

PRELIMINARY HYDROLOGY STUDY  
FOR  
LOT 31 RANCHO DEL SOL  
SAN DIEGO, CALIFORNIA

Prepared For:

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Revised 12-20-20



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- Appendix B: Existing and Proposed Drainage Map
- Appendix C: Existing Drainage Basin Calculations
- Appendix D: Proposed Drainage Basin Calculations

## **Introduction**

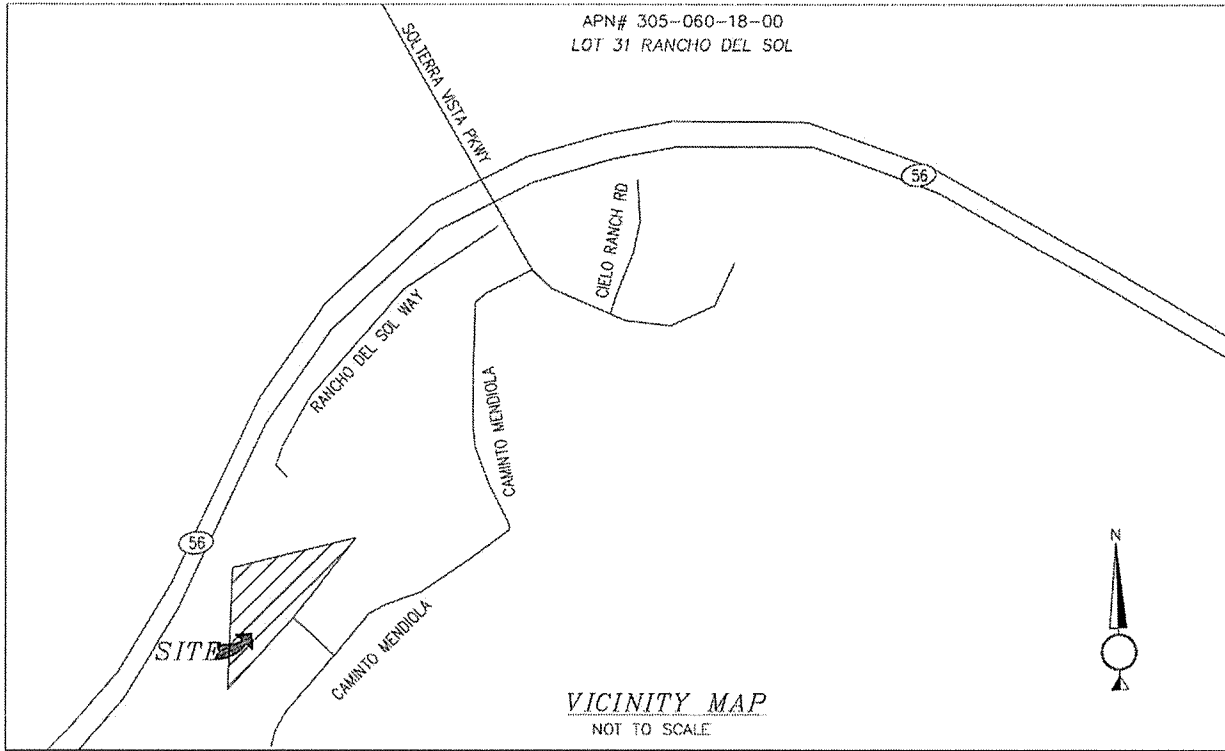
The existing site is located at Assessor's Parcel No. 305-060-18-00, lot 31, located in the Rancho del Sol Unit 1 subdivision, according to Recorded Map No. 12477, in the City and County of San Diego, State of California. For the location of the subject site, refer to the Vicinity Map

This new development proposes to construct a single residential home and a horse barn. The property is bordered on the north by a southeast descending, relatively undisturbed hillside with five residential properties bordering a small portion of the very northwest property boundary; on the west by a relatively undisturbed southerly descending hillside; and on the southeast by existing residential properties lower in elevation.

## **Existing Conditions**

The existing site is vacant with no storm drains within the lot. The property sheet flows from north to south to an existing brow ditch located behind the existing homes and eventually discharging to curb inlets located at Caminito Mendiola street. The site has no offsite runoff from the neighbor on the north side.

**Vicinity Map**



**Proposed Improvements**

The proposed development will drain via a graded swale as shown on the grading plans to a proposed detention basins. This will allow the runoff to be treated before leaving the site via an 18-inch concrete pipe. Additionally, the driveway has been designed with porous pavement to provide treatment for the proposed runoff. The runoff will then be captured and conveyed via three-inch pipes to an 18-inch pipe that will discharge on Caminito Mendiola as well. The open space will keep the same sheet flow and will be conveyed via new concrete brow ditch.

<b>EXISTING CONDITIONS</b>	<b>NODE 400</b>	<b>Q100=2.31</b>
<b>EXISTING CONDITIONS</b>	<b>NODE 300</b>	<b>Q100=10.49</b>
<b>PROPOSED DEVELOPMENT</b>	<b>NODE 200</b>	<b>Q100=1.80</b>
<b>PROPOSED DEVELOPMENT</b>	<b>NODE 100</b>	<b>Q100=10.86</b>

**Conclusion**

The proposed sheet flow has been redirected slightly to discharge to on Caminito Mendiola and the peak runoff will decrease. Therefore, it is our opinion that the redirection of the runoff will not impact the neighbors because the overall location of discharge will remain the same. The project will also install retention basins to collect storm water runoff to be used for treatment and control peak flow purposes. This will decrease any runoff the development site may produce. The site is located next to an open space. However, the project is not required to obtain a 401/404 Permit as required by the Regional Water Quality Control Board under the Federal Clean Water Act.

### **Rational Method Methodology**

The County of San Diego's Drainage Manual requires that the Rational Method be used for hydrologic analysis of a watershed less than 0.5 square-miles.

The Rational Method computer program developed by Advanced Engineering Software (AES) was used for this study because it satisfies the County of San Diego's design criteria. The hydrologic model is developed by creating independent node-link models of each interior drainage basin and linking these sub-models together at confluence points. The program has the capability to perform calculations for 15 hydrologic processes. These processes are assigned code numbers that appear in the results. The code numbers and their significance are as follows:

- Code 1: Confluence analysis at a node
- Code 2: Initial subarea analysis
- Code 3: Pipe flow travel time (computer estimated pipe sizes)
- Code 4: Pipe flow travel time (user specified pipe size)
- Code 5: Trapezoidal channel travel time
- Code 6: Street flow analysis through a subarea
- Code 7: User specified information at a node
- Code 8: Addition of the subarea runoff to mainline
- Code 9: V-Gutter flow through subarea
- Code 10: Copy mainstream data onto a memory bank
- Code 11: Confluence a memory bank with the mainstream memory
- Code 12: Clear a memory bank
- Code 13: Clear the mainstream memory
- Code 14: Copy a memory bank onto the mainstream memory
- Code 15: Hydrologic data bank storage functions

## **Criteria**

In order to perform the hydrologic analysis; base information for the study area is required. This information includes the existing drainage facility locations and sizes, existing land uses, flow patterns, drainage basin boundaries, and topographic elevations. Drainage basin boundaries, flow patterns, and topographic elevations were determined from the Drainage Maps which are located in Appendix B. The hydrologic conditions were analyzed in accordance with the County of San Diego's design criteria as follows:

Design Storm: 100-year 6-hour event

Runoff Coefficients: "C" Valves varied from 0.55 to 0.95 depending on the land use

Soil Type: "B"

Rainfall Intensity: County of San Diego *Drainage Design Manual*

## **References**

City of San Diego Drainage Design Manual, 2018.



