masters stainless
Salerno Louis © 282-7333 4973 Frienly T V & Appliances sls & serv 280-9335	26 Manganiello G L 583-2559 27 Kotthoff Jeanette Mrs 583-3925 28 Bills Geraldine Mrs	aluminum 582-2755 ZIP CODE 92115
4977 San Diego Lather's Trust Funds 281-7729	29 Vacant 30*Nicholson Geoffrey 583-5390	5602 Del Capri Terrace conva 583-1993 Horton Cordia Mrs
Tile Marble & Terrazzo Shopmn &	31★Shure Irving 287-7328	CHOLLAS PARKWAY INT
Fnshrs Local No 28 284-6151	32★Goldman Leonora	5722 University Terrace Apar
Wood Wire & Metal Lathers Intl	33★Harrer Robt 287-8277	582-8304
Union Local 260-281-6654	34★Clayton Phillip C	*Flahide Richd A 287-4:
4979 Bob's Carpet Service layers 282-2041	35 Escalante Carmen Mrs 286-3454	2 Vacant
4979a Thomas Furniture Co 283-7109	36★Burkes James C	3 Brotherson Shirley Mrs
4979b Fashion Carpets & Drapes 282-9487	37 Orter June M 287-6167	4★Howlett Susan D 286-1
4981 Finch Cookie Co baked gds distrs	38 Vacant	5★Dussman Harry 582-54
280-2042	39 No Return	6 Hetu Maurice J 283-28
4983 Mickey's Beauty Salon 281-7138	40★Yip Chas Y 287-4509	7 Boone James A 583-26
50TH ST INTERSECTS	41★Torrison John R 287-8478	8 No Return
5027 Apartments	42 Hassan Beverly J 287-6457	9★Mc Mikle Ernest C 28
1★Boone Leon	43★Baker P D 286-7795	10 Vacant
2★Dunn Edw J	44★Torrison Leonard L 583-2371	11 Jones J L 583-0356
3★Ladd Thelma Mrs	45★Wilson Pamela A 287-7541	12 Crockett Mary Mrs 2{
4 Simons Alvin J	46 No Return	13 Kruger Betty M Mrs
5*Hall Paul	47#Lingh Stoven 583 7358	14*Matthows Albort
6 Nelson Bernard	48★Woolery K S 287-9788	15 Vacant
7 Colio Dolores Mrs 283-1988	49 Vacant	16+Krauth Robert 286-48
8 No Return	50 Goldin Robt S 583-1771	17 Rosenberg Bryan D
9★Houle Benj	5428 Apartments	18*Kidd Bruce S 287-64§
10★Gaines Melvin R	51 Kler Michael 287-7096	19 Vacant
11★Harper Douglas N	52 Vacant	20 No Return
12 Pulley Frances Mrs 282-4147	53★Geiger Debbie A 582-8326	21 Pehling Robt E
5045 Moore Wilson K 281-4897	54★Schoff P J 287-5895	22*Abell Harry M 583-6'
5047 Capps Dora Mrs	55★Brady Robt P 286-4629	23 Eisenberg Henry 582-
5049*Martin Keith E 283-9570	56 No Return	24 No Return
5051★Hall Heidi S Mrs 283-4531	57 Brienzo Barbara 583-1947	25 Vacant
5053★Alberts Gary 283-1187	58★U'Ren Stuart C 582-1976	26 Gonzales Edna
j Helm Kenneth 284-8056	59*Walton Wip 286-4183	27★De Water Susan
5057 Reilly Michl J 281-8541	60*Blitstein R 582-6785	28 Winget Gerald
5085 Hienkel Janice © 281-9826	61 Vacant	29 Silberzweig Thelma N
5109 P T S Electronics (Work Shop)	62 Riley Jerry D 287-3616	30*Dupont Barbara 287-
5111 P T S Electronics telev sups whol	63 Moriarty John J III 287-9409	31 Roth Pauline Mrs 58:
280-7070	64 Geus Harold F 287-0256	32 Zegart Joseph 583-719
5150 San Diego Pipe & Supply Co Inc	65 Dries Nancy 286-4602	33 Nemeroff Max 287-39
282-8154	66 Eldridge Peter 583-3644	34 Ortega Alicia Mrs
52D ST INTERSECTS	67★Kalman Helene B Mrs 286-9900	35 Snouffer Lydia Mrs
5202 County Dept Pub Health (east s d	68★Rapay Elmer D 287-6079	36 Vacant
health center) 582-6433	5430 Apartments	37 Ten Broek Howard 5
5211 S G S Real Estate Co Inc 583-6050	69 Maloney James D	38★Sherbondy Mark A
5219 J M Electric contr 583-4021	70*Gardiner Emory 287-3640	39 No Return
Action Plumbing sup 583-2442	71★Bulgin R G	40 Vacant
5221 Mexicali Restaurant 287-8470	72 No Return	41 Hopkins Robt
5227 Antofte Studios stained glass	73 Vacant	42*Rainne Larry W 287
286-2892	74*Shap Linda C	43*Williams Stanley 286
5229 Geo's Custom Jewelry mfrs 583-0413	75 Hoffmann Ernst K 287-6764	44 Vacant
5231 Milano Restaurant & Italian	76 Bates Lodi Mrs	45 Conry Clare A Mrs ≨
Delicatessen 583-7850	77★Phillips Lisa 583-6772	46★Sukramule Choosak
5241 George's Bill House Of Furniture	78★Mulroy Mary E 583-7944	47*Pedermann Alina 58
582-1616	79 Sexual Freedom League 582-5143	48*Rollefson Jerry G 58
5255 Acacia Unfinished & Patio Furniture	80★Berg Michl D 582-1979	49 Hetu Joe
582-1616	81 Thompson Mary Mrs 286-7933	50 Vacant
5257 Pudggie's Pub tavern 583-9925	82★Kalman James 286-7073	51 Vacant
5259 Vacant	83 No Return	52 Irvine Mary F Mrs
5263 Vacant	84★Parks David W 286-8165	53 Matthews James F 5
5265 Southern California Books 583-9746	85 Vacant	54 Mark R 286-1298
5295 Transitainer truck rentals 287-4860	86*Rubin Michl L	55 Chapman Clara Mrs
Bus That Goes In Circles Inc The	5450 Apartments	56 Spahn Eliz Mrs 583-
tour buses 583-6393	1 Parks Kenneth R 287-9081	5757 Aegean Apartments 28
5303 University Wood Yard firewood	2 Vacant	1*Clarke C A 286-7877
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5354 Alcala Travel travel agency 287-1530	5450b Webster Don Agency genl ins	7★Mitchell Karen
5360 Transamerica Title Insurance Co	583-0221	8★Jones P L
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YOUR \$2 BILLION FAMILY FINANCIAL CENTER



