

APPENDIX D1-D5

Geotechnical Reports

APPENDIX D1

60% DESIGN DEVELOPMENT REPORT

GEOTECHNICAL REPORT PUMP STATION AND CUT & COVER SECTIONS MORENA PUMP STATION, WW FORCE MAIN, AND BRINE/CENTRATE CONVEYANCE PREDESIGN (NC01)

SAN DIEGO, CALIFORNIA

Prepared for

City of San Diego
Public Utilities Department
San Diego, California

September 19, 2017
AECOM Project No. 60530732

Prepared by

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September 19, 2017

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Subject: 60% Design Development Report
Geotechnical Report
Pump Station and Cut & Cover Sections
Morena Pump Station, WW Force Main, and
Brine/Centrifuge Conveyance Predesign (NC01)
San Diego, California
AECOM Project No. 60530732

Dear Ms. Nasrawi:

AECOM Technical Services Inc. (AECOM) has prepared this Geotechnical Report to support the 60 percent Design Development for Morena Pump Station and the cut and cover portions of the pipeline for the above referenced project. This report provides the findings from previous and recent subsurface explorations, a discussion of geologic and geotechnical conditions, and conclusions and recommendations pertaining to the geotechnical aspects of design and construction.

The purpose of this report is to provide geologic and geotechnical information for 60% design of the Morena Pump Station, connections with the existing sewer main, and the cut and cover pipeline sections. Results of field subsurface explorations completed to date are described in this report. The report will be supplemented based on additional explorations planned within the pump station property limits once the property is available for ongoing design. Geotechnical and geologic conclusions and recommendations related to fault hazards and the specific tunneled portions of the pipeline alignment are presented under separate cover.

We appreciate the opportunity to work with you on this project. If you have any questions, please contact us.

Sincerely,

AECOM Technical Services, Inc.



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LIST OF ACRONYMS AND ABBREVIATIONS

AECOM	AECOM Technical Services, Inc.
ASCE	American Society of Civil Engineers
ASTM	ASTM International, formerly American Society for Testing and Materials
bgs	below ground surface
BMPs	Best Management Practices
CBC	California Building Code
CDSM	Cement Deep Soil Mixing
CGS	California Geological Survey
CPTs	Cone Penetration Tests
CRR	cyclic resistance ratio
CSM	Cutter Soil Mixing
CSR	cyclic stress ratio
EI	Expansion Index
EPA	Environmental Protection Agency
EQFZ	Earthquake Fault Zone
FS	Factor of Safety
ft	feet
g	Gravitational Acceleration
IBC	International Building Code
MCE	Maximum Considered Earthquake
mgd	million gallons per day
MPS	Morena Pump Station
MWWD	Metropolitan Waste Water Department
NCWPF	North City Water Purification Facility
NCWRP	North City Water Reclamation Plant
NGVD 29	National Geodetic Mean Datum of 1929
NPDES	National Pollutant Discharge Elimination System
OCPs	organochlorine pesticides
PCBs	polychlorinated biphenyls
pcf	pounds per cubic foot
PGA	Peak Ground Acceleration
pci	pounds per cubic inch
PLWTP	Point Loma Wastewater Treatment Plant
psf	pounds per square foot
PVC	polyvinyl chloride
RCFZ	Rose Canyon Fault Zone
SAM	Site Assessment and Mitigation
SPT	Standard Penetration Test
SVOC	semi-volatile organic compound
TPH	total petroleum hydrocarbons
URS	URS Corporation
USA	Underground Service Alert
USGS	United States Geological Survey
VOCs	volatile organic compounds
WW	Wastewater

1.0 INTRODUCTION

This geotechnical report prepared by AECOM Technical Services, Inc. (AECOM) supports 60% Design Development for the Morena Pump Station (MPS), Wastewater (WW) Force Main, and Brine/Centrates Conveyance Predesign (NC01) Project (“Project”) for Pure Water San Diego. Specifically, the report provides a summary of geotechnical conditions and design considerations for the Morena Pump Station, connections to the existing sewer main (including tunnels) via diversion structure pipelines, and the cut-and-cover (open trench) portions of the pipeline alignments. Geotechnical recommendations for the design and construction of the project are provided. The planned locations of the pump station and pipelines are shown on Figure 1.

This report does not address the fault hazards associated with the project, or the tunnel crossings at Interstate-805 (I-805), San Clemente Canyon, and Rose Creek/North County Transit District (NCTD) Railroad Crossing that are proposed for portions of the pipeline alignments. The field explorations for the tunnel crossings and fault investigation are discussed below as they are related to the other portions of the alignment as well; however, these topics are addressed in greater detail under separate cover (Faulting Report and Tunnel Report).

1.1 PROJECT DESCRIPTION

The planned Project will convey an average of 37.7 million gallons per day (mgd) of raw wastewater to the North City Water Reclamation Plant (NCWRP). Wastewater will be conveyed from MPS via a new 48-inch-diameter force main approximately 10.7 miles north to NCWRP. A new 30-inch-diameter brine/centrate pipeline will convey 13.5 mgd of brine/centrate (generated from the North City Water Purification Facility [NCWPF]) south, by gravity flow (by way of MPS) for treatment at the Point Loma Wastewater Treatment Plant (PLWTP). The brine/centrate line will run parallel to the force main in a joint trench for approximately 95 percent of the overall alignment. A detailed description of the Project is provided in the Draft Final Design Development Basis of Design Report. A summary of the planned foundations and below-grade features is provided below as it relates to the geotechnical considerations discussed in this report.

The layout of the primary components of the Morena Pump Station is shown on Figure 2. The planned finished grade for the Morena Pump Station is approximately Elevation 16 feet National Geodetic Vertical Datum of 1929 (NGVD 29). The Intake Screening Building has a planned bottom of foundation elevation of -18.25 feet, which will require an approximate excavation depth of 34 feet. The Pump Station Building will require an excavation depth of about 51 feet to accommodate the foundation (bottom elevation of -32.5 feet NGVD 29) and underlying base layer. The Energy Dissipator Structure has a planned bottom of foundation elevation of -7.33 feet NGVD 29, which will require an excavation of about 25 feet deep. Other supporting facilities, including the Electrical Building, Maintenance Building, transformers, Odor Control structures and High Purity Oxygen Injection Skid will be constructed at or near finished grade.

The general alignment of the wastewater and brine/centrate pipelines is shown on Figure 1. Shorter stretches of cut-and-cover pipeline and jack and bore installation will be constructed extending south of the planned pump station site to connect with existing infrastructure. The planned pipe invert depths for the cut-and-cover portions of the pipelines generally range from 10 to 30 feet below grade.

1.2 SUBSURFACE INFORMATION

The conclusions and recommendations presented in this report are based on previous data performed by others and additional subsurface data collected by AECOM as part of the ongoing geotechnical investigation.

Previous studies include the “Desktop Geotechnical Study Report, Pure Water Program Task 7, Morena Pump Station, WW Force Main and Brine Conveyance Pre-Design, City of San Diego” prepared by Allied Geotechnical Engineers (Allied, 2015), as well as other geotechnical reports available in AECOM’s files. The references reviewed are listed in Section 7 of this report. The geotechnical evaluation also included a review of published geologic maps, and City and County of San Diego hazard maps.

1.2.1 Current Subsurface Explorations

The field explorations for this 60% Design phase of the project were conducted between March and August 2017. Field explorations and geologic reconnaissance were performed for the tunnel crossings, the pump station, the diversion structure, and the trench portions of the alignment.

1.2.1.1 Summary

AECOM prepared a Safe Work Plan for our work prior to commencing our field investigation. Underground Service Alert was notified prior to advancing all the borings.

Logs of the borings are provided in Appendix A, which also provides details of the field exploration program. A summary of the borings is provided as Table A-1 in Appendix A. The borings were numbered based on spacing along the project alignment. Some of the planned boring locations were eliminated based on nearby available information or due to utility conflicts, and therefore the numbering of the borings is not sequential.

1.2.1.2 Pump Station

The initial explorations were focused at the Morena Pump Station to investigate the general subsurface and ground conditions, as well as the potential for fault hazards. Three soil borings PS-1 through PS-3 were advanced around the perimeter of the MPS. Boring PS-2 was converted to a monitoring well. Boring logs are attached to this report. Cone Penetration Tests (CPTs) were also performed around the perimeter of the pump station site; CPT logs are provided in the Faulting Report. Approximate locations of the borings and CPTs adjacent to the MPS are shown on Figure 2. Due to access restrictions, recent explorations for the pump station were performed in the streets around the perimeter of the site. Additional explorations are planned within the pump station property once access to the site is available.

The borings were extended to depths ranging between 61.5 and 81 feet. Drilling was performed with a truck-mounted drill rig equipped to drill with hollow-stem augers methods. The test borings were drilled, logged, sampled and backfilled under supervision of an AECOM engineering geologist. The borings were located in the City right-of-way in accordance with City Traffic Control Permits. The borings were backfilled in accordance with County of San Diego

Department of Environmental Health requirements, and the surfaces were restored with concrete to match existing conditions.

1.2.1.3 Tunnel Borings

Borings were advanced for the four originally planned tunnel crossings, including the I-805 tunnel crossing, the Rose Creek/NCTD tunnel crossing, the San Clemente Canyon tunnel crossing, and the Tecolote Creek tunnel crossing. The Tecolote Creek tunnel crossing has been modified to be a conventional cut and cover (open trench) construction crossing. The field investigation for the tunnels consisted of advancing eight exploratory borings (EX-1; RC-1, RC-2, RC-3 and RC-4; and SC-1, SC-2 and SC-3) and collecting soil/core samples for more detailed review and laboratory testing. The field investigation for the tunnel crossings is described in more detail in our tunnel report.

The borings for the Tecolote Creek crossing (TC-1 and TC-2) were drilled to depths 96.5 feet. Drilling was performed with a truck-mounted drill rig equipped to drill with hollow-stem augers and mud rotary drilling methods. Hollow-stem auger methods were used until groundwater was encountered. Mud rotary drilling methods were used below the groundwater. Test borings were drilled, logged, sampled and backfilled under supervision of an AECOM engineering geologist.

These borings were located in the City right-of-way in accordance with City Traffic Control Permits. Monitoring wells were constructed and borings were backfilled in accordance with County of San Diego Department of Environmental Health requirements. Disturbed surfaces were restored to match existing conditions.

1.2.1.4 Cut-and-Cover (Open Trench)

AECOM advanced 67 borings along the Morena Pipeline alignment where conventional cut-and-cover, open trenching methods are planned for construction of the force main, the water main, and the smaller water lines. The borings were extended to depths up to 19.5 or 20 feet. Drilling was performed with truck-mounted drill rigs equipped to drill with hollow-stem auger methods. The test borings were drilled, logged, sampled and backfilled under supervision of an AECOM engineering geologist or engineer. The borings were located in the City right-of-way in accordance with City Traffic Control Permits. The borings were backfilled with a combination of soil cuttings and hydrated bentonite. The surface was capped with concrete.

1.2.1.5 Diversion Structures

Four borings were advanced along Friars Road, south of the Morena Pump Station location, for the diversion structure tie-ins. The borings were extended to depths ranging between 20 and 61.5 feet. Drilling was performed with a truck-mounted drill rig equipped to drill with hollow-stem augers methods. The test borings were drilled, logged, sampled and backfilled under supervision of an AECOM engineer. The borings were located in the City right-of-way in accordance with City Traffic Control Permits. The borings were backfilled in accordance with County of San Diego Department of Environmental Health requirements, and the surfaces were restored with concrete to match existing conditions.

1.2.2 Laboratory Testing

Geotechnical laboratory tests were performed on selected samples to aid in estimating soil properties and validate visual classifications of the materials. The tests include grain size, plasticity characteristics, strength and corrosivity. Test results are presented in Appendix B and are summarized at the corresponding sample location on the logs of the borings in Appendix A.

1.2.3 Groundwater Sampling

Groundwater sampling was performed three days after the installation of the monitoring well to evaluate the groundwater for construction dewatering and groundwater disposal. Analytical laboratory testing was performed on the selected groundwater sample for total petroleum hydrocarbons (TPH) extended range by EPA Modified Method 8015B, title 22 metals including mercury by EPA Methods 6010B and 7470A, organochlorine pesticides (OCPs) by EPA Method 8081A, polychlorinated biphenyls (PCBs) by EPA Method 8082, semi-volatile organic compounds (SVOCs) by EPA Method 8270C, and volatile organic compounds (VOCs) by EPA Method 8260B. The results of the analytical testing are presented in Appendix C.

Additional sampling and testing will be performed as the geotechnical investigation for the Project progresses and additional monitoring wells are installed within the pump station property.

1.2.4 Previous Ground Improvement

We understand that ground improvement was performed for other projects in the area south of the MPS. Stone columns were installed across the San Diego River Valley for the North Metro Interceptor Sewer to improve the ground. Stone columns may be encountered for the diversion tie-in structures. In addition, we understand that grouting may have been performed for the Mid-Coast Trolley project along Friars Road. Grouting may be encountered in the subsurface near the Diversion Structures for the Project. Records of these ground improvement projects are not available but these ground improvement projects are suspected in the area.

2.0 SITE CONDITIONS

This section of the report summarizes the site and subsurface conditions at the Morena Pump Station and along the pipeline alignments. Additional detail on geologic and seismic setting is provided in the Fault Hazard Report and the Tunnel Report

2.1 GEOLOGIC AND SEISMIC SETTING

The project physiographic/geologic setting includes the 1) former floodplain of the San Diego River and 2) upland mesas with canyons.

Historically, the San Diego River deposited thick alluvium over a broad floodplain delta extending from Old Town north along the easterly margins of Mission Bay. Land development over the years, including freeways and railroads, required raising and leveling the former floodplain surface with fill soil, locally more than 10 feet deep. Along most of the southerly reach of the planned pipeline (between Ingulf Street and Friars Road) the combined thickness of fill and alluvium is greater than pipeline trench depths, except for short reaches within the Bay Point Formation, a soft Pleistocene age sedimentary formation consisting mostly of dense sand, hard clay and gravel.

The Morena Pump Station and the southern portion of the pipeline alignment are considered to lie within the Rose Canyon Fault Zone (RCFZ). The on-shore portion of the RCFZ extends along the northeast flank of Mount Soledad at La Jolla and continues southward along the eastern margins of Mission Bay (just west of Interstate 5) towards downtown San Diego. Between Mission Bay and San Diego Bay, the zone appears to widen and diverge. Active faults (i.e. Holocene-age fault rupture) and potentially active faults (i.e. Quaternary-age fault rupture) are present in the Project area.

The RCFZ has been known for years to have the potential to produce a large damaging earthquake (M7 or greater). The pipeline and pump station will likely experience strong seismic shaking, and possibly fault rupture during their design life. The saturated alluvium below the groundwater table is potentially liquefiable under strong seismic shaking from a local or distant earthquake.

2.2 GEOLOGIC UNITS

The geologic units present across the Project alignment include fill soil, recent alluvium, Quaternary older alluvium/Bay Point Formation, and older formational materials. These units are described below. The surface geology along the planned pipeline route is shown on Figures 3a through 3e.

2.2.1 Fill

Fill soils are present at the ground surface at many locations across the Project, due to the construction of both public infrastructure and private development. Most of the Morena/West Morena Boulevard reach is underlain by varying thicknesses of fill. The fill has been placed in conjunction with land-filling along former low-lying areas, road grading and underground utility construction. Fill soils tend to be erratic mixtures of sand, clay, gravel and sometimes

construction debris. The fill contains a wide range of particle sizes, up to boulder sized. The fill along the alignment is considered undocumented, i.e. compaction records are not available. The fill may have been hydraulically placed at the southern end of the alignment and in the vicinity of the pump station.

2.2.2 Alluvium

Alluvial deposits, predominantly loose to dense silty sands, clean sands and sandy gravels underlie the former floodplain areas and the inland canyon-creek crossings. The Morena/West Morena Boulevard pipeline reach is underlain by alluvium at greater depth than the pipeline. Between the Morena Pump Station and Ingulf Street, the composition of the alluvium varies considerably, with more fine-grained silts and some clays present within the alluvium near the pump station. In the vicinity of Tecolote Creek, the alluvium was characterized as loose to medium dense sand and stiff clay, although in nearby previous borings, young estuarine deposits (primarily silts and clays with some sands and organic deposits) were logged above the alluvium. Toward the northern portion of the Morena Boulevard stretch, the material below the fill may be more colluvial in nature due to its proximity to the hills to the east.

The inland natural canyons at San Clemente Creek and Rose Creek are mapped as underlain by alluvium (Kennedy, 1975). Recent borings suggest alluvium is relatively thin, less than about 15 feet thick.

2.2.3 Older Deposits and Formational Materials

The other materials present along the Project alignment, arranged from younger to older deposits, are briefly described below:

Old Paralic Deposits (formerly called Bay Point Formation) – late to middle Pleistocene aged, marine and non-marine poorly consolidated sandstone (medium dense to very dense sand, silty sand and clayey sand, some localized zones of gravel and cobbles).

Very Old Paralic Deposits (formerly called Lindavista Formation) – middle to early Pleistocene, interfingering strandline, beach, estuarine and colluvial deposits (siltstone, sandstone, conglomerate), can have strong cementation, cobbles.

Stadium Conglomerate – Eocene aged, cobble conglomerate in silty sand matrix with some sandstone, strongly cemented.

Friars Formation – middle to late Eocene aged, marine and non-marine sandstone, siltstone and claystone. Claystone portions can be highly expansive and prone to landslide hazards.

Scripps Formation – Eocene aged, silty sandstone and sandy siltstone with occasional cobble conglomerate beds, strong cementation/concretions.

Ardath Shale – lower to middle Eocene aged, sandy siltstone and claystone with local concreted zones, claystone portions are potentially expansive and prone to landslide hazards.

Old Paralac Deposits, Very Old Paralac Deposits, and Scripps Formation were encountered in borings performed for the project.

2.3 SUBSURFACE CONDITIONS – MORENA PUMP STATION

The preliminary available data indicates that within the depth of the explorations (maximum depth about 80 feet below ground surface [bgs]), the site is underlain by a thin fill layer over alluvium. The fill ranges from about 3 to 5 feet in depth and consists primarily of silty sand. The underlying alluvium varies significantly, and is highly interlayered in some locations.

Within the upper portion of the alluvium to depths ranging from about 19 to 29 feet, the soil is mostly poorly graded sand to silty sand that is primarily loose with some zones of very loose and medium dense material. At some exploration locations, significant interbeds of low-plasticity silt were present within this upper zone of the alluvium.

The upper sandy zone is generally underlain by a fine-grained zone that extends to a depth of about 50 feet bgs. It consists primarily of silt, with interbeds of clay, as well as silty sand. The consistency ranges from soft to stiff.

A deeper granular zone of sand to silty sand is present below a depth of about 50 feet. It is generally medium dense to dense, with some looser zones. There are some zones of fine-grained soil within the deeper granular layer, and in many locations, there is a bed approximately 5 feet thick of stiff silt at depths that range from 55 to 65 feet bgs. Boring PS-3 encountered gravel at a depth of 76 feet bgs. Some of the CPTs reached practical refusal between depths of about 60 to 80 feet, possibly in gravel layers or denser portions of the lower sand layer.

2.4 SUBSURFACE CONDITIONS – PIPELINE ALIGNMENT

Along most of the southerly pipeline reach (Morena/West Morena Boulevard between Friars Road and Ingulf Street), planned trench depths are expected to be within fill over alluvium and/or estuarine deposits, except for short reaches within the Bay Point Formation. In general, fill thicknesses range from about 5 to 10 feet and are generally silty and clayey sands with some gravel. Alluvium (and colluvium, within the northern portion of the stretch) is generally very loose to dense sands with some silts and clays, and potentially some cobbles and boulders. Estuarine-type deposits consisting mostly of sands, clays and some very soft organic soil are present below the fill along West Morena/Morena Boulevard between Dorcas Street on the south and Savannah Street on the north. Bay Point Formation has been logged as shallow as 2 feet bgs along the northern portion of the Morena Boulevard reach, and as shallow as about 2 feet bgs along Ingulf Street as ground elevations rise.

From where the pipeline route ascends up to the coastal mesa at Clairemont Drive toward the NCWRP, the route is mostly within dense sedimentary formations including the Lindavista Formation and the Scripps Formation. Trenches may be classified as CalTrans Class A soils for trench stability (to be verified by the contractor's geotechnical engineer), although these

formations are known to contain cemented zones and cobble intervals that can pose difficult excavation conditions for trenching. Some canyon fill will be encountered along discreet sections of the alignment.

Subsurface conditions along the cut-and-cover portions of the pipeline alignment are summarized in Table 1 and presented in Appendix D.

2.5 GROUNDWATER

Within the mesas, groundwater is assumed to be deeper than 100 feet bgs, with shallow perched water present at varying depths along the alignment. Throughout the portion of the alignment along the coastal plain, groundwater has been measured at depths ranging from 10 to 20 feet bgs. Groundwater was measured at depths of about 9 to 10 feet bgs in the recent pump station borings during drilling, and the initial stabilized reading in monitoring well PS-2 measured groundwater at a depth of about 8 feet bgs (about Elevation 5 to 6 feet NGVD 29). Groundwater was measured at a depth of 15 feet near Tecolote Creek. Groundwater can be assumed to be as shallow as 2 feet within the active stream channel. Groundwater depths will vary with rainfall and seasonal influences.

The groundwater table is mostly above anticipated trench depths along the southern coastal portion of the pipeline alignment and will require dewatering for construction. Groundwater (and soil) contamination may be present along West Morena Boulevard associated with former leaking underground storage tanks.

3.0 GEOLOGIC AND SEISMIC HAZARDS

3.1 FAULTING

Active and potentially active faults are present in the Project vicinity. The planned pipeline alignment generally parallels the active RCFZ along east Mission Bay. Locations of active strands of the RCFZ are known near Balboa Avenue, at Buenos Avenue, and at Old Town; otherwise active faults in the southern Project area are generally mapped as suspected or “concealed” (i.e., not well located) on geologic and fault maps by the City of San Diego. Active faults are not mapped or suspected within the Project area east of the RCFZ; fault rupture does not present a potential geologic hazard in these areas. The potential for faulting as it relates to the planned Project is discussed in the Faulting Report.

Faults crossing the pipeline alignment may require the use the flexible connections or other measures to accommodate fault-related displacement. Fault hazards and mitigations are discussed further in the Faulting Report.

3.2 LIQUEFACTION

Liquefaction is a phenomenon in which loose to medium dense, saturated, granular materials undergo matrix rearrangement, develop high pore water pressure, and lose shear strength because of cyclic ground vibrations induced by earthquakes. This rearrangement and strength loss is followed by a reduction in bulk volume of the liquefied soils. The effects of liquefaction can include the loss of bearing capacity below foundations, settlement in level ground, large horizontal deformations of relatively level ground with an unconfined vertical face (referred to as lateral spreading) and instability in areas of sloping ground (also known as flow sliding).

A potential for liquefaction exists at the Morena Pump Station site within the alluvial sand and silt layers present below the groundwater table (about 9 feet bgs). The borings and CPTs within Sherman Street and Custer Street were used to evaluate liquefaction potential at the pump station site. The evaluation methodology and the results of the evaluations are presented below. The evaluations will be refined pending the results of laboratory testing and additional subsurface explorations within the pump station property.

A potential for liquefaction also exists within the fill and alluvium present along the southern portion of the pipeline alignment (south of Ingulf Street and crossing Tecolote Creek), and potentially within some of the canyon crossings (Rose Creek and San Clemente Creek). Liquefaction potential and possible effects within the cut-and-cover portions of the pipeline alignment are discussed in this report in a qualitative manner. Liquefaction potential within the planned tunnel segments is discussed in the Tunnel Report.

3.2.1 Methodology

The procedure for evaluating liquefaction potential is empirical and is based on data and observations at sites that have, and have not, liquefied during an earthquake. In the procedure, the capacity of the soil to resist liquefaction is expressed as the cyclic resistance ratio (CRR). The seismic demand is a function of the anticipated Peak Ground Acceleration (PGA) at the site

and is termed the cyclic stress ratio (CSR). The potential for liquefaction was assessed at each sample (or CPT measurement) depth for the pump station site in terms of a Factor of Safety (FS). This factor of safety is defined as the CRR divided by the CSR generated by the design ground motion. A potential for liquefaction was interpreted to exist at each subsurface layer where the FS against liquefaction is less than 1.1. Settlement in each layer where the FS was less than 1.1 was then calculated using empirical correlations of the liquefaction potential to volumetric strain.

The potential for liquefaction and the amount of associated settlement are evaluated differently for the borings and CPTs, as described below:

SPT Method – This method uses penetration resistance of the sampler (blow counts per foot) from the borings to obtain the resistance of the soil to liquefaction. SPT N-Values are obtained from the blow counts. A correction factor of 0.65 was applied to blow counts obtained from the 2.5-inch inner diameter sampler to account for the difference in size from the SPT sampler. Laboratory test results provided fines contents for use in the analyses.

CPT Method - The analysis was performed using the CLiq software (developed by GeoLogismiki, in collaboration with Gregg Drilling and Dr. Peter Robertson). The software uses the tip resistance and sleeve friction obtained from the CPT soundings to interpret soil type, relative density/consistency and other properties to calculate the FS against liquefaction and assess secondary effects.

The SPT method incorporates the procedures outlined by Idriss and Boulanger (2008) for liquefaction triggering and the empirical method proposed by Ishihara and Yoshimine (1992) to estimate liquefaction induced settlements. Within the CLiq program, the CPT method used the updated procedures in Boulanger and Idriss (2014) for liquefaction triggering and the calibrated procedures for estimating liquefaction-induced ground settlement by Zhang et al. (2002).

The following input parameters were used in the liquefaction analysis for the MPS:

- Earthquake Magnitude, M_w : 7.0
- Peak Ground Acceleration (PGA): 0.59g
- Depth to groundwater: 9 feet bgs

The PGA used for liquefaction analyses is the Risk-Targeted PGA adjusted for Site Class D from the 2016 California Building Code. The PGA is roughly equivalent to a ground motion with a 1% probability of exceedance in 50 years and is intended to correspond to the ground motion associated with structure collapse. PGA was evaluated for both Site Classes D and E, with Site Class D resulting in a higher PGA of 0.59g, which was used in the analyses, compared to 0.53g for Site Class E. See Section 4.3 of this report for additional discussion.

3.2.2 Results

The results of the analyses for the MPS site suggest that liquefaction is expected to occur within the very loose to medium dense sand layer that is present from near the ground surface to depths

generally ranging from 19 to 29 feet bgs. Other underlying sandy layers are also subject to liquefaction and are expected to contribute to a smaller extent to liquefaction-induced settlement.

Surface expression of liquefaction is generally considered to occur as a result of liquefaction within soil layers present within about 50 feet of the ground surface. Engineering analyses of the recent borings and CPTs around the Morena Pump Station perimeter suggest that surface expression of liquefaction could result in ground surface settlements ranging from about 5 to 11 inches. Differential settlement over relatively short horizontal distances could occur due to the highly variable subsurface conditions. Settlements at the lower end of this range are expected to occur at the locations where more layering of fine-grained soils is present in the upper, potentially liquefiable layer.

The below-grade portions of some of the planned structures will extend through the upper granular layer where much of the liquefaction is expected to occur. The excavation for the Pump Station Building is expected to extend to a depth of about 50 feet bgs. However, some of the subsurface explorations around the site perimeter indicate that the sand layer that is present below about 50 feet is sufficiently loose and has a relatively low fines content, such that liquefaction could also occur in the soil at this depth. Engineering analyses suggest that settlement within this lower granular layer could result in settlement up to about 5 inches that could impact the deeper structures.

Detailed results of the liquefaction analyses are presented in Appendix E.

Although no detailed analyses were performed, the potential for soil liquefaction is suspected along the southern portion of the pipeline alignments south of Ingulf Street within the alluvial soils. Mitigation of potential liquefaction along the pipeline alignments south of Ingulf Street and north of the MPS is not planned.

3.2.3 Performance of Fine-Grained Soils

The loss of shear strength in fine-grained soil from strong ground shaking can also cause potential surface deformations and adversely impact the performance of foundations. A potential for liquefaction susceptibility in fine-grained soils can be evaluated using the criteria presented by Idriss and Boulanger (2008). The majority of the fine-grained soils encountered in our explorations are silts that are expected to exhibit sand-like behavior and hence considered to potentially liquefy. Preliminary visual observations of soil samples indicate that there are likely to be limited soil layers that exhibit clay-like behavior, which should not be susceptible to liquefaction.

Strength loss in the fine-grained soils was estimated using the correlation between plasticity characteristics and soil sensitivity (Idriss and Boulanger, 2008). However, due to the relatively limited presence of clays, it is unlikely that significant surface deformations will arise from cyclic softening of quick clays.

3.3 LATERAL SPREADING

Lateral spreading is a phenomenon where surficial soil displaces along a shear zone that has formed within an underlying liquefied layer. Upon reaching mobilization, the surficial blocks are

transported downslope or in the direction of a free face by earthquake and gravitational forces. Lateral spreading is thought to occur on slopes as level as ½ percent, or on level ground with a “free face”, such as a stream bank.

The pump station site itself is relatively flat, and a levee about 8 to 10 feet high borders the site on the south. However, south of the berm, the ground surface slopes down about 13 feet to Friars Road. South of Friars Road, a more gradual slope is present between the road and the San Diego River.

It is expected that the potentially liquefiable layers present at the pump station site may be laterally continuous to the south in the vicinity of the river. Engineering evaluations suggest that there is a potential for lateral spreading to impact the Morena Pump Station and surrounding pipelines, particularly to those to the south of the pump station site. Engineering considerations associated with lateral spreading are discussed in Sections 4 and 5.

3.4 SEISMIC SETTLEMENT OF DRY SAND/SEISMIC COMPACTION

Strong ground motion can also cause the densification of soils above the groundwater level, resulting in settlement of the ground surface. This phenomenon is known as seismically-induced settlement or seismic compaction, which typically occurs in dry, loose cohesionless soils. During an earthquake, soil grains may become more tightly packed due to the collapse of voids or pore spaces, resulting in a reduction in the thickness of the soil column. Several feet of loose alluvial soils are present between the thin surficial fill layer and the groundwater table; however, considering the relatively large liquefaction-induced settlements expected, the amount of seismic compaction should be relatively low compared to the estimated liquefaction-induced settlement. Further, any liquefaction mitigation performed at the site should also mitigate the potential for liquefaction-induced settlement.

3.5 OTHER HAZARDS

Other seismic and geologic hazards that could impact the Project have been considered. In general, the local geologic conditions indicate that other seismic and geologic hazards are not likely to affect the site. Slopes in the Project vicinity are generally sufficiently flat and/or far enough from the improvements that stability is not expected to be a significant issue. Static loading from new improvements could initiate settlement within the underlying deposits; however, surface loading is relatively small and settlement should occur within relatively short time frames. Settlement is further discussed within the context of foundation design (Section 4.4). Potentially expansive and/or collapsible soil has not been identified in significant quantities along the project alignment, and should not impact Project design.

Given the location of the site, the potential for seiches or tsunamis affecting the site is considered low (County of San Diego, 2009). While the Morena Pump Station site is protected from flooding of the San Diego River by the adjacent levee, some flooding potential exists along West Morena/Morena Boulevard to the north of the pump station. From West Morena Boulevard at Tecolote Creek (just north of the Tecolote Road overcrossing) approximately 2,000 feet north along West Morena/Morena Boulevard, the Project area is within the 100-year floodplain (Federal Emergency Management Agency, 2012). This area could experience flood depths

between 1 to 2 feet, however, this hazard should not constitute constraints to the planned pipelines in this area.

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4.0 CONCLUSIONS & RECOMMENDATIONS – PUMP STATION

The primary geotechnical considerations for the design of the Morena Pump Station are the presence of potentially liquefiable soil and the need for deep excavations to facilitate construction of below-grade facilities, combined with the presence of a relatively shallow groundwater table. These considerations are discussed below. Geotechnical recommendations for design are also provided.

Based on our geotechnical investigation and analyses, it is our opinion that the site location shown on Figure 2 is suitable for development of the pump station and associated structures from a geotechnical and geologic standpoint provided the recommendations provided below are incorporated into the design and construction of the project.

4.1 LIQUEFACTION MITIGATION

As discussed in Section 3, ground surface settlements ranging from about 5 to 11 inches could occur at the MPS site during the design seismic ground motion. These settlements could be erratic with large differential settlement occurring within short horizontal distances. Some lateral movements could also occur due to lateral spreading toward the San Diego River.

Possible structural solutions to mitigate the effects of liquefaction settlement include supporting the structures on stiffened shallow foundations (mats) or deep foundations such as drilled piers or piles. Relatively thick stiffened shallow or mat foundations would limit differential settlement across the foundation, and may reduce total settlements slightly, however, significant settlement and some tilting could still occur, and it is understood that structure settlements of this magnitude are not considered acceptable from a structural standpoint. Deep foundations to support the numerous facilities would need to extend to depths estimated to range from 60 to 100 feet bgs, which could be costly. In addition, considering the large amount of below-grade infrastructure planned for the pump station site (in addition to the main structures), it is expected that it would be difficult to accommodate differential settlement between the structures and the other infrastructure.