**APPENDIX A**  
**City of San Diego Design Charts**



APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

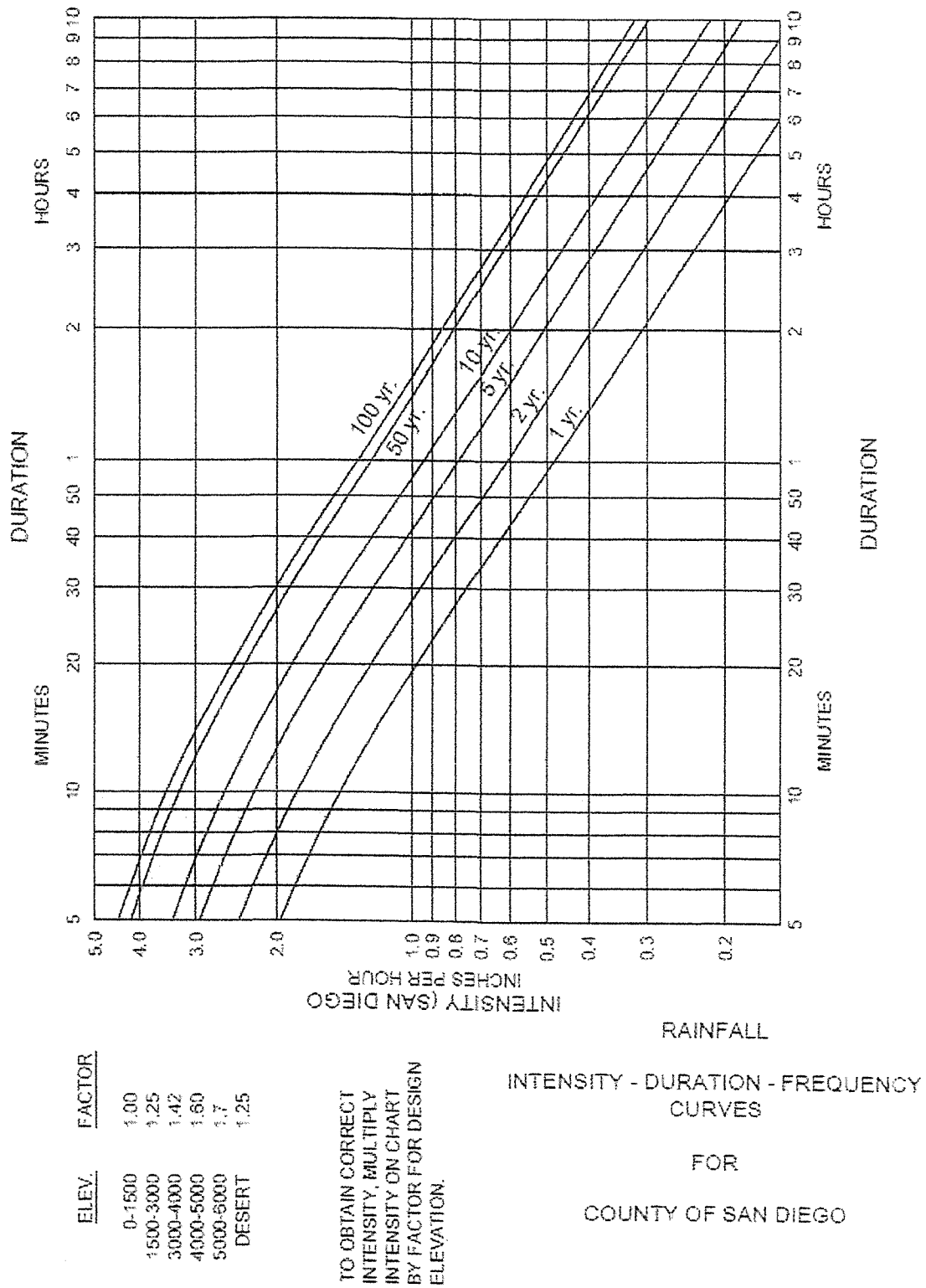


Figure A-1. Intensity-Duration-Frequency Design Chart



APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

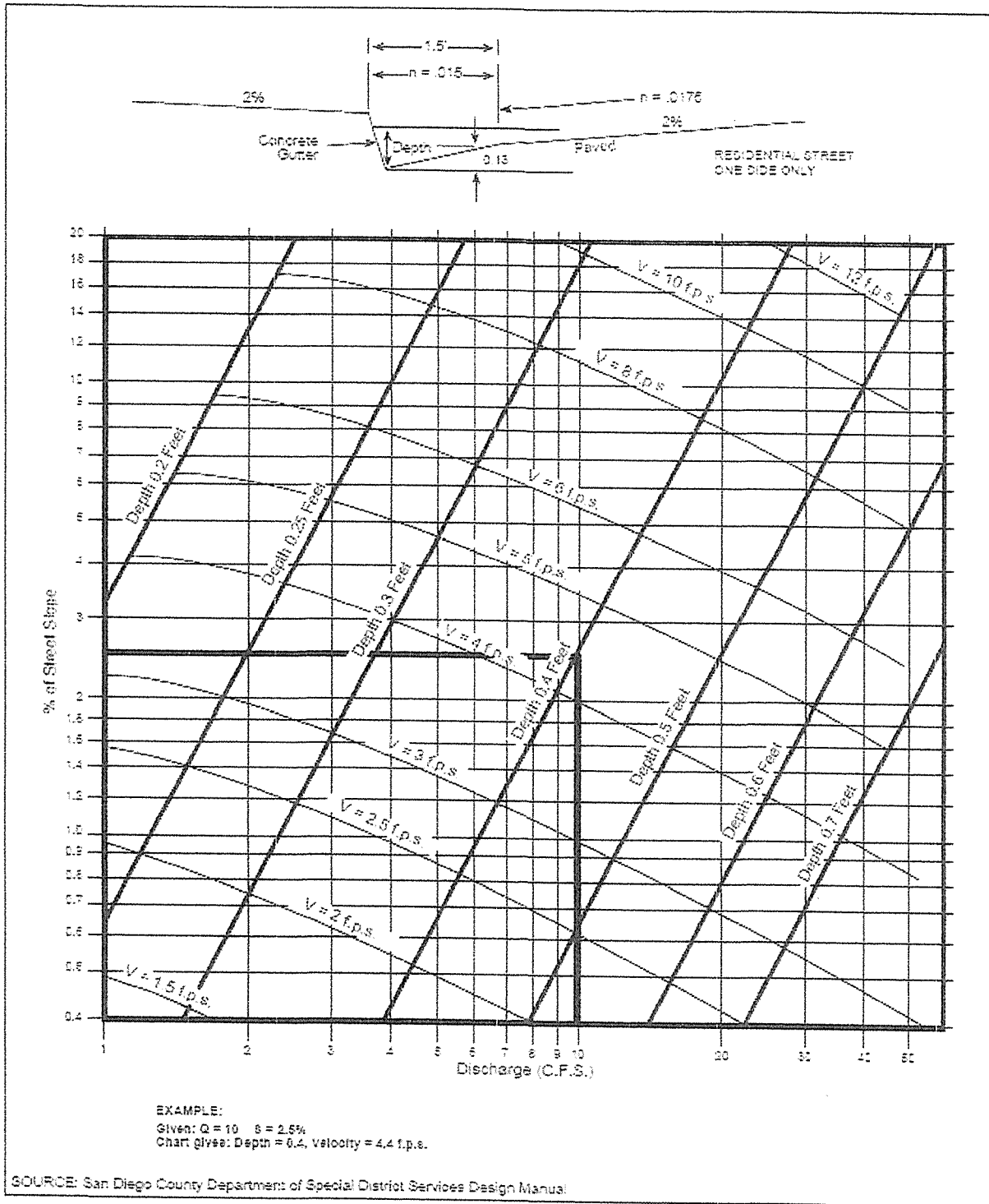


Figure A-5. Gutter and Roadway Discharge - Velocity Chart

APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

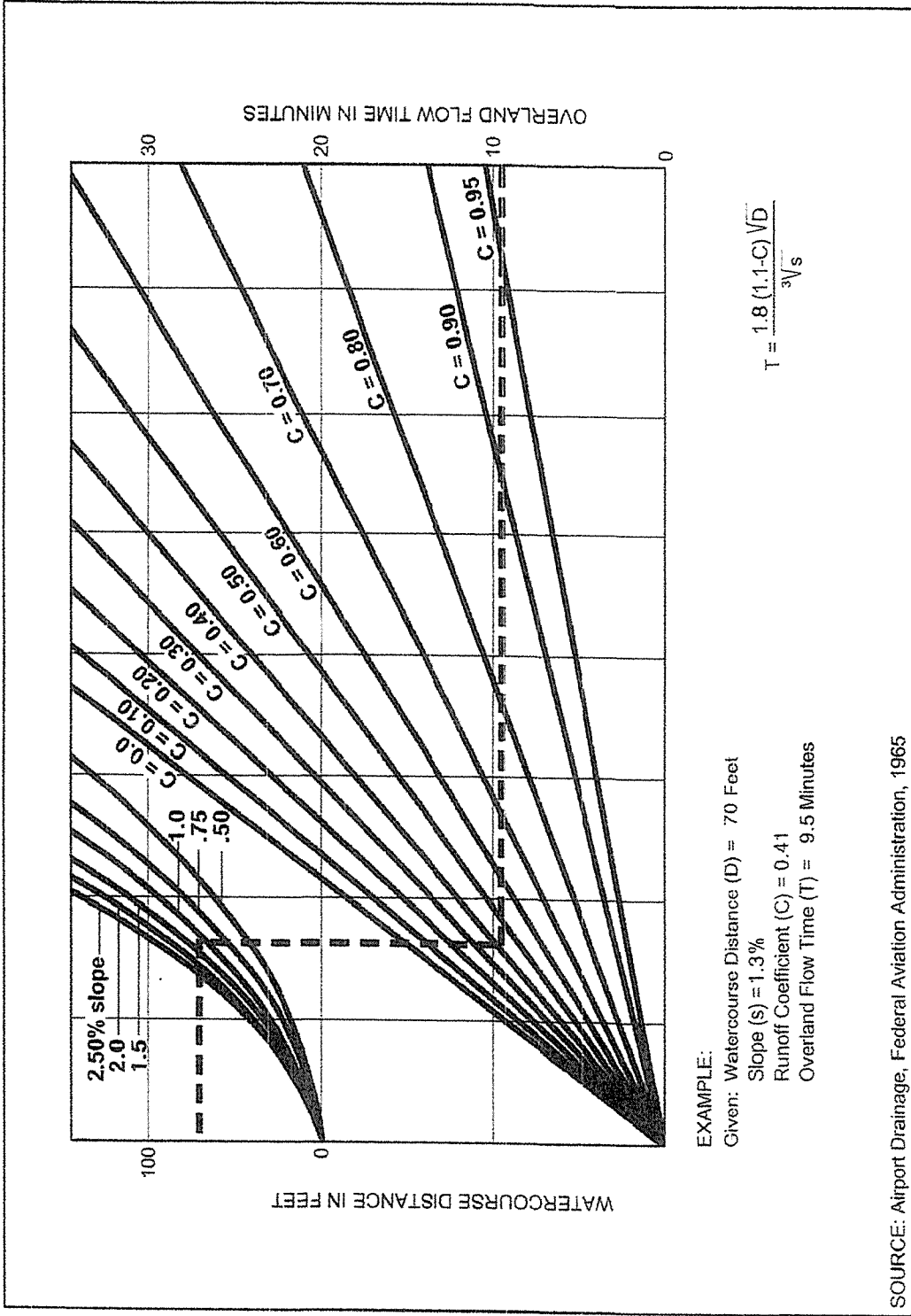


Figure A-4. Rational Formula - Overland Time of Flow Nomograph

Note: Use formula for watercourse distances in excess of 100 feet.



APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

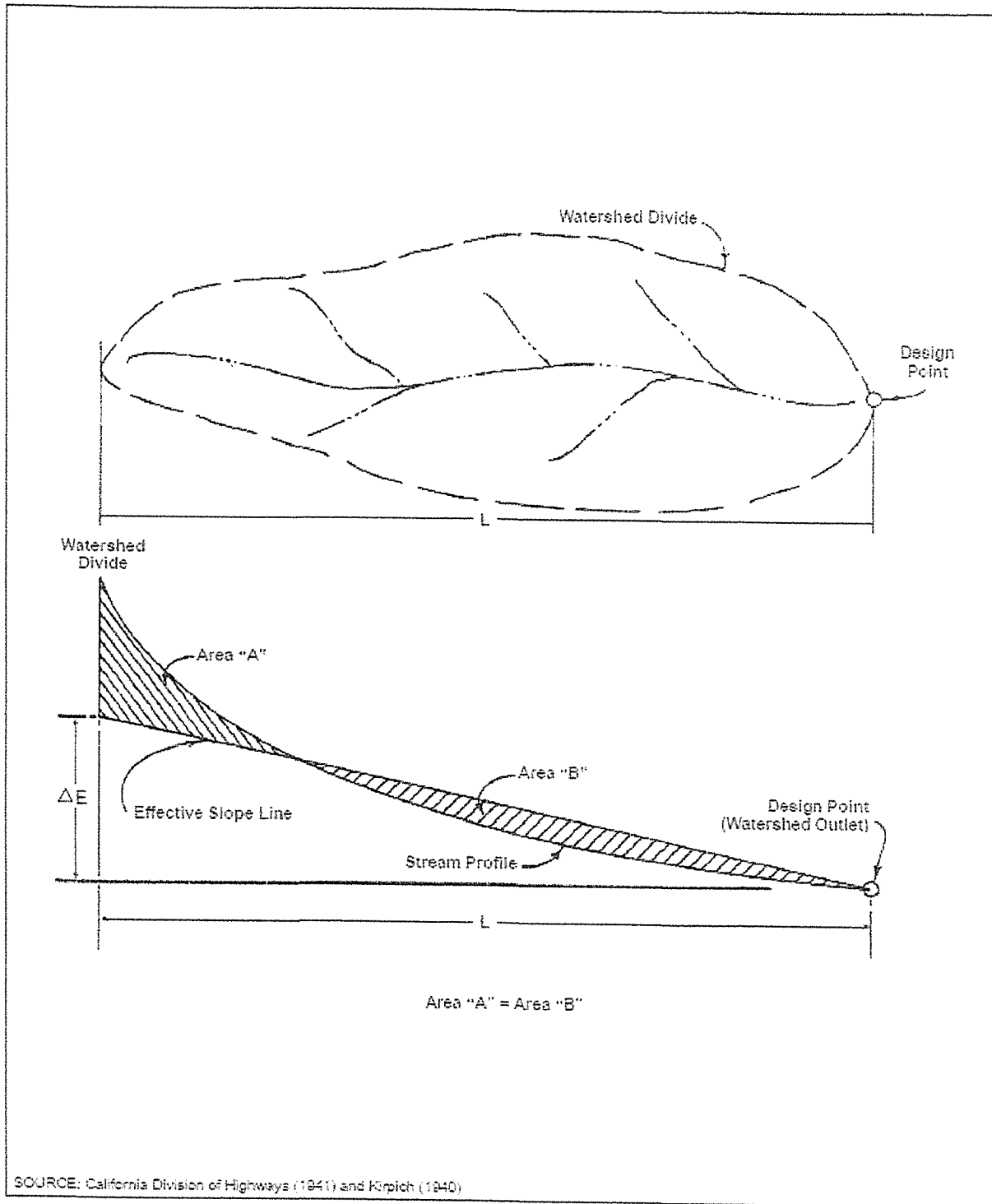


Figure A-3. Computation of Effective Slope for Natural Watersheds

APPENDIX A: RATIONAL METHOD AND MODIFIED RATIONAL METHOD

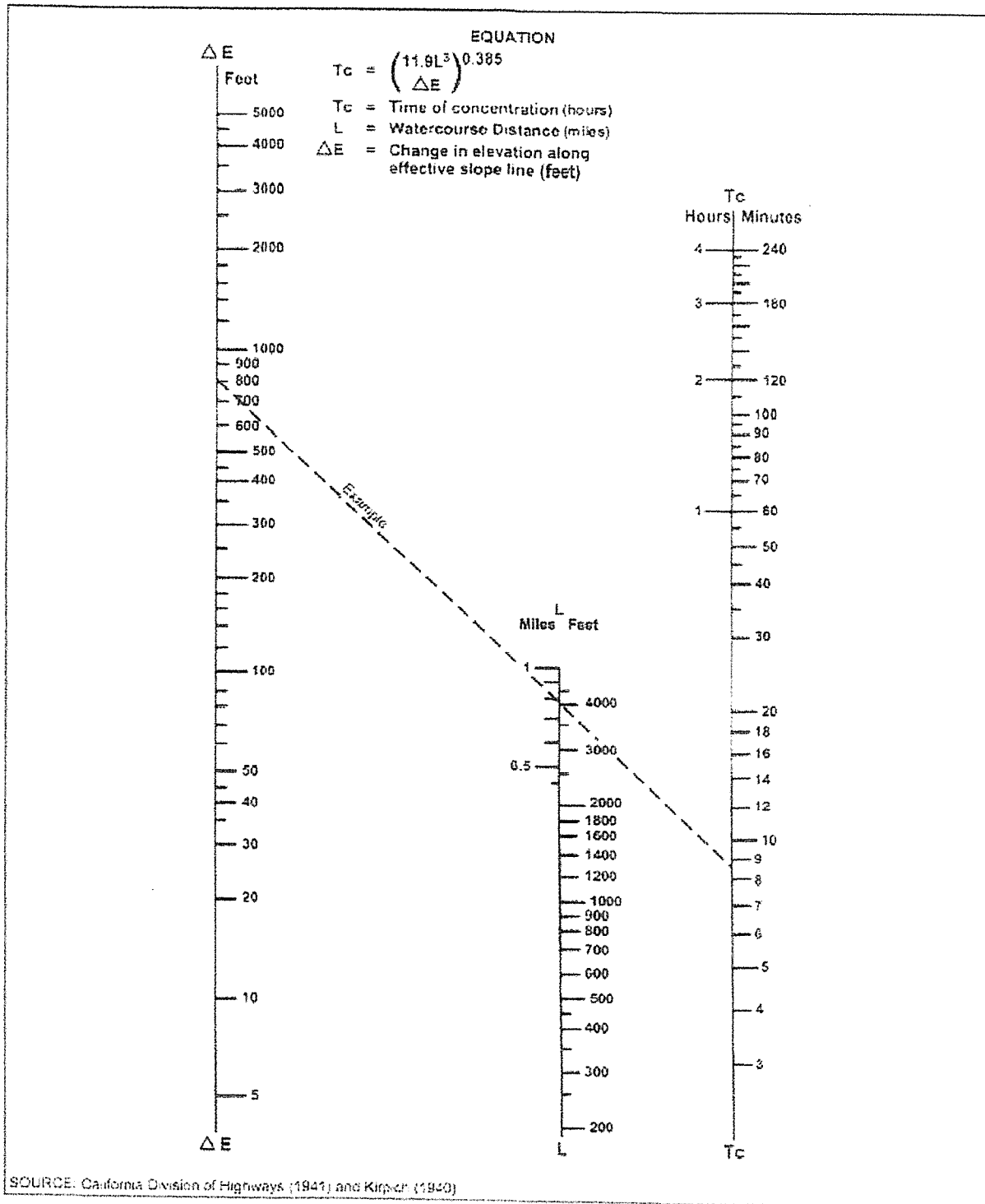


Figure A-2. Nomograph for Determination of  $T_c$  for Natural Watersheds

**Note:** Add ten minutes to the computed time of concentration from Figure A-2.

### 3.2.2.4 Grated Inlets in Sag

A grated inlet in a sag location operates as a weir at shallower depths and as an orifice at larger depths. The designer shall estimate the capacity of the inlet under both weir flow and orifice flow conditions, then adopt a design capacity equal to the smaller of the two results. Figure 3-5 provides a nomogram for calculating the capacity of grated inlets in sag locations.

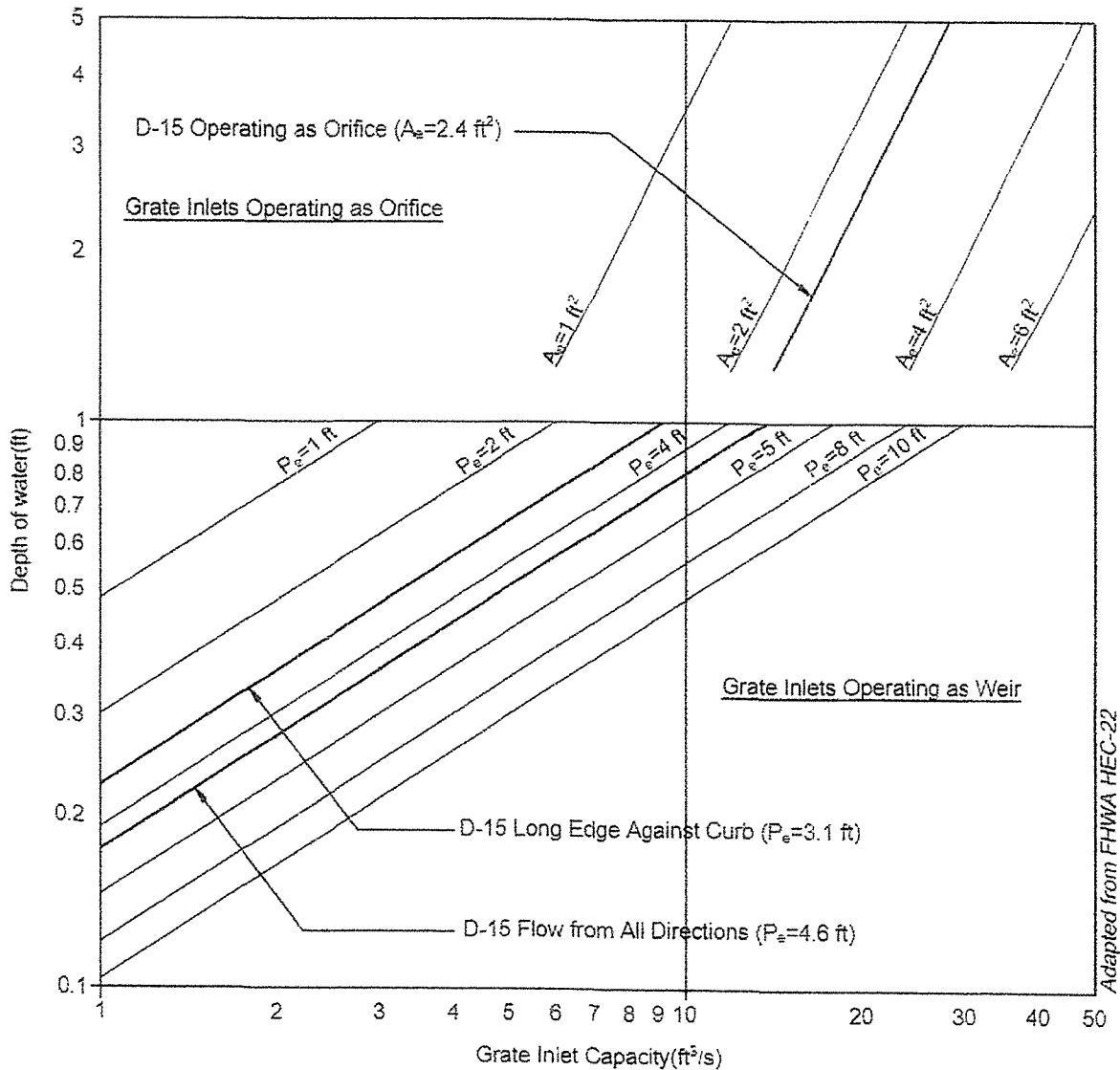


Figure 3-5. Capacity of Grate Inlets in Sag Locations

- Step 1.** Calculate the capacity of a grate inlet operating as a weir using the weir equation (Equation 3-10) with a length equivalent to perimeter of the grate. When the grate is located next to a curb, disregard the length of the grate against the curb.