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5570	u	u	Tuxedo Junction	rentals
5580	u	u	Not listed	
5586	u	u	Humphrey Appliance	refgr-freezer sls
			John's TV & Appliance	
5590	u	u	Vacant	
5592	u	u	Not listed	

<mark>1984</mark>

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Appendix B

Building Permits

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	<u>ermit Ap</u>	plication	APFLICANT FILL INSIDE HEAVY LINE	s i	PLAN FILI	E,	PERMI	r	106
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02 15 2003 Building Permit Application INSIDE HEAVY LINES PLAN FILE PERMIT NUMBER NUMBER OWNER'S JOB-NAME ADDRES MAIL SIDE SET REAR ADDRESS YARD BACK 0% YARD TEL. NO USE MAP VACANT YE CITY a. 3. ZON NO. SITE NK BLS ARCHITECT of ECONOMIC LOCATION CENSUS ENGINEER CODE EA. LBD. TAX TRACT YARIANCE NO STREET BUILDING LOT ADDRESS AREA AREA 1852.3 YEL. NO. PERMIT NUMBER STATE Encroachment Yes ST. GRADE LICENSE NU. Permit Reg d No-CHECK METER CLEARANCE CHECKED BY: BUILDING CONTRACTOR SIZE REQUESTED METER LOCATION EXISTING [] CENTER LINE STREET OTHER ADDRESS OF PROPERTY an in TEL. NO. TYPE OF BOOK VERIFIED BY CITY CONNECTION FAGE FIRE ZONE Type of Construction STATE LICENSE NO. STREET YE IMPROVED N 1 53 i li 111 IV V 2 SPECIAL. JOB DESCRIPTION YES 🗖 OCCUPANCY GROUP INSPECTOR REQ'D. NO 🗆 A B C D E F G HY 🗍 LEGAL DESCRIPTION: (Attach Metes & Bounds if Necessary) PLAN CHECKED BY PLAN CHECK LOT 💭 🖗 BLOCK TRACT RECEIPT NO BUILDING BUILDING ADDRESS / Jak VALUATIO NEW 📋 ALTER DEMOLISH [m BUILDING ADD D REPAIR MOVE PERMIT FEE RESIDENTIAL I NUMBER OF NUMBER OF LESS MON-RESIDENTIAL DWELLING UNITS PLAN-CHECK FEE COUNTY SANITATION DISTRICT PRIVATE DISPOSAL APPROVA PECLIPT NO. SEWER FEE STATEMENT OF PROPOSED USE AMOUNT DUE TTENTION: APPLIC THIS FERMIT I hereby scknowledge that I have read this application; that the THIS PERMIT DOES NOT BECOME intornation given is correct; and that I am the owner, or the duly authorized agent of the owner. I agree to comply with city and state AUTHORIZES τ¢. UNTIL SIGNED BY THE DIRECTO ONLY THE lay's regulating construction; and in doing the work authorized there-by no person will be employed in violation of the Labor Code of the State of California relating to Workmen's Compensation insurance. BUILDING INSPECTION, OR HIS DE WORK NOTED AND FEES ARE PAID, AND RECEI ACKNOWLEDGED IN SPACE PROVIDE SIGNATURE OF INSPECTION OWNER or AGENT DEPARTMENT 5 ADDRESS 32 EVIDENCE OF AGENCY NOTED PLOT PLAN CHECK & APPROVED INSPECTOR HEALTH DEPT. APPROVAL ζ, CITY OF SAN DIEGO FORN NO. 87

02 15 2003 29584 108 : DOTESS 15 544 NYe PLAN FILE A. A. HOUSE NO. 2175-NO PLAN FILE NO. 5586 Cla APPLICATION FOR BUILDING PERMIT APPLICANT: PRINT WITHIN HEAVY LINES ONLY COMPLETE LEGAL DESCRIPTION OF PROPERTY TION DE SUBDIVISO Por. 29 mon Villa TYPE OF BUILDING TO BE CONSTRUCTED Kill OWELLANG 9- \square BARADE NUMBER OF NOW ON THIS SITET NO 🗌 YES W RESIDENTIAL D Commercial D NUNGER OF IF "YES" IS IT OTHER ā CLASS OF WORK FLOOR SPACE VALUATION OF JOB IN SO.FT. REPAIR NEN G \Box ADDITION NOVE ALTERATION DENOLISH NAME AND MAILING ADDRESS 2 Domain Sprinkly OWN 34EL ST. 5151 TELFI 700 . 44 4 2 NAME AND ADDRESS TRACTOR COM STATE LICENSE NO. TEL NO I HYREBY CERTIFY THAT THE ABOVE APPLICATION IS CORRECT. THAT CONSTRUCTION WILL COMPLY WITH ALL LAWS AND I WILL NOT EMPLOY ANY PERSON IN VIOLATION OF THE STATE OF CALIFORNIA LABOR CODE RELATING TO WORKWAN'S COMPENSATION INSURANCE. DIGNATURE OF APPLICANT TATE Marke C. ON PLANNING COMMISSION USE ONLY USE PACT RO M P 2 4 5 C A FOR BUILDING INSPECTION DEPARTMENT USE CHLY CONSTRUCTION PE OF VALUATION OF AL STREET 1 FINE RESISTANCE 1 N.R 9,850 de OCCUPANCY GROUP DE FGH BC 1 J DIVISION IRE ZONE 1 2 5-54