Ground improvement is considered the most feasible approach to limit total settlement of structures to acceptable levels, provide improved foundation support, and limit differential settlement between structures and interconnected utilities and structures. At a minimum, the treatment would need to improve the potentially liquefiable sandy layer that extends to depths ranging from 19 to 29 feet. Deeper treatment is recommended to address deeper granular layers that were encountered in some locations, as well as below the Pump Station Building and other below-grade structures.

The soil liquefaction mitigation (ground improvement) will mitigate the potential for lateral spreading and seismic settlement of dry sands at the pump station property. Total post-liquefaction and dry sand seismic settlements are estimated to be less than 2 inches, which is within the tolerable settlement limits for the structures provided by the project structural engineer.

Selection of the type of ground improvement depends on the soil conditions, surface conditions, and the desired level of improvement. Some options are discussed below. Regardless of the option selected, flexible connections on pipelines exiting the pump station site will be required. If the entire pump station site is not improved, flexible connections on utilities within the site would also be needed. Due to the presence of nearby buildings, strong vibration-induced densification techniques were not considered because of the possibility of settlement of those structures.

The ground improvement methods discussed below are all considered technically feasible for the soil conditions present at the site. Cost, schedule, and plan area to be improved (below individual structures vs. entire site) should be considered to select the most appropriate method. It may also be possible to combine shoring of the deep excavations with the ground improvement system. The recommended method (vibro-replacement) is considered the most technically appropriate and cost-effective, however, other techniques can be considered based on discussions with the selected ground improvement contractor.

4.1.1 Compaction Grouting

Compaction grouting injects a low-mobility grout under relatively high pressures into the soil to displace and compact soils, improve shear strength and reinforce fine-grained soils. Each depth increment of grout injection is called a “stage” and is typically on the order of 1 to 2 feet. The process can be optimized by sequencing primary and secondary injection points, which are grouted in an alternating pattern. The injection points are typically located within planned building footprints, with the treatment area extending a minimum of five feet horizontally outside the edge of the planned foundations.

The purpose of the compaction grouting would be to increase the relative density of the potentially liquefiable layers to reduce the estimated settlement to an amount that can be tolerated by the structures. The design of compaction grouting program, which is typically executed by a specialty geotechnical contractor, is typically dependent on the following criteria: 1) allowable post-improvement settlement (liquefaction-induced); 2) depth of soil improvement; 3) lateral limits of soil improvement, 4) construction monitoring tolerances and requirements, and 5) verification testing acceptance criteria (e.g., post improvement CPTs). It is common to design a compaction grouting program such that the post-improvement liquefaction-induced settlement is limited to less than an inch.

4.1.2 Vibro-Replacement

Vibro-replacement involves inserting a probe into the ground to densify the surrounding soils and filling the void created with gravel to create a “stone column” or “aggregate pier”. The procedure is repeated along a grid pattern over the area to be improved. The minimum diameter of the stone columns is typically 3 feet. The column spacing is typically 6 to 10 feet.

Vibro-replacement can mitigate the potential for liquefaction by: 1) densifying the soils, 2) reinforcing the soil mass with columns of gravel, and 3) providing increased drainage to alleviate the excess pore pressures developed by strong ground shaking (Idriss and Boulanger 2008).

Vibro-replacement is suitable for sands, silts and clays (Idriss and Boulanger 2008), although drainage and liquefaction mitigation are maximized in sands. The method improves bearing capacity and typically allows for shallow foundations for support of structures due to the reduced potential for liquefaction-induced settlement.

A specialty geotechnical contractor designs and constructs the vibro-replacement using performance criteria specified by the Geotechnical Engineer. The criteria are typically:

- Allowable vertical bearing pressure (post improvement).
- Allowable liquefaction-induced settlement (post improvement).
- Depth of soil improvement.
- Lateral limits of soil improvement.
- Acceptance Testing (e.g., post improvement CPTs).

4.1.3 Soil Mixing

Various methods and types of soil mixing are available and typically involve in-situ mixing of cement slurry with existing soil to create columns of improved soil.

Deep Soil Mixing produces relatively large diameter soil-cement columns (up to about 9 feet in diameter). It can typically extend to depths of about 80 feet, and is well suited to liquefaction mitigation. Variations of this method can be used to construct reinforced and/or tied-back secant and tangent pile walls for basement excavations.

Other methods use multiple cutting heads or augers to create overlapping or continuous columns of improved soil. Cement Deep Soil Mixing (CDSM) is often used to create “cells” of improved soil to mitigate liquefaction. Cutter Soil Mixing (CSM) is generally used to create soil-cement panels, similar to a diaphragm wall, which, when reinforced, can be used as basement wall excavations. The selection of the appropriate cutting tool should consider the presence of the gravel layers in the subsurface.

4.1.4 Selected Ground Improvement Option

Based on the site constraints (limited site size and surrounding improvements), the subsurface conditions, and cost considerations, we recommend bottom feed vibro-replacement (stone columns) be utilized to improve the ground conditions for mitigation of liquefaction potential. Due to the numerous planned below-grade utility connections within the pump station facility, ground improvement should extend across the entire pump station site (to the property line) as shown on Figure 4a. We understand that flexible connections are also planned between the underground utilities and buildings within the pump station facility. We further understand that the flexible connections will allow for approximately 1 inch of differential movement between these utilities and the buildings.

To reduce total and differential liquefaction-induced settlement to acceptable levels, stone columns should be installed on a grid pattern of approximately 10 feet by 10 feet. The recommended stone column depths across the site are shown on Cross Section A-A' on Figure

4b. Across the majority of the pump station site, stone columns should be installed to a depth of 50 feet to limit ground surface settlement and associated impacts to at-grade structures and near-surface utilities. Within the footprint of the Pump Station Dry Well/Wet Well (foundation depth of about 50 feet) and the Intake Screening Building (foundation depth of about 33 feet), stone columns should be installed to approximately 75 feet bgs to densify sand layers that are present below those relatively deep foundations. Stone column recommendations will be refined based on additional planned field exploration and engineering analyses, and a pre-production ground improvement test program will be implemented to define the actual stone column spacing for production.

Evaluation of the test section and acceptance of the ground improvement will be based on the results of post-ground improvement CPTs, which should be performed on a grid pattern of about 40 feet by 40 feet in between the stone columns. The calculated post-liquefaction settlement based on the CPTs will be required to be less than 2 inches, which should provide a differential settlement of less than 1 inch over a 20 foot horizontal distance. The post ground improvement CPT results will be evaluated using the procedures outlined by Idriss and Boulanger (2008) for liquefaction triggering and the empirical method proposed by Ishihara and Yoshimine (1992) to estimate liquefaction induced settlements.

4.2 BELOW GRADE DESIGN & CONSTRUCTION

The three main planned below-grade structures at the Morena Pump Station, the Intake Screening Building (excavation depth of about 34 feet), Pump Station Building (about 51 feet) and Energy Dissipator Structure (about 25 feet deep) will require significant excavation depths that will extend into the alluvium and below the groundwater table. Due to the depths and the presence of surrounding structures, the below-grade excavations will require temporary or permanent shoring and temporary groundwater control during construction. The permanent below-grade walls will need to be designed to resist lateral earth pressures, groundwater and seismic effects. Several feasible shoring and dewatering methods, as well as recommendations for below-grade wall design, are presented in the following sections.

4.2.1 Shoring Types

Much of the below-grade construction in previous developments in and near downtown San Diego has used soldier piles (driven steel H-piles or drilled and concreted H-piles) with wood or steel lagging between the soldier piles to provide temporary support for “bottom-up” construction. Soldier pile and lagging shoring systems generally require global dewatering (discussed further in the following section), which could require significant pumping to achieve acceptable conditions at the pump station site due to the predominantly sandy conditions. While the shallower Energy Dissipator Structure may be practical to construct with soldier piles and lagging, the deeper structures may require excessive dewatering using this method. Further, the length of the soldier piles needed for the Pump Station Building may be impractical. If used, soldier piles should be drilled rather than driven due to presence of gravel layers and to minimize the potential for settlement of adjacent structures due to densification of loose sand layers during pile driving.

Shoring generally requires lateral support to construct a basement that extends more than one level. Most San Diego based contractors use temporary ground anchors (tiebacks) behind the

wall, rather than internal bracing (struts and walers) in front of the wall to maximize accessibility within the excavation, however, internal bracing could be practical for some of the structures planned for this site. The most common anchors are auger drilled and pressure grouted tendons. The number of levels of tiebacks would depend on the depth of excavation, lateral and other loads, and the serviceability requirements of nearby infrastructure.

Sheet pile walls may also be feasible to retain the shallower excavations planned for the site. Structural sections of piles, typically panels of interlocking “Z” or “W” shapes, are driven into the ground sequentially to form a relatively water-tight excavation. Dewatering would still be required within the excavation. The sands and silts present at the site should result in acceptable installation conditions for sheet pile walls, although additional investigation is needed to determine the depth and extent of gravel layers at the site. Installation is typically accomplished by vibrating or driving the sheet piles into the ground. Care would be needed to avoid damaging nearby structures due to vibrations. While tiebacks can be used to provide lateral restraint of sheet piles, this type of shoring is typically used for shallow excavations where lateral restraint is not needed. Alternatively, internal bracing could be used for smaller excavations.

For the Pump Station Building and the Intake Screening Building, several types of reinforced concrete walls, such as diaphragm walls or secant/tangent walls, should be considered. These methods use various excavation techniques to create a water-tight shoring/cut-off wall that can also act as the permanent below-grade wall. Similar to sheet pile walls, dewatering is limited to within the excavation. For diaphragm walls, excavation is typically achieved using a clamshell excavator bucket to create a wall that is several feet thick. A secant or tangent wall is generally a series of overlapping drilled shafts. Considering the subsurface conditions and shallow groundwater level, excavation for both types of walls would likely be advanced using drilling fluid to maintain stability of the excavation until concrete is placed. Various types of reinforcement can be set in the concrete. As discussed previously, soil-cement can also be used to construct basement walls, and could be used in combination with liquefaction mitigation.

Regardless of the shoring method selected, monitoring of the infrastructure surrounding the site, including public roads, neighboring buildings, and the trolley tracks, will require careful monitoring to check that the excavation is not significantly impacting these features. Preliminary lateral earth pressures for the design of temporary tied-back shoring systems are presented on Figures 5 and 6 for excavation depths of 30 and 50 feet, respectively.

4.2.2 Dewatering

Considering the shallow depth to groundwater, dewatering of the area ahead of the major excavations will be required to reduce the possibility of soil instability at the bottom of the excavation and to allow for dry, firm working conditions. The groundwater surface should remain at least 5 feet below the bottom of the excavation until the foundations are poured and adequate structure loads are in place to resist hydrostatic uplift. Dewatering volumes could be substantial due to the predominate presence of sands and silts within the planned excavation depths.

The typical approach would be to globally dewater the excavation area with closely-spaced wells around the perimeter of the excavation and possibly within the excavation as well. This method would be required if soldier pile and lagging is used for shoring. Other shoring methods that

create a water-tight shoring wall, such as sheet piles, diaphragm wall and contiguous pile or soil-cement walls, should require less pumping to lower the groundwater level within the excavation. Alternate dewatering methods such as ground freezing, which would reduce or eliminate groundwater discharge concerns, could be considered but are costly.

Dewatering for the Pump Station Building will likely need to consider permeabilities within both the upper and lower sandy alluvial layers. The excavations for the Intake Screening Building and Energy Dissipator Structure may extend into the finer grained intermediate layer between depths of about 20 to 30 feet and 50 feet; this layer is much siltier and will be less transmissive than the more sandy layers above and below this depth, which will be a factor in dewatering design.

We have performed a preliminary evaluation of the groundwater pumping rate for construction dewatering that will likely be required to provide dry excavation for below-grade construction. The subsurface stratigraphy used in this evaluation was based on Boring PS-2 advanced on March 28, 2017. The subsurface was modeled to include 4 layers as shown in Table 2 below:

Table 2
Subsurface Layering for Groundwater Pumping Rate Evaluation

Layer	Elevation (feet, NGVD 29)	Soil Description
1	+16 to -10	Poorly graded sand with silt (SP-SM)
2	-10 to -35	Silt/clay (ML/CL)
3	-35 to -55	Poorly graded sand with silt (SP-SM)
4	-55 to -100	Silt/clay (ML/CL)

The design groundwater elevation is +5 feet, NGVD 29 based on measurements for the well development on March 31, 2017 and the slug tests performed on July 26, 2017. Slug tests were performed to evaluate the hydraulic conductivity of the subsurface. Correlations with index properties were also used to evaluate the hydraulic conductivity.

We used the computer program GeoSlope SEEP/W to evaluate the water flow rate during dewatering. Various boundary conditions and subsurface conditions were modeled for both a cross sectional model and a plan model. The cross sectional models assumed a 100-foot wide excavation with a row of dewatering wells on each side of the excavation. The plan models assumed an 80 by 100 foot excavation with dewatering wells spaced at 20 feet around the perimeter of the excavation. A summary of the SEEP/W analyses is presented in Table 3 below, and the results of the analyses are presented in Appendix F.

Table 3
Model Assumptions and Flow Rates for Groundwater Pumping Rate Evaluation

Model	Model Conditions	Flow Rate (gpd ¹)
1	Cross Sectional Model, 1000-ft model width and individual soil layers	93,070
2	Cross Sectional Model, 500-ft model width and individual soil layers	208,631
3	Cross Sectional Model, 500-ft model width and composite soil layer	56,876
4	Plan View Model with 500-ft width, composite soil layer, and $K_h/K_v=1$	168,689
5	Plan View Model with 500-ft width, composite soil layer, and $K_h/K_v=5$	430,447

Note:

1. gpd = gallons per day

Based on these evaluations, we recommend that a flow rate between 168,000 and 430,000 gpd be assumed for planning and permitting purposes. The flow rate and total volume of water extracted may be reduced depending on the shoring method used to perform the excavation. Different shoring methods can reduce the flow of water into the side walls of the excavation and reduce the dewatering requirements.

Pump tests and additional borings and laboratory testing may be performed within the pump station property once access is permitted. Pump tests may be performed to provide additional data on soil permeability at various depths for design of the dewatering system and estimation of disposal estimates.

Analytical laboratory testing was performed on the groundwater sample collected from the monitoring well PS-2. The metals barium and molybdenum were detected at concentrations of 0.142 and 0.0287 milligrams per liter (mg/l), respectively. Total TPH was detected at 73 micrograms per liter (µg/l); and the VOCs cis-1,2-dichloroethene (c-1,2-DCE) and trans-1,2-dichloroethene (t-1,2-DCE) were detected at 480 and 2.6 µg/l, respectively. No PCBs, OCPs or SVOCs were detected above their respective laboratory detection limits.

The analytical results were compared to State and Federal hazardous waste criteria. Based on the metals, VOCs and TPH concentrations detected, the wastewater is not considered Federal Resource Conservation and Recovery Act (RCRA) or California hazardous. However, for project dewatering purposes, pumped groundwater is anticipated to either be disposed to the municipal stormwater or sewer systems. Both disposal options will require a discharge permit and additional analytical testing to satisfy permit requirements. Additionally, because of the presence of VOCs and TPH, predisposal treatment of the wastewater from construction dewatering would likely be required. However, given the low concentration of contaminants detected, it is likely treatment will be successful in meeting permit requirements. Permit requirements will also include the identification of a discharge location, estimation of the volume and rate of discharge and discharge duration, and payment of related discharge fees. Dewatering will require a permit from the Metropolitan Waste Water Department (MWWD) to divert effluent to the sewer or a National Pollutant Discharge Elimination System (NPDES) permit if effluent is diverted to a storm drain. Further analytical testing of groundwater is planned as access to the pump station site is provided.

The dewatering contractor is responsible for designing, installing, and operating a dewatering system capable of lowering and maintaining the groundwater to the desired depths during

construction. As part of final design, AECOM should be consulted to evaluate the depth of required drawdown to check that the shoring system is stable against geotechnical failure mechanisms, such as external stability, foundation heave and hydraulic failure, and also estimate settlement of the adjacent ground due to dewatering.

4.2.3 Below-Grade Wall Design

To construct the permanent subterranean walls, cast-in-place concrete walls can be placed within the shoring system, or a finished surface can be placed over the permanent shoring wall. Permanent subterranean walls should be designed for earth, hydrostatic, seismic, and surcharge pressures. A preliminary lateral pressure diagram for the design of below-grade walls is presented as Figure 7.

The subterranean walls along the streets will be subject to traffic loading (car and truck traffic). The vertical surcharge load to account for traffic was assumed to be the equivalent of 2 feet of soil overburden or 250 pounds per square foot (psf). While loading from the Trolley is not expected to impact below-grade wall design, considering the currently planned pump station layout, the vertical surcharge load from the Trolley is considered to be 1,880 psf, based on our previous experience working with similar projects in Downtown San Diego.

When designing the walls for the static case only, the restrained earth pressure increment should be added to the active earth pressure in addition to the hydrostatic and surcharge pressures. When designing the walls for seismic case only, the seismic pressure increment should be added to the active earth pressure in addition to the hydrostatic and surcharge pressures.

The recommended seismic pressure increment was estimated as an equivalent fluid, and was developed by considering recommendations by Lew and others (2010). They completed a comprehensive review of the methods used to assess seismic earth pressures on deep subterranean retaining walls and provided updated recommendations for design. The seismic pressure increment was developed using a conservative estimate of the pseudostatic acceleration coefficient (K_h) that is equal to 0.5 PGA (PGA=0.59G) as recommended by Lew and others (2010).

4.2.4 Subsurface Drainage and Waterproofing

Design of the portion of the permanent subterranean walls above the groundwater level should incorporate adequate drainage behind the wall to collect water from sources such as irrigation or surface runoff to reduce the potential for hydrostatic pressure to build up behind the wall. The wall drainage should extend at least 5 feet below the design groundwater elevation. The drainage system should consist of a prefabricated geocomposite drainage structure, such as Miradrain or equivalent, or filter material behind the wall. Filter materials should conform to the gradation requirements specified in Section 300-3.5.2 (Pervious Backfill) of the current “Standard Specifications for Public Works Construction.” If filter material is used behind the wall, the horizontal width should be at least 18 inches and the material should extend up to within 18 inches of the ground surface.

Basement walls should be waterproofed for end use. Because of the potential for increased moisture from landscaping and underground utilities, it may be necessary to place the

waterproofing over the entire height of the walls, depending on the functionality of the wall surface needed. A limited degree of waterproofing is expected to be required for the Project; this typically allows damp patches and minor leakage through construction joints. A high degree of waterproofing may be needed if functionality requires the interior of the basement wall surface to be free of all leakage, seepage and damp patches. The Project Architect and qualified waterproofing consultant should review and select waterproofing requirements relative to the desired functionality.

4.3 SEISMIC DESIGN

The Project area will likely be subject to moderate to severe ground shaking in response to a local or more distant large-magnitude earthquake occurring during the expected life of the planned facilities. We understand seismic design parameters are required for design of the Morena Pump Station facilities. Parameters can also be provided for other locations along the pipeline alignment, if needed. For design in accordance with the 2016 CBC (based on ASCE 7-10), the following parameters should be used. These parameters are developed in the code based on Risk-Targeted Maximum Considered Earthquake (MCE_R) ground motion response accelerations.

Based on the presence of potentially liquefiable soils, the Site Class would be F in accordance with ASCE 7-10. However, since we understand the fundamental period of the structures will be 0.5 seconds or less, Site Class D may be used in lieu of F for structural design. In addition, if ground improvement is used to mitigate the potentially liquefiable soils, Site Class D may also be appropriate. Seismic design parameters for Site Class E were evaluated to aid in the evaluation of PGA for liquefaction studies; however since the short period acceleration for Site Class D is higher than Site Class E, the Site Class D acceleration was used for engineering evaluations. The seismic design coefficients for Site Classes D and E are summarized in Table 4.

Table 4
2016 CBC Seismic Coefficients

Parameter	Value	Value	Reference
Site Class	D	E	ASCE 7-10, Table 20.3-1
Mapped Spectral Acceleration - Short Period, S_s (g)	1.291	1.291	2016 CBC Figure 1613.3.1(2) ¹
Mapped Spectral Acceleration - 1 Sec. Period, S_1 (g)	0.500	0.500	2016 CBC Figure 1613.5(4) ¹
Site Coefficient - Short Period, F_a	1.000	0.900	2016 CBC Table 1613.3.3(1) ¹
Site Coefficient - 1 Sec. Period, F_v	1.500	2.400	2016 CBC Table 1613.3.3(2) ¹
MCE ² Spectral Response Acceleration - Short Period, S_{MS} (g)	1.291	1.162	2016 CBC Equation 16-37, $S_{MS}=F_a S_s$
MCE ² Spectral Response Acceleration - 1 Sec. Period, S_{M1} (g)	0.750	1.200	2016 CBC Equation 16-38, $S_{M1}=F_v S_1$
Design Spectral Response Acceleration - Short Period, S_{DS} (g)	0.861	0.775	2016 CBC Equation 16-39, $S_{DS}=2/3 * S_{MS}$
Design Spectral Response Acceleration - 1 Sec. Period, S_{D1} (g)	0.500	0.800	2016 CBC Equation 16-40, $S_{D1}=2/3 * S_{M1}$

Table 4
2016 CBC Seismic Coefficients

Parameter	Value	Value	Reference
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Notes:

1. Calculated using U.S. Seismic Design Maps web application developed by USGS.
2. MCE – Maximum Considered Earthquake.
3. Site coordinates estimated from 'Google Earth' computer program used to evaluate coefficients: 32.76331; -117.20021.

4.4 FOUNDATION DESIGN

Structure foundations at the Morena Pump Station will vary between near-surface to depths of about 50 feet bgs. Therefore, foundation recommendations have been developed for the near-surface soil above the groundwater level, the upper sandy zone below the groundwater level, the finer grained/silty intermediate zone, and the (relatively denser) lower sandy zone.

Near-surface foundations may consist of conventional continuous strip and spread footings founded on existing, recompacted or improved fill or alluvium. Mat foundations may also be used to support near-surface structures. Foundations below the groundwater level should be supported on thick mat foundations. Tiedowns may be required if structure weights are insufficient to resist buoyancy effects due to the shallow groundwater level. Pad foundations (small mats) may be used to support lightly-loaded equipment at grade.

4.4.1 Continuous Strip and Spread Footings

Continuous strip and spread footings above the groundwater should be embedded at least 18 inches below lowest adjacent grade and should be at least 12 inches wide. Footings dimensioned as recommended may be designed using the following allowable soil bearing pressure.

Table 5
Allowable Bearing Pressures for Footing Design

Footing Depth (feet, bgs)	Allowable Bearing Pressure (psf) ^a	Increase per Foot of Embedment (psf) ^b	Increase per Foot of Width (psf) ^c	Maximum Allowable Bearing Pressure (psf)
0 to 6 (above groundwater)	2,000	1,000	500	3,000
6 to 9 (above groundwater)	1,800	NA	NA	1,800

Notes:

- a. Allowable bearing pressure may be increased by one-third for loads that include wind or seismic forces.
- b. Per foot of embedment beyond 18 inches below lowest adjacent grade.
- c. Per foot of width beyond 12 inches.

Adjacent footings bearing at different elevations should be located such that the slope from bearing level to bearing level is flatter than 1:1 horizontal:vertical (H:V).

For foundations designed as recommended above, the total settlement due to foundation loads only is estimated to range from about ½ to 1 inch, with differential settlements approximately half that amount between adjacent columns. The majority of this settlement should occur when

structure loads are applied. The estimated settlement should be re-evaluated by the Geotechnical Engineer when the structure design is complete, especially where the footings are not similarly proportioned, embedded, and loaded.

The upper 12 inches of material in areas not protected by hardscape should not be included in design for passive resistance. If friction is to be used to resist lateral loads, we recommend using an allowable coefficient of friction of 0.4 between the soil and foundation concrete. If it is desired to combine frictional and passive resistance in design, we recommend using an allowable friction coefficient of 0.3. Passive resistance may be increased by one-third for loads that include wind or seismic forces.

4.4.2 Mat Foundations

Mat foundations with a minimum width of 5 feet can be designed for the following allowable bearing pressures, based on the depth of the mat below grade.

Table 6
Allowable Bearing Pressures for Mat Foundation Design

Depth of Mat Foundation (feet, bgs)	Allowable Bearing Pressure (psf) ^{a,b}
0 to 55	4,000
>55	6,000

Notes:

- a. Allowable bearing pressure may be increased by one-third for loads that include wind or seismic forces.
- b. Based on an assumed foundation width of 10 feet.

The immediate settlement at the center of the mat subject to these bearing pressure is estimated to be ¾- to 1 inch.

The geotechnical analysis of large mat foundations typically uses the Modulus of Vertical Subgrade Reaction (k) developed from evaluations of settlement that consider actual loads, the plan layout of the mat and local variations in subsurface conditions. AECOM recommends a Modulus of Vertical Subgrade Reaction of 75 pounds per cubic inch (pci) for foundations below the groundwater level, if ground improvement techniques are not employed. The installation of stone columns, as recommended in the ground improvement section above, will densify the subsurface soils across the entire pump station site and below proposed foundations. Based on the presence of stone columns, we recommend that the mat foundations planned for the structures founded below the groundwater level be designed for a Modulus of Vertical Subgrade Reaction of 125 pounds per cubic inch (pci). Shallow mat foundations founded in fill soils above the groundwater may be designed for a Modulus of Vertical Subgrade Reaction of 150 pounds per cubic inch (pci). The densification of the sandy soils should be verified as part of the stone column program; the Modulus of Vertical Subgrade Reaction may need to be reevaluated at that time. We should review the plots of mat contact pressures and deflections for compatibility with the geotechnical assumptions used to develop the modulus and the strength and stiffness characteristics interpreted to exist within the materials supporting the foundation.

4.4.3 Lateral Resistance

Resistance to lateral loads on the foundations can be provided by passive resistance along the edge of the footings and by frictional resistance along the bottom of the footings. For passive resistance, we recommend using the following allowable equivalent fluid weights for footings or grade beams poured neat against the excavation or properly backfilled.

Table 7
Foundation Allowable Passive Resistance

Depth of Footing (feet, bgs)	Allowable Passive Resistance (pcf) ^{a, b}
0 to 9 (above groundwater)	250
9 to 25 (below groundwater)	115
25 to 55 (below groundwater)	100
>55 (below groundwater)	175

Notes:

- a. Allowable bearing pressure may be increased by one-third for loads that include wind or seismic forces.
- b. pcf – pounds per cubic foot.

4.5 RETAINING WALL DESIGN

We understand small gravity retaining walls may be used to retain surface improvements at the site. Retaining walls should be designed for earth, seismic, and surcharge pressures (seismic pressure can be neglected for walls less than 6 feet high). Hydrostatic pressures should not develop on the near-surface retaining walls considering the depth to groundwater and assuming proper drainage behind the walls. Foundations for retaining walls can be designed using the recommendations for continuous strip and spread footing presented in Section 4.4.1 of this report. The following sections provide specific recommendations for subsurface drainage and wall design.

4.5.1 Subsurface Drainage

Retaining walls should have adequate drainage behind the wall to avoid developing hydrostatic pressure from leaking underground utilities or surface runoff. Retaining walls should have a perimeter drain at the base of the wall. The base drain should be a minimum four-inch diameter perforated pipe. The pipe should be surrounded with at least one cubic foot per lineal foot of pipe of $\frac{3}{4}$ -inch crushed rock wrapped with filter fabric, such as Mirafi 140NL, or approved equivalent.

Adequate weep holes or collector pipes need to be incorporated into the system to provide an outlet for the drained water. Drainage from collector pipes should be directed to a suitable outlet. Existing or proposed water bearing utilities, or surface conditions that could promote infiltration (e.g., irrigated landscaping) behind walls, may require additional subsurface drainage.

4.5.2 Lateral Earth Pressures

Lateral earth pressures behind retaining walls depend on the allowable wall movement, wall inclination, backfill material and backfill slope. Retaining walls that are free to deflect at the top should be designed for an equivalent fluid weighing 35 pcf for level backfill. This earth pressure assumes the free standing walls will have a vertical back and will mainly retain properly processed, placed and compacted coarse grained soils. They also assume compaction within four feet of the wall will be completed with light hand-held or equivalent equipment; the lateral pressures would be higher if heavy equipment is used for soil compaction next to the walls.

For typical car and truck traffic, the lateral pressure can be estimated as a vertical surcharge equivalent to 2 feet of soil overburden or 250 psf.

Walls with heights over 6 feet should be designed for a seismic pressure increment applied as an inverted triangular distribution equal to 27 pcf. To develop the seismic pressure, the seismic pressure increment should be added to the active earth pressure. The seismic pressure increment was developed using the Mononobe-Okabe pseudo-static approach and a repeatable ground acceleration of 0.3g.

4.6 CORROSIVITY

Analytical testing for corrosion potential was performed on select soil samples at the pump station and along the project alignment. The results of the analytical testing are presented in

Appendix B; sample locations are shown on the boring logs (Appendix A). Due to the marine depositional environment, it is expected that the on-site soil may be somewhat corrosive, and that concrete and steel design will need to consider potential effects of corrosion. We understand that a corrosion engineer is part of the project design team. The analytical tests for corrosion potential have been forwarded to the corrosion engineer for evaluation and development of design recommendations.

Additional testing of groundwater samples may be required to provide data on corrosivity as part of the application for groundwater disposal for dewatering.

4.7 STORMWATER INFILTRATION

Infiltration testing has not been performed, as access to the site will not be permitted by the current property owner until October 2017. Based on the AECOM field explorations near the Morena Pump Station, the soils underlying the site are shallow fills overlying alluvium consisting of interbedded silty sands, clays, and sands. These soils are most likely attributable to Hydraulic Group D soils (City of San Diego 2016 Storm Water Standards). These soils are expected to provide an infiltration rate between 0.01 and 0.5 inches per hour, consistent with partial infiltration. However, the measured groundwater at the site is approximately 9 feet below the existing ground surface. Based on the City of San Diego Storm Water Standards, Part 1: BMP Design Manual, the depth to the seasonal high groundwater table must be greater than 10 feet for infiltration Best Management Practices (BMPs) for stormwater infiltration to be allowed.

Soil and groundwater contamination have been mapped in the vicinity of the site. Analytical laboratory tests were performed on one groundwater sample recovered from the monitoring well developed in Custer Street near the pump station location. Total petroleum hydrocarbons (TPH) and volatile organic compounds (VOCs) were detected in the groundwater sample. The presence of on-site groundwater contamination further restricts the feasibility of stormwater infiltration at the site due to the potential for spreading contamination. Analytical test data are discussed in Section 4.2.2 above and presented in Appendix C. In addition, and as discussed above, groundwater was measured at 9 feet below the ground surface in the monitoring well installed in Custer Street. In accordance with Section C.3 of the Storm Water Standards, this should also preclude infiltration of stormwater at the site. Both of these issues are addressed in Criteria 3 and 7 of Worksheet C.4-1 (Appendix G).

Due to shallow groundwater and potential contaminants in the subsurface, infiltration of stormwater is not recommended at the pump station site. Infiltration of stormwater could also lead to mounding of groundwater in the area and increase the risk of soil liquefaction on neighboring properties. As indicated in Worksheet C.4-1, if any of the criteria "...from rows 5-8 is no then infiltration of any volume is considered to be infeasible within the drainage area."

The above evaluation is summarized in the attached Worksheet C.4-1 (Appendix D of the San Diego Storm Water Standards). Partial infiltration is indicated as feasible per this worksheet from a subsurface soil standpoint, however, due to the shallow groundwater, the potential liquefaction and the presence of groundwater contaminants, infiltration of stormwater is not recommended at this site.

5.0 CONCLUSIONS & RECOMMENDATIONS – CUT & COVER PIPELINES

The primary geotechnical considerations for the design of the pipelines are the presence of shallow groundwater at the southern end of the project and potential concretions in the formation soils. In addition, pipeline sections south of the MPS may be installed using trenchless techniques. These considerations are discussed below. Geotechnical recommendations for design are also provided.

Based on our geotechnical investigation and analyses, it is our opinion that the pipeline alignments shown on Figures 3a through 3e are suitable for pipeline development from a geotechnical and geologic standpoint provided the recommendations provided below are incorporated in the design and construction of the project.

5.1 LIQUEFACTION

A potential for liquefaction exists for the portions of the pipelines both south of the Morena Pump Station and extending north from the pump station along Morena/West Morena Boulevard to Ingulf Street, as shown in Table 1. Liquefaction-induced ground surface settlement is expected to vary across the alignment, but magnitudes could be as high as what is estimated at the pump station site (5 to 11 inches). Mitigation for soil liquefaction along the pipeline alignments outside of the MPS pump station facility will not be performed; some differential settlement may be experienced along the pipelines near the edge of the improved area adjacent to MPS. Liquefaction may also be experienced along the southern portions of the pipeline alignments in the Morena Blvd. and W. Morena Blvd. areas.

Generally, the pipelines will settle along with the ground surface, with the pipelines somewhat moderating the differential settlement that will occur along the alignment. In general, since liquefaction can essentially “fluidize” the soil, pipelines below the groundwater table can “float” during post-liquefaction conditions, typically resulting in apparent heave at the ground surface. However, we understand that the pipeline will essentially be continuously full of water/effluent. The weight of the filled pipeline will be similar to the weight of the liquefied soil displaced by the pipeline, if soil liquefaction occurs. Therefore, the potential for the pipe to float as a result of liquefaction is considered negligible. From an operational standpoint, we understand that the pumps will be shut-off in the event of a significant earthquake, reducing the volume of fluid that could leak if the pipe ruptures.