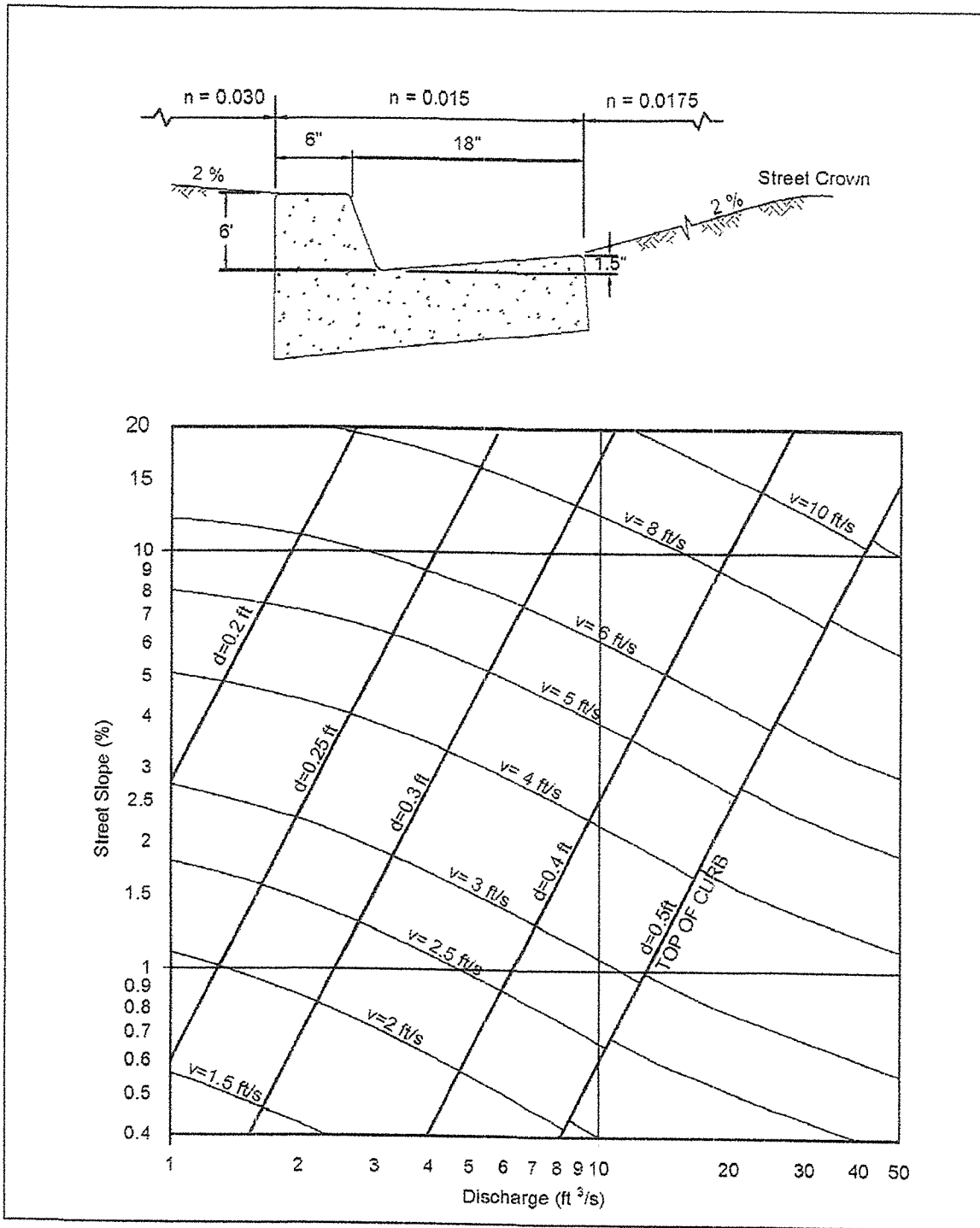


Figure 3-2: Gutter and Roadway Discharge-Velocity Chart (6" Curb)

**TABLE 2**  
**RUNOFF COEFFICIENTS (RATIONAL METHOD)**  
**DEVELOPED AREAS (URBAN)**

<u>Land Use</u>	<u>Coefficient, C</u> <u>Soil Type (1)</u>
Residential:	<u>D</u>
Single Family	.55
Multi-Units	.70
Mobile Homes	.65
Rural (lots greater than 1/2 acre)	.45
Commercial (2) 80% Impervious	.85
Industrial (2) 90% Impervious	.95

**NOTES:**

- (1) Type D soil to be used for all areas.
- (2) Where actual conditions deviate significantly from the tabulated imperviousness values of 80% or 90%, the values given for coefficient C, may be revised by multiplying 80% or 90% by the ratio of actual imperviousness to the tabulated imperviousness. However, in no case shall the final coefficient be less than 0.50. For example: Consider commercial property on D soil.

Actual imperviousness	=	50%
Tabulated imperviousness	=	80%
Revised C	=	$\frac{50}{80} \times 0.85 = 0.53$



6 Selection of Riprap and Filter Blanket (3)  
 200-1.6.1 Riprap and Filter Blanket Material

Vel. Ft/Sec (1)	Rock Class (2)	Riprap Thick-ness (3)	Filter Blanket (3)			Lower Layer (6)
			Upper Layer(s)			
			Opt. 1 Sec. 200 (4)	Opt. 2 Sec. 400 (4)	Opt. 3 (5)	
6-7	No. 3 Back-ling	.6	3/16"	C2	D.O.	---
7-8	No. 2 Back-ling	1.0	1/4"	B3	D.O.	---
8-9.5	Fec-ling	1.4	3/8"	---	D.O.	---
9.5-11	Light	2.0	1/2"	---	3/4", 1 1/2" P.B.	---
11-13	1/4 Ton	2.7	3/4"	---	3/4", 1 1/2" P.B.	Sand
13-15	1/2 Ton	3.4	1"	---	3/4", 1 1/2" P.B.	Sand
15-17	1 Ton	4.3	1 1/2"	---	Type B	Sand
17-20	2 Ton	5.4	2"	---	Type B	Sand

Practical use of this table is limited to situations where "V" is less than D.  
 (1) Average velocity in pipe or bottom velocity in energy dissipator, whichever is greater.

- (2) If desired riprap and filter blanket class is not available, use next larger class.
- (3) Filter blanket thickness = 1 Foot or "V", whichever is less.

- (4) Standard Specifications for Public Works Construction.
- (5) D.O. = Disintegrated Granite, 1 MM to 10 MM
- P.B. = Processed Miscellaneous Base
- Type B = Type B bedding material, (minimum 75% crushed particles, 100% passing 2 1/2" sieve, 10% passing 1" sieve)
- (6) Sand 75% retained on #200 sieve.

SECTION 201 - CONCRETE, MORTAR AND RELATED MATERIALS  
 201-1 PORTLAND CEMENT CONCRETE  
 201-1.1.2 Concrete Specified By Class (Pg. 88)

- In "Concrete Class Use Table" modify as follows:
- (1) **Revised Concrete Pavement (not Integral with curb)** 570-A-2500
  - To Read: Concrete Pavement (not Integral with curb), Cross Gutter and Alley Aprons** 560-C-3250
  - (2) **Revised Curb, Integral Curb and Pavement, Gutter, Walk, Alley Aprons** 570-O-2500
  - To Read: Curb and Gutter (separate or combined) and Walks** 570-C-2500
  - (3) **Change concrete class for "Shoulder Surface Drainage Facilities" from "200-C-2500" to "270-O-2500".**

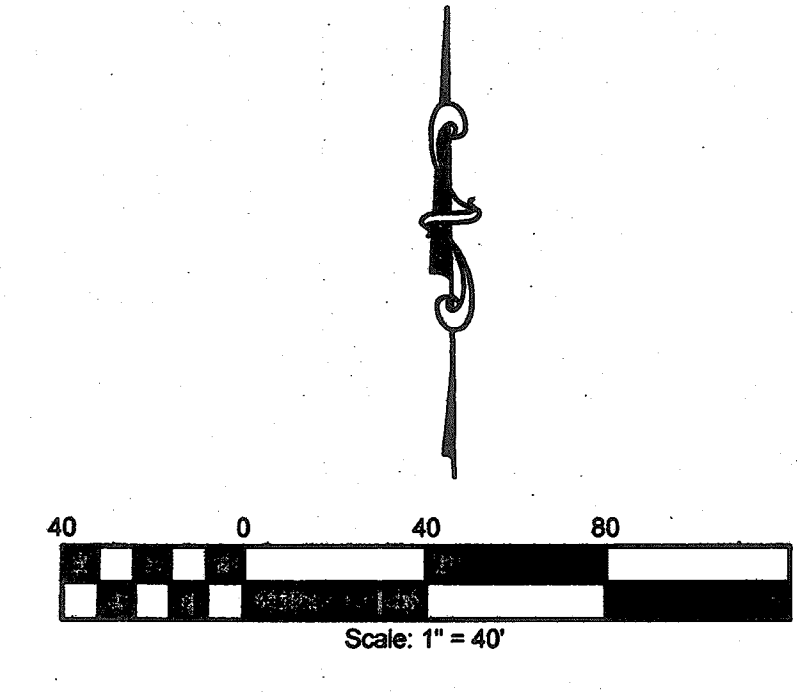
# APPENDIX B

## Existing and Proposed Drainage Maps





- LEGEND**
- SUBDIVISION BOUNDARY ————
  - BASIN LIMITS ————
  - FLOW DIRECTION ————
  - PAD ELEVATION — PAD 000
  - NODE NUMBER — [XXX]
  - NODE NUMBER — (AC)



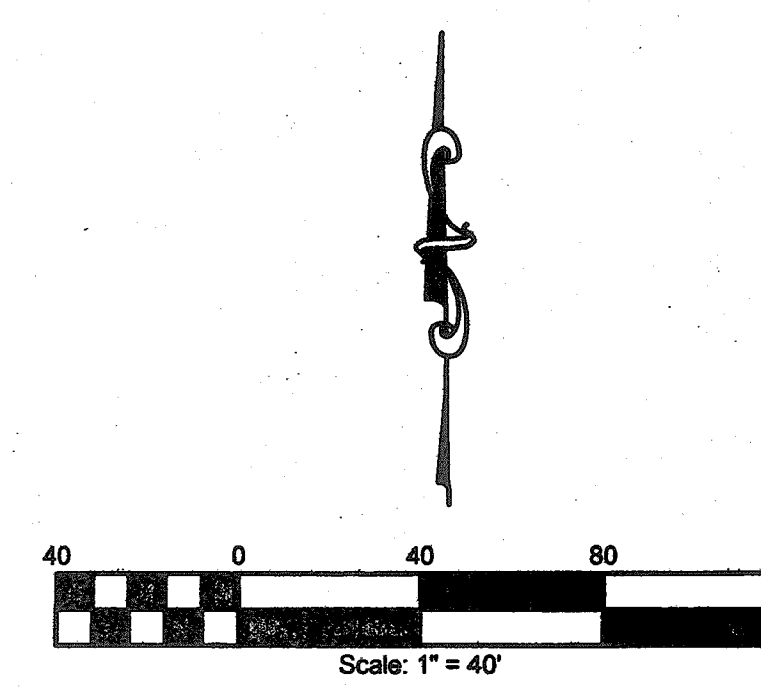
**LOT 31, RANCHO DEL SOL UNIT 1  
PRE DEVELOPMENT DRAINAGE MAP**

PREPARED BY: ARC CONSTRUCTION & ENGINEERING INC.	REVISION 1:	12-20-20
ADDRESS: 11879 VIA FRIEL SAN DIEGO, CA 92128	REVISION 2:	
PHONE NO.: 628-575-9480	REVISION 3:	
PROJECT ADDRESS: 13185 CAMINITO MENDIOLA SAN DIEGO, CA 92130	REVISION 4:	
PROJECT NAME: RANCHO DEL SOL	REVISION 5:	
SHEET TITLE: PRELIMINARY POST DEVELOPMENT DRAINAGE MAP	REVISION 6:	
SHEET NO.: PRE DEVELOPMENT	REVISION 7:	
	REVISION 8:	
	REVISION 9:	
	REVISION 10:	
	ORIG. DATE:	4-20-20
	SHEET	1 OF 2
	P.T.S. NO.	508590
	PERMIT NO.	