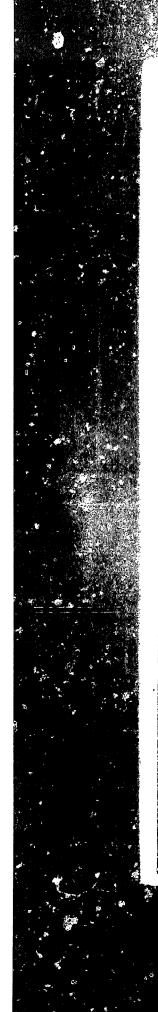
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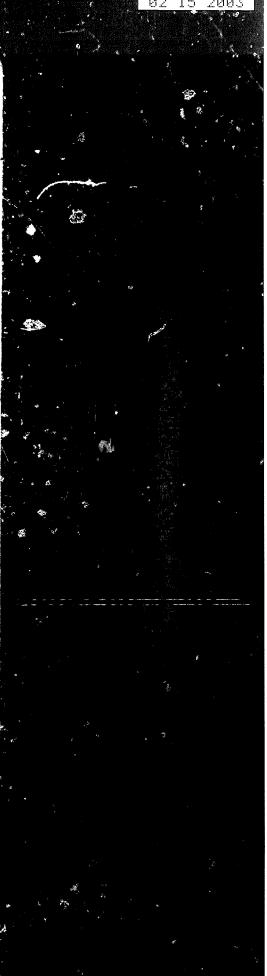
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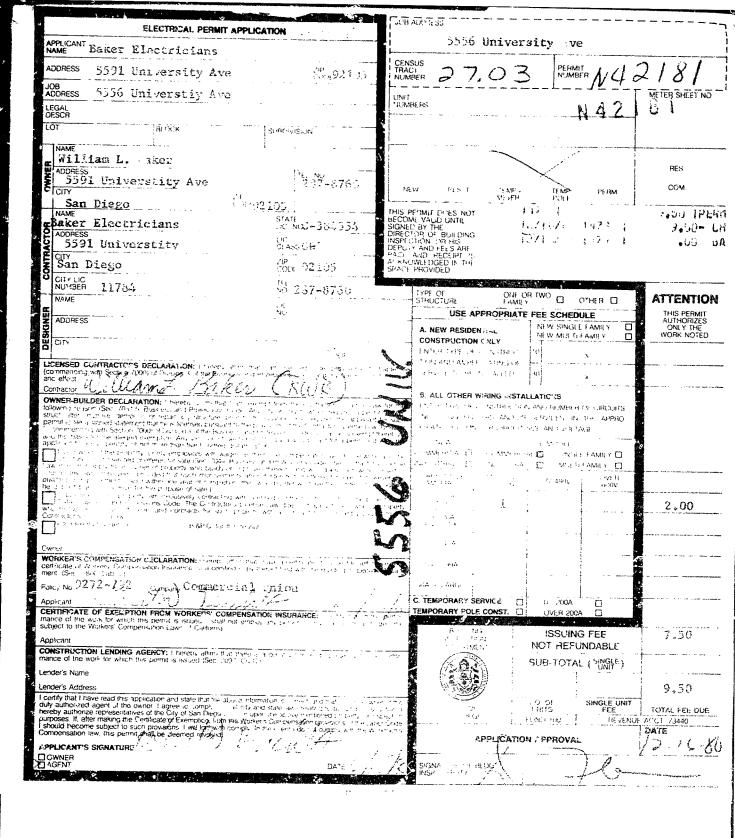
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JOB ADDRESS SEPARATE APPLICATION **ELECTRICAL** Permit Application 5551 REQUIRED FOR EACH 0 BUILDING CENSUS PERMIT TRACT NUMBER NAME OF NAME OF BUSINESS! NUMBER £ 7.02 omeo Offin ŝ MAILING ADDRESS (NUMBER) (STREET) METER SHEET N NA0 UNIT se. 4c= NUMBERS MICERS TELEPHONE NUMBER (CITY) ZIP 462-1096 CA NAME RES. PCIFIC Inecas NEW RESET TEMP TEMP FERM. CCM. CTOR ADDRESS (NUMBER) (STREET) METER POLE // In 71 E a CONTRA TELEPHONE NUMBER ZIP 2 123 2925242 /_ 10 1-7 PAID: (5132 +++++++++ ICLASS NO STATE LICENSE NO CITY LICENSE NUMBER **ZEO 235** کے مع JOB ADDRESS 5556 EN 5.20 Sec. 10 * 64 CALL STREET TON THE CAMERAN Fees TYPE OF For New Residential No Amps Schedule ONE OR TWO FAMILY OTHER 🖄 STRUCTURE 10 Construction Only x \$.25 Check appropriate block FEE SCHEDULES DREDULE A FOR NEW For each ampere of service USENT 955 D 3Ø x \$ 45 А - 10000 FEEDENTIAL CONSTRUCTION CNLY fuse or bleaker size NEW RENOCES AND REP SCHEDULE B. FOR NEW TOM AND MY AND ALL REMODELS 115 230V 126 208V Voltage 277-480 Children Size Over лų: 8 600 V 8 Anipelies $\Delta r_{1} = \frac{1}{2} r_{1}$ f eer hierbir 0-20A F. inclumber of prouts faeder, and or services installing in the left-hand column by the correct France \$ 3 00 \$ 6.00 \$ 1.50 (to 100 incl 10 m. \tilde{T} 0-20A 75 1.50 6.00 CIRU THE TO BE THE APPROPRIATE COLUMN -More than 100 DESERT DOD- TO BE DONE 25 - 35A 2.00 4.00 8.00 Clever. 