5.2 EXCAVATION AND SHORING

Considerations related to pipeline trench excavation, dewatering and shoring are presented in this section. While not discussed in this geotechnical report, trenchless technology could be considered in some of the areas of high groundwater to reduce the need for dewatering and addressing difficult shoring and excavation conditions. Trenchless technology may also be considered at crossings of major intersections to address logistical concerns.

5.2.1 Excavatibility

Trench excavatibility will vary along the alignment. Near the Morena Pump Station, the fill, alluvium, and Bay Point Formation should mostly be excavatable using conventional earth-moving equipment. Some cobbles and boulders or other obstructions could be present within the fill and alluvium; these could require hoe-rams or other equipment to break up the oversized material for removal. Excavation will also extend below the groundwater level in much of this area.

As the alignment extends east from Morena Boulevard and Ingulf Street into older formational material, excavation conditions will be more difficult. Larger equipment will likely be required for excavation. Hoe-rams and breaker bars will likely be required in some areas due to zones of cementation/concretions, as well as cobbles and boulders within the various units.

5.2.2 Dewatering

Excavation along much of the southern stretch of the alignment will require excavation below the groundwater table. It will be necessary to lower the groundwater level below the bottom of the trench a minimum of 2 feet. The method of dewatering will depend on the type of shoring selected (see next section), but options include installing dewatering wells along the alignment or placing sumps within the bottom of the excavation. The soil below the groundwater level along much of this stretch consists of granular material with relatively high permeability and therefore high flow rates. Estimated permeability is expected to be similar to conditions at the pump station site (see Section 4.2).

5.2.3 Shoring

Shoring east of the Morena Boulevard/Ingulf Street intersection will need to retain primarily dense formational material, with some areas of fill soil. The formational soils can likely be retained using standard trench shields. Some caving could occur within the fill soils as well as within zones of clean sand within the more granular formations. These sections will likely require continuous shoring to retain the soil.

South of the Morena Boulevard/Ingulf Street intersection, shoring will need to retain potentially loose and granular soil, with possible cobbles and boulders, and will extend below the groundwater level in some reaches of the pipeline alignments. Where cobbles and boulders are not present, sheet piles would likely be the most appropriate shoring method due to its relatively water-tight seal (with the benefit of reduced dewatering demands) and ease of installation in loose and soft soils. However, oversize rock could cause refusal to sheet pile installation. Where oversize rock is encountered or suspected, or where denser materials may be present within the excavation depths, pre-augering can be used in advance of sheet pile installation to disturb the ground and potentially remove obstructions. Sheet piles are typically driven or vibrated into place; settlement of nearby improvements due to vibrations should be considered and monitoring may be required.

In areas of denser ground, deeper excavations, or significant zones of oversize rock, other shoring methods should be considered. Driven or drilled soldier piles and lagging, with internal bracing or tiebacks as needed, would likely be an appropriate shoring method, although

additional dewatering would be required. Other water-tight walls such as secant pile or tangent walls, while costly, could also be considered.

Trench box or braced shoring should be designed to resist at-rest soil pressures. An equivalent fluid pressure (EFP) of 60 pounds per cubic foot (pcf) may be assumed for at-rest lateral pressures above the water table. Below the water table, an EFP of 29 pcf and 30 pcf may be assumed for fill/alluvium and formational soils, respectively. A hydrostatic pressure of 62.4 pcf should be assumed below the water table if construction dewatering is not employed (i.e. use of sumps to control groundwater in the bottom of the trenches).

Lateral forces may be resisted by passive resistance. An EFP of 240 pcf, and 260 pcf may be assumed for fill/alluvium and formational soils above the water table, respectively. An EFP of 115 pcf, and 130 pcf may be assumed for fill/alluvium and formational soils below the water table, respectively.

5.3 EARTHWORK

Earthwork for the pipelines is expected to consist of trench excavation, placement of the pipe bedding and pipe, trench backfill, and replacement of the pavement section or other existing surface. Earthwork should be completed according to the applicable and most recent versions of the City of San Diego Grading requirements, the California Building Code and the Standard Specifications for Public Works Construction.

5.3.1 Trench Backfill Materials

Soil to be used as trench backfill outside of the pipe bedding zone should meet the criteria listed below:

- No oversize materials greater than 6 inches in maximum dimension.
- An Expansion Index (EI) less than 30 and a Plasticity Index less than 25.
- A relatively well-graded particle size distribution with at least 60 percent (by weight) passing a 1-inch sieve and fines content (percent, by weight, passing the No. 200 sieve) not exceeding 30 percent.

Most of the soil excavated from the trenches should meet the above requirements, although north of Ingulf Street, some of the native formational material may need to be screened to remove oversize material.

Bedding within the pipe bedding zone (12 inches below pipes to 6 inches above the pipes) should consist of clean sand in accordance with the Green Book. Class 2 Aggregate Base will be required within roadway sections. The Geotechnical Engineer should review and test all import sources.

Geotechnical laboratory R-value testing for the project is not complete. Pavement restoration above the pipe trenches will be addressed in an update to this report.

5.3.2 Excavation and Disposal Considerations

Prior to excavating trenches, any existing pavements, structures, abandoned utilities and improvements, vegetation, and other debris and rubble should be removed and disposed of offsite. Soils containing organic matter should be removed from the planned development area.

Following site preparation, excavated soil should be considered for re-use as trench backfill. See Section 5.3.1 for fill material requirements. Soil below the groundwater level will likely be too wet for use as backfill (even if the trench area has been dewatered) unless it can be spread out and dried before re-use, which may require a large processing area. Material that is suitable for re-use will require an area for stockpiling. Excess soil will need to be exported or stored for future use.

5.3.3 Fill Placement and Compaction

Fill soils should be placed in loose horizontal lifts that do not exceed eight inches, or as needed to obtain adequate compaction. Each layer should be spread evenly and thoroughly mixed during spreading to uniformly obtain a moisture content that is above the optimum moisture content for compaction. After each layer has been mixed and spread, it should be compacted to a relative compaction of not less than 90 percent. Relative compaction is defined as the ratio (expressed in percent) of the in-place dry unit weight of the compacted fill divided by the maximum dry unit weight evaluated in accordance with ASTM D1557.

Within paved areas, the upper 12 inches of backfill (below the Class 2 Aggregate base and pavement) should be compacted to a minimum of 95 percent relative compaction. Class 2 Aggregate base for roadway pavement should be compacted to a minimum of 95 percent relative compaction.

5.4 TRENCHLESS INSTALLATION

We understand that several sections of the pipeline alignments for the influent and effluent connections to the existing sewer main south of the MPS along Friars Road may be installed using trenchless construction techniques (tunneling methods). These portions of the pipelines would be installed below the existing groundwater level in the area in loose alluvial soils. Due to the subsurface conditions, the use of an open-face tunneling method may not be appropriate. Below the groundwater, the Tunnelman's Ground Classification for Soils may be considered to be flowing sands. Above the groundwater level, the Tunnelman's Ground Classification for Soils may be considered to be running or slow raveling sands. The use of pressurized face microtunneling techniques are recommended for tunnels in this portion of the project.

6.0 GENERAL CONDITIONS AND LIMITATIONS

Geotechnical engineering and the geological sciences require interpretations based on limited subsurface data. A relatively small portion of the pertinent soil and groundwater conditions along the planned Project route have been observed. The recommendations made herein are based on the assumption that soil and groundwater conditions will not deviate appreciably from those found during our current field investigation and previous explorations. Professional judgment discussed herein is based on an understanding of the planned construction and partly on general experience. Actual subsurface conditions encountered during construction will likely vary from those discussed in this report. If variations in subsurface conditions are encountered during construction, AECOM should be consulted for further recommendations.

This report is intended for design purposes. If reviewed by building contractors they should make their own interpretation of the data contained and referenced in this report.

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7.0 REFERENCES

- Allied Geotechnical Services, Inc., 2015. “Geotechnical Desktop Study Report, Pure Water Program Task 7, Morena Pump Station, WW Force Main and Brine Conveyance Pre-Design (NC05), City of San Diego,” dated December 14, 2015.
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- Ishihara, K., and Yoshimine, M., 1992. “Evaluation of settlements in sand deposits following liquefaction during earthquakes,” Soils and Foundations, Vol. 32, No. 1, pp. 173–88.
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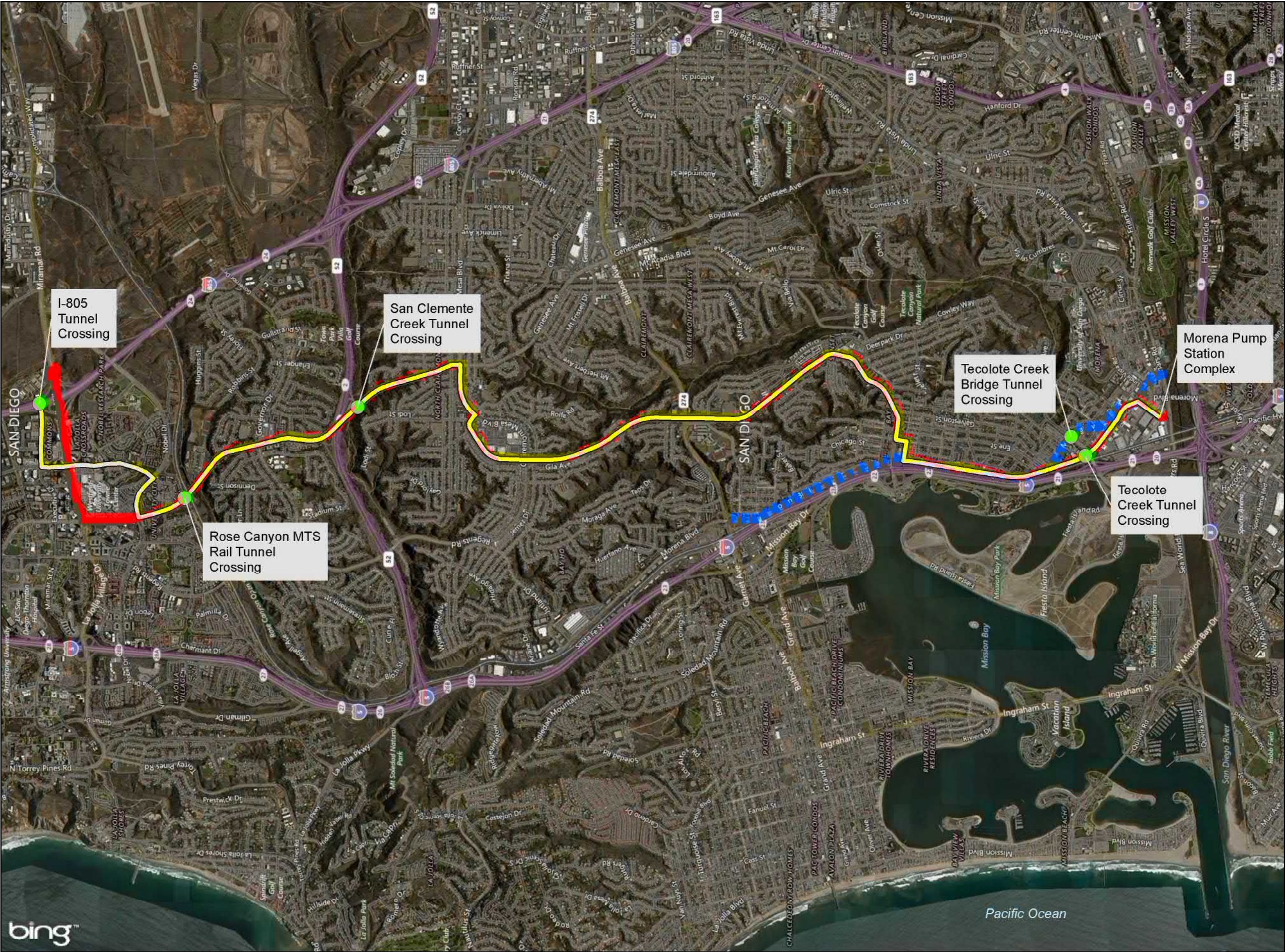
FIGURES

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Figure 1
Alignment Map
Morena Pipeline Project
San Diego, CA

Alignments

- City Alternative No. 1a (20151123)
- City Alternative No. 2 (20151123)
- Alvarado Morena Pipeline
- Tunnel Crossing



0 1,750 3,500
1:42,000 1 inch = 3,500 feet






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bing™



Reference: KEH 30% Design 4-7-17

LEGEND

-  SH-01 Cone Penetration Test (CPT), 2017
-  PS-01 Exploratory Boring, 2017
-  PS-02 Exploratory Boring with Monitoring Well, 2017



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20 0 20 40
SCALE: 1" = 40'

MORENA PUMP STATION SITE PLAN MORENA PIPELINE PROJECT SAN DIEGO, CALIFORNIA

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PM: JL	PROJ. NO: 60530732	2

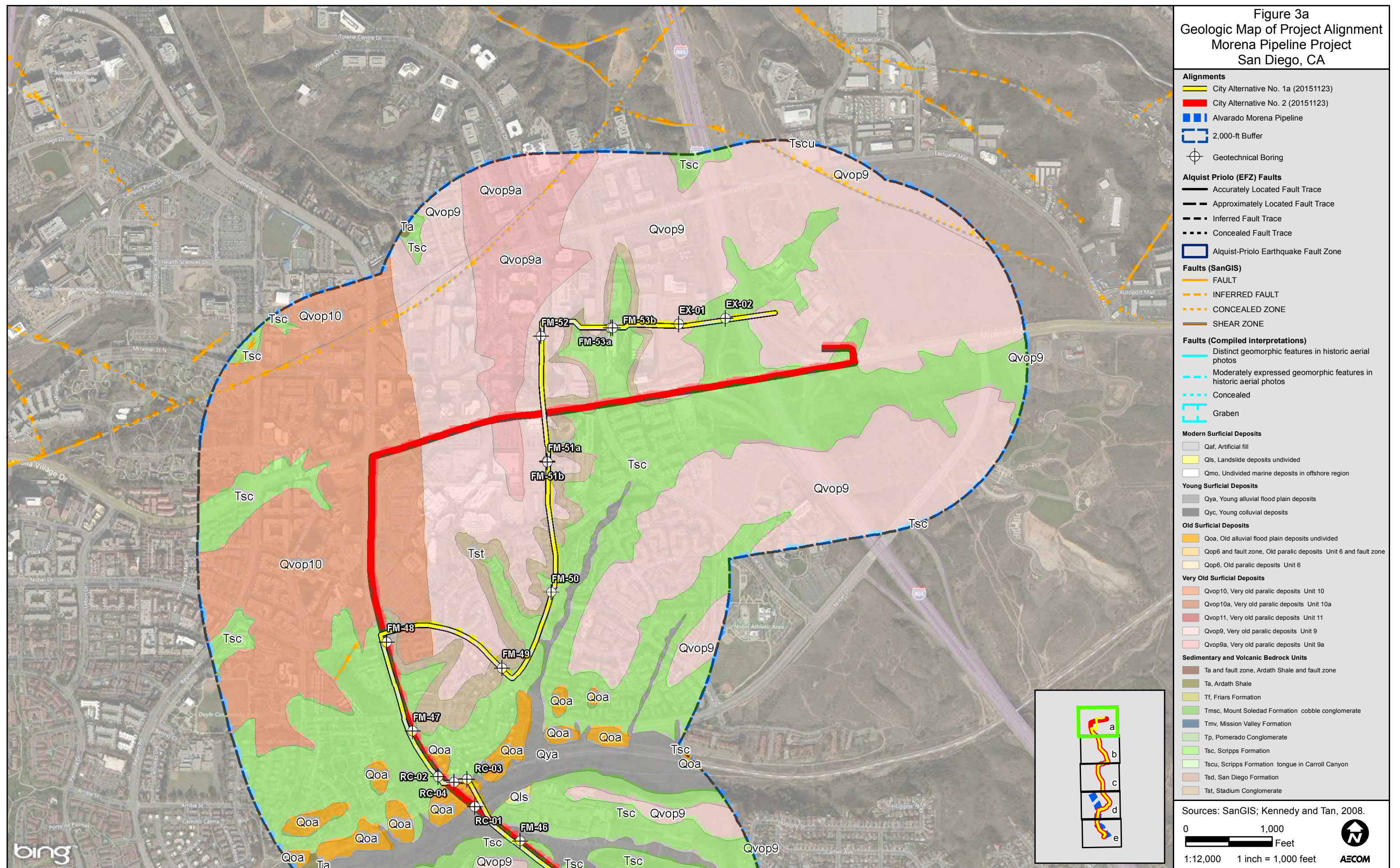
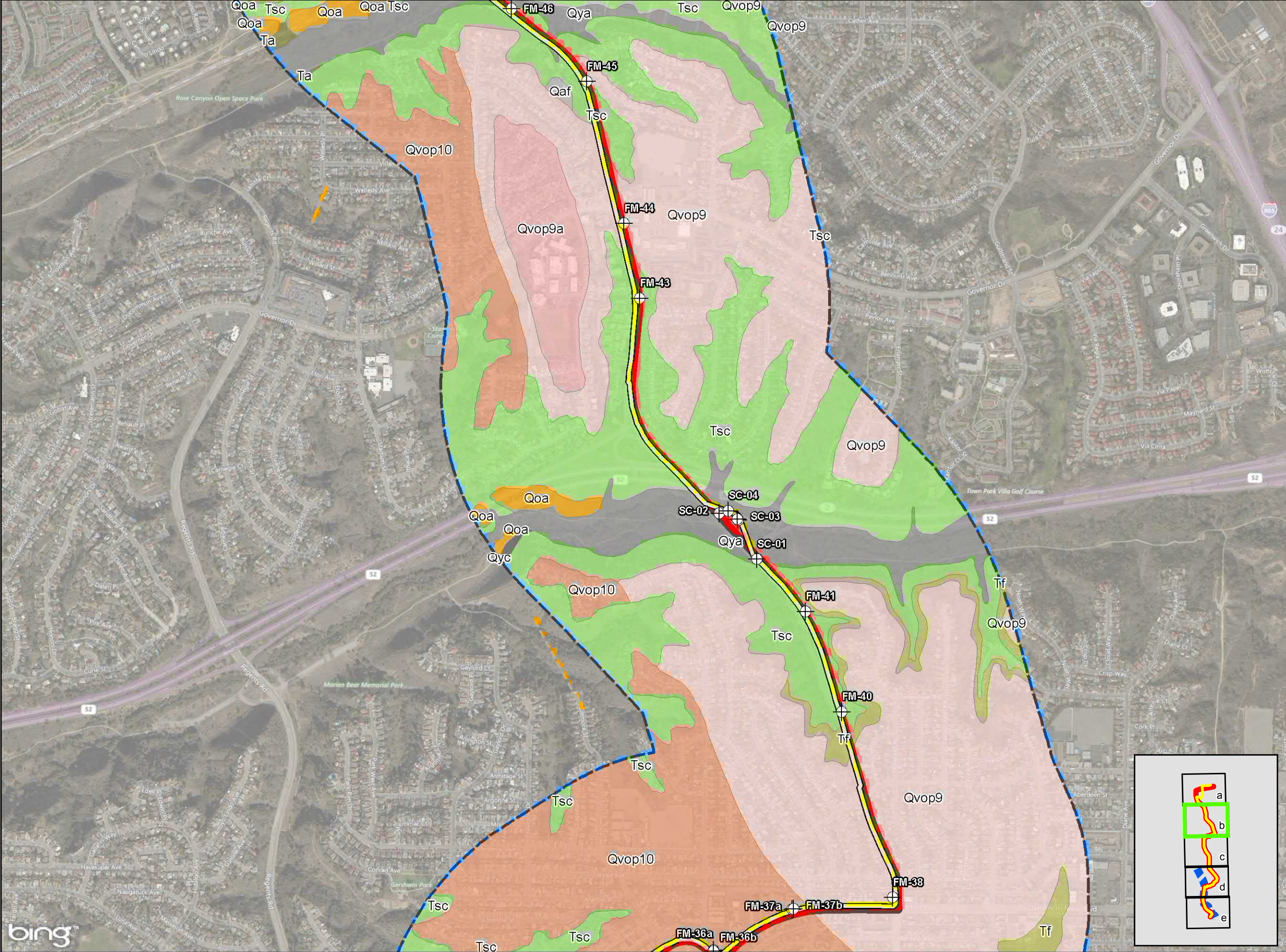


Figure 3b
Geologic Map of Project Alignment
Morena Pipeline Project
San Diego, CA



Alignments

- City Alternative No. 1a (20151123)
- City Alternative No. 2 (20151123)
- Alvarado Morena Pipeline
- 2,000-ft Buffer
- Geotechnical Boring

Alquist Priolo (EFZ) Faults

- Accurately Located Fault Trace
- Approximately Located Fault Trace
- Inferred Fault Trace
- Concealed Fault Trace
- Alquist-Priolo Earthquake Fault Zone

Faults (SanGIS)

- FAULT
- INFERRED FAULT
- CONCEALED ZONE
- SHEAR ZONE

Faults (Compiled interpretations)

- Distinct geomorphic features in historic aerial photos
- Moderately expressed geomorphic features in historic aerial photos
- Concealed
- Graben

Modern Surficial Deposits

- Qaf, Artificial fill
- Qls, Landslide deposits undivided
- Qmo, Undivided marine deposits in offshore region

Young Surficial Deposits

- Qya, Young alluvial flood plain deposits
- Qyc, Young colluvial deposits

Old Surficial Deposits

- Qoa, Old alluvial flood plain deposits undivided
- Qop6 and fault zone, Old paralic deposits Unit 6 and fault zone
- Qop6, Old paralic deposits Unit 6

Very Old Surficial Deposits

- Qvop10, Very old paralic deposits Unit 10
- Qvop10a, Very old paralic deposits Unit 10a
- Qvop11, Very old paralic deposits Unit 11
- Qvop9, Very old paralic deposits Unit 9
- Qvop9a, Very old paralic deposits Unit 9a

Sedimentary and Volcanic Bedrock Units

- Ta and fault zone, Ardath Shale and fault zone
- Ta, Ardath Shale
- Tf, Friars Formation
- Tmso, Mount Soledad Formation cobble conglomerate
- Tmv, Mission Valley Formation
- Tp, Pomerado Conglomerate
- Tsc, Scripps Formation
- Tscu, Scripps Formation tongue in Carroll Canyon
- Tsd, San Diego Formation
- Tst, Stadium Conglomerate

Sources: SanGIS; Kennedy and Tan, 2008.

0 1,000 Feet

1:12,000 1 inch = 1,000 feet


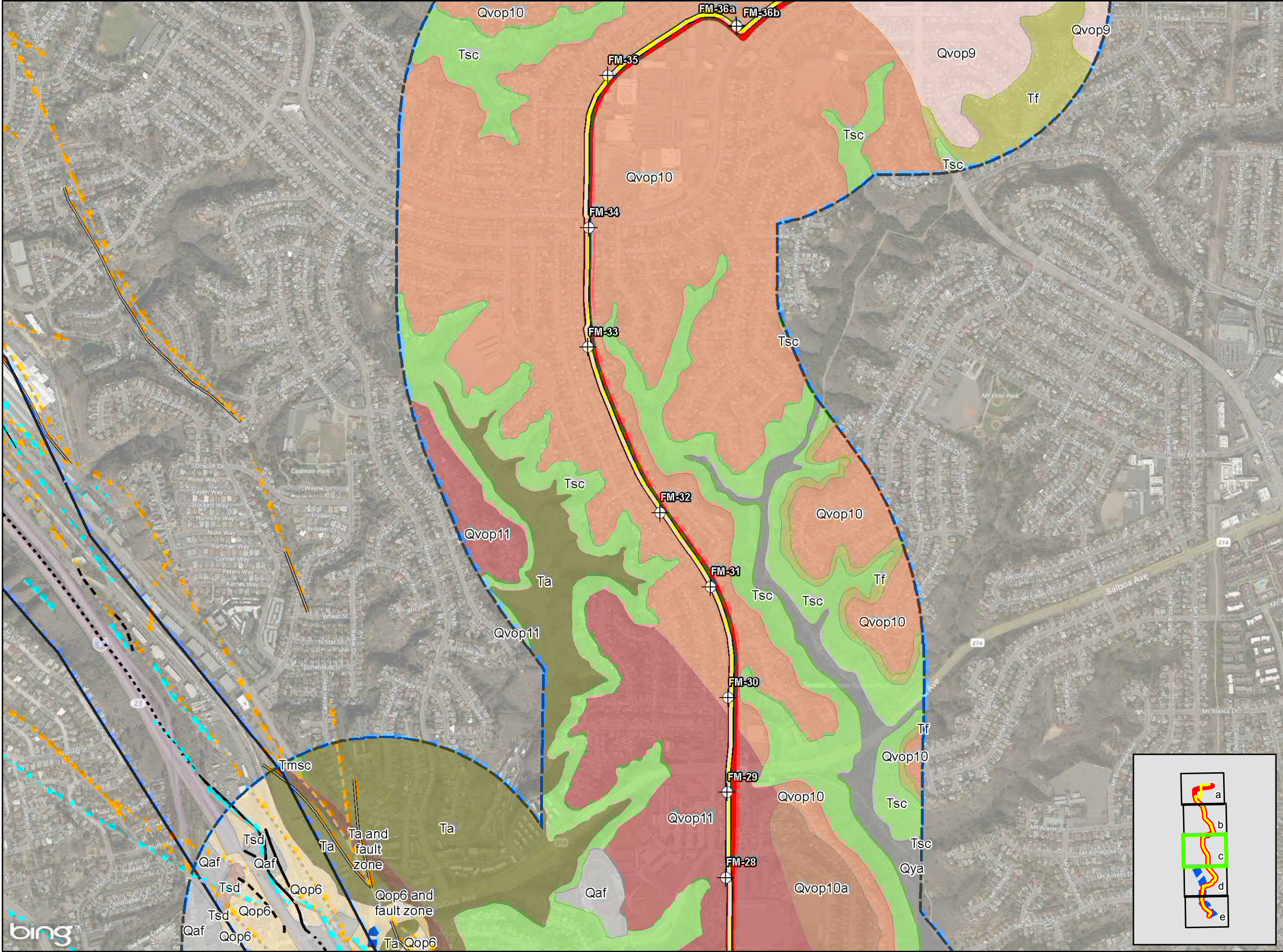

AECOM

Figure 3c
Geologic Map of Project Alignment
Morena Pipeline Project
San Diego, CA



Alignments

- City Alternative No. 1a (20151123)
- City Alternative No. 2 (20151123)
- Alvarado Morena Pipeline
- 2,000-ft Buffer
- Geotechnical Boring

Alquist Priolo (EFZ) Faults

- Accurately Located Fault Trace
- Approximately Located Fault Trace
- Inferred Fault Trace
- Concealed Fault Trace
- Alquist-Priolo Earthquake Fault Zone

Faults (SanGIS)

- FAULT
- INFERRED FAULT
- CONCEALED ZONE
- SHEAR ZONE

Faults (Compiled interpretations)

- Distinct geomorphic features in historic aerial photos
- Moderately expressed geomorphic features in historic aerial photos
- Concealed
- Graben

Modern Surficial Deposits

- Qaf, Artificial fill
- Qls, Landslide deposits undivided
- Qmo, Undivided marine deposits in offshore region

Young Surficial Deposits

- Qya, Young alluvial flood plain deposits
- Qyc, Young colluvial deposits

Old Surficial Deposits

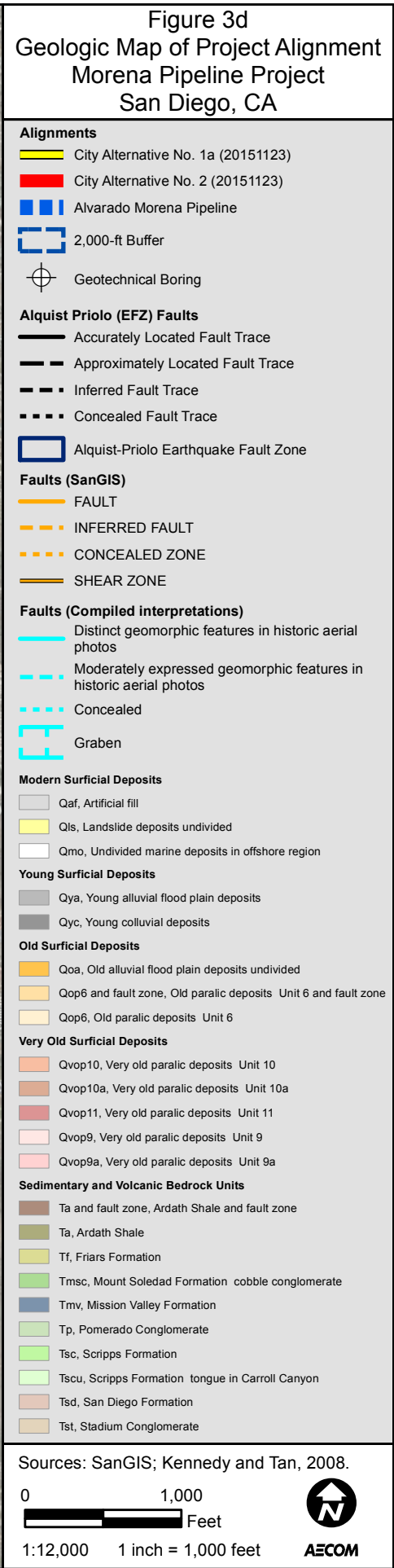
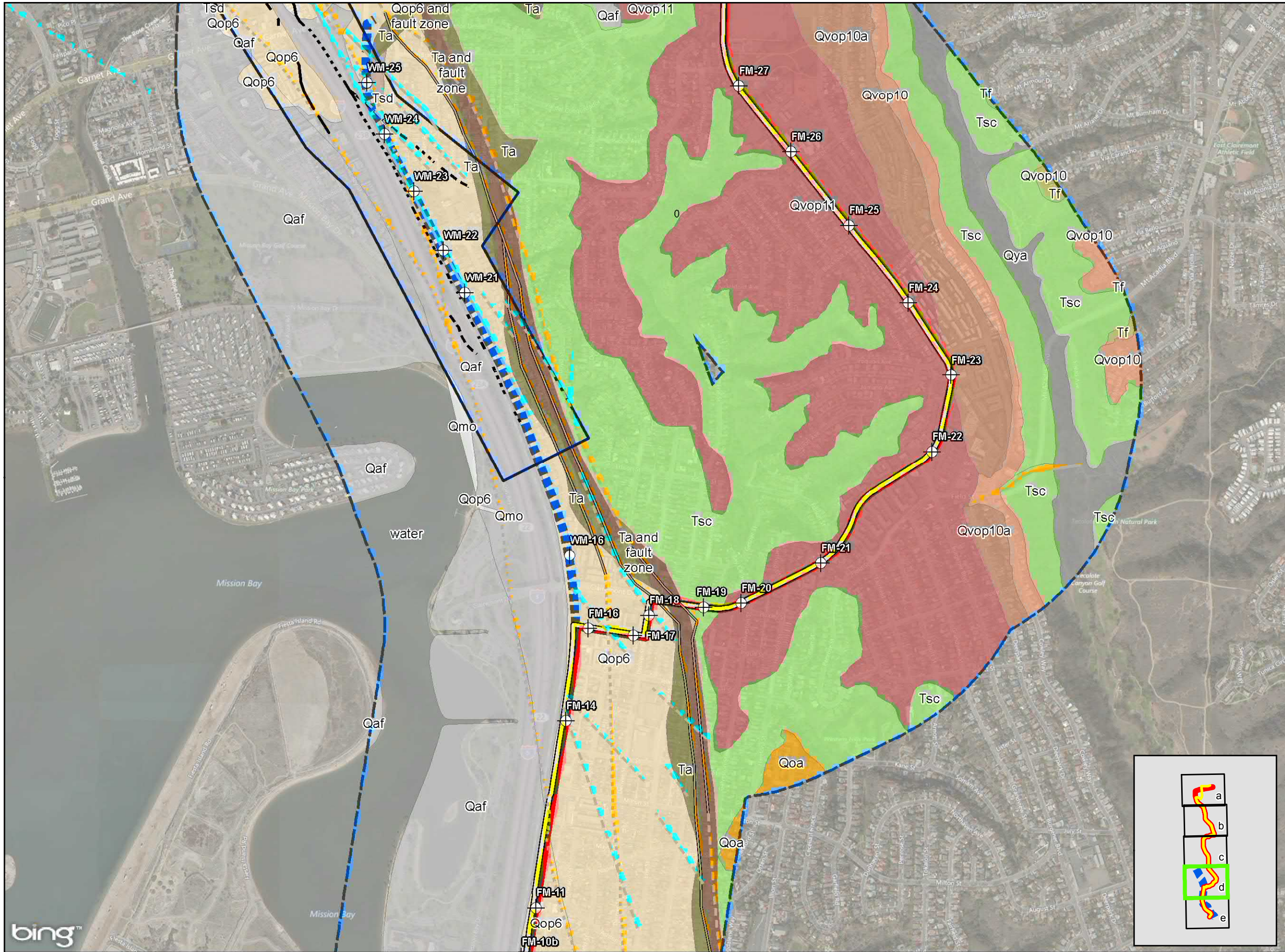
- Qoa, Old alluvial flood plain deposits undivided
- Qop6 and fault zone, Old paralic deposits Unit 6 and fault zone
- Qop6, Old paralic deposits Unit 6