**LEGEND**

- SUBDIVISION BOUNDARY \_\_\_\_\_
- BASIN LIMITS \_\_\_\_\_
- FLOW DIRECTION \_\_\_\_\_
- PAD ELEVATION \_\_\_\_\_ PAD 000
- NODE NUMBER \_\_\_\_\_ XXX
- NODE NUMBER \_\_\_\_\_ AC



**LOT 31, RANCHO DEL SOL UNIT 1  
POST DEVELOPMENT DRAINAGE MAP**

PREPARED BY: ABC CONSTRUCTION & ENGINEERING INC.	REVISION 1:	12-20-20
ADDRESS: 11879 VIA FIRIL SAN DIEGO, CA 92128	REVISION 2:	_____
PHONE NO.: 858-875-8450	REVISION 3:	_____
PROJECT ADDRESS: 13185 CAMINITO MENDOCINO SAN DIEGO, CA 92130	REVISION 4:	_____
PROJECT NAME: RANCHO DEL SOL	REVISION 5:	_____
SHEET TITLE: PRELIMINARY POST DEVELOPMENT DRAINAGE MAP	REVISION 6:	_____
SHEET NO.: POST DEVELOPMENT	REVISION 7:	_____
	REVISION 8:	_____
	REVISION 9:	_____
	REVISION 10:	_____
	ORIG. DATE:	4-20-20
	SHEET 1	OF 2
	P.T.S. NO.	508580
	PERMIT NO.	_____



**APPENDIX C**

**Existing Drainage Basin Calculations**

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RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT  
2003,1985,1981 HYDROLOGY MANUAL  
(c) Copyright 1982-2014 Advanced Engineering Software (aes)  
Ver. 21.0 Release Date: 06/01/2014 License ID 1667

Analysis prepared by:

ARC CONSTRUCTION & ENGINEERING INC.  
10948 ELDERWOOD LANE  
SAN DIEGO CA 92131

\*\*\*\*\* DESCRIPTION OF STUDY  
\*\*\*\*\*

\* 100 YEAR EVENT  
\*  
\* LOT 31 RANCHO DEL SOL  
\*  
\* DATE:4/20/20  
\*

\*\*\*\*\*  
\*

FILE NAME: LOT32.DAT  
TIME/DATE OF STUDY: 07:55 04/21/2020

-----  
USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:  
-----

-----  
USER SPECIFIED STORM EVENT(YEAR) = 100.00  
SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00  
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE =  
0.95

RAINFALL-INTENSITY ADJUSTMENT FACTOR = 1.000

\*USER SPECIFIED:

NUMBER OF [TIME,INTENSITY] DATA PAIRS = 9

- 1) 5.000; 4.410
- 2) 10.000; 3.480
- 3) 15.000; 3.000
- 4) 20.000; 2.530
- 5) 25.000; 2.400
- 6) 30.000; 2.000
- 7) 40.000; 1.650
- 8) 50.000; 1.500

9) 60.000; 1.400  
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD  
 NOTE: ONLY PEAK CONFLUENCE VALUES CONSIDERED  
 \*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW

MODEL\*

MANNING		HALF- CROWN TO STREET-CROSSFALL:			CURB GUTTER-GEOMETRIES:				
NO.	WIDTH (FT)	CROSSFALL (FT)	IN- / SIDE	OUT- / SIDE	PARK- / WAY	HEIGHT (FT)	WIDTH (FT)	LIP (FT)	HIKE (FT)
1	30.0	20.0	0.018	0.018	0.020	0.67	2.00	0.0312	0.167

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET  
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
  2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)
- \*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*

\*\*\*\*\*  
 \*\*\*

FLOW PROCESS FROM NODE 300.00 TO NODE 301.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
 SOIL CLASSIFICATION IS "D"  
 S.C.S. CURVE NUMBER (AMC II) = 88  
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 116.00  
 UPSTREAM ELEVATION(FEET) = 287.00  
 DOWNSTREAM ELEVATION(FEET) = 259.00  
 ELEVATION DIFFERENCE(FEET) = 28.00  
 URBAN SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267  
 WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN  
 THE MAXIMUM OVERLAND FLOW LENGTH = 100.00  
 (Reference: Table 3-1B of Hydrology Manual)  
 THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.174  
 SUBAREA RUNOFF(CFS) = 0.38  
 TOTAL AREA(ACRES) = 0.26 TOTAL RUNOFF(CFS) = 0.38

\*\*\*\*\*  
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FLOW PROCESS FROM NODE 302.00 TO NODE 303.00 IS CODE = 51

-----  
-----  
>>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<<  
>>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<<

=====  
=====  
ELEVATION DATA: UPSTREAM(FEET) = 259.00 DOWNSTREAM(FEET) =  
189.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 819.84 CHANNEL SLOPE = 0.0854  
CHANNEL BASE(FEET) = 50.00 "Z" FACTOR = 10.000  
MANNING'S FACTOR = 0.015 MAXIMUM DEPTH(FEET) = 1.00  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.413  
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 88  
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 5.48  
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 3.08  
AVERAGE FLOW DEPTH(FEET) = 0.04 TRAVEL TIME(MIN.) = 4.43  
Tc(MIN.) = 10.70  
SUBAREA AREA(ACRES) = 8.50 SUBAREA RUNOFF(CFS) = 10.15  
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350  
TOTAL AREA(ACRES) = 8.8 PEAK FLOW RATE(CFS) =  
10.46

END OF SUBAREA CHANNEL FLOW HYDRAULICS:  
DEPTH(FEET) = 0.05 FLOW VELOCITY(FEET/SEC.) = 4.10  
LONGEST FLOWPATH FROM NODE 300.00 TO NODE 303.00 = 935.84  
FEET.

=====  
=====  
END OF STUDY SUMMARY:  
TOTAL AREA(ACRES) = 8.8 TC(MIN.) = 10.70  
PEAK FLOW RATE(CFS) = 10.46

=====  
=====  
END OF RATIONAL METHOD ANALYSIS



\*\*\*\*\*  
\*\*\*

RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT  
2003,1985,1981 HYDROLOGY MANUAL  
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Ver. 21.0 Release Date: 06/01/2014 License ID 1667

Analysis prepared by:

ARC CONSTRUCTION & ENGINEERING INC.  
10948 ELDERWOOD LANE  
SAN DIEGO CA 92131

\*\*\*\*\* DESCRIPTION OF STUDY  
\*\*\*\*\*

\* 100 YEAR EVENT  
\*  
\* LOT 31 RANCHO DEL SOL  
\*  
\* DATE:4/20/20  
\*

\*\*\*\*\*  
\*

FILE NAME: LOT33.DAT  
TIME/DATE OF STUDY: 06:30 04/21/2020

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USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:  
-----

-----  
USER SPECIFIED STORM EVENT(YEAR) = 100.00  
SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00  
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE =  
0.95

RAINFALL-INTENSITY ADJUSTMENT FACTOR = 1.000

\*USER SPECIFIED:

NUMBER OF [TIME,INTENSITY] DATA PAIRS = 9

1) 5.000; 4.410  
2) 10.000; 3.480  
3) 15.000; 3.000  
4) 20.000; 2.530  
5) 25.000; 2.400  
6) 30.000; 2.000  
7) 40.000; 1.650  
8) 50.000; 1.500

9) 60.000; 1.400  
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD  
 NOTE: ONLY PEAK CONFLUENCE VALUES CONSIDERED  
 \*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW

MODEL\*

MANNING FACTOR NO. (n)	HALF- WIDTH (FT)	CROWN TO CROSSFALL (FT)	STREET-CROSSFALL: IN- / OUT- / PARK- SIDE / SIDE / WAY	CURB HEIGHT (FT)	GUTTER-GEOMETRIES: WIDTH (FT)	LIP (FT)	HIKE (FT)
	1 0.0150	30.0	20.0	0.018/0.018/0.020	0.67	2.00	0.0312

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET  
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
  2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)
- \*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*

\*\*\*\*\*  
 \*\*\*

FLOW PROCESS FROM NODE 400.00 TO NODE 401.00 IS CODE = 21

-----  
 >>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

=====

NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
 SOIL CLASSIFICATION IS "D"  
 S.C.S. CURVE NUMBER (AMC II) = 88  
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 133.00  
 UPSTREAM ELEVATION(FEET) = 291.50  
 DOWNSTREAM ELEVATION(FEET) = 270.00  
 ELEVATION DIFFERENCE(FEET) = 21.50  
 URBAN SUBAREA OVERLAND TIME OF FLOW(MIN.) = 6.267  
 WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN  
 THE MAXIMUM OVERLAND FLOW LENGTH = 100.00  
 (Reference: Table 3-1B of Hydrology Manual)  
 THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.174  
 SUBAREA RUNOFF(CFS) = 0.28  
 TOTAL AREA(ACRES) = 0.19 TOTAL RUNOFF(CFS) = 0.28

\*\*\*\*\*  
 \*\*\*

FLOW PROCESS FROM NODE 402.00 TO NODE 403.00 IS CODE = 52

-----  
-----  
>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA<<<<<

=====  
=====  
ELEVATION DATA: UPSTREAM(FEET) = 270.00 DOWNSTREAM(FEET) =  
219.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 298.37 CHANNEL SLOPE = 0.1709  
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION  
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION  
CHANNEL FLOW THRU SUBAREA(CFS) = 0.28  
FLOW VELOCITY(FEET/SEC) = 4.74 (PER LACFCD/RCFC&WCD HYDROLOGY  
MANUAL)  
TRAVEL TIME(MIN.) = 1.05 Tc(MIN.) = 7.31  
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 403.00 = 431.37  
FEET.

\*\*\*\*\*  
\*\*\*  
FLOW PROCESS FROM NODE 402.00 TO NODE 403.00 IS CODE = 51  
-----

-----  
-----  
>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

=====  
=====  
ELEVATION DATA: UPSTREAM(FEET) = 219.00 DOWNSTREAM(FEET) =  
209.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 266.91 CHANNEL SLOPE = 0.0375  
CHANNEL BASE(FEET) = 2.00 "Z" FACTOR = 1.000  
MANNING'S FACTOR = 0.013 MAXIMUM DEPTH(FEET) = 1.00  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.819  
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 88  
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.31  
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.17  
AVERAGE FLOW DEPTH(FEET) = 0.12 TRAVEL TIME(MIN.) = 0.86  
Tc(MIN.) = 8.17  
SUBAREA AREA(ACRES) = 1.54 SUBAREA RUNOFF(CFS) = 2.06  
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350  
TOTAL AREA(ACRES) = 1.7 PEAK FLOW RATE(CFS) =  
2.31

END OF SUBAREA CHANNEL FLOW HYDRAULICS:  
DEPTH(FEET) = 0.17 FLOW VELOCITY(FEET/SEC.) = 6.24  
LONGEST FLOWPATH FROM NODE 400.00 TO NODE 403.00 = 698.28  
FEET.