1 72. 12 00 40 - 45A 3.00 6.00 176-676516-1 50 APP 1 00 8 00 16.00 Hora E TLEIEE 5.0010 00 20.00 5ాల 1051 1994 acknowledge that I to a read this application ation given is correct and that The owner or the July authorized 200 1200 24 00 3994 600 adent of the owner I agree to comply with city and state that requiating analized on in the event Latinot comply with the Workmen. Comput-460 5091 100 14:00 28.00 ation May this permit shall be deemed revoked 1F 90455 TURE LEINAS ôGO. 799,1 200 16 00 32 90 ilit Star and 15 800 - 999A 9.00 18 00 36 00 ANT FOR AT L 1000A & Larger 10.00 20.00 40.00 HESS 0---200 Amperes 67 TEMPORASY SERVICE OTE: CONTRACTORS ARE AUTHORIZED TO CONSTRUCT NLY WORK RECOGNIZED BY THE STATE CONTRACTORS ICENSE BOARD AS BEING WITHIN THEIR CLASSIFICATION. Fee---5.00 Over 200A TEMPORARY POLE CONST Fee-25.00 CALL 236-6256 FOR INSPECTION a state of the second secon DATE 18 15-1 APPLICATION APPROVAL PERMIT DOES NOT BECOME VALIE SIGNED BY THE DIRECTOR OF DING INSPECTION OF HIS DEPUTY FLES ARE PAID, AND RECEIPT IS NOWLEDGED, IN SPACE PROVIDED Min. Fee \$5.00 e si na NOT REFUNDABLE ATTENTION SUB-TOTAL ISINGLEUNIS 25.54 L'EARTARY, الاختصار فال الاستار فال الاحتار فال الاستار 6-1-70 Seil in TURE OF BLDG a. 5:00 DEPUNY SIN BLE UNIT NO OF UNITS `+. FFV 3.781 TOTAL FEL DUE REVENUE ACCT 7344 FUND 100 112 SAN DIEGO