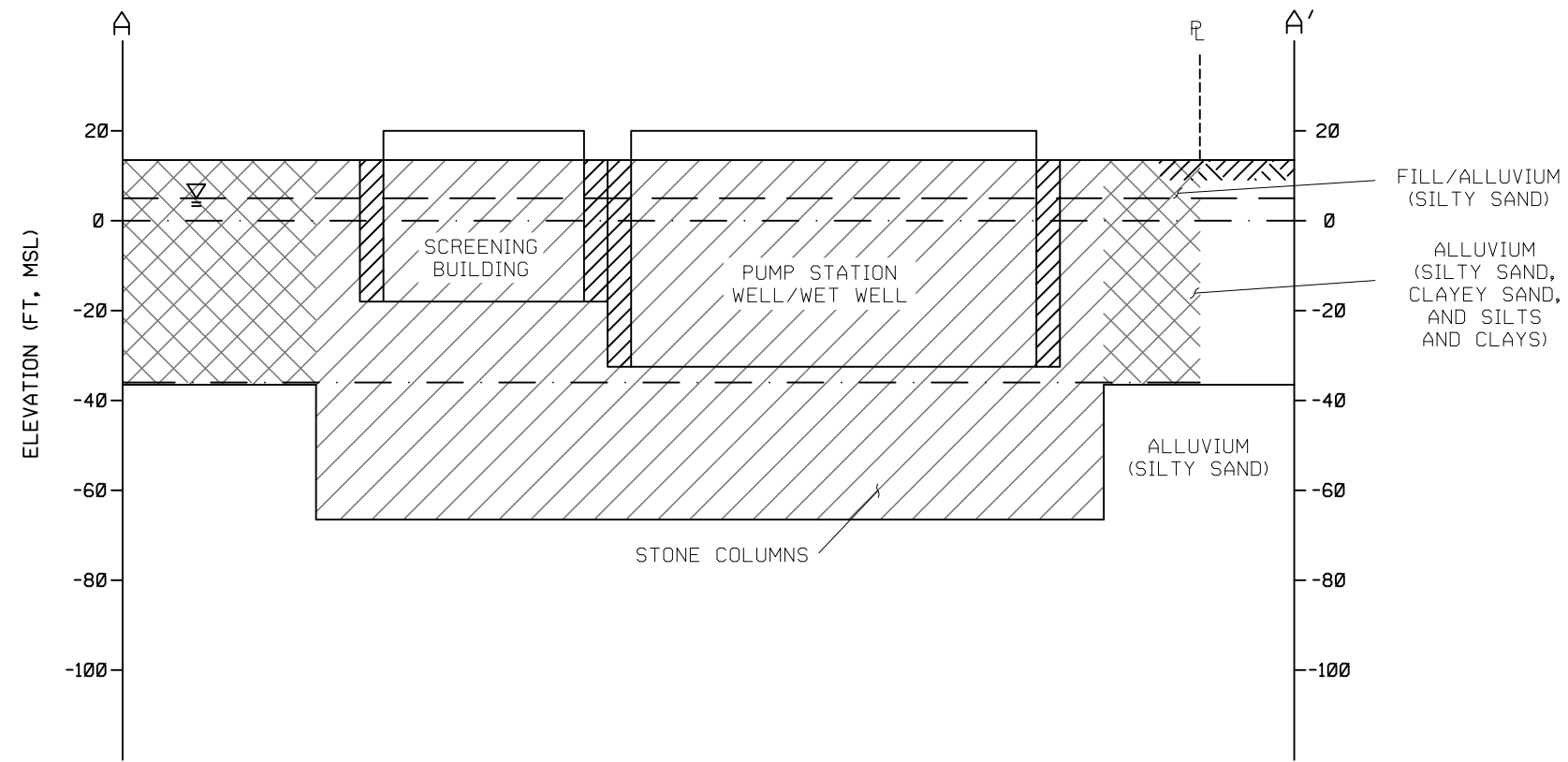
Very Old Surficial Deposits

- Qvop10, Very old paralic deposits Unit 10
- Qvop10a, Very old paralic deposits Unit 10a
- Qvop11, Very old paralic deposits Unit 11
- Qvop9, Very old paralic deposits Unit 9
- Qvop9a, Very old paralic deposits Unit 9a

Sedimentary and Volcanic Bedrock Units

- Ta and fault zone, Ardat Shale and fault zone
- Ta, Ardat Shale
- Tf, Friars Formation
- Tmsc, Mount Soledad Formation cobble conglomerate
- Tmv, Mission Valley Formation
- Tp, Pomerado Conglomerate
- Tsc, Scripps Formation
- Tscu, Scripps Formation tongue in Carroll Canyon
- Tsd, San Diego Formation
- Tst, Stadium Conglomerate



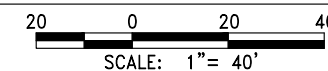


LEGEND

— · — · — · —	GEOLOGIC CONTACT
	STONE COLUMNS TO 50 FEET BELOW GROUND SURFACE
	STONE COLUMNS TO 80 FEET BELOW GROUND SURFACE



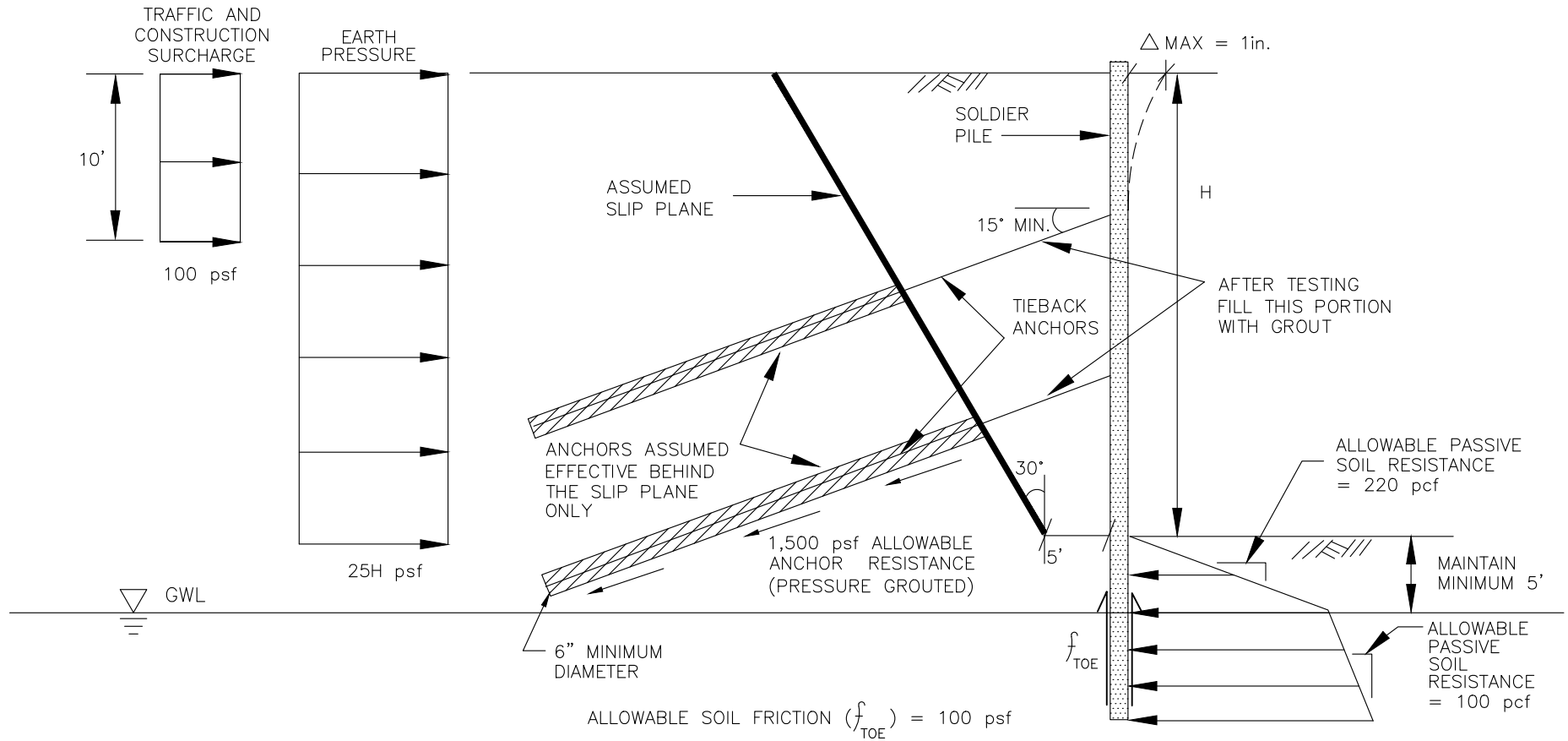
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CROSS SECTION A-A MORENA PIPELINE PROJECT SAN DIEGO, CALIFORNIA

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NOTES:

- ASSUMES LEVEL BACKFILL AND NO HYDROSTATIC PRESSURE.
- FOR PRELIMINARY DESIGN.
- GWL — DESIGN GROUNDWATER LEVEL DURING CONSTRUCTION PER GEOTECHNICAL REPORT (5' BELOW BOTTOM OF EXCAVATION).

**LATERAL EARTH PRESSURES AND RESISTANCES
FOR TEMPORARY TIEBACK WALLS (0' - 30' bgs)
MORENA PIPELINE PROJECT
SAN DIEGO, CALIFORNIA**

AECOM

NOT TO SCALE

CHECKED BY: PB

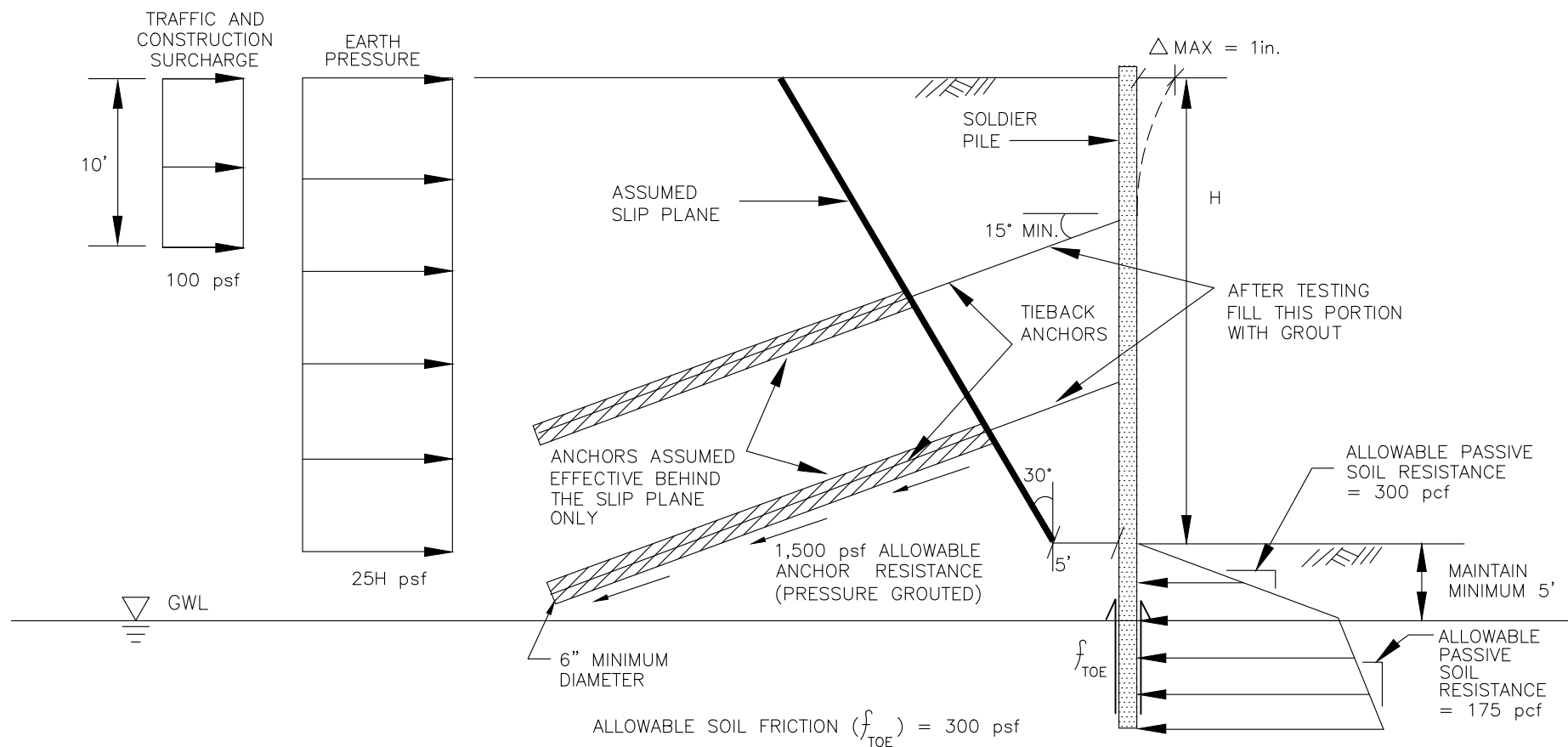
DATE: 5-17-17

FIG. NO:

PM: JL

PROJ. NO: 60530732

5



NOTES:

- ASSUMES LEVEL BACKFILL AND NO HYDROSTATIC PRESSURE.
- FOR PRELIMINARY DESIGN.
- GWL — DESIGN GROUNDWATER LEVEL DURING CONSTRUCTION PER GEOTECHNICAL REPORT (5' BELOW BOTTOM OF EXCAVATION).

**LATERAL EARTH PRESSURES AND RESISTANCES
FOR TEMPORARY TIEBACK WALLS (0' - 50' bgs)
MORENA PIPELINE PROJECT
SAN DIEGO, CALIFORNIA**

AECOM

NOT TO SCALE

CHECKED BY: PB

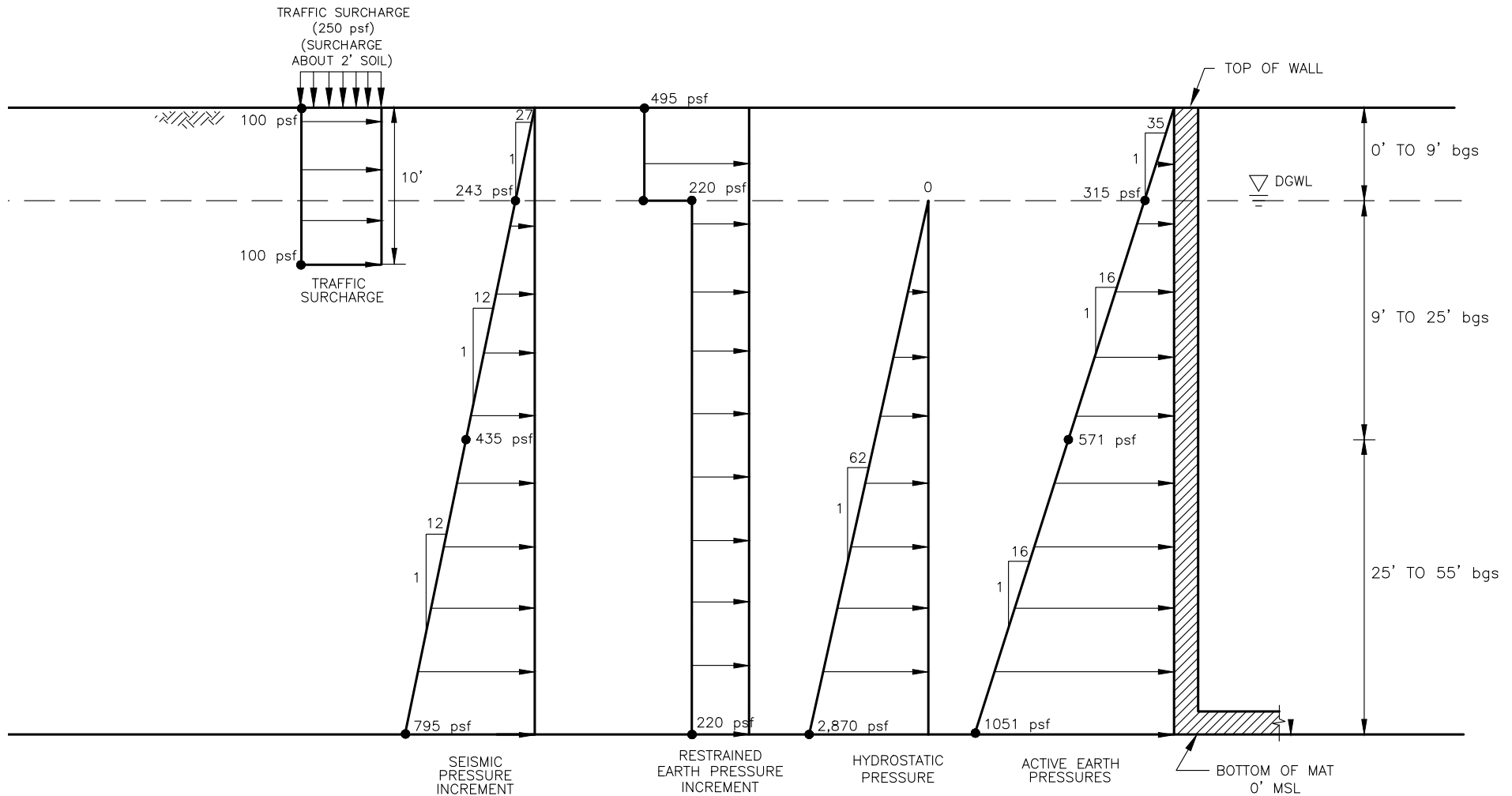
DATE: 5-17-17

FIG. NO:

PM: JL

PROJ. NO: 60530732

6



NOTES:

1. DGWL: DESIGN GROUNDWATER LEVEL PER GEOTECHNICAL REPORT
2. STATIC AT REST PRESSURE = ACTIVE EARTH PRESSURE + RESTRAINED EARTH PRESSURE INCREMENT
3. SEISMIC PRESSURE = ACTIVE EARTH PRESSURE + SEISMIC PRESSURE INCREMENT

**LATERAL EARTH PRESSURES FOR PERMANENT
SUBTERRANEAN WALL
MORENA PIPELINE PROJECT
SAN DIEGO, CALIFORNIA**

AECOM

NOT TO SCALE

CHECKED BY: PB

DATE: 5-17-17

FIG. NO:

PM: JL

PROJ. NO: 60530732

7

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TABLES

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Table 1
Summary of Geotechnical Conditions Along Cut & Cover Pipeline Alignment

Location	Soils/Formations	Groundwater Depth (feet)	Geologic Hazards	Construction Considerations
Friars Rd. between the I-5 and Napa St.	Fill over deep alluvium	7 to 9 feet	Liquefaction, lateral spreading, flooding	Shallow groundwater, soft soils, shoring
South of Morena Pump Station	Fill over deep alluvium (assumed)	6 to 10 feet (estimated)	Liquefaction, lateral spreading	Shallow groundwater, soft soils, shoring
Morena Pump Station to Morena Blvd.	Fill over alluvium	9 to 10 feet (at MPS) becoming deeper (10 to 15 feet) to the north	Liquefaction	Shallow groundwater, soft soils, shoring
Morena Blvd. near Napa St. to West Morena Blvd. near Vega St.	Fill over Bay Point Formation	10 to 14 feet	Flooding	Shallow groundwater, gravel & possible cobble layer
West Morena Blvd. near Vega St. to Morena Blvd. near Kane St.	Shallow to deep fill over alluvium or Bay Point Formation	10 to 18 feet	Liquefaction, flooding	Shallow groundwater, soft soils, shoring, possible cementation/concretions
Ingulf St. between Morena Blvd. and Denver St.	Shallow fill over Bay Point Formation	19.5 feet (near Morena), otherwise greater than trench depths	-	-
Denver St. and Clairemont Dr. to near Galveston St.	Shallow to deep fill over Scripps/Ardath Formation, some with Lindavista over Scripps Formation	Greater than trench depths	-	Possible cementation/concretions, gravel
Clairemont Dr. near Clairemont Ct. to Clairemont Mesa Blvd. and Genessee Ave.	Shallow to no fill over Lindavista Formation	19.5 feet (near Lakehurst Ave), otherwise greater than trench depths	-	Possible cementation/concretions, cobbles & boulders
Genessee Ave. near Appleton St. to near Radcliffe Ln.	Deep fill, with some over Scripps Formation or alluvium	Greater than trench depths	-	Soft soils, shoring, possible cementation/concretions, cobbles & boulders
Genessee Ave. near Governor Dr. to near Centurion Square	Lindavista Formation, deep fill over alluvium, or shallow to deep fill over Scripps Formation	Greater than trench depths	-	Soft soils, shoring, possible cementation/concretions, cobbles & boulders
Genessee Ave. near Decoro St. to Towne Centre Dr. near Executive Dr.	Shallow to deep fill over Scripps Formation	Greater than trench depths	-	Possible cementation/concretions, cobbles & boulders, possible contamination (on Towne Centre Dr. near La Jolla Village Dr.)

Note:

- ~~Subsurface data is based on preliminary sources.~~

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APPENDIX A

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Key to Logs


















Sheet 1 of 1

Elevation, feet	Depth, feet	SAMPLES			Graphic Log	MATERIAL DESCRIPTION	Dry Density, pcf	Dry Density, pcf	REMARKS AND OTHER TESTS
		Type	Number	Blows per foot					
1	2	3	4	5	6	7	8	9	10





COLUMN DESCRIPTIONS

- | | |
|---|--|
| <p>1 Elevation: Elevation in feet referenced to NAVD88 or site datum.</p> <p>2 Depth: Depth in feet below the ground surface.</p> <p>3 Sample Type: Type of soil sample collected at depth interval shown; sampler symbols are explained below.</p> <p>4 Sample Number: Sample identification number.</p> <p>5 Blows per foot: Number of blows required to advance driven sampler 12 inches beyond first 6-inch interval, or distance noted, using a 140-lb hammer with a 30-inch drop.</p> <p>6 Graphic Log: Graphic depiction of subsurface material encountered; typical symbols are explained below.</p> <p>7 Material Description: Description of material encountered; may include relative density/consistency, moisture, color, particle size; texture, weathering, and strength of formation material. If shown, designation in parentheses denotes Munsell color classification.</p> | <p>8 Water Content: Water content of soil sample measured in laboratory, expressed as percentage of dry weight of specimen.</p> <p>9 Dry Unit Weight: Dry density of soil sample measured in laboratory, in pounds per cubic foot.</p> <p>10 Remarks and Other Tests: Comments and observations regarding drilling or sampling made by driller or field personnel.</p> <p>SA Sieve analysis, %<#200 sieve
 WA Three-point wash sieve, %<#200 sieve
 LL Liquid limit (from Atterberg limits test), %
 PI Plasticity Index [LL - PL], %; NP=nonplastic
 DS Direct Shear test
 CORR Corrosivity Test suite</p> |
|---|--|





TYPICAL MATERIAL GRAPHIC SYMBOLS

 FILL	 Lean CLAY (CL)	 Fat CLAY (CH)	 Silty CLAY (ML-CL)
 Clayey SILT (CL-ML)	 SILT (ML)	 Elastic SILT (MH)	 Clayey SAND (SC)
 Silty SAND (SM)	 Well-graded SAND with silt (SW-SM)	 Well-graded SAND (SW)	 Poorly-graded SAND with clay (SP-SC)
 Poorly-graded SAND with silt (SP-SM)	 Poorly-graded SAND (SP)	 Well-graded GRAVEL with silt (GW-GM)	 Poorly-graded GRAVEL with silt (GP-GM)
 Poorly-graded GRAVEL (GP)			

TYPICAL SAMPLER GRAPHIC SYMBOLS

 2.5" I.D. sampler	 Standard Penetration sampler
 Core Sample	 Bulk Sample

OTHER GRAPHIC SYMBOLS

-  First water encountered at time of drilling and sampling (ATD)
-  Water level measured at specified time after completion of drilling and sampling
-  General change in material
-  Inferred or gradational contact between strata

GENERAL NOTES

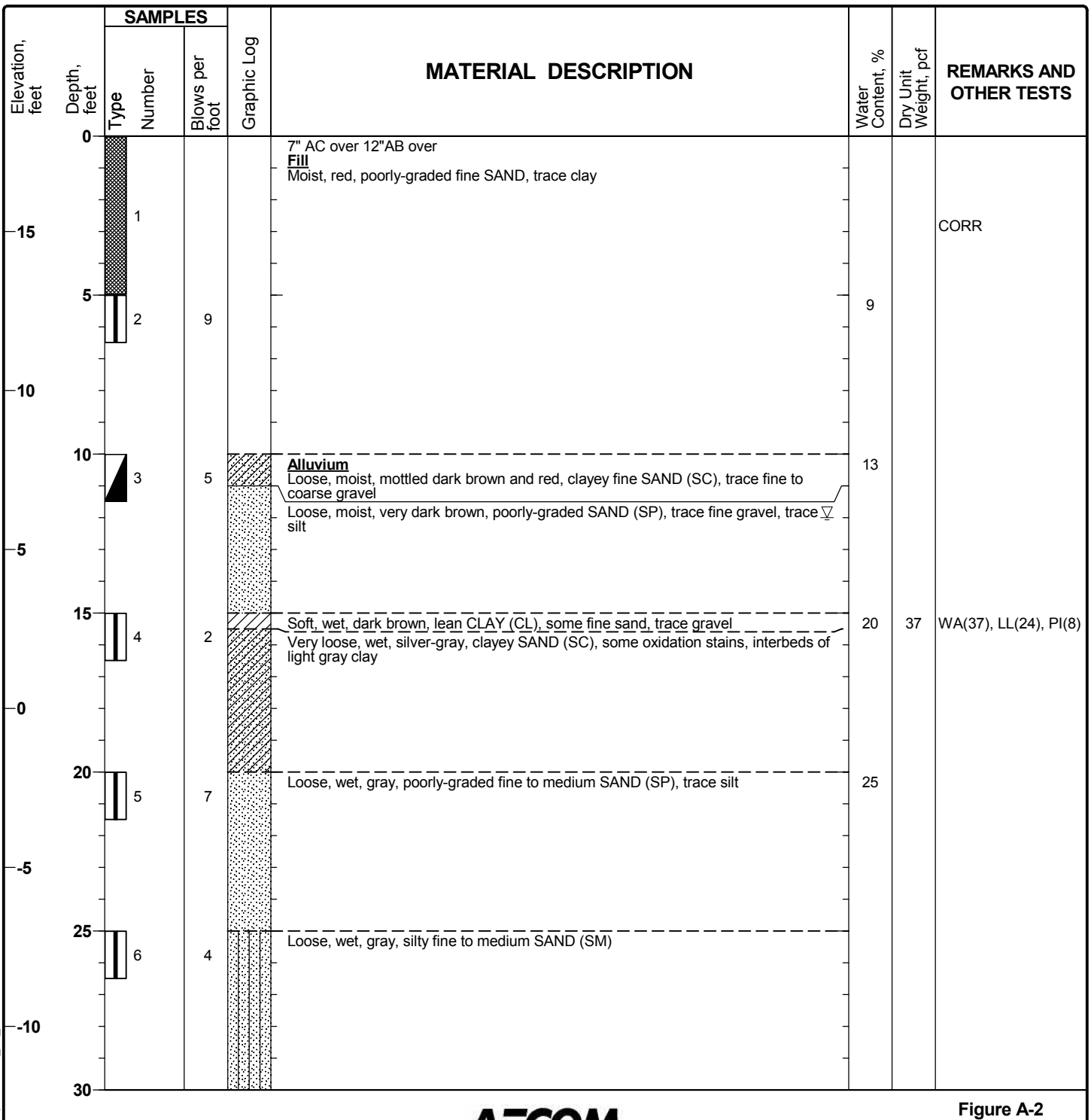
- Soil classifications are based on the Unified Soil Classification System. Descriptions and stratum lines are interpretive; actual lithologic changes may be gradual. Field descriptions may have been modified to reflect results of lab tests.
- Descriptions on these logs apply only at the specific boring locations and at the time the borings were advanced. They are not warranted to be representative of subsurface conditions at other locations or times.

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring DS-01

Sheet 1 of 2

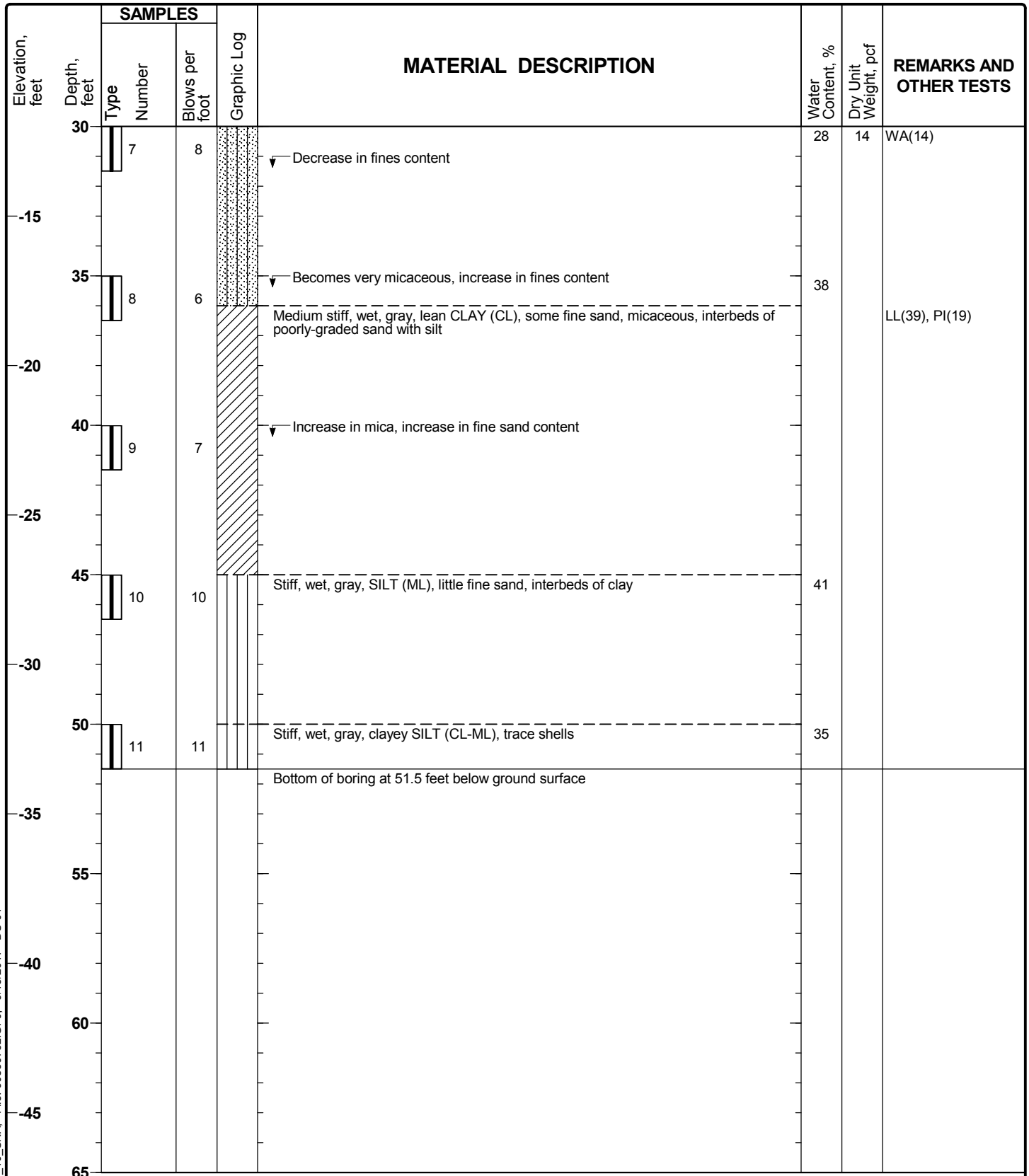
Date(s) Drilled	08/02/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	51.5 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	18 feet
Water Level Depth	12 ft	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cement/Bentonite Grout	Location	N32.763304, W117.195882		



Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring DS-01

Sheet 2 of 2



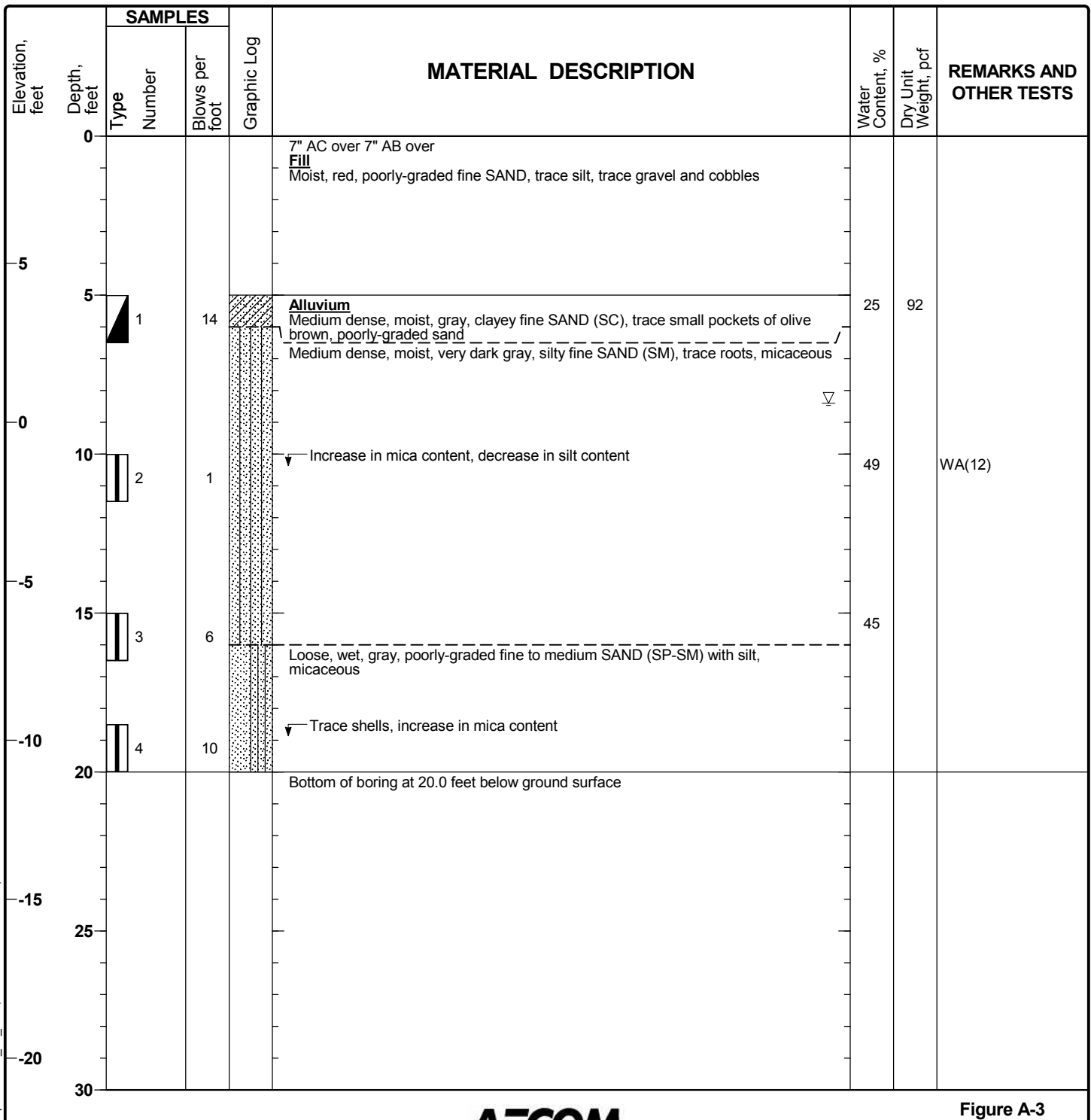
Report: GEO_10_SNA: File: 60530732.GPJ: 9/15/2017 DS-01

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring DS-02

Sheet 1 of 1

Date(s) Drilled	08/02/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	9 feet
Water Level Depth	8.4 ft	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.762757, W117.198883		

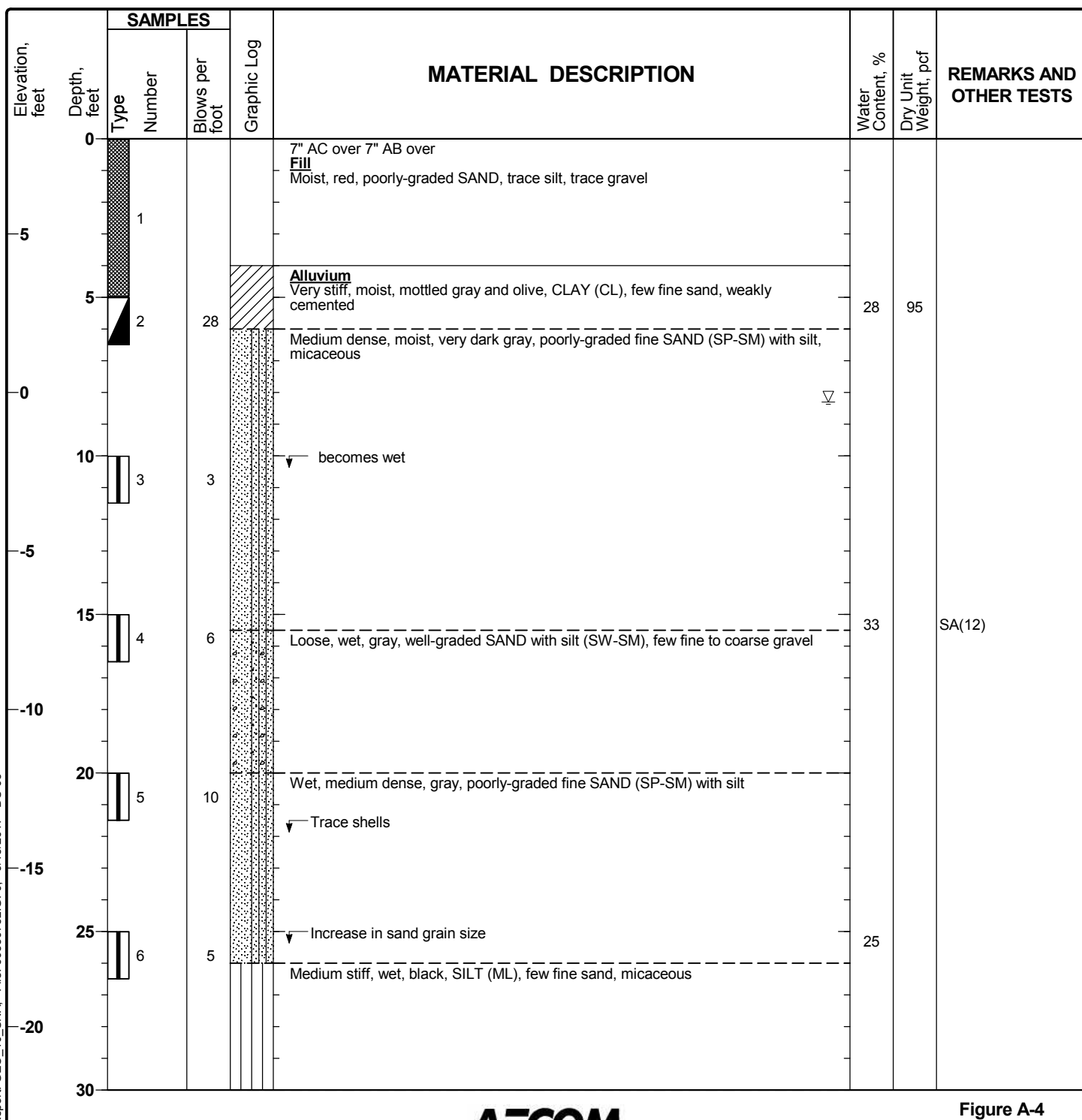


Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring DS-03

Sheet 1 of 2

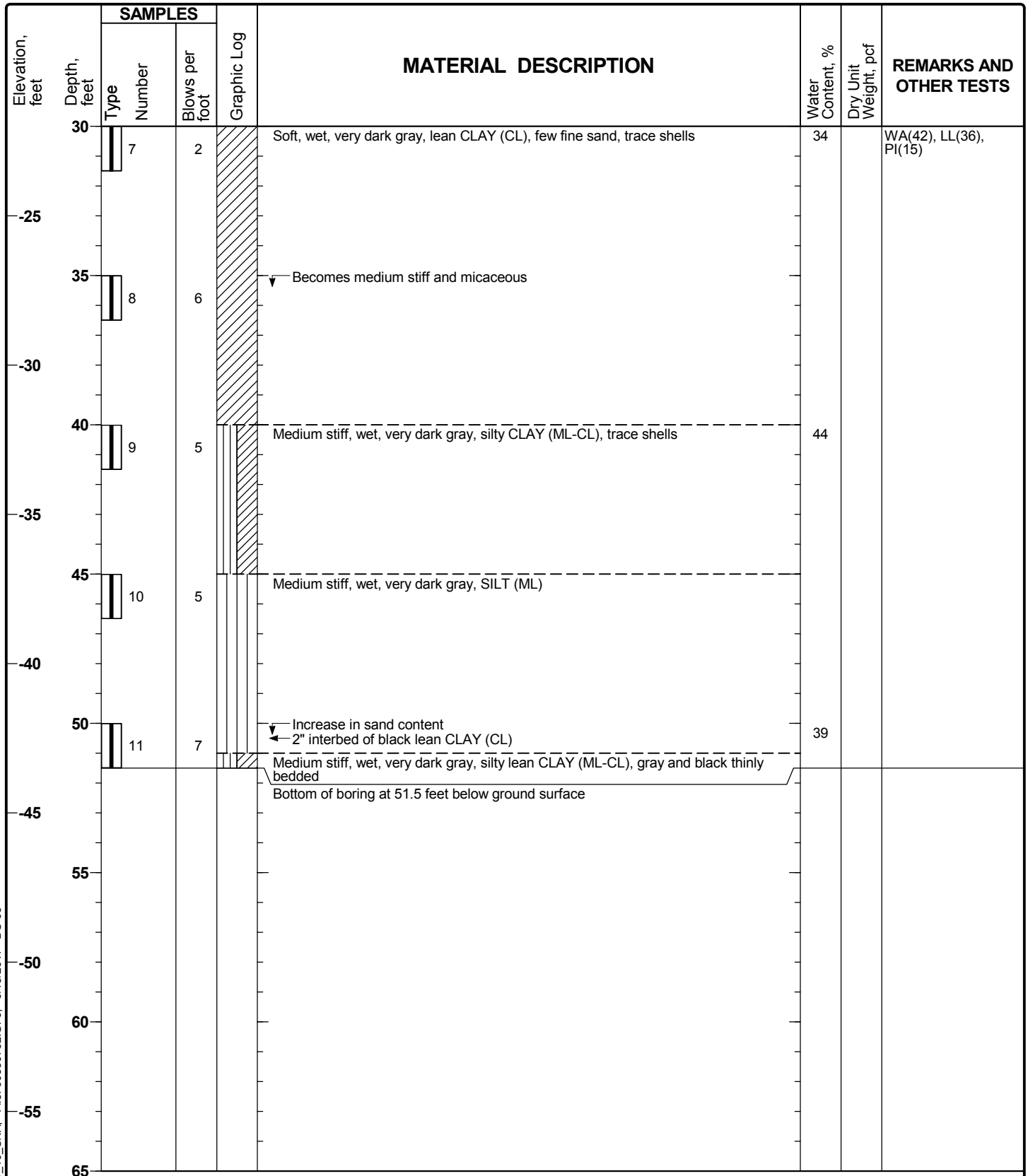
Date(s) Drilled	08/03/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	51.5 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	8 feet
Water Level Depth	8.3 ft	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cement/Bentonite Grout	Location	N32.76249, W117.201137		



Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring DS-03

Sheet 2 of 2



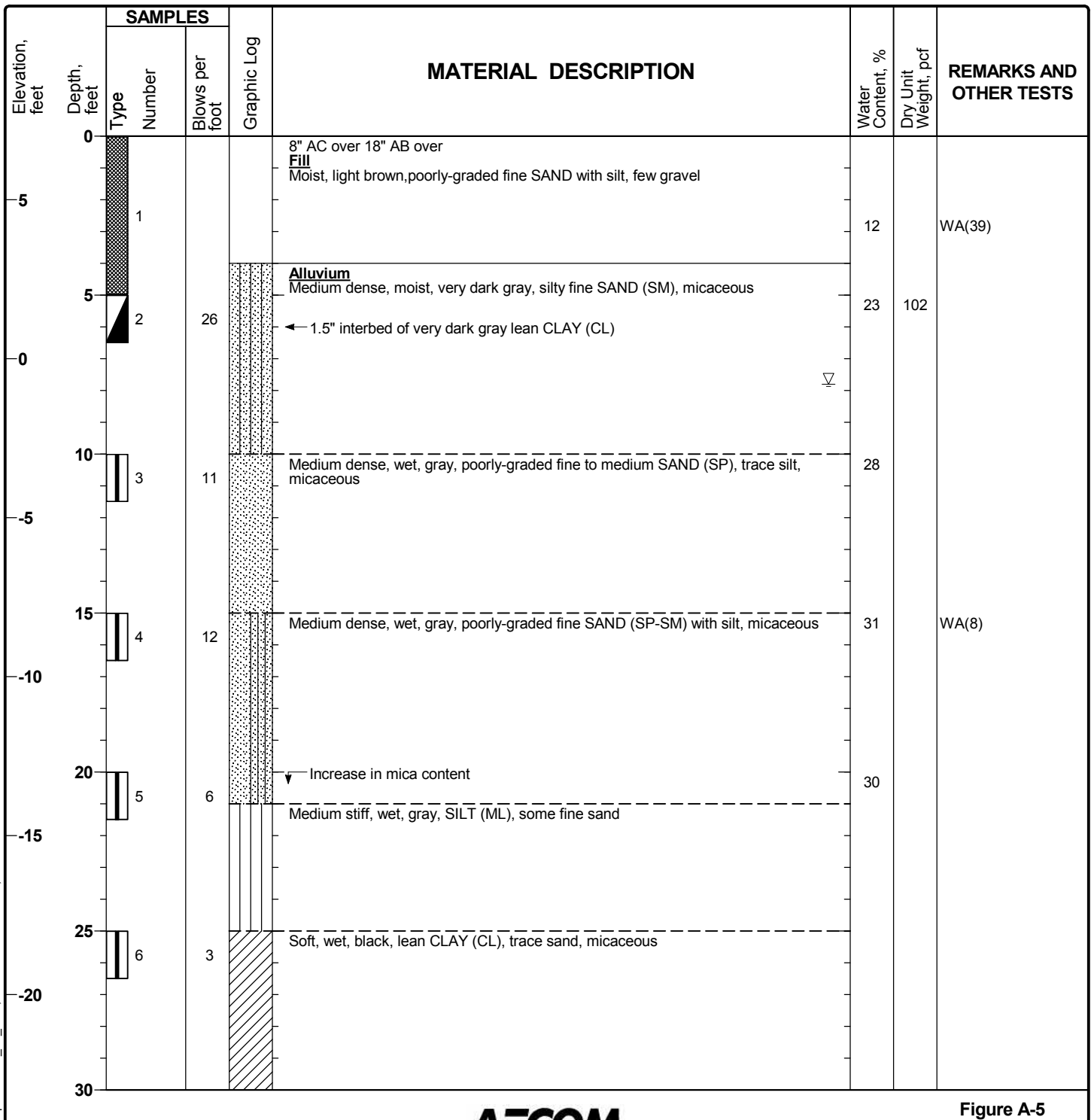
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Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring DS-06

Sheet 1 of 2

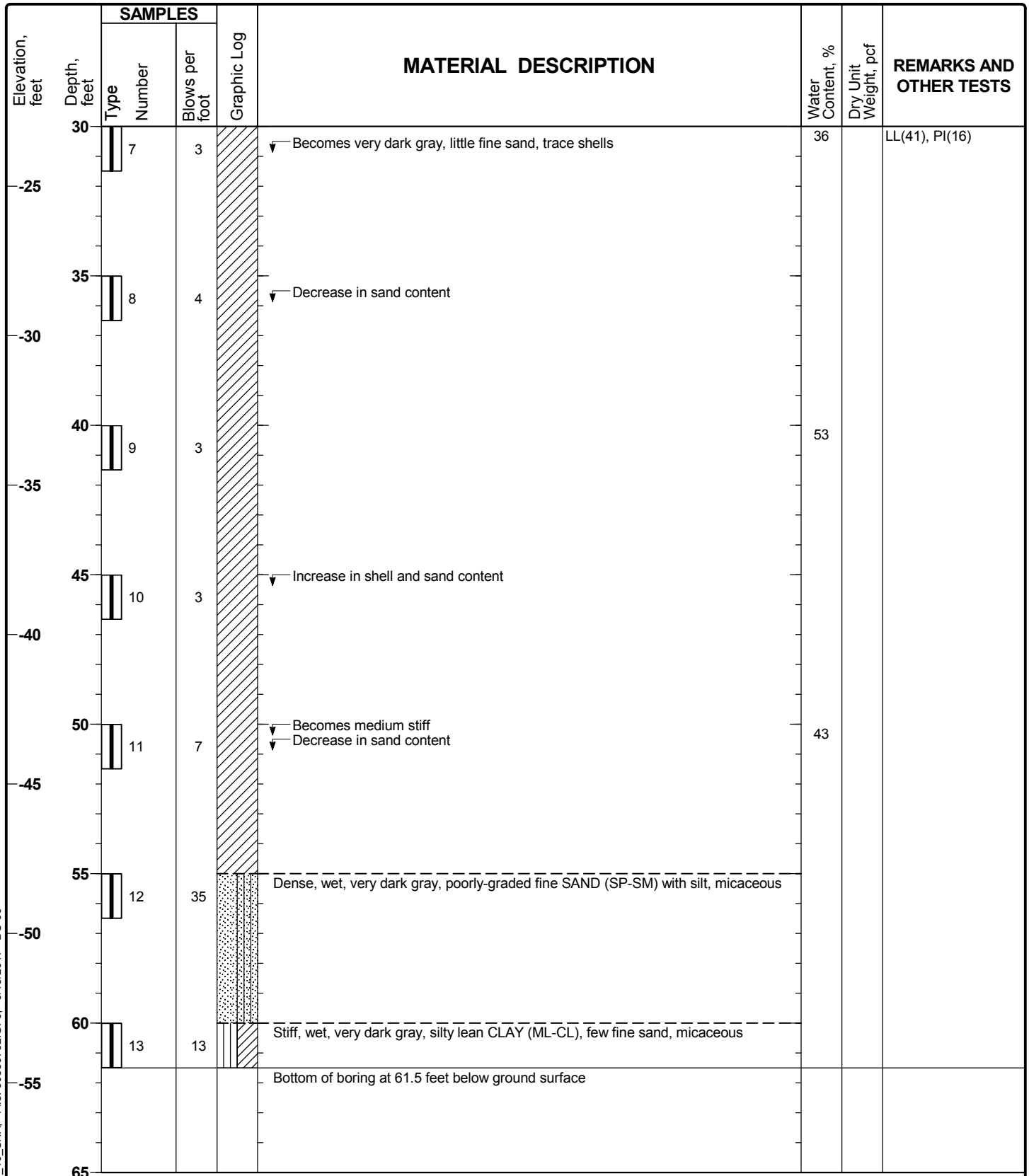
Date(s) Drilled	08/04/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	61.5 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	7 feet
Water Level Depth	7.8 ft	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cement/Bentonite Grout	Location	N32.76218, W117.20404		



Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring DS-06

Sheet 2 of 2



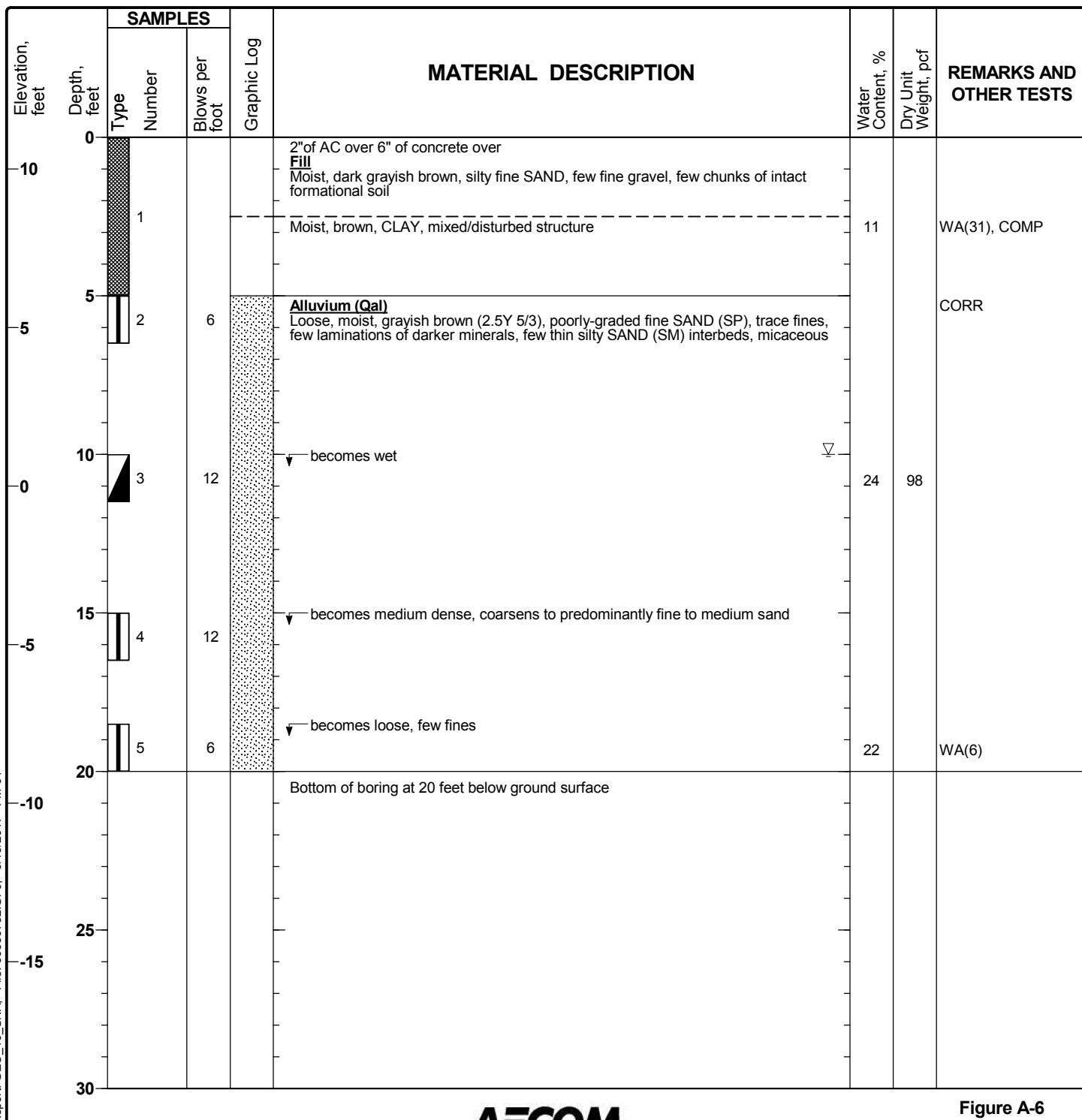
Report: GEO_10_SNA: File: 60530732.GPJ: 9/15/2017 DS-06

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-01

Sheet 1 of 1

Date(s) Drilled	04/27/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	11 feet
Water Level Depth	10 fbgs (during drilling)	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.76413, W117.19996		

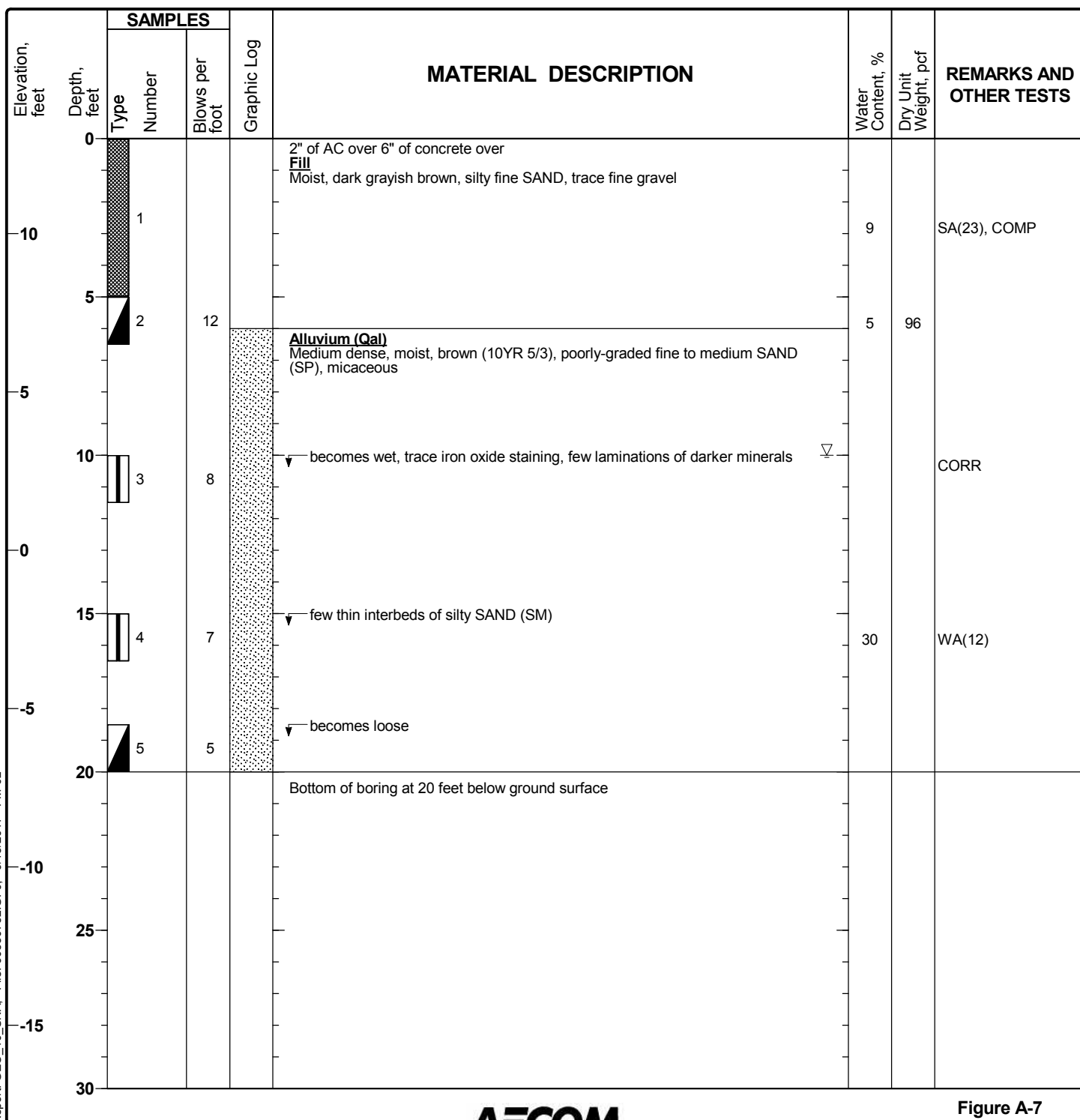


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-02

Sheet 1 of 1

Date(s) Drilled	04/27/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	13 feet
Water Level Depth	10 fbgs (during drilling)	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.76485, W117.19935		

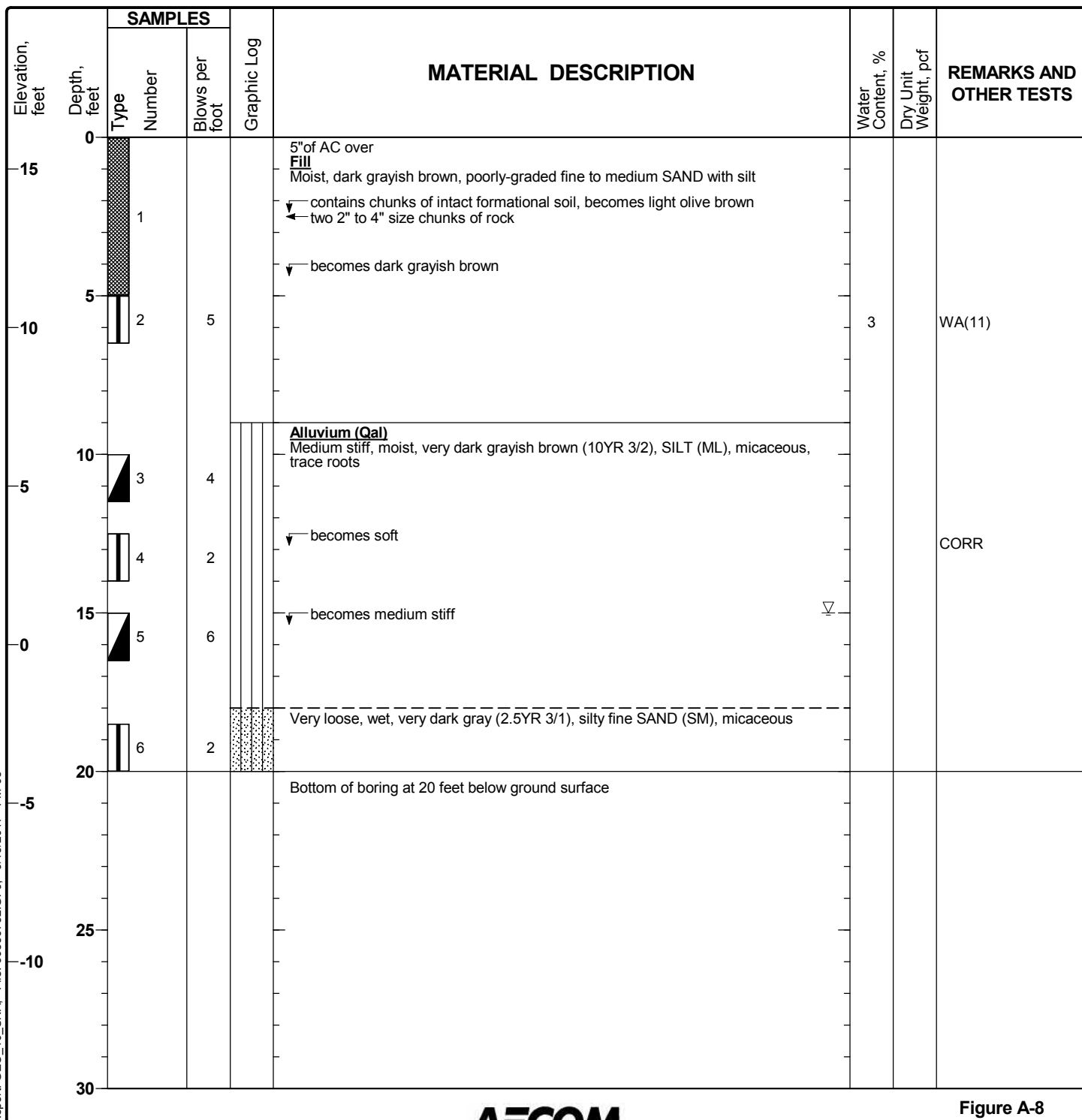


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-03

Sheet 1 of 1

Date(s) Drilled	04/27/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	16 feet
Water Level Depth	15 fbgs (after drilling)	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.765444, W117.198803		

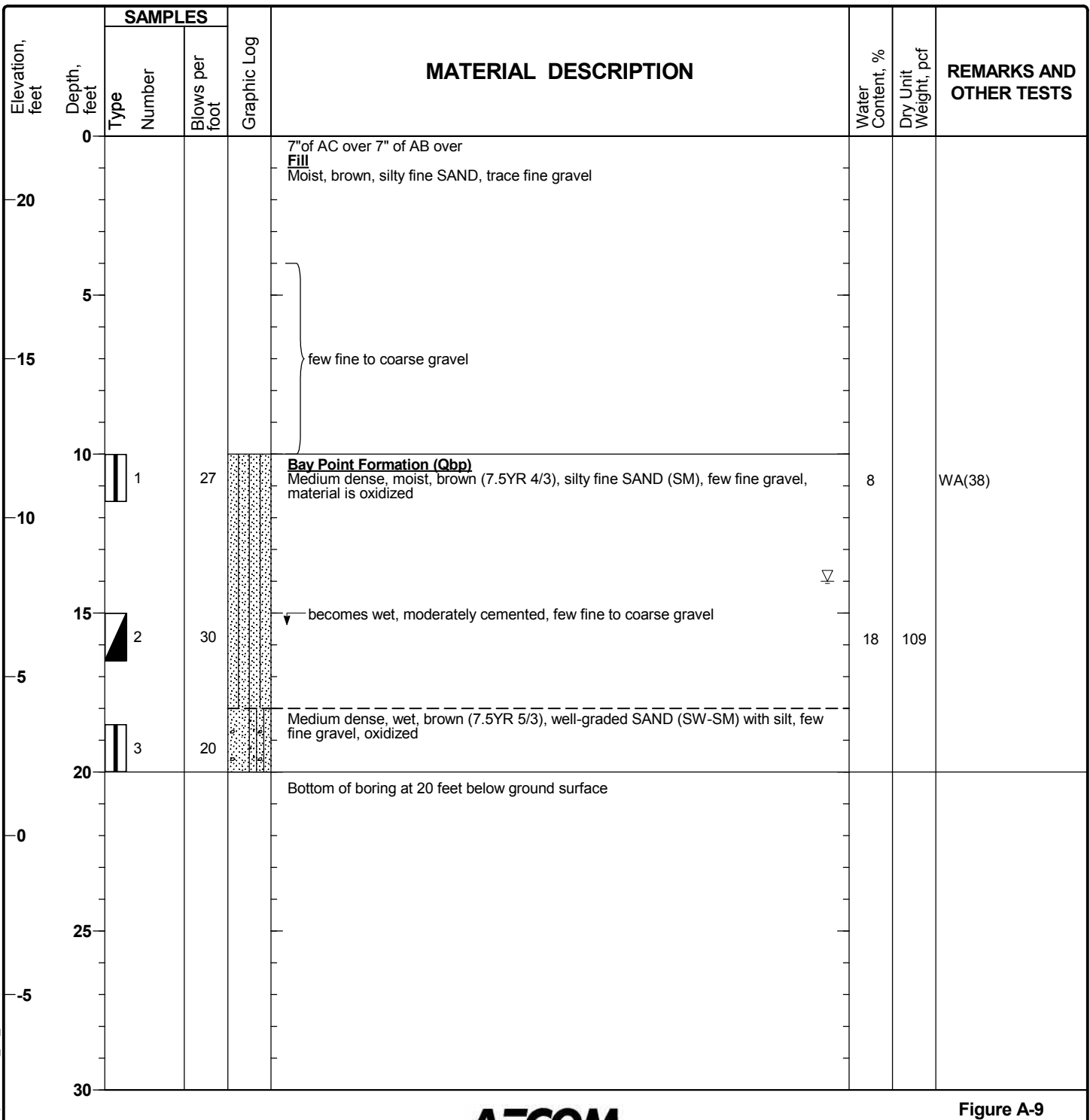


Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-04

Sheet 1 of 1

Date(s) Drilled	05/02/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	22 feet
Water Level Depth	14 fbgs (during drilling)	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.76653, W117.19873		