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END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 1.7 TC (MIN.) = 8.17

PEAK FLOW RATE (CFS) = 2.31

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END OF RATIONAL METHOD ANALYSIS

# APPENDIX D

## Proposed Drainage Basin Calculations

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RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT  
2003,1985,1981 HYDROLOGY MANUAL  
(c) Copyright 1982-2014 Advanced Engineering Software (aes)  
Ver. 21.0 Release Date: 06/01/2014 License ID 1667

Analysis prepared by:

ARC CONSTRUCTION & ENGINEERING INC.  
10948 ELDERWOOD LANE  
SAN DIEGO CA 92131

\*\*\*\*\* DESCRIPTION OF STUDY  
\*\*\*\*\*

\* 100 YEAR EVENT  
\*  
\* LOT 31 RANCHO DEL SOL  
\*  
\* DATE:12/20/20  
\*

\*\*\*\*\*  
\*

FILE NAME: LOT30.DAT  
TIME/DATE OF STUDY: 10:55 12/28/2010

-----  
-----  
USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:  
-----  
-----

USER SPECIFIED STORM EVENT(YEAR) = 100.00  
SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00  
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE =  
0.95

RAINFALL-INTENSITY ADJUSTMENT FACTOR = 1.000

\*USER SPECIFIED:

NUMBER OF [TIME,INTENSITY] DATA PAIRS = 9

1) 5.000; 4.410  
2) 10.000; 3.480  
3) 15.000; 3.000  
4) 20.000; 2.530  
5) 25.000; 2.400  
6) 30.000; 2.000  
7) 40.000; 1.650  
8) 50.000; 1.500

9) 60.000; 1.400  
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD  
 NOTE: ONLY PEAK CONFLUENCE VALUES CONSIDERED  
 \*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW

MODEL\*

MANNING		CROWN TO STREET-CROSSFALL:			CURB GUTTER-GEOMETRIES:				
NO.	WIDTH	CROSSFALL	IN- / SIDE	OUT- / SIDE	PARK- WAY	HEIGHT	WIDTH	LIP	HIKE
(n)	(FT)	(FT)				(FT)	(FT)	(FT)	(FT)
1	30.0	20.0	0.018/0.018/0.020			0.67	2.00	0.0313	0.167

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:  
 1. Relative Flow-Depth = 0.00 FEET  
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)  
 2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)  
 \*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN  
 OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*

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 FLOW PROCESS FROM NODE 100.00 TO NODE 101.00 IS CODE = 21  
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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

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 NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
 SOIL CLASSIFICATION IS "D"  
 S.C.S. CURVE NUMBER (AMC II) = 88  
 INITIAL SUBAREA FLOW-LENGTH (FEET) = 87.73  
 UPSTREAM ELEVATION (FEET) = 291.00  
 DOWNSTREAM ELEVATION (FEET) = 273.00  
 ELEVATION DIFFERENCE (FEET) = 18.00  
 URBAN SUBAREA OVERLAND TIME OF FLOW (MIN.) = 5.870  
 WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc  
 CALCULATION!  
 100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 4.248  
 SUBAREA RUNOFF (CFS) = 0.15  
 TOTAL AREA (ACRES) = 0.10 TOTAL RUNOFF (CFS) = 0.15

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 \*\*\*  
 FLOW PROCESS FROM NODE 101.00 TO NODE 102.00 IS CODE = 52  
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>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 273.00 DOWNSTREAM(FEET) =  
238.50  
CHANNEL LENGTH THRU SUBAREA(FEET) = 193.46 CHANNEL SLOPE = 0.1783  
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION  
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION  
CHANNEL FLOW THRU SUBAREA(CFS) = 0.15  
FLOW VELOCITY(FEET/SEC) = 4.74 (PER LACFCD/RCFC&WCD HYDROLOGY  
MANUAL)  
TRAVEL TIME(MIN.) = 0.68 Tc(MIN.) = 6.55  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 102.00 = 281.19  
FEET.

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FLOW PROCESS FROM NODE 102.00 TO NODE 103.00 IS CODE = 51

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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 238.50 DOWNSTREAM(FEET) =  
195.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 481.74 CHANNEL SLOPE = 0.0903  
CHANNEL BASE(FEET) = 2.00 "Z" FACTOR = 1.000  
MANNING'S FACTOR = 0.013 MAXIMUM DEPTH(FEET) = 1.00  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.883  
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 88  
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.05  
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 6.24  
AVERAGE FLOW DEPTH(FEET) = 0.08 TRAVEL TIME(MIN.) = 1.29  
Tc(MIN.) = 7.84  
SUBAREA AREA(ACRES) = 1.33 SUBAREA RUNOFF(CFS) = 1.81  
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350  
TOTAL AREA(ACRES) = 1.4 PEAK FLOW RATE(CFS) =  
1.94

END OF SUBAREA CHANNEL FLOW HYDRAULICS:  
DEPTH(FEET) = 0.12 FLOW VELOCITY(FEET/SEC.) = 7.69  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 103.00 = 762.93  
FEET.

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FLOW PROCESS FROM NODE 103.00 TO NODE 104.00 IS CODE = 31



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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 195.00 DOWNSTREAM(FEET) = 194.30  
FLOW LENGTH(FEET) = 23.52 MANNING'S N = 0.013  
DEPTH OF FLOW IN 9.0 INCH PIPE IS 5.5 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 6.81  
ESTIMATED PIPE DIAMETER(INCH) = 9.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 1.94  
PIPE TRAVEL TIME(MIN.) = 0.06 Tc(MIN.) = 7.89  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 104.00 = 786.45  
FEET.

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FLOW PROCESS FROM NODE 104.00 TO NODE 104.00 IS CODE = 10  
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>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<  
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FLOW PROCESS FROM NODE 105.00 TO NODE 106.00 IS CODE = 21  
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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<  
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NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 88  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 85.52  
UPSTREAM ELEVATION(FEET) = 291.00  
DOWNSTREAM ELEVATION(FEET) = 273.00  
ELEVATION DIFFERENCE(FEET) = 18.00  
URBAN SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.795  
WARNING: THE MAXIMUM OVERLAND FLOW SLOPE, 10.%, IS USED IN Tc  
CALCULATION!  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.262  
SUBAREA RUNOFF(CFS) = 0.15  
TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.15

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FLOW PROCESS FROM NODE 106.00 TO NODE 107.00 IS CODE = 52

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>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 273.00 DOWNSTREAM(FEET) =  
238.50  
CHANNEL LENGTH THRU SUBAREA(FEET) = 195.21 CHANNEL SLOPE = 0.1767  
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION  
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION  
CHANNEL FLOW THRU SUBAREA(CFS) = 0.15  
FLOW VELOCITY(FEET/SEC) = 4.74 (PER LACFCD/RCFC&WCD HYDROLOGY  
MANUAL)  
TRAVEL TIME(MIN.) = 0.69 Tc(MIN.) = 6.48  
LONGEST FLOWPATH FROM NODE 105.00 TO NODE 107.00 = 280.73  
FEET.

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FLOW PROCESS FROM NODE 107.00 TO NODE 108.00 IS CODE = 51

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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 238.50 DOWNSTREAM(FEET) =  
205.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 380.00 CHANNEL SLOPE = 0.0882  
CHANNEL BASE(FEET) = 2.00 "Z" FACTOR = 1.000  
MANNING'S FACTOR = 0.013 MAXIMUM DEPTH(FEET) = 1.00  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.009  
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 88  
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 3.47  
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 9.37  
AVERAGE FLOW DEPTH(FEET) = 0.17 TRAVEL TIME(MIN.) = 0.68  
Tc(MIN.) = 7.16  
SUBAREA AREA(ACRES) = 4.73 SUBAREA RUNOFF(CFS) = 6.64  
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350  
TOTAL AREA(ACRES) = 4.8 PEAK FLOW RATE(CFS) =  
6.78

END OF SUBAREA CHANNEL FLOW HYDRAULICS:

DEPTH(FEET) = 0.25 FLOW VELOCITY(FEET/SEC.) = 11.80

LONGEST FLOWPATH FROM NODE 105.00 TO NODE 108.00 = 660.73 FEET.

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FLOW PROCESS FROM NODE 108.00 TO NODE 108.00 IS CODE = 10

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>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 2 <<<<<

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FLOW PROCESS FROM NODE 111.00 TO NODE 112.00 IS CODE = 21

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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

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NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 88  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 117.00  
UPSTREAM ELEVATION(FEET) = 292.50  
DOWNSTREAM ELEVATION(FEET) = 291.00  
ELEVATION DIFFERENCE(FEET) = 1.50  
URBAN SUBAREA OVERLAND TIME OF FLOW(MIN.) = 10.707  
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN  
THE MAXIMUM OVERLAND FLOW LENGTH = 74.23  
(Reference: Table 3-1B of Hydrology Manual)  
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.412  
SUBAREA RUNOFF(CFS) = 0.13  
TOTAL AREA(ACRES) = 0.11 TOTAL RUNOFF(CFS) = 0.13

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FLOW PROCESS FROM NODE 112.00 TO NODE 113.00 IS CODE = 52

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>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 291.00 DOWNSTREAM(FEET) =  
229.30  
CHANNEL LENGTH THRU SUBAREA(FEET) = 266.00 CHANNEL SLOPE = 0.2320

NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION  
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION  
CHANNEL FLOW THRU SUBAREA(CFS) = 0.13  
FLOW VELOCITY(FEET/SEC) = 4.74 (PER LACFCD/RCFC&WCD HYDROLOGY  
MANUAL)  
TRAVEL TIME(MIN.) = 0.93 Tc(MIN.) = 11.64  
LONGEST FLOWPATH FROM NODE 111.00 TO NODE 113.00 = 383.00  
FEET.

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FLOW PROCESS FROM NODE 113.00 TO NODE 108.00 IS CODE = 51  
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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 229.30 DOWNSTREAM(FEET) =  
206.00  
CHANNEL LENGTH THRU SUBAREA(FEET) = 286.41 CHANNEL SLOPE = 0.0814  
CHANNEL BASE(FEET) = 2.00 "Z" FACTOR = 1.000  
MANNING'S FACTOR = 0.013 MAXIMUM DEPTH(FEET) = 1.00  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.237  
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 88  
TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 0.79  
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY(FEET/SEC.) = 5.36  
AVERAGE FLOW DEPTH(FEET) = 0.07 TRAVEL TIME(MIN.) = 0.89  
Tc(MIN.) = 12.53  
SUBAREA AREA(ACRES) = 1.17 SUBAREA RUNOFF(CFS) = 1.33  
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350  
TOTAL AREA(ACRES) = 1.3 PEAK FLOW RATE(CFS) =  
1.45

END OF SUBAREA CHANNEL FLOW HYDRAULICS:  
DEPTH(FEET) = 0.10 FLOW VELOCITY(FEET/SEC.) = 6.76  
LONGEST FLOWPATH FROM NODE 111.00 TO NODE 108.00 = 669.41  
FEET.