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O. W. CURFIS, DIRECTOR INSPECTION DEPARIMENT

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THE CITY OF SAN DIEGO Development Services Department 1222 First Avenue, San Diego, CA 92101-4154

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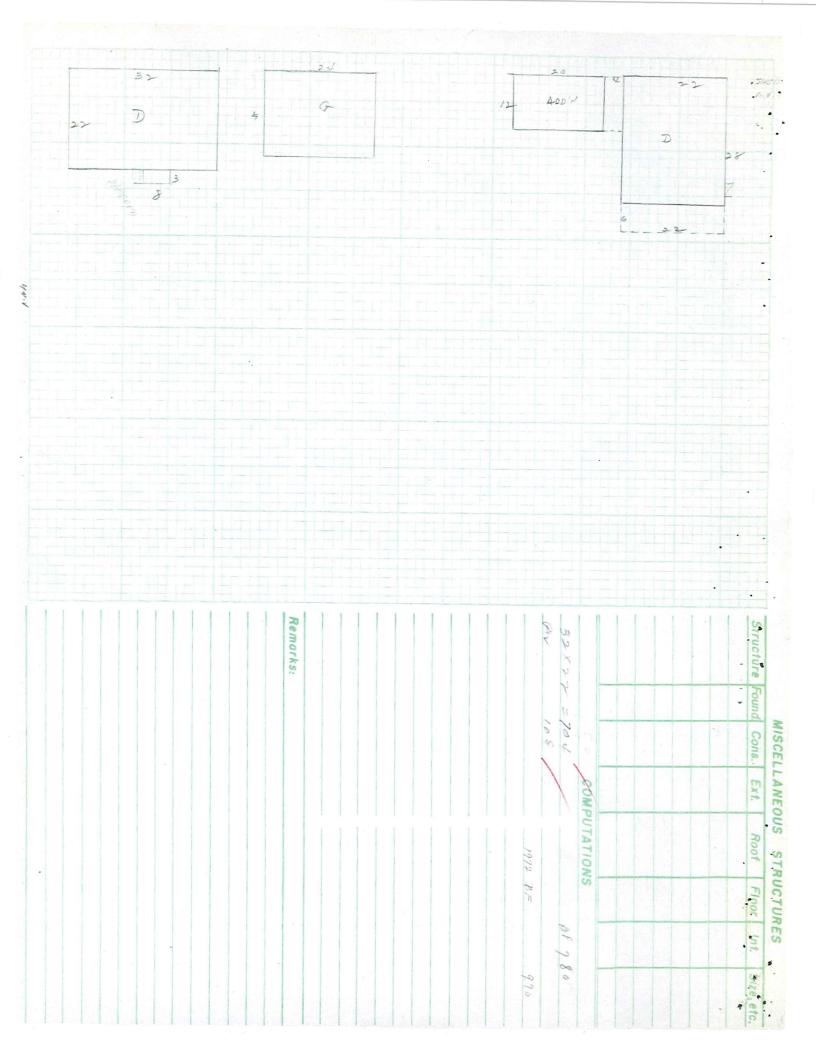
Appendix C

Assessor's Building Records

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┊╪╡ ┾┅┪┥ ┥┝┽┽╎┞╌┼╌ <u>╂╟┝┽╶┼╴┾╼┨╼┽╾</u> ┝╍╅ <mark>╾</mark> ┝╍	╾┫═╀ ╴╏╴┨╶ ╬╕╎╼╞═╬═╊╼╞ _╧ ┽╍╞ _╍ ┾╾┾╌╡╺┿╼╋╶┽╍╎╺┿╍╎╾╋╍┾╸╎╾┽╌┠╶┨╍┼╾┽╸╏╾╎╍╏╍╎╌┤╶┥	╺╴╴┎╍┼╍┿╼╎╺╎╍┼╍┥┥┥┥╌┥╌┥╌┥╶┥╶┥╸┥╴╸
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┟┊╞┽╍╏╍╎╌╎╌╎╌╎╌╎╴╎╸┥╼╎╸╎	╺┋╕┊┊╴╵╍╡╔╶┇╼┇╶┇╸┇╸╈ ┙╫╬╍╁╎╌╎╌╡┠╌┼╌╃╶╀╎╲┨╴┢╌┽╼╎╌┡╌╄╶┨╶╎╶╎╾╆╌╂╼┝╼┼╼╎	╎╼╉╼┼╾╡╎╾┥┥┥╎╌┽╌┶╎╌╋╍┝╍╋╍┾╍╎╍┽╌┥┝╍╡┝╼┨
	╶╬╾╣╞╾╢┟╷╫╌╣┣╌╢┟╍╢┝╢┨╍╣╏╎╌╢╞╾╎╌┑╏╎╏╏┝╌┝╼┝╾╊╍┠╍┼╍╵╍╬╍╊╼┾╾┝╼╣	╴╪╾┨╸╞╾┤╾╪╼┾╌╪╾┝╍┾╍┠╍╬╍╂╍╊╸╎╌╡╌┥╶┼╌┥