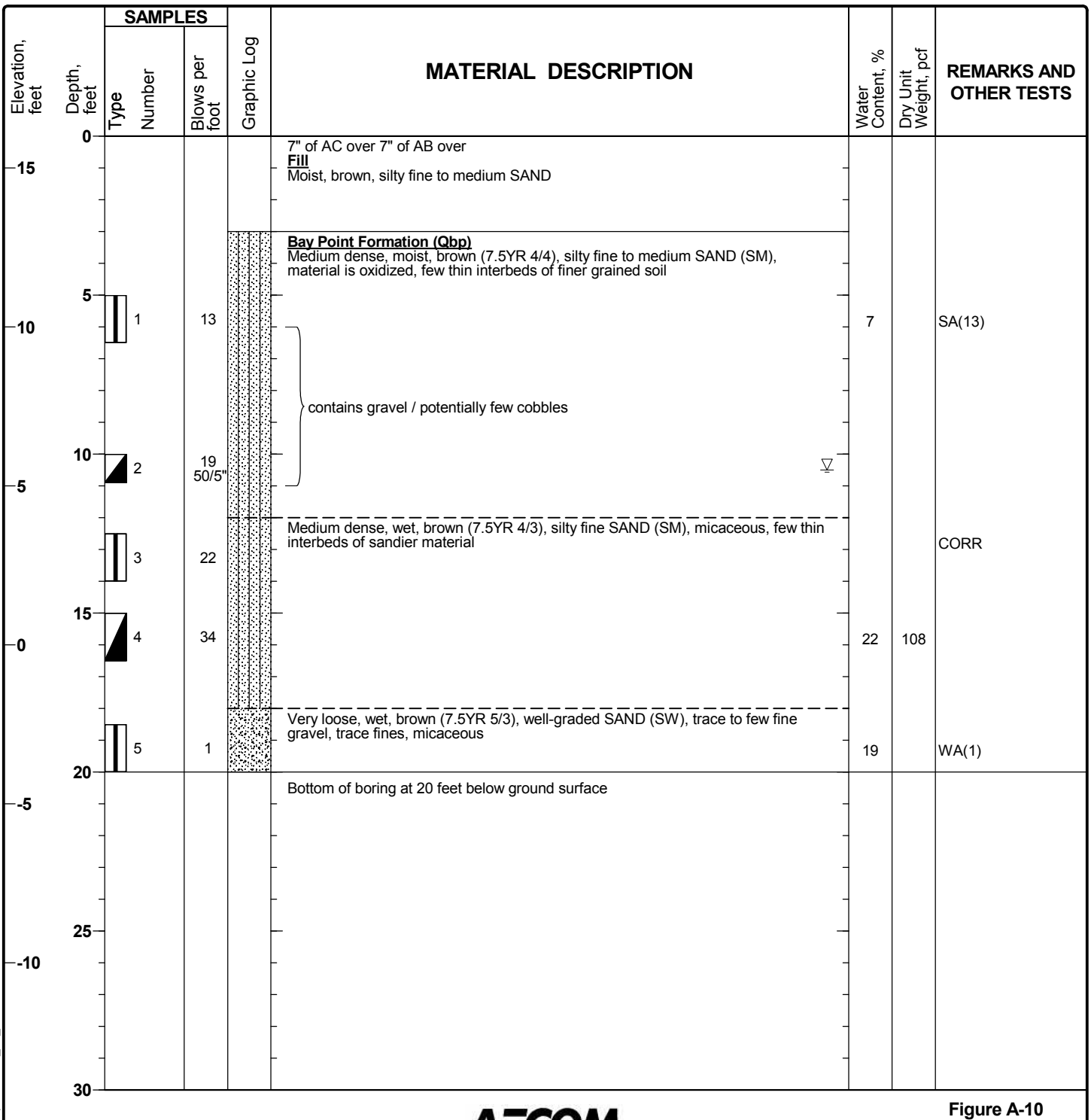


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-06

Sheet 1 of 1

Date(s) Drilled	05/02/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	16 feet
Water Level Depth	10.5 fbs (during drilling)	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.769601, W117.203201		

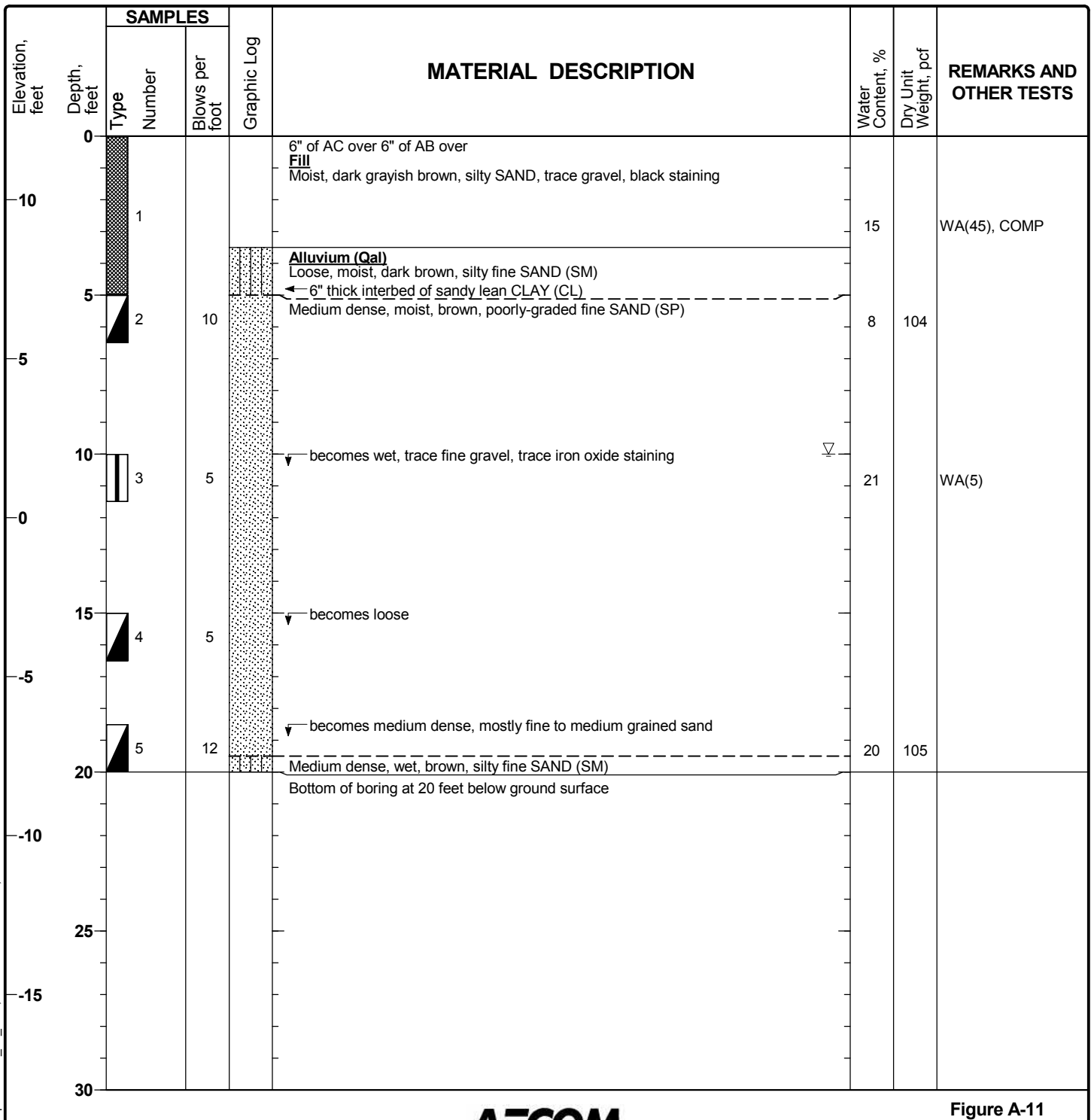


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-07

Sheet 1 of 1

Date(s) Drilled	05/02/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	12 feet
Water Level Depth	10 fbg (during drilling)	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.77364, W117.2062		

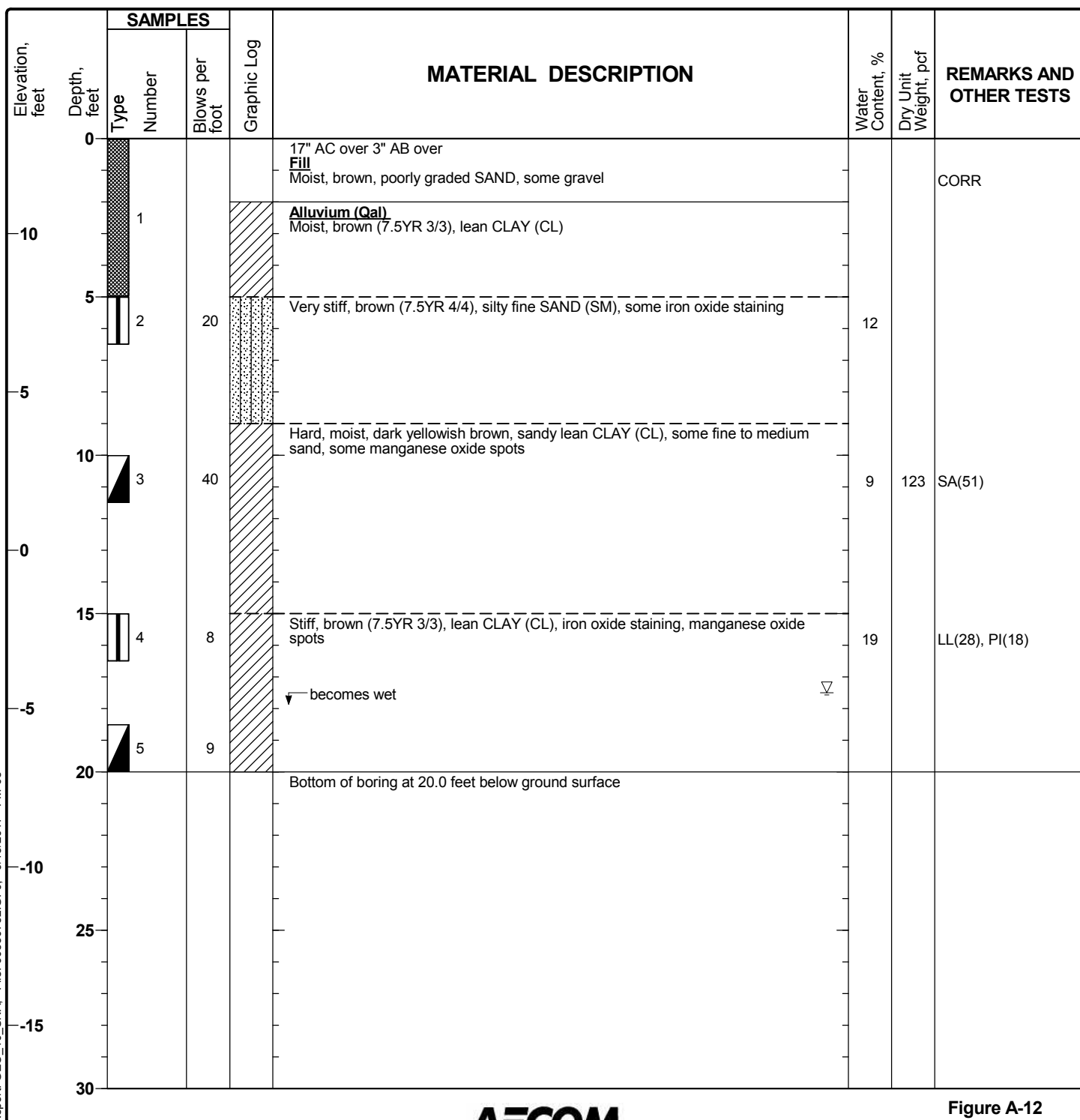


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-08

Sheet 1 of 1

Date(s) Drilled	05/10/17	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch / 4" drag bit and tri-cone bit	Total Depth of Borehole	20.0 feet
Drill Rig Type	Diedrich D-50 Turbo	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	13 feet
Water Level Depth	17.5 fbs (during drilling)	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.77599, W117.20729		

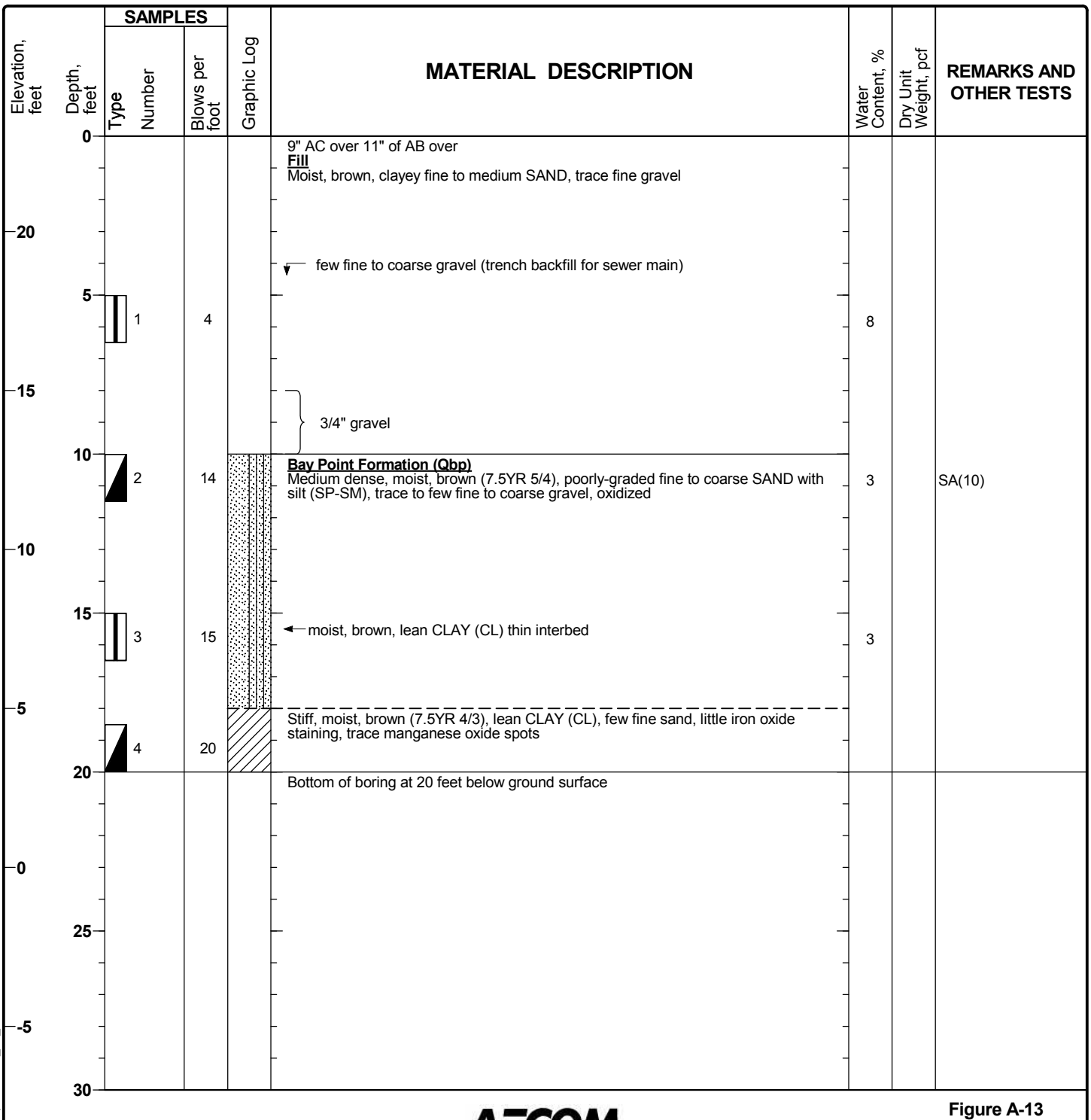


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-09

Sheet 1 of 1

Date(s) Drilled	05/17/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	23 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.77778, W117.20776		



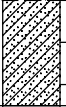


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-10a

Sheet 1 of 1

Date(s) Drilled	05/10/17	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch / 4" drag bit and tri-cone bit	Total Depth of Borehole	7.5 feet
Drill Rig Type	Diedrich D-50 Turbo	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	20 feet
Water Level Depth	not encountered	Sampling Method(s)	2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.77999, W117.20762		

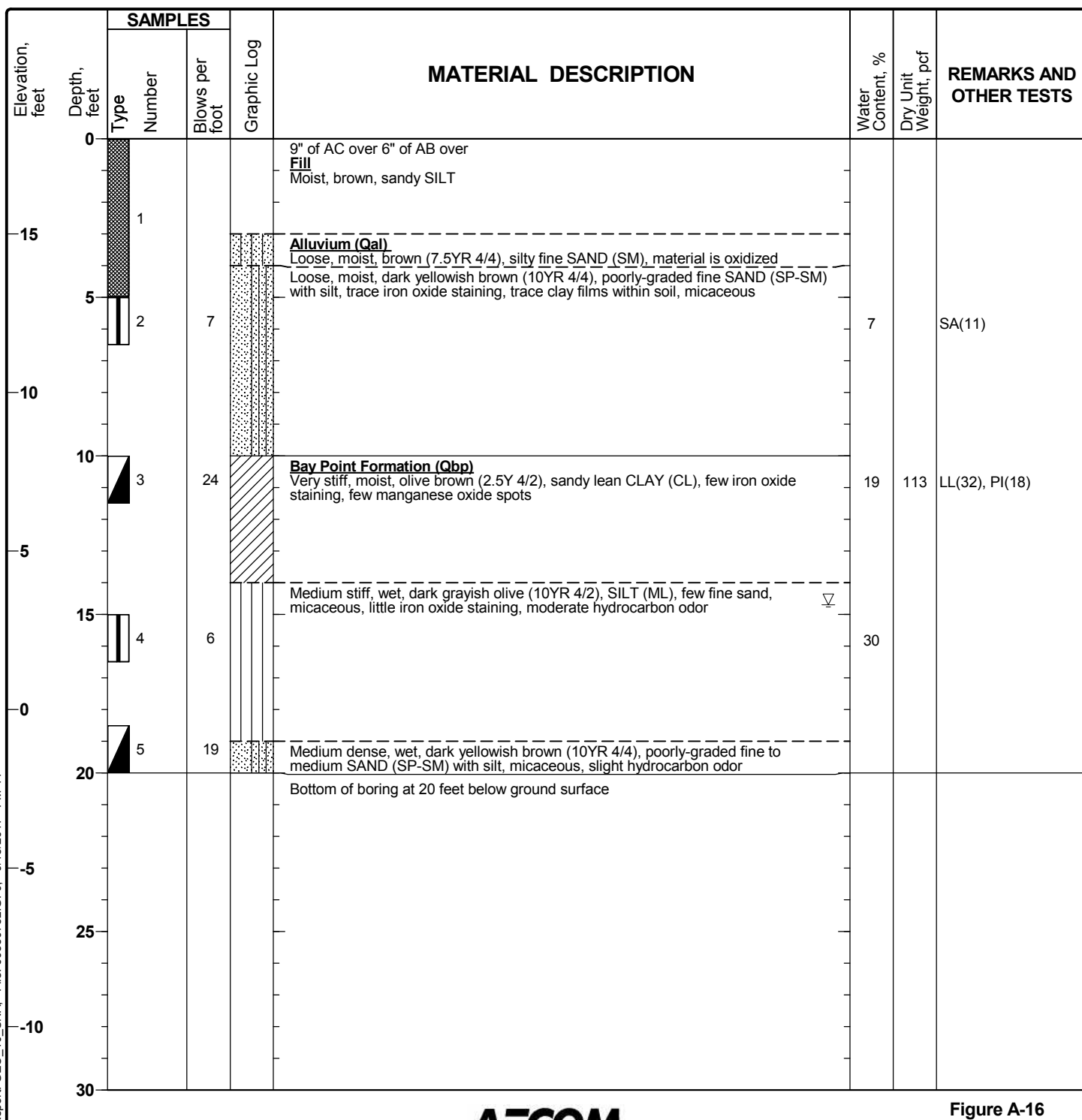
Elevation, feet	Depth, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
		Type	Number					
20	0		1		14" AC over 3" AB over Fill Moist, brown, sandy SILT, some fine sand	17		CORR
15	5		2		Alluvium (Qal) Medium dense, moist, brown (7.5YR 4/4), silty fine SAND (SM), iron oxide staining			
					Medium dense, moist, brown (7.5YR 3/3), clayey fine to medium SAND (SC)			
					Bottom of boring at 7.5 feet below ground surface (refusal on hard material)			
10	10							
5	15							
0	20							
-5	25							
-10	30							

Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-11

Sheet 1 of 1

Date(s) Drilled	05/09/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	18 feet
Water Level Depth	14.7 fbs (during drilling)	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.78142, W117.20714		

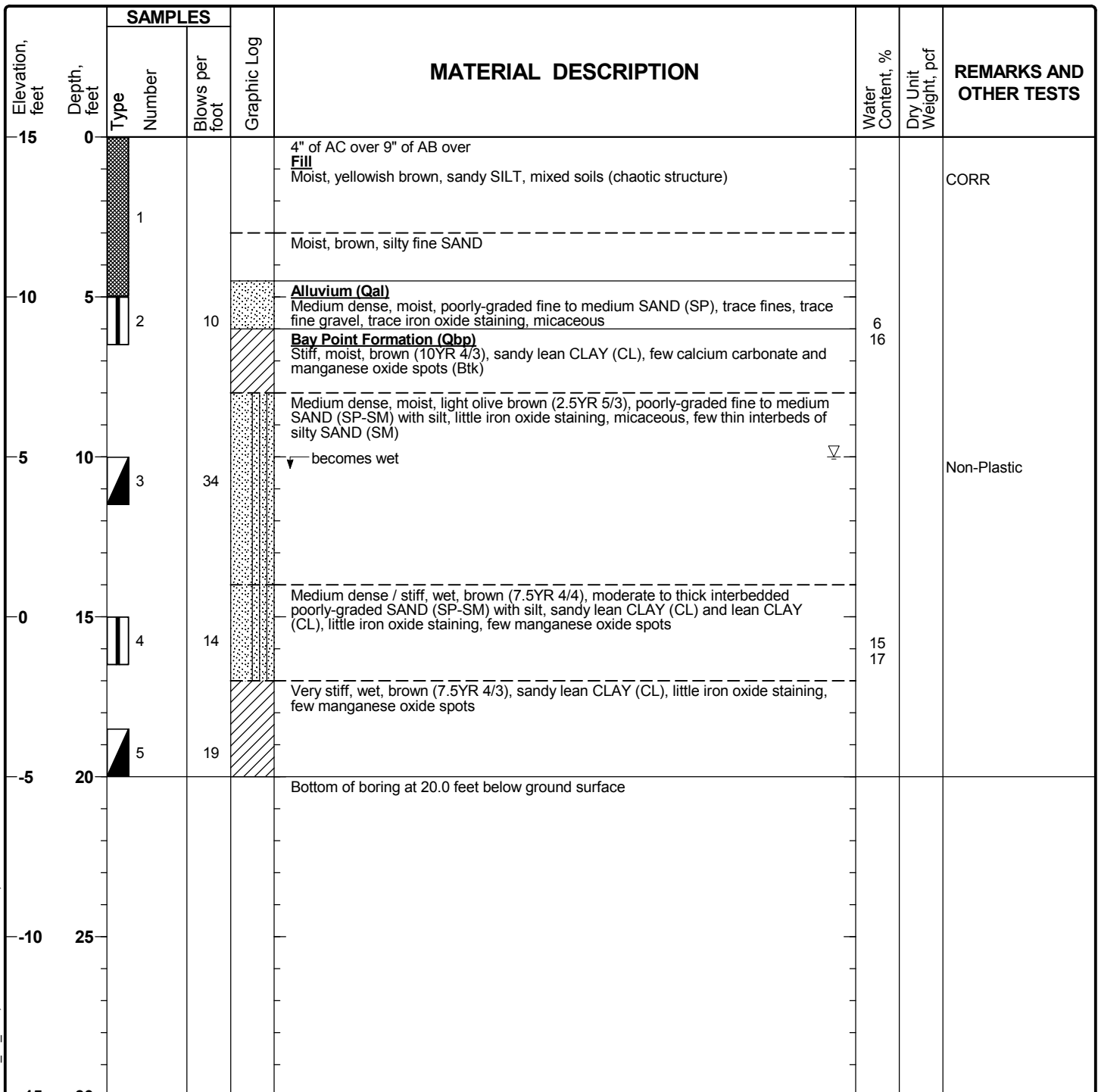


Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-14

Sheet 1 of 1

Date(s) Drilled	05/09/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	15 feet
Water Level Depth	10 fbgs (during drilling)	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.78684, W117.20617		

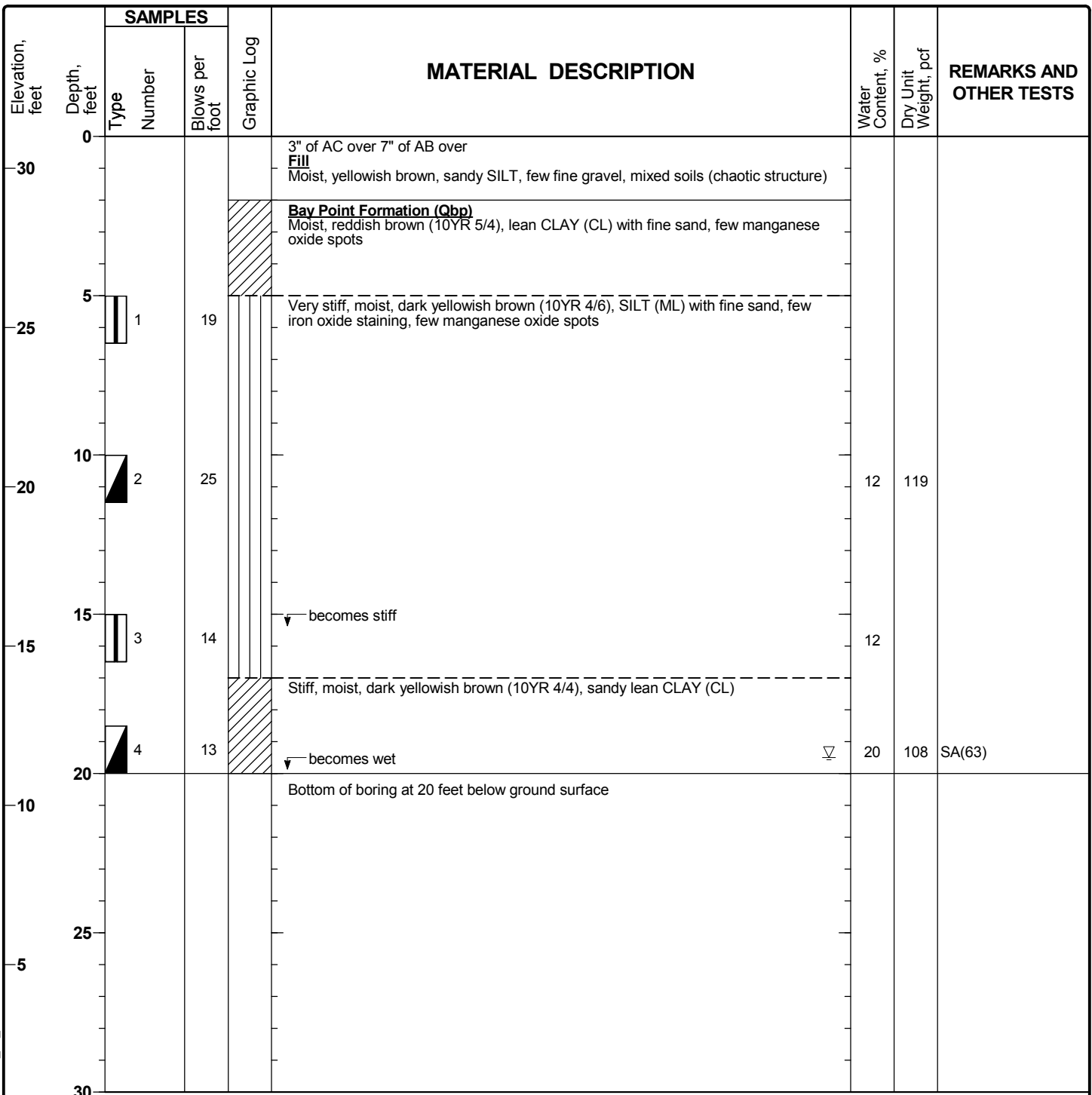


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-16

Sheet 1 of 1

Date(s) Drilled	05/09/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	31 feet
Water Level Depth	19.5 fbg (during drilling)	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.78952, W117.20543		

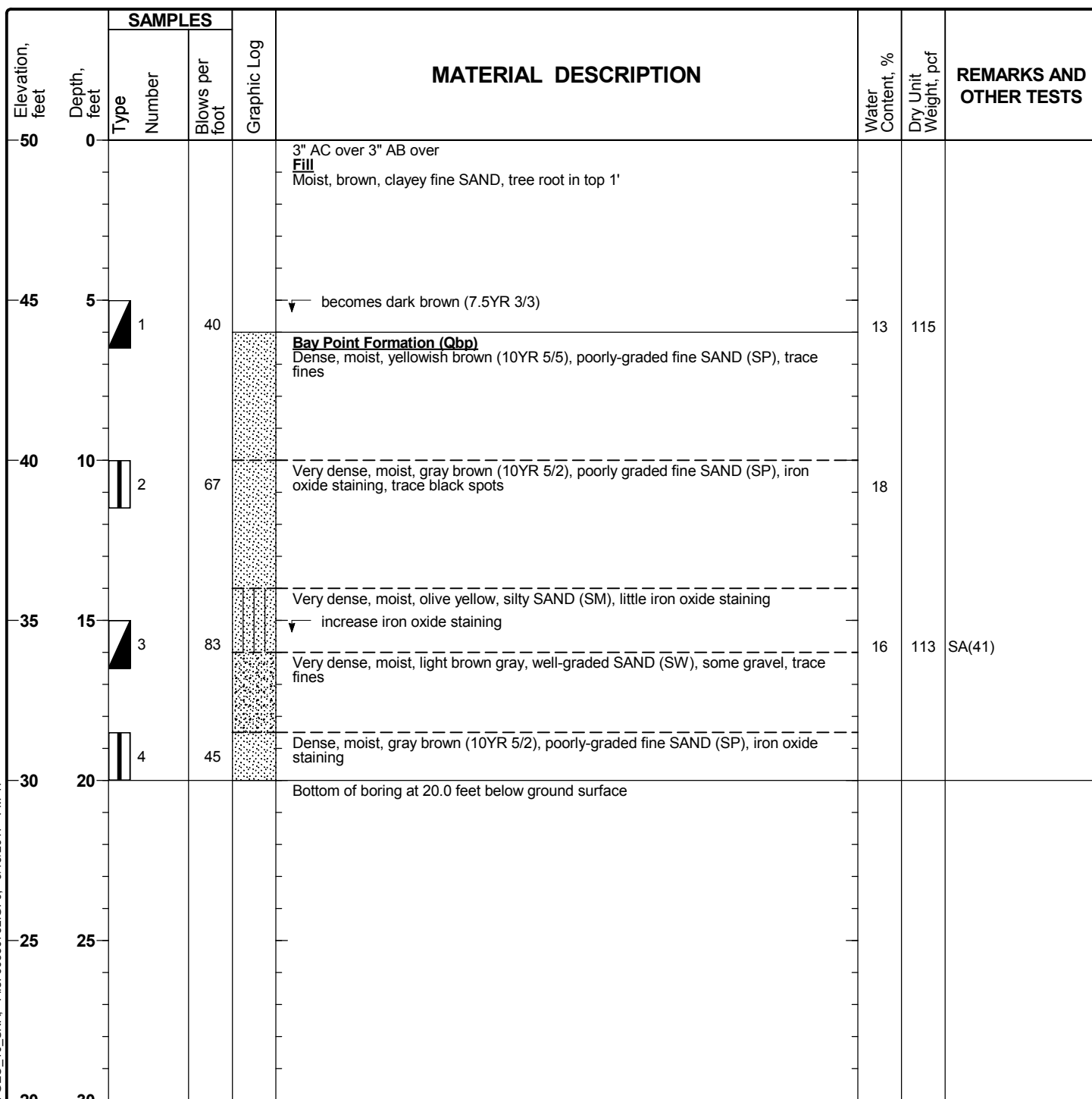


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-17

Sheet 1 of 1

Date(s) Drilled	05/10/17	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch / 4" drag bit and tri-cone bit	Total Depth of Borehole	20.0 feet
Drill Rig Type	Diedrich D-50 Turbo	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	50 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.78933, W117.20388		