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FLOW PROCESS FROM NODE 108.00 TO NODE 108.00 IS CODE = 11  
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>>>>CONFLUENCE MEMORY BANK # 2 WITH THE MAIN-STREAM MEMORY<<<<<

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\*\* MAIN STREAM CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.45	12.53	3.237	1.28

LONGEST FLOWPATH FROM NODE 111.00 TO NODE 108.00 = 669.41 FEET.

\*\* MEMORY BANK # 2 CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	6.78	7.16	4.009	4.83

LONGEST FLOWPATH FROM NODE 105.00 TO NODE 108.00 = 660.73 FEET.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	7.61	7.16	4.009
2	6.92	12.53	3.237

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 7.61 Tc (MIN.) = 7.16  
TOTAL AREA (ACRES) = 6.1

\*\*\*\*\*  
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FLOW PROCESS FROM NODE 108.00 TO NODE 104.00 IS CODE = 31

-----  
->>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

=====  
ELEVATION DATA: UPSTREAM (FEET) = 200.00 DOWNSTREAM (FEET) = 194.00  
FLOW LENGTH (FEET) = 59.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 12.0 INCH PIPE IS 7.3 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 15.19  
ESTIMATED PIPE DIAMETER (INCH) = 12.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 7.61  
PIPE TRAVEL TIME (MIN.) = 0.06 Tc (MIN.) = 7.22  
LONGEST FLOWPATH FROM NODE 111.00 TO NODE 104.00 = 728.41 FEET.

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FLOW PROCESS FROM NODE 104.00 TO NODE 104.00 IS CODE = 11

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->>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

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\*\* MAIN STREAM CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	7.61	7.22	3.997	6.11

LONGEST FLOWPATH FROM NODE 111.00 TO NODE 104.00 = 728.41 FEET.

\*\* MEMORY BANK # 1 CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.94	7.89	3.872	1.43

LONGEST FLOWPATH FROM NODE 100.00 TO NODE 104.00 = 786.45 FEET.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	9.38	7.22	3.997
2	9.31	7.89	3.872

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 9.38 Tc (MIN.) = 7.22  
TOTAL AREA (ACRES) = 7.5

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FLOW PROCESS FROM NODE 104.00 TO NODE 114.00 IS CODE = 31

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->>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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ELEVATION DATA: UPSTREAM (FEET) = 194.00 DOWNSTREAM (FEET) = 182.00  
FLOW LENGTH (FEET) = 200.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 15.0 INCH PIPE IS 8.4 INCHES  
PIPE-FLOW VELOCITY (FEET/SEC.) = 13.18  
ESTIMATED PIPE DIAMETER (INCH) = 15.00 NUMBER OF PIPES = 1  
PIPE-FLOW (CFS) = 9.38  
PIPE TRAVEL TIME (MIN.) = 0.25 Tc (MIN.) = 7.47  
LONGEST FLOWPATH FROM NODE 100.00 TO NODE 114.00 = 986.45 FEET.

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FLOW PROCESS FROM NODE 114.00 TO NODE 114.00 IS CODE = 10

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>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 3 <<<<<

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FLOW PROCESS FROM NODE 115.00 TO NODE 116.00 IS CODE = 21

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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

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RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 82  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 114.30  
UPSTREAM ELEVATION(FEET) = 227.50  
DOWNSTREAM ELEVATION(FEET) = 226.30  
ELEVATION DIFFERENCE(FEET) = 1.20  
URBAN SUBAREA OVERLAND TIME OF FLOW(MIN.) = 10.279  
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN  
THE MAXIMUM OVERLAND FLOW LENGTH = 70.75  
(Reference: Table 3-1B of Hydrology Manual)  
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.453  
SUBAREA RUNOFF(CFS) = 0.52  
TOTAL AREA(ACRES) = 0.37 TOTAL RUNOFF(CFS) = 0.52

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\*\*\*  
FLOW PROCESS FROM NODE 116.00 TO NODE 119.00 IS CODE = 61

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>>>>COMPUTE STREET FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>(STANDARD CURB SECTION USED)<<<<<

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UPSTREAM ELEVATION(FEET) = 226.30 DOWNSTREAM ELEVATION(FEET) =  
204.00  
STREET LENGTH(FEET) = 416.00 CURB HEIGHT(INCHES) = 6.0  
STREET HALFWIDTH(FEET) = 10.00  
  
DISTANCE FROM CROWN TO CROSSFALL GRADEBREAK(FEET) = 1.50  
INSIDE STREET CROSSFALL(DECIMAL) = 0.020  
OUTSIDE STREET CROSSFALL(DECIMAL) = 0.020  
  
SPECIFIED NUMBER OF HALFSTREETS CARRYING RUNOFF = 2

STREET PARKWAY CROSSFALL(DECIMAL) = 0.020  
Manning's FRICTION FACTOR for Streetflow Section(curbs-to-curbs) =  
0.0130  
Manning's FRICTION FACTOR for Back-of-Walk Flow Section = 0.0200

\*\*TRAVEL TIME COMPUTED USING ESTIMATED FLOW(CFS) = 1.03  
STREETFLOW MODEL RESULTS USING ESTIMATED FLOW:  
STREET FLOW DEPTH(FEET) = 0.16  
HALFSTREET FLOOD WIDTH(FEET) = 1.50  
AVERAGE FLOW VELOCITY(FEET/SEC.) = 5.04  
PRODUCT OF DEPTH&VELOCITY(FT\*FT/SEC.) = 0.79  
STREET FLOW TRAVEL TIME(MIN.) = 1.38 Tc(MIN.) = 11.65  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.321  
STREETS & ROADS (CURBS/STORM DRAINS) RUNOFF COEFFICIENT = .8700  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 98  
AREA-AVERAGE RUNOFF COEFFICIENT = 0.634  
SUBAREA AREA(ACRES) = 0.35 SUBAREA RUNOFF(CFS) = 1.01  
TOTAL AREA(ACRES) = 0.7 PEAK FLOW RATE(CFS) = 1.52

END OF SUBAREA STREET FLOW HYDRAULICS:  
DEPTH(FEET) = 0.17 HALFSTREET FLOOD WIDTH(FEET) = 2.29  
FLOW VELOCITY(FEET/SEC.) = 4.44 DEPTH\*VELOCITY(FT\*FT/SEC.) = 0.76  
LONGEST FLOWPATH FROM NODE 115.00 TO NODE 119.00 = 530.30  
FEET.

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FLOW PROCESS FROM NODE 118.00 TO NODE 119.00 IS CODE = 81  
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<

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100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.321  
STREETS & ROADS (CURBS/STORM DRAINS) RUNOFF COEFFICIENT = .8700  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 98  
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6809  
SUBAREA AREA(ACRES) = 0.18 SUBAREA RUNOFF(CFS) = 0.52  
TOTAL AREA(ACRES) = 0.9 TOTAL RUNOFF(CFS) = 2.04  
TC(MIN.) = 11.65

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FLOW PROCESS FROM NODE 119.00 TO NODE 120.00 IS CODE = 31  
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<

>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<



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ELEVATION DATA: UPSTREAM(FEET) = 191.50 DOWNSTREAM(FEET) = 190.00
FLOW LENGTH(FEET) = 5.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 6.0 INCH PIPE IS 3.6 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 16.38
ESTIMATED PIPE DIAMETER(INCH) = 6.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.04
PIPE TRAVEL TIME(MIN.) = 0.01 Tc(MIN.) = 11.66
LONGEST FLOWPATH FROM NODE 115.00 TO NODE 120.00 = 535.30
FEET.
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FLOW PROCESS FROM NODE 117.00 TO NODE 120.00 IS CODE = 81
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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<
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100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.321
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500
SOIL CLASSIFICATION IS "D"
S.C.S. CURVE NUMBER (AMC II) = 88
AREA-AVERAGE RUNOFF COEFFICIENT = 0.6112
SUBAREA AREA(ACRES) = 0.24 SUBAREA RUNOFF(CFS) = 0.28
TOTAL AREA(ACRES) = 1.1 TOTAL RUNOFF(CFS) = 2.31
TC(MIN.) = 11.66
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FLOW PROCESS FROM NODE 121.00 TO NODE 114.00 IS CODE = 31
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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<
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ELEVATION DATA: UPSTREAM(FEET) = 186.00 DOWNSTREAM(FEET) = 182.00
FLOW LENGTH(FEET) = 30.00 MANNING'S N = 0.013
DEPTH OF FLOW IN 9.0 INCH PIPE IS 3.9 INCHES
PIPE-FLOW VELOCITY(FEET/SEC.) = 12.52
ESTIMATED PIPE DIAMETER(INCH) = 9.00 NUMBER OF PIPES = 1
PIPE-FLOW(CFS) = 2.31
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 11.70
LONGEST FLOWPATH FROM NODE 115.00 TO NODE 114.00 = 565.30
FEET.
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FLOW PROCESS FROM NODE 114.00 TO NODE 114.00 IS CODE = 11

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>>>>CONFLUENCE MEMORY BANK # 3 WITH THE MAIN-STREAM MEMORY<<<<<

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\*\* MAIN STREAM CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	2.31	11.70	3.317	1.14

LONGEST FLOWPATH FROM NODE 115.00 TO NODE 114.00 = 565.30 FEET.

\*\* MEMORY BANK # 3 CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	9.38	7.47	3.950	7.54

LONGEST FLOWPATH FROM NODE 100.00 TO NODE 114.00 = 986.45 FEET.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	10.86	7.47	3.950
2	10.19	11.70	3.317

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:

PEAK FLOW RATE (CFS) = 10.86 Tc (MIN.) = 7.47  
TOTAL AREA (ACRES) = 8.7

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END OF STUDY SUMMARY:

TOTAL AREA (ACRES) = 8.7 TC (MIN.) = 7.47  
PEAK FLOW RATE (CFS) = 10.86

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END OF RATIONAL METHOD ANALYSIS



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RATIONAL METHOD HYDROLOGY COMPUTER PROGRAM PACKAGE  
Reference: SAN DIEGO COUNTY FLOOD CONTROL DISTRICT  
2003,1985,1981 HYDROLOGY MANUAL  
(c) Copyright 1982-2014 Advanced Engineering Software (aes)  
Ver. 21.0 Release Date: 06/01/2014 License ID 1667

Analysis prepared by:

ARC CONSTRUCTION & ENGINEERING INC.  
10948 ELDERWOOD LANE  
SAN DIEGO CA 92131

\*\*\*\*\* DESCRIPTION OF STUDY  
\*\*\*\*\*

\* 100 YEAR EVENT  
\*  
\* LOT 31 RANCHO DEL SOL  
\*  
\* DATE:12/20/20  
\*

\*\*\*\*\*  
\*

FILE NAME: LOT31.DAT  
TIME/DATE OF STUDY: 10:56 12/28/2010

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---  
USER SPECIFIED HYDROLOGY AND HYDRAULIC MODEL INFORMATION:  
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USER SPECIFIED STORM EVENT(YEAR) = 100.00  
SPECIFIED MINIMUM PIPE SIZE(INCH) = 6.00  
SPECIFIED PERCENT OF GRADIENTS(DECIMAL) TO USE FOR FRICTION SLOPE =  
0.95