		╵╏╏╘╎┊┥╏ ╎ ┆┝╞ ╏╵┾╬╏┥┥┥┾╢╄┫
	┿┨╡╬┙╚┈┤╡╻╶╶╴╢ ╤┨╬┑╲╗┿┼┨╪┼╍┍┽┥╿┥╞┑╴┼┯┨╴╎┵┨╵╕┽╸╸╶╌┨╞╎┟┼╼╴╵╉┾┯┼	
	┦╾┇╍╞╍ ┝╩ <mark>╼┾╾╋╋╪╼╬╞┿╣╤╈╞╌┽┼╌╂╌┼╍╌┽┽╶╂╌┉╍╌╴╞╸┨╌┼┾╌┼┼┨╌┼╶╴</mark>	╾┽┨┾┿┿╇╋┿╧╧╧╋┼╧╵┼╊┧╴┼┶┟╶╆┥╄┛
	┊╏╾ <u>┝╾</u> ┝╼┨╼╉╼╎╦┨╾╊╌╡╾╇╌┝╼┥┥┿┱╂╼┥╾╍╸┡╌╄╍╋╴╹╾╍┝╼┨╼╉╌╬╾┶╌┼╌└╌┨╌╽╌┼╼╂	╼╞╴╉╾┾╍┾╾╬╼┠╸┢╸┝╸┝╸┨╼╎╼╎╾┼╾┦═┥═╋╧╋╧╋╧╋╧╋
┠╍╁╍┼╶┼╾╽╾┿╾┼╾┥╾┥╍╢╾┤╾┾╾╎╶┥╾┝╸╿╴ <u>┠╌┿╾┊╼┥</u> ╸	<u>┆╞╼╬┯╧╼╪╾╋╦╧╋╪╌┶╼╪╍╵</u> ╽╪╌╬╌╬╌╢╴┾╌┨╴╎╴╴╴╎╴┥╴╎╞╼╎┑┥╼	─ ┢╌┨╍╞╌┨╍╞╌┨╼┣ ┊┲┣ ╴╪╼╞╌┨ ┝ ╼╎╼╎╼╎╼╏╺┖╶┧╶┼╸┠╸ ┨
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┢╍┼╍┼╍┨╍┨╍╢╍╬╍┼╍╂╍╉╍╢	╞ ╋╗╋╶┝╶┇╴╏╺┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥┥	╾╉╾╬╾┾╴┾╼╬╾╋╼╪╌┊╍╬╸┫╸╄═╬╧╡╴╉╴╋╴╋╴┥╴╣╶┫╴
	╽┞╸┝╌╪╌╽╌╡╌┨╌┨╶╁╴┟╍┧╶┧╾╄╴║╼┝╌╎╴┝╴┨╴┍╾┼╌┽╌┼╴╽╶╻╶┾╾╾┶╍┝╼┼╶╂═┤╾╎╾╆╸╋╍┾╾┩	╺┥╼╋╼╋╾╅╴┝╴┟╼┧╼┝╶┝╌┼╌┥╼┫╍┨╼┨╼┨╸┨╸╽╼┟╼┠╼┫
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Appendix D