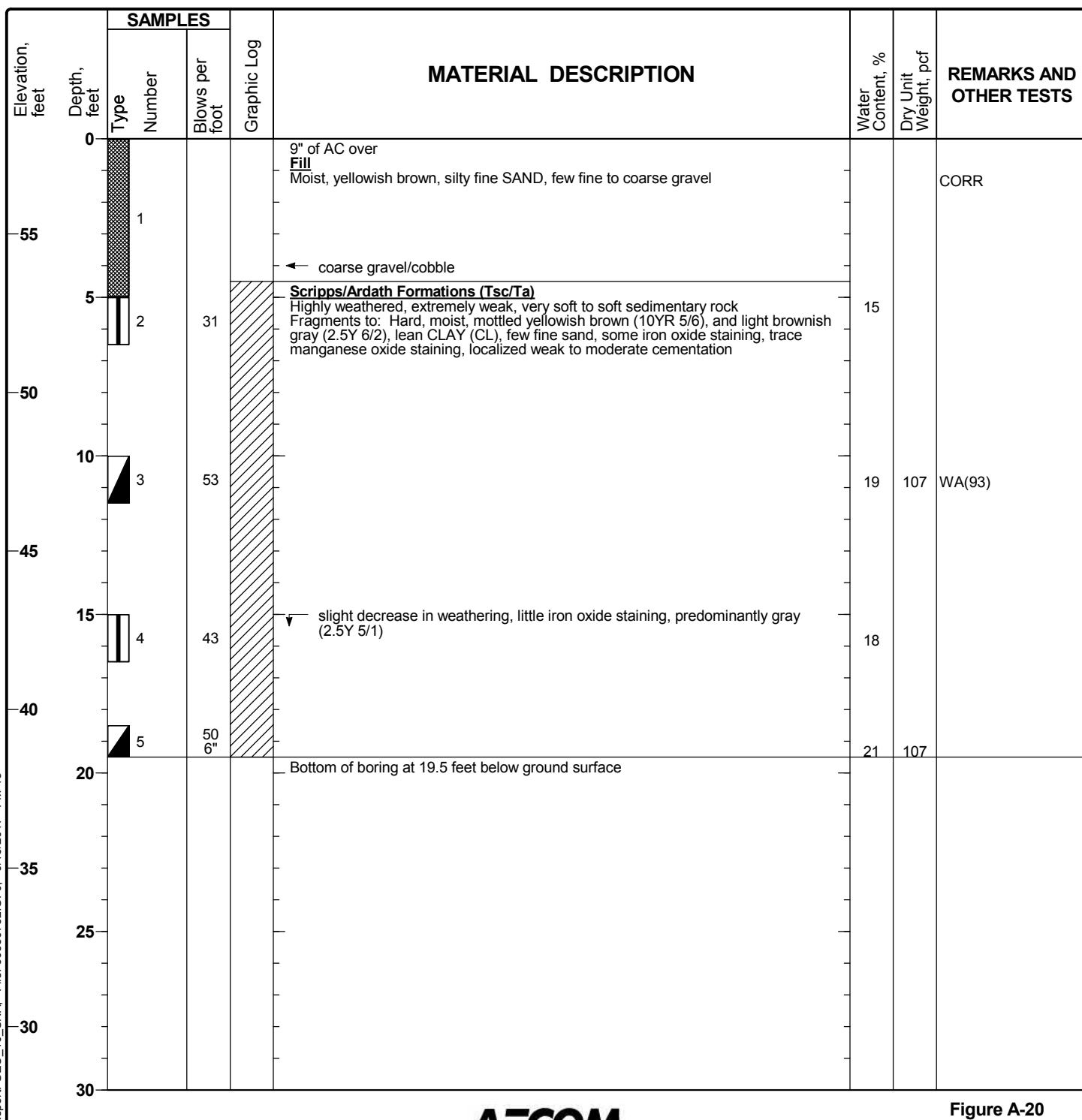


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-18

Sheet 1 of 1

Date(s) Drilled	05/17/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	19.5 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	58 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.78992, W117.20335		



Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-19

Sheet 1 of 1

Date(s) Drilled	06/26/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	105 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.790166, W117.201486		

Elevation, feet	Depth, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
		Type	Number					
105	0				5" AC over 6" AB over Fill Moist, dark yellowish brown, SILT, little to some sand, trace gravel, some chunks of Scripps Fm. material, trace chunks of clay			
100	5	1	33			18		SA(71)
95	10	2	40			14	115	
90	15	3	70		<u>Scripps Formation (Tsc)</u> Hard, moist, dark yellowish brown (10YR 4/4), SILT (ML) with gravel			
85	20	4	77					
					Bottom of boring at 20.0 feet below ground surface			
80	25							
75	30							

Report: GEO_10_SNA- File: 60530732.GPJ- 9/15/2017 FM-19

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-20

Sheet 1 of 1

Date(s) Drilled	06/26/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.8 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	151 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap		Location N32.79029, W117.200206		

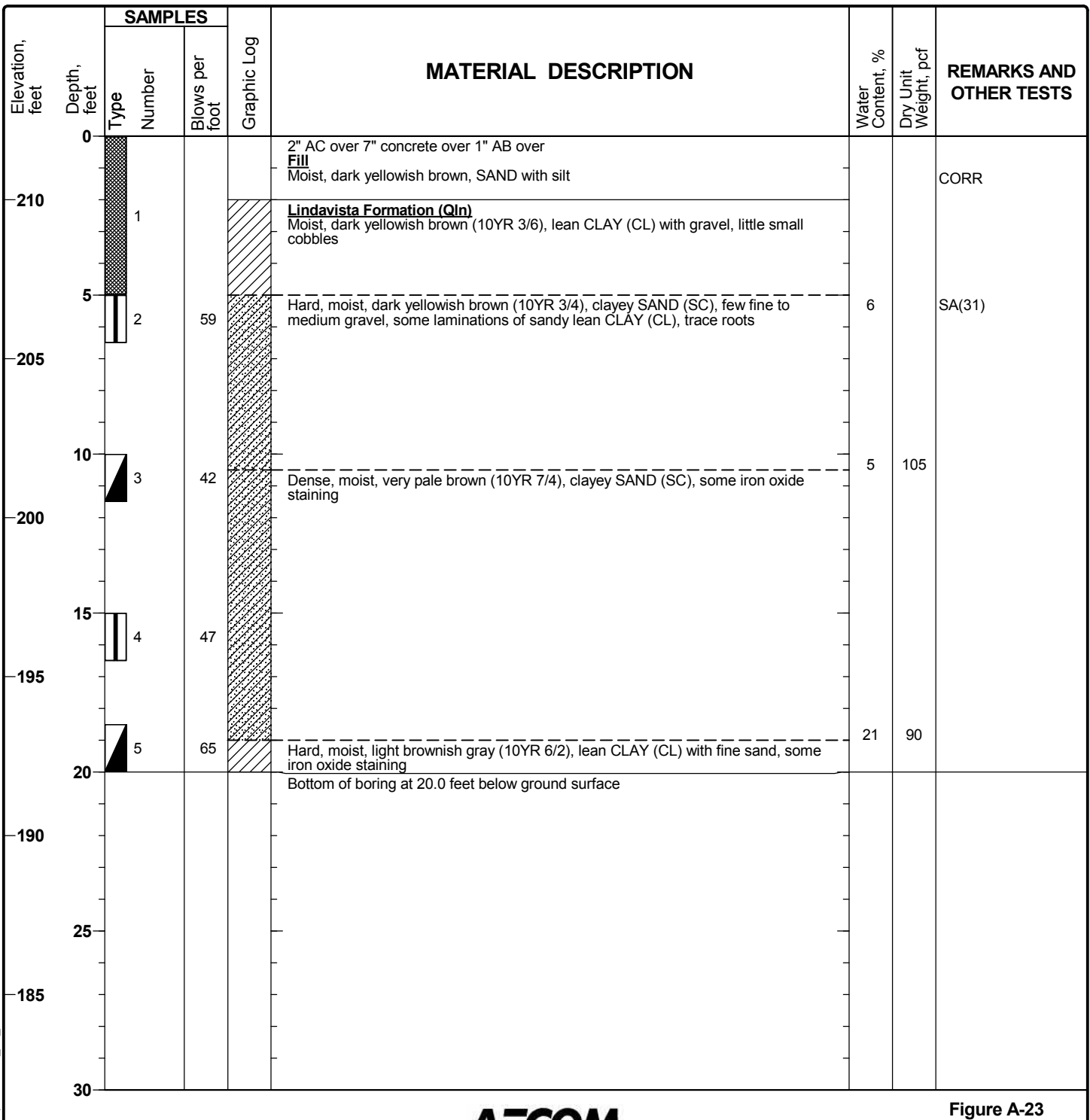
Elevation, feet	SAMPLES			Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
	Type	Number	Blows per foot					
0					5" AC over 4" AB over			
150		1			Fill Moist, dark yellowish brown, SILT, trace sand, some harder fragments of silty SAND			CORR
5								
145		2	95 12"		Lindavista Formation (Qln) Very dense, moist, dark yellowish brown (10YR 4/6), clayey SAND (SC), trace gravel, little laminations of darker-grained minerals, some iron oxide staining, interbeds of silty sand, little manganese oxide nodules	18		SA(48)
10								
140		3	97 12"		Hard, moist, brown (10YR 4/3), sandy lean CLAY (CL), little iron oxide staining	19	101	LL(47), PI(24)
15								
135		4	87		Scripps Formation (Tsc) Hard, moist, pale brown (10YR 6/5), lean CLAY (CL) with sand, trace gravel, laminations of iron-oxide rich materials and darker-grained minerals	18		SA(82)
20		5	84 9"			19	97	
130					Bottom of boring at 19.8 feet below ground surface			
25								
125								
30								

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-21

Sheet 1 of 1

Date(s) Drilled	06/26/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	212 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.791472, W117.197492		

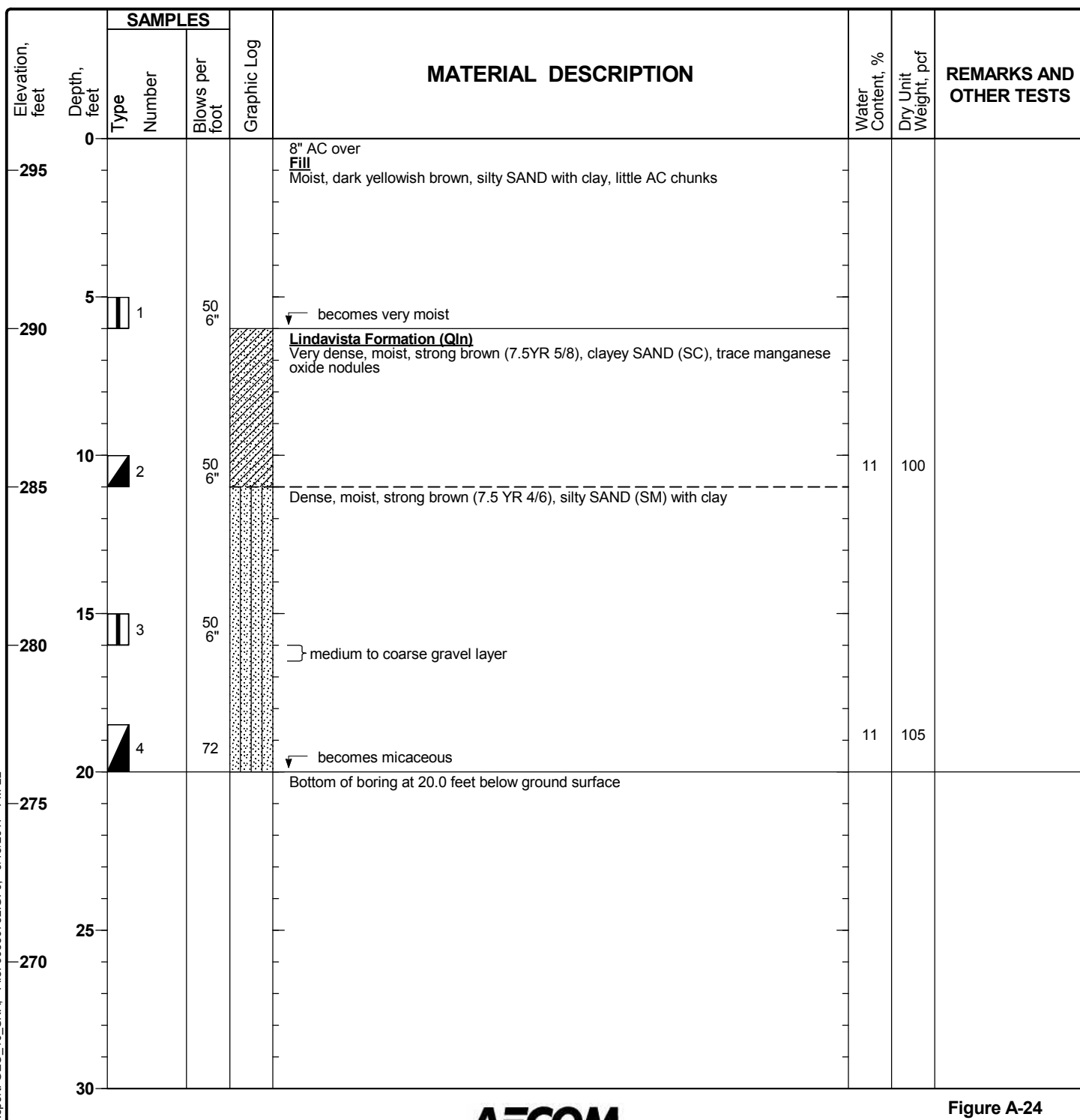


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-22

Sheet 1 of 1

Date(s) Drilled	06/27/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	296 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.79472, W117.19372		

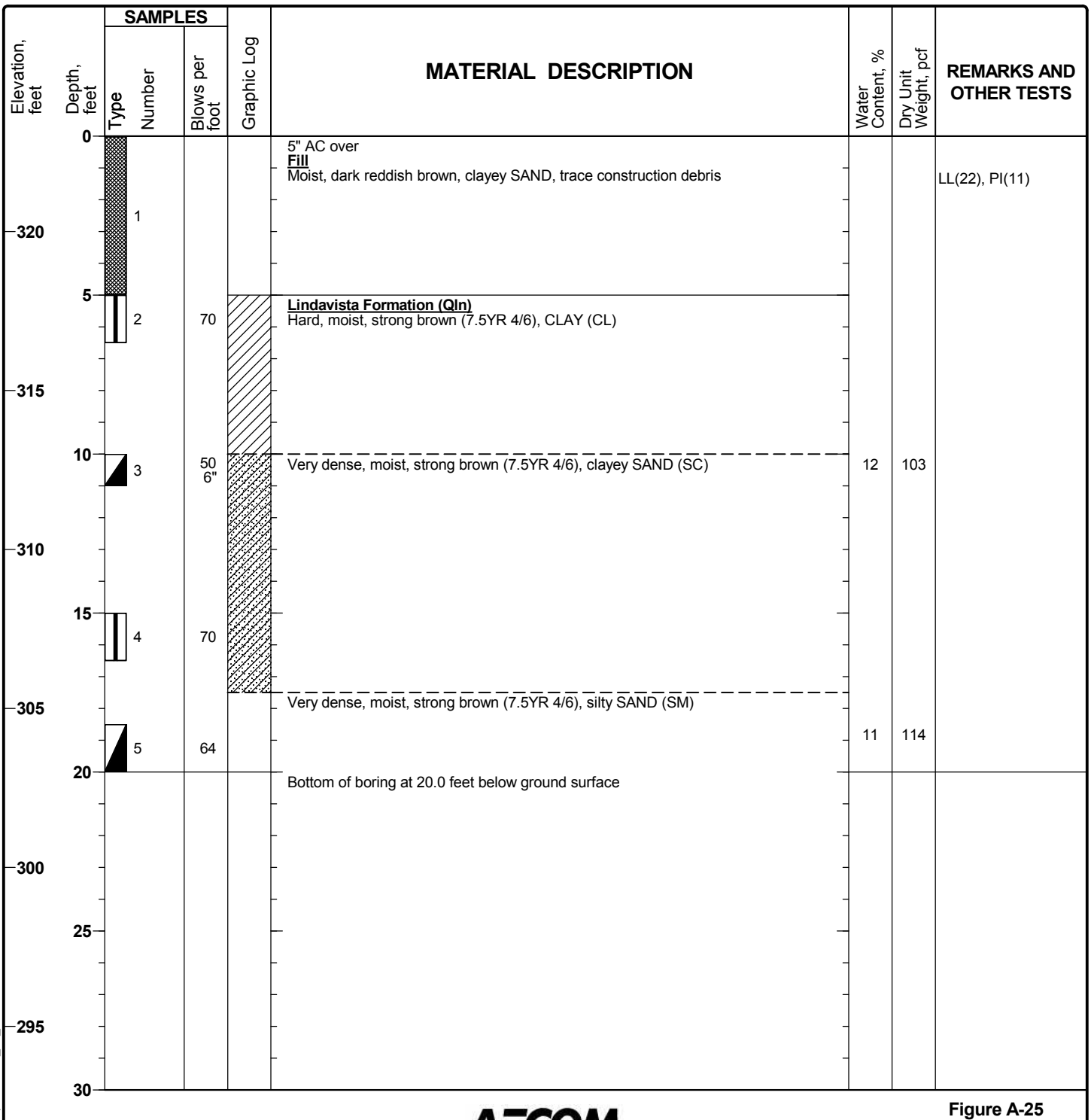


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-23

Sheet 1 of 1

Date(s) Drilled	06/27/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	323 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.796941, W117.193087		

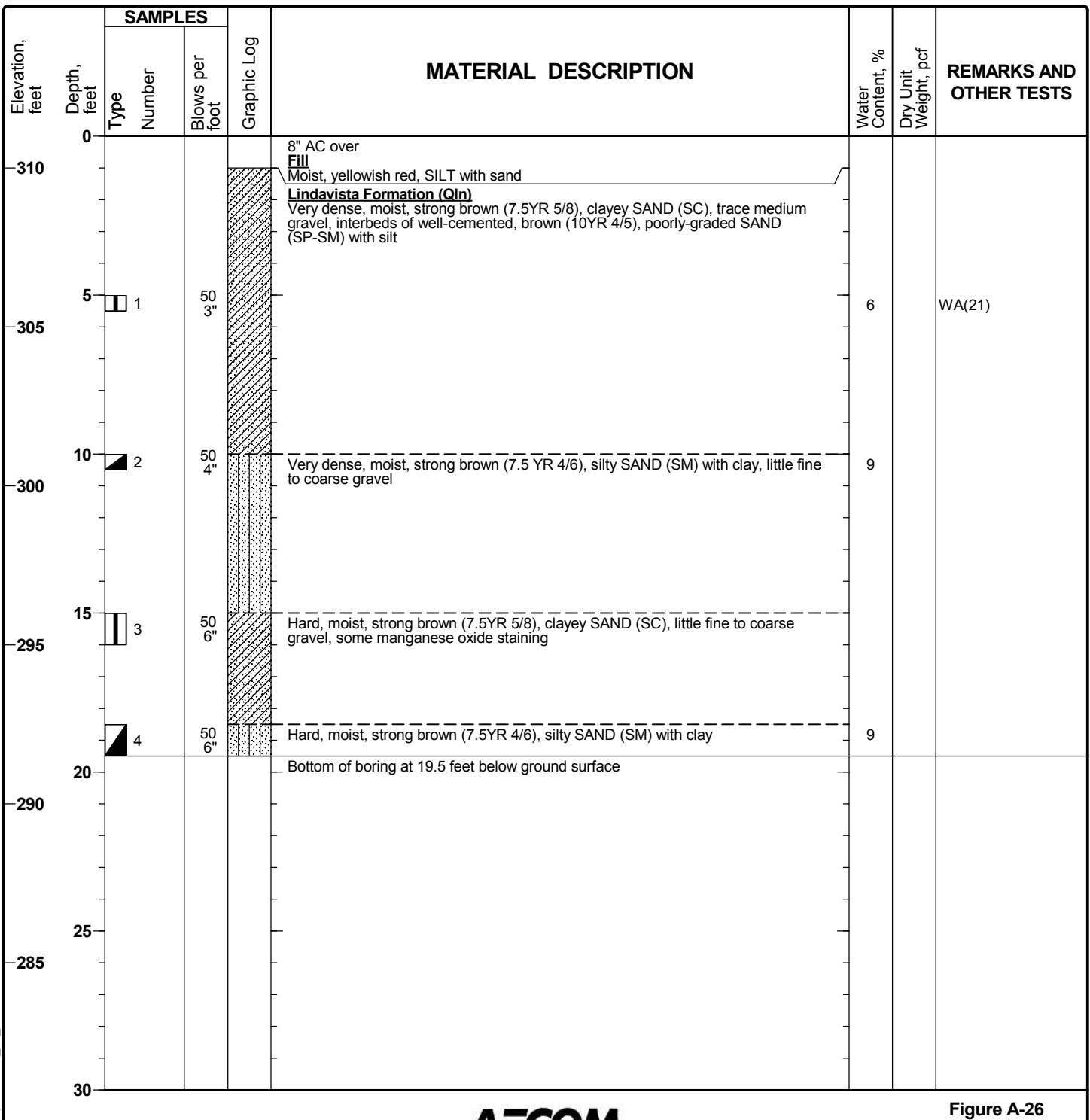


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-24

Sheet 1 of 1

Date(s) Drilled	06/27/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	311 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.79902, W117.19457		

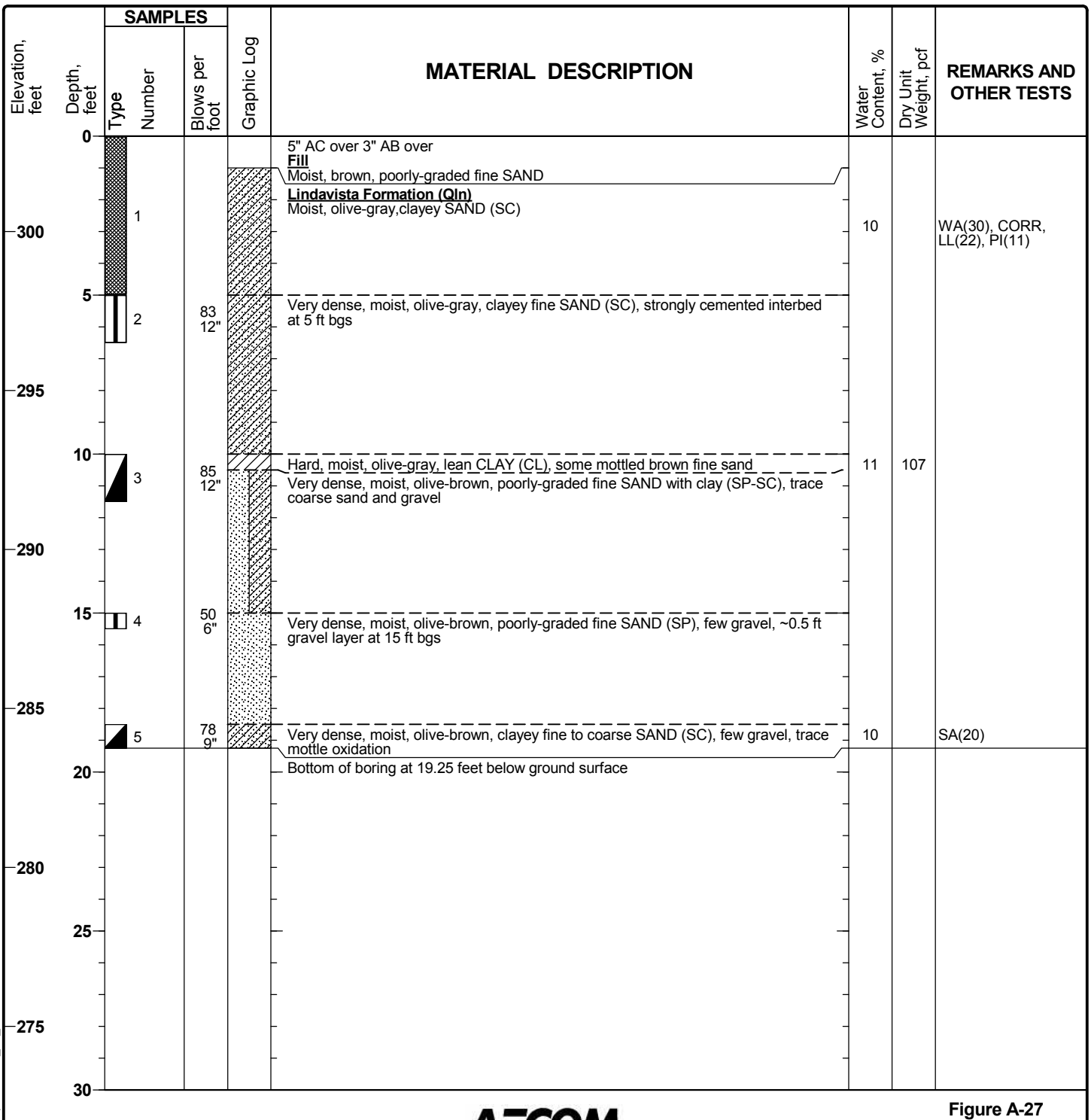


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-25

Sheet 1 of 1

Date(s) Drilled	07/27/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.3 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	303 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.80125, W117.196628		

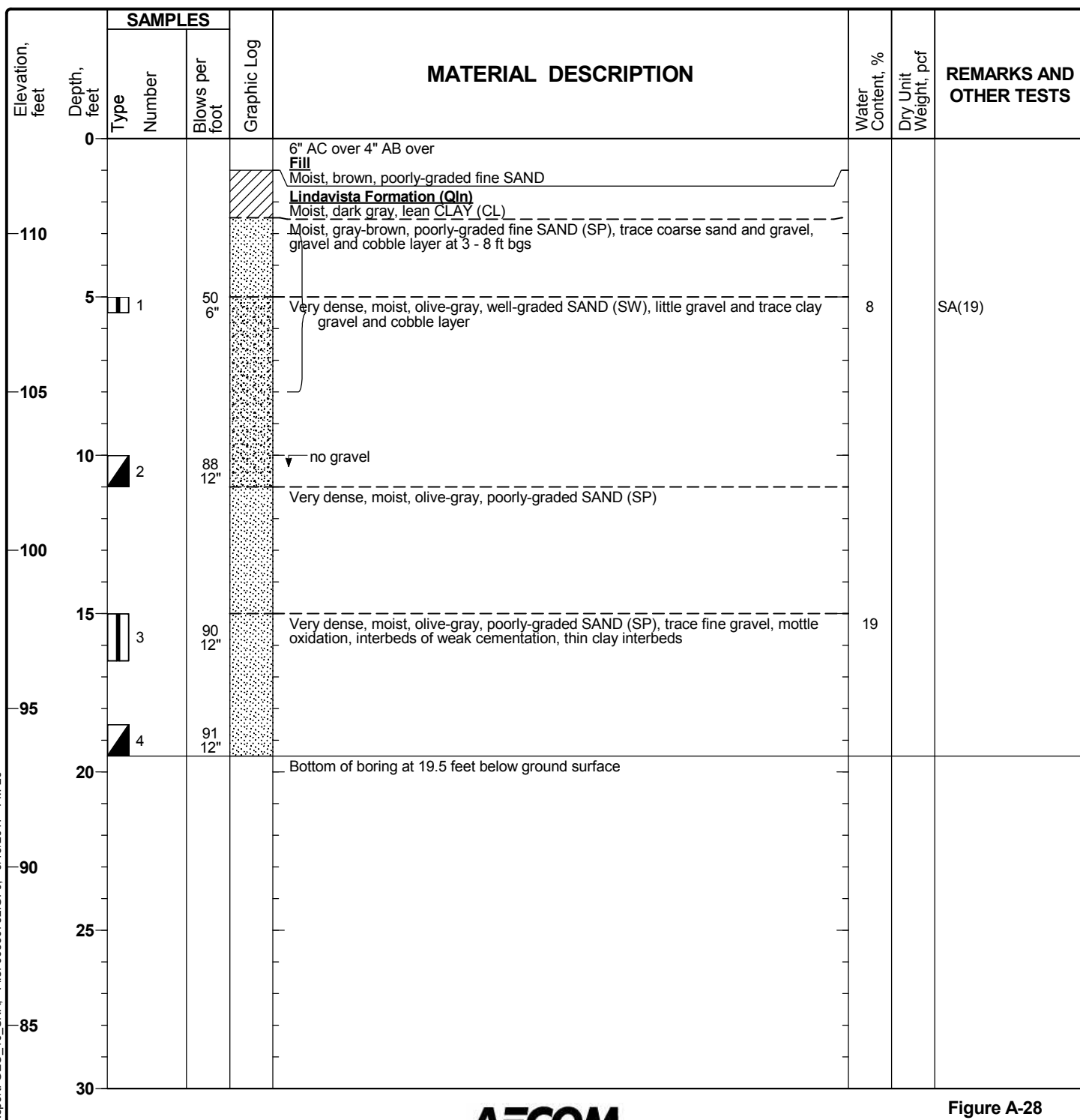


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-26

Sheet 1 of 1

Date(s) Drilled	07/27/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	113 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.803355, W117.198636		

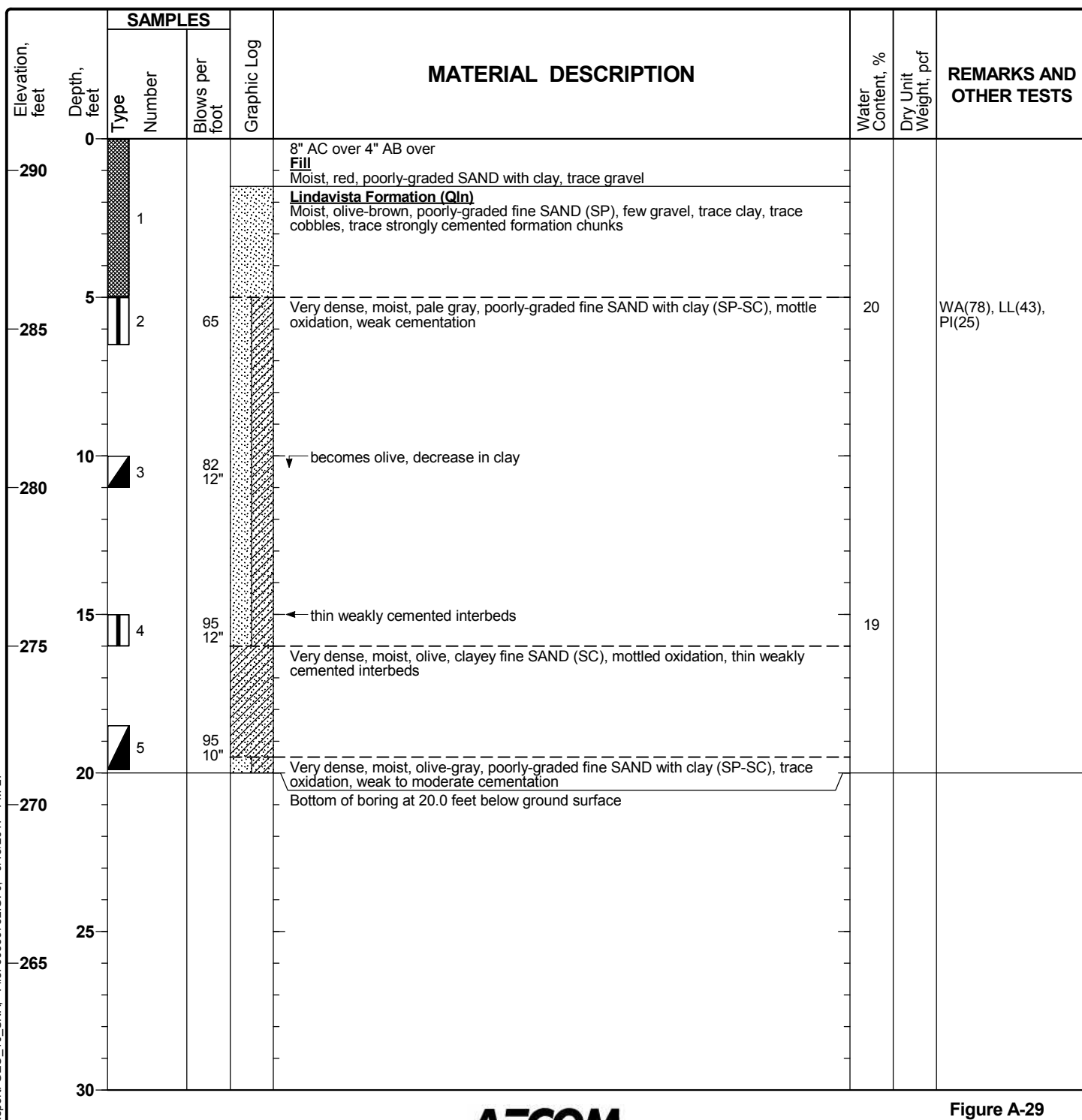


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-27

Sheet 1 of 1

Date(s) Drilled	07/28/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	291 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.80525, W117.200454		