RAINFALL-INTENSITY ADJUSTMENT FACTOR = 1.000

\*USER SPECIFIED:

NUMBER OF [TIME,INTENSITY] DATA PAIRS = 9

1) 5.000; 4.410  
2) 10.000; 3.480  
3) 15.000; 3.000  
4) 20.000; 2.530  
5) 25.000; 2.400  
6) 30.000; 2.000  
7) 40.000; 1.650  
8) 50.000; 1.500

9) 60.000; 1.400  
 SAN DIEGO HYDROLOGY MANUAL "C"-VALUES USED FOR RATIONAL METHOD  
 NOTE: ONLY PEAK CONFLUENCE VALUES CONSIDERED

\*USER-DEFINED STREET-SECTIONS FOR COUPLED PIPEFLOW AND STREETFLOW MODEL\*

MANNING		HALF- CROWN TO STREET-CROSSFALL:			CURB GUTTER-GEOMETRIES:			
NO.	WIDTH	CROSSFALL	IN- /	OUT-/PARK-	HEIGHT	WIDTH	LIP	HIKE
(n)	(FT)	(FT)	SIDE /	SIDE/	(FT)	(FT)	(FT)	(FT)
			WAY	WAY				
1	30.0	20.0	0.018/0.018/0.020		0.67	2.00	0.0312	0.167
	0.0150							

GLOBAL STREET FLOW-DEPTH CONSTRAINTS:

1. Relative Flow-Depth = 0.00 FEET  
 as (Maximum Allowable Street Flow Depth) - (Top-of-Curb)
2. (Depth)\*(Velocity) Constraint = 6.0 (FT\*FT/S)

\*SIZE PIPE WITH A FLOW CAPACITY GREATER THAN OR EQUAL TO THE UPSTREAM TRIBUTARY PIPE.\*

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FLOW PROCESS FROM NODE 200.00 TO NODE 201.00 IS CODE = 21

>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<

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NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
 SOIL CLASSIFICATION IS "D"  
 S.C.S. CURVE NUMBER (AMC II) = 88  
 INITIAL SUBAREA FLOW-LENGTH(FEET) = 72.20  
 UPSTREAM ELEVATION(FEET) = 291.00  
 DOWNSTREAM ELEVATION(FEET) = 285.00  
 ELEVATION DIFFERENCE(FEET) = 6.00  
 URBAN SUBAREA OVERLAND TIME OF FLOW(MIN.) = 5.664  
 100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 4.287  
 SUBAREA RUNOFF(CFS) = 0.15  
 TOTAL AREA(ACRES) = 0.10 TOTAL RUNOFF(CFS) = 0.15

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FLOW PROCESS FROM NODE 201.00 TO NODE 202.00 IS CODE = 52

>>>>COMPUTE NATURAL VALLEY CHANNEL FLOW<<<<<

>>>>TRAVELTIME THRU SUBAREA<<<<<

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ELEVATION DATA: UPSTREAM (FEET) = 285.00 DOWNSTREAM (FEET) = 229.30  
CHANNEL LENGTH THRU SUBAREA (FEET) = 231.50 CHANNEL SLOPE = 0.2406  
NOTE: CHANNEL FLOW OF 1. CFS WAS ASSUMED IN VELOCITY ESTIMATION  
NOTE: CHANNEL SLOPE OF .1 WAS ASSUMED IN VELOCITY ESTIMATION  
CHANNEL FLOW THRU SUBAREA (CFS) = 0.15  
FLOW VELOCITY (FEET/SEC) = 4.74 (PER LACFCD/RCFC&WCD HYDROLOGY MANUAL)  
TRAVEL TIME (MIN.) = 0.81 Tc (MIN.) = 6.48  
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 202.00 = 303.70 FEET.

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FLOW PROCESS FROM NODE 202.00 TO NODE 203.00 IS CODE = 51

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>>>>COMPUTE TRAPEZOIDAL CHANNEL FLOW<<<<<  
>>>>TRAVELTIME THRU SUBAREA (EXISTING ELEMENT)<<<<<

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ELEVATION DATA: UPSTREAM (FEET) = 229.30 DOWNSTREAM (FEET) = 212.00  
CHANNEL LENGTH THRU SUBAREA (FEET) = 243.00 CHANNEL SLOPE = 0.0712  
CHANNEL BASE (FEET) = 2.00 "Z" FACTOR = 1.000  
MANNING'S FACTOR = 0.013 MAXIMUM DEPTH (FEET) = 1.00  
100 YEAR RAINFALL INTENSITY (INCH/HOUR) = 3.997  
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 88  
TRAVEL TIME COMPUTED USING ESTIMATED FLOW (CFS) = 0.83  
TRAVEL TIME THRU SUBAREA BASED ON VELOCITY (FEET/SEC.) = 5.44  
AVERAGE FLOW DEPTH (FEET) = 0.07 TRAVEL TIME (MIN.) = 0.74  
Tc (MIN.) = 7.22  
SUBAREA AREA (ACRES) = 0.97 SUBAREA RUNOFF (CFS) = 1.36  
AREA-AVERAGE RUNOFF COEFFICIENT = 0.350  
TOTAL AREA (ACRES) = 1.1 PEAK FLOW RATE (CFS) = 1.50

END OF SUBAREA CHANNEL FLOW HYDRAULICS:  
DEPTH (FEET) = 0.11 FLOW VELOCITY (FEET/SEC.) = 6.47  
LONGEST FLOWPATH FROM NODE 200.00 TO NODE 203.00 = 546.70 FEET.

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FLOW PROCESS FROM NODE 203.00 TO NODE 203.00 IS CODE = 10

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>>>>MAIN-STREAM MEMORY COPIED ONTO MEMORY BANK # 1 <<<<<<

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FLOW PROCESS FROM NODE 204.00 TO NODE 205.00 IS CODE = 21

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>>>>RATIONAL METHOD INITIAL SUBAREA ANALYSIS<<<<<<

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RESIDENTIAL (1. DU/AC OR LESS) RUNOFF COEFFICIENT = .4100  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 82  
INITIAL SUBAREA FLOW-LENGTH(FEET) = 114.45  
UPSTREAM ELEVATION(FEET) = 225.00  
DOWNSTREAM ELEVATION(FEET) = 224.50  
ELEVATION DIFFERENCE(FEET) = 0.50  
URBAN SUBAREA OVERLAND TIME OF FLOW(MIN.) = 11.065  
WARNING: INITIAL SUBAREA FLOW PATH LENGTH IS GREATER THAN  
THE MAXIMUM OVERLAND FLOW LENGTH = 50.00  
(Reference: Table 3-1B of Hydrology Manual)  
THE MAXIMUM OVERLAND FLOW LENGTH IS USED IN Tc CALCULATION!  
100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.378  
SUBAREA RUNOFF(CFS) = 0.26  
TOTAL AREA(ACRES) = 0.19 TOTAL RUNOFF(CFS) = 0.26

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FLOW PROCESS FROM NODE 205.00 TO NODE 206.00 IS CODE = 31

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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<<  
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW) <<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 223.37 DOWNSTREAM(FEET) = 219.30  
FLOW LENGTH(FEET) = 18.00 MANNING'S N = 0.013  
ESTIMATED PIPE DIAMETER(INCH) INCREASED TO 6.000  
DEPTH OF FLOW IN 6.0 INCH PIPE IS 1.3 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 8.49  
ESTIMATED PIPE DIAMETER(INCH) = 6.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 0.26  
PIPE TRAVEL TIME(MIN.) = 0.04 Tc(MIN.) = 11.10  
LONGEST FLOWPATH FROM NODE 204.00 TO NODE 206.00 = 132.45  
FEET.

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FLOW PROCESS FROM NODE 207.00 TO NODE 203.00 IS CODE = 31

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>>>>COMPUTE PIPE-FLOW TRAVEL TIME THRU SUBAREA<<<<<  
>>>>USING COMPUTER-ESTIMATED PIPESIZE (NON-PRESSURE FLOW)<<<<<

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ELEVATION DATA: UPSTREAM(FEET) = 213.75 DOWNSTREAM(FEET) = 213.00  
FLOW LENGTH(FEET) = 30.00 MANNING'S N = 0.013  
DEPTH OF FLOW IN 6.0 INCH PIPE IS 2.3 INCHES  
PIPE-FLOW VELOCITY(FEET/SEC.) = 3.85  
ESTIMATED PIPE DIAMETER(INCH) = 6.00 NUMBER OF PIPES = 1  
PIPE-FLOW(CFS) = 0.26  
PIPE TRAVEL TIME(MIN.) = 0.13 Tc(MIN.) = 11.23  
LONGEST FLOWPATH FROM NODE 204.00 TO NODE 203.00 = 162.45  
FEET.

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FLOW PROCESS FROM NODE 208.00 TO NODE 203.00 IS CODE = 81

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>>>>ADDITION OF SUBAREA TO MAINLINE PEAK FLOW<<<<<

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100 YEAR RAINFALL INTENSITY(INCH/HOUR) = 3.362  
NATURAL DESERT LANDSCAPING RUNOFF COEFFICIENT = .3500  
SOIL CLASSIFICATION IS "D"  
S.C.S. CURVE NUMBER (AMC II) = 88  
AREA-AVERAGE RUNOFF COEFFICIENT = 0.3808  
SUBAREA AREA(ACRES) = 0.18 SUBAREA RUNOFF(CFS) = 0.21  
TOTAL AREA(ACRES) = 0.4 TOTAL RUNOFF(CFS) = 0.47  
TC(MIN.) = 11.23

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FLOW PROCESS FROM NODE 203.00 TO NODE 203.00 IS CODE = 11

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>>>>CONFLUENCE MEMORY BANK # 1 WITH THE MAIN-STREAM MEMORY<<<<<

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\*\* MAIN STREAM CONFLUENCE DATA \*\*

STREAM	RUNOFF	Tc	INTENSITY	AREA
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NUMBER	(CFS)	(MIN.)	(INCH/HOUR)	(ACRE)
1	0.47	11.23	3.362	0.37
LONGEST FLOWPATH FROM NODE			204.00 TO NODE	203.00 = 162.45

FEET.

\*\* MEMORY BANK # 1 CONFLUENCE DATA \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)	AREA (ACRE)
1	1.50	7.22	3.997	1.07
LONGEST FLOWPATH FROM NODE			200.00 TO NODE	203.00 = 546.70

FEET.

\*\* PEAK FLOW RATE TABLE \*\*

STREAM NUMBER	RUNOFF (CFS)	Tc (MIN.)	INTENSITY (INCH/HOUR)
1	1.80	7.22	3.997
2	1.73	11.23	3.362

COMPUTED CONFLUENCE ESTIMATES ARE AS FOLLOWS:  
 PEAK FLOW RATE (CFS) = 1.80 Tc (MIN.) = 7.22  
 TOTAL AREA (ACRES) = 1.4

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 END OF STUDY SUMMARY:  
 TOTAL AREA (ACRES) = 1.4 TC (MIN.) = 7.22  
 PEAK FLOW RATE (CFS) = 1.80

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 END OF RATIONAL METHOD ANALYSIS