Master Property Records

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Appendix E

Notice of Completion

September 21, 2017

Notices of Completion could not be located.

Appendix F

Site Map and Photograph Key



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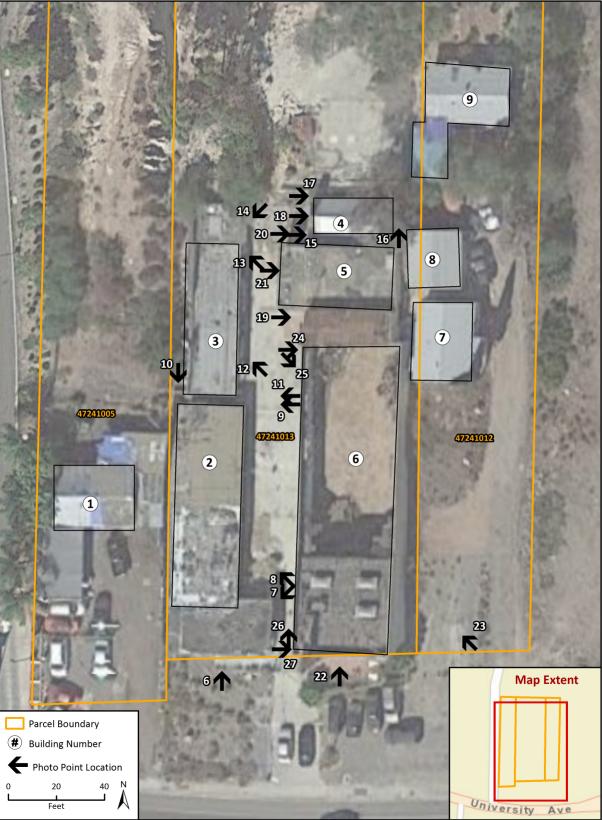


Fig X Site Map Photo Key_CenterParce



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