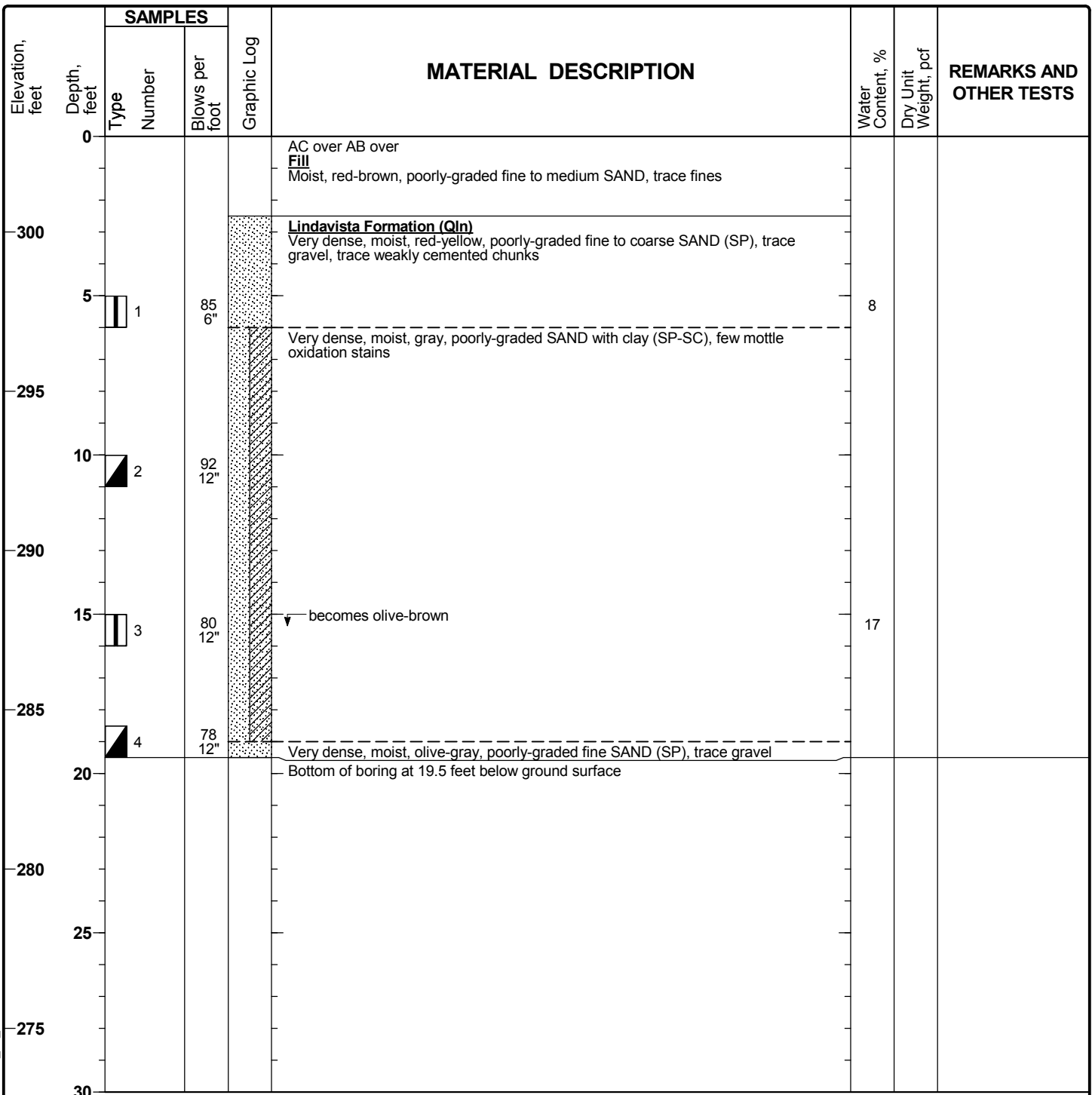


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-28

Sheet 1 of 1

Date(s) Drilled	07/26/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	303 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.808603, W117.201152		

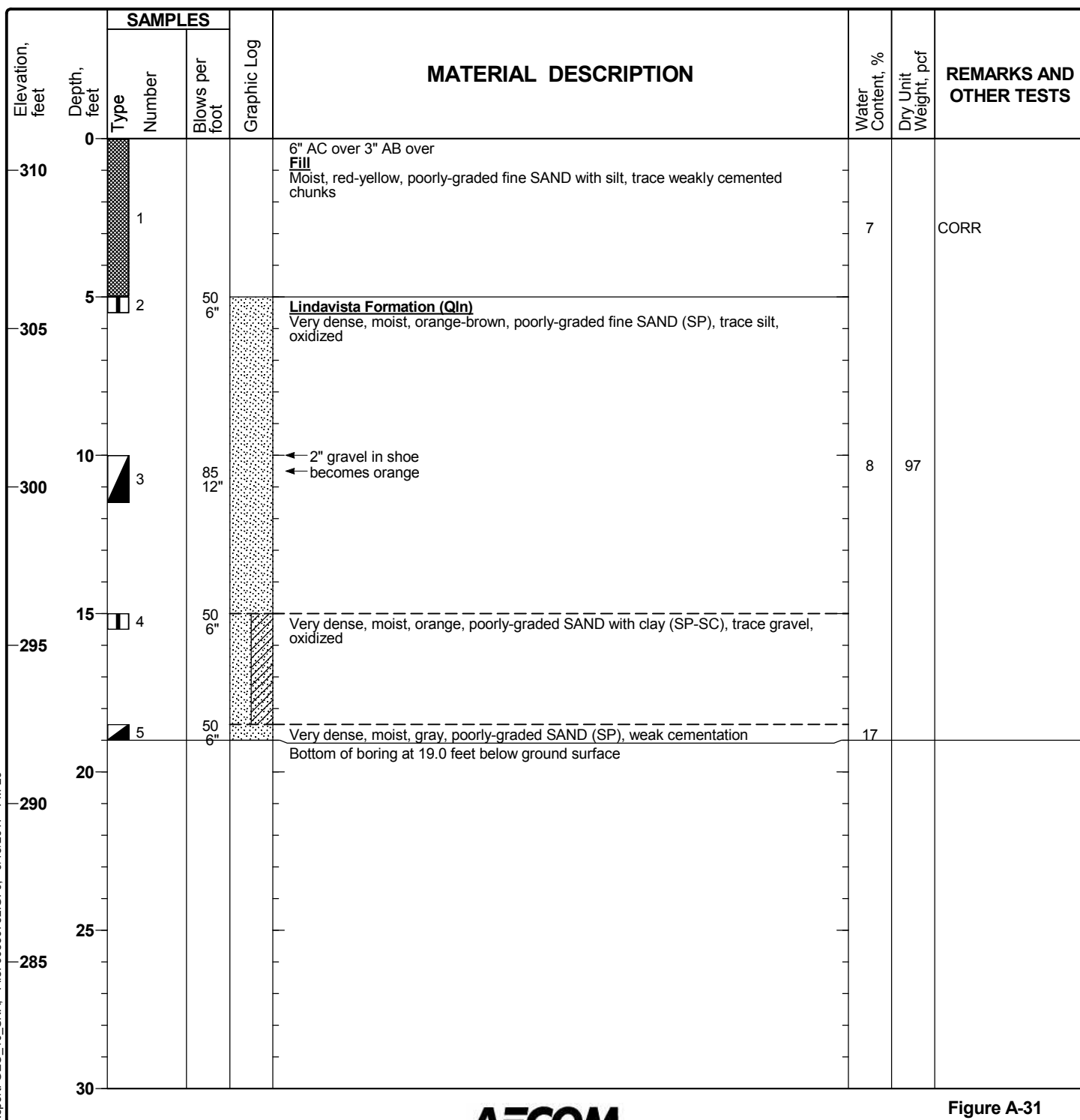


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-29

Sheet 1 of 1

Date(s) Drilled	07/26/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	311 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.811064, W117.201125		

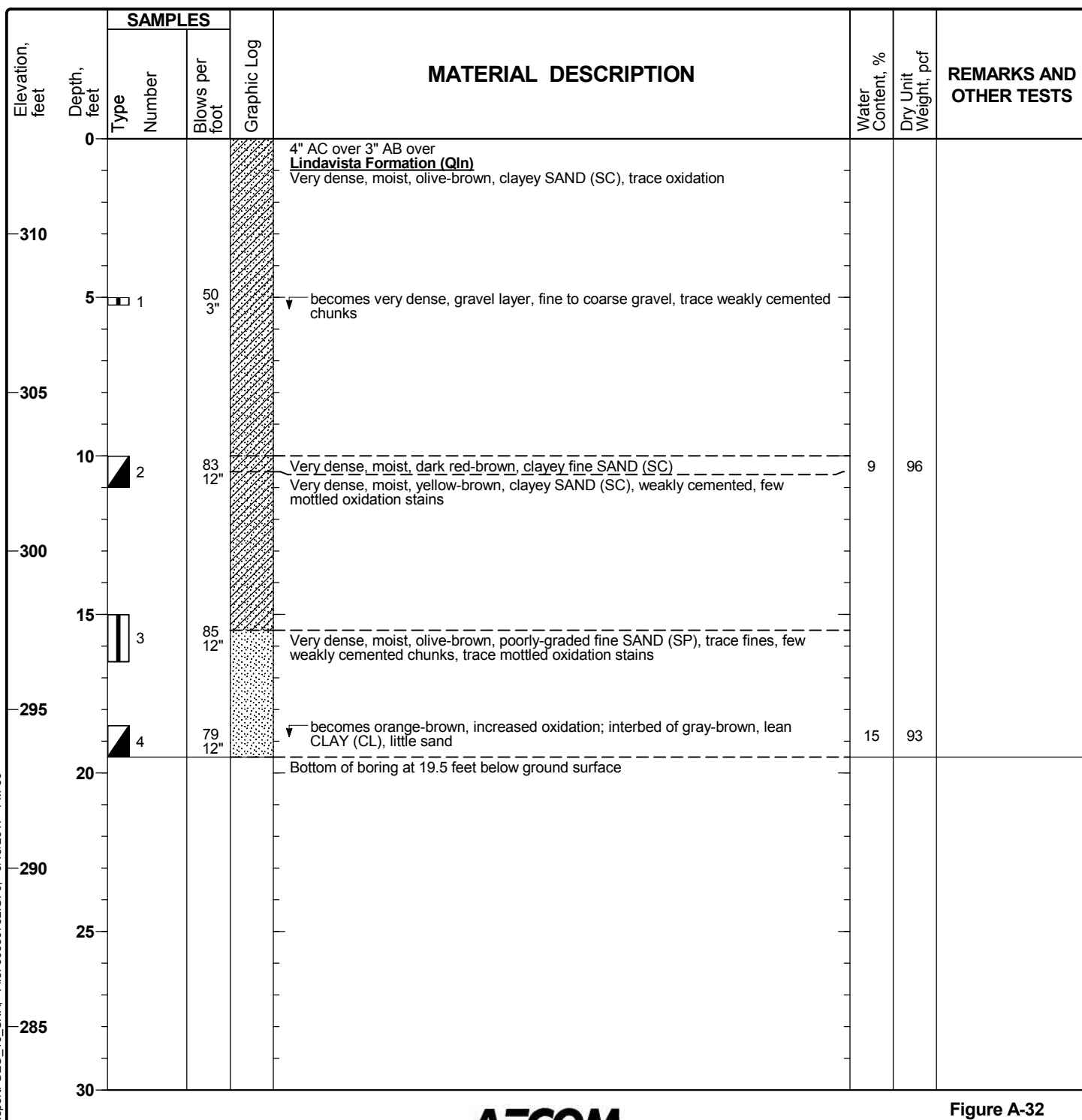


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-30

Sheet 1 of 1

Date(s) Drilled	07/26/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	313 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.813809, W117.201113		

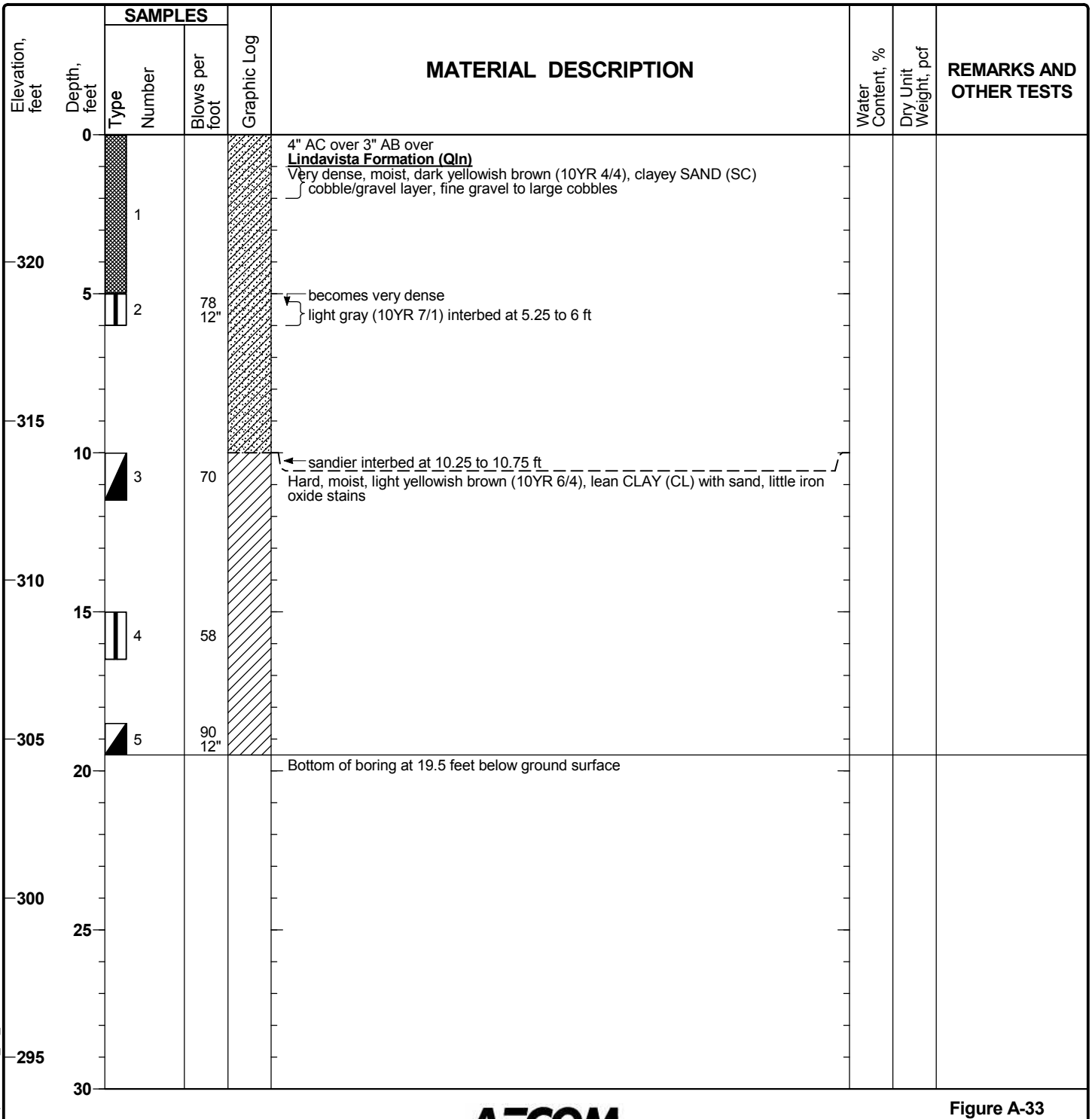


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-31

Sheet 1 of 1

Date(s) Drilled	7/25/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	324 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.817006, W117.201764		

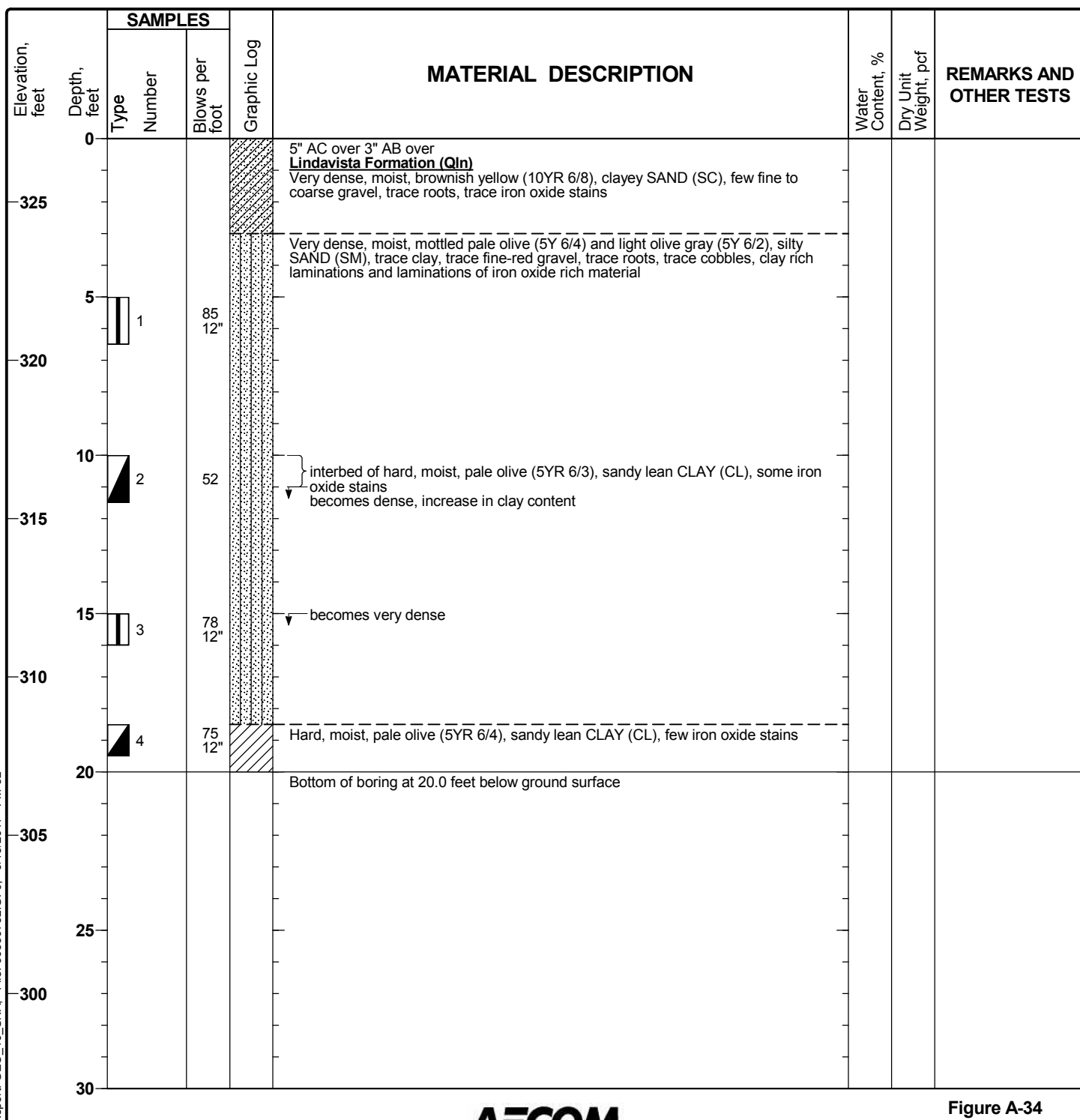


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-32

Sheet 1 of 1

Date(s) Drilled	7/25/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	327 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.819132, W117.20351		

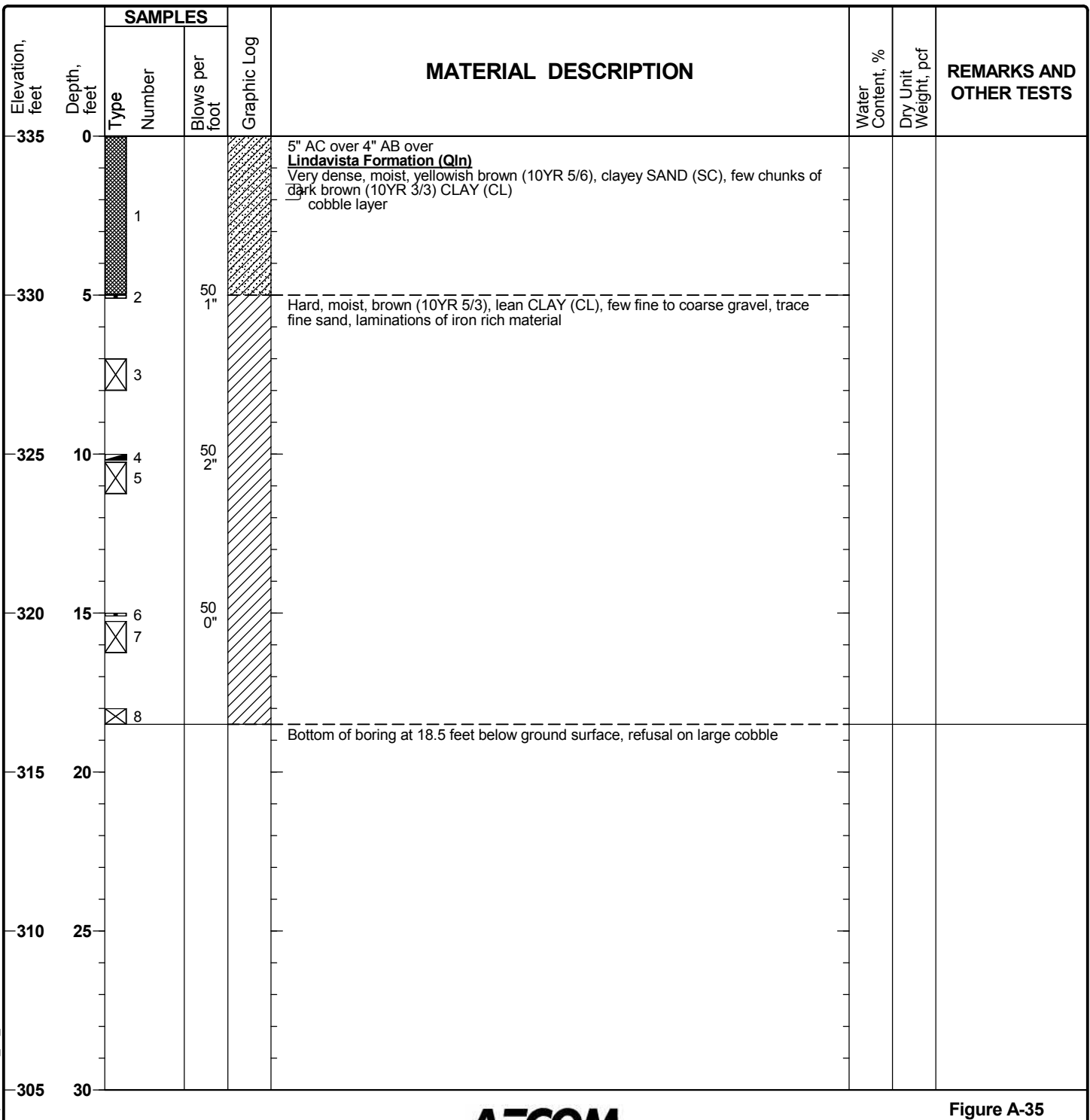


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-33

Sheet 1 of 1

Date(s) Drilled	7/25/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	18.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	335 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk / Grab	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.823902, W117.206029		



Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-34

Sheet 1 of 1

Date(s) Drilled	06/28/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	340 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.827359, W117.206037		

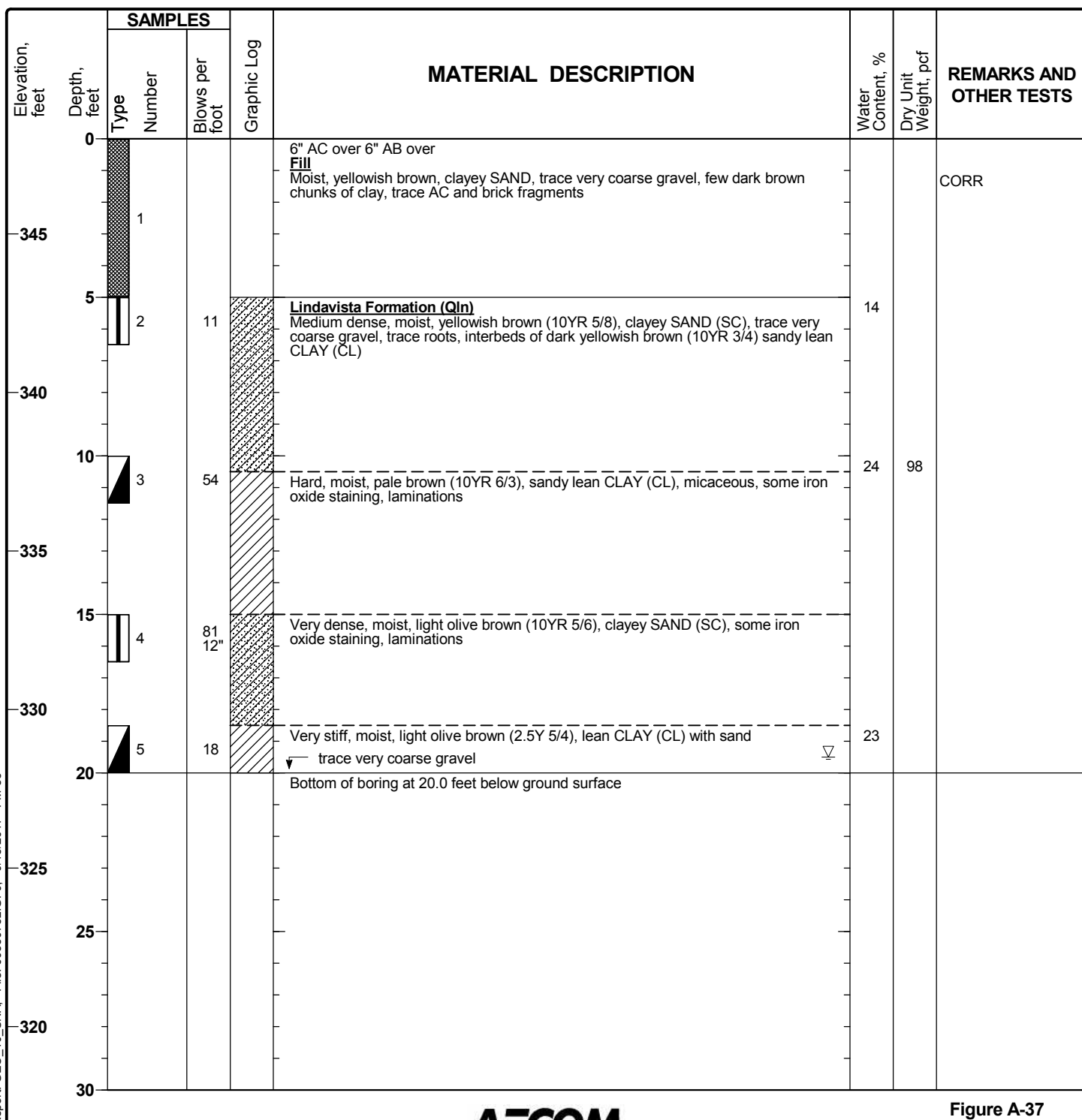
Elevation, feet	Depth, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
		Type	Number	Blows per foot				
340	0				6" AC over 8" AB over Lindavista Formation (Qln) Very dense, moist, olive yellow (2.5Y 6/6), clayey SAND (SC), <2" chunks of moist, dark olive brown CLAY (CL), few roots, trace fine to medium gravel } gravel layer			
335	5	□	1	50 6"	Very dense, moist, yellow (10YR 7/8), clayey SAND (SC), laminations of iron-oxide rich layers			
330	10	◼	2	50 6"	Very dense, moist, olive yellow (2.5YR 6/6), silty SAND (SM), few to little gravel	10	107	SA(27)
325	15	□	3	50 6"	Very dense, moist, olive (5YR 4/4), clayey SAND (SC), interbeds of very dark grayish brown (10YR 3/2) CLAY (CL)	14		
		◼	4	50 6"	Very hard, moist, olive (5YR 5/4), lean CLAY (CL), trace fine sand, micaceous, laminations, interbeds of SILT (ML)	12		
320	20				Bottom of boring at 19.5 feet below ground surface			
315	25							
310	30							

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-35

Sheet 1 of 1

Date(s) Drilled	06/28/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	348 feet
Water Level Depth	19.5 fbg (during drilling)	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.31759, W117.205434		



Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-36a

Sheet 1 of 1

Date(s) Drilled	06/29/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	2.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	356 feet
Water Level Depth	not encountered	Sampling Method(s)	none	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.833234, W117.20105		

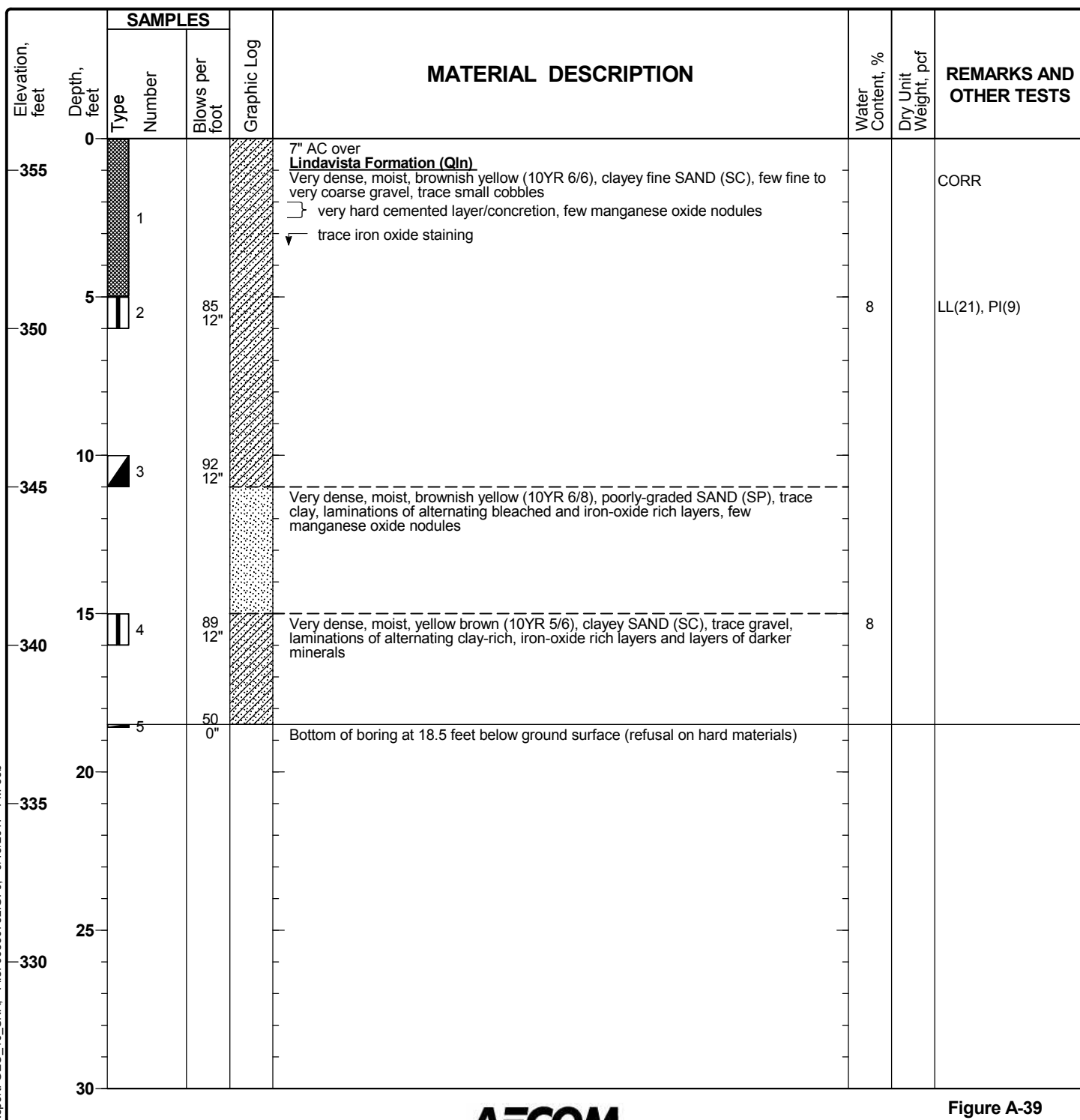
Elevation, feet	Depth, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
		Type	Number					
0					6" AC over Lindavista Formation (Qln)			
355					Dense, moist, yellowish brown (10YR 5/8), sandy SILT (ML), trace fine sand, few fine to very coarse gravel, some chunks of well cemented material, trace small cobbles			
					Bottom of boring at 2 feet below ground surface (refusal on hard material)			
5								
350								
10								
345								
15								
340								
20								
335								
25								
330								
30								

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-36b

Sheet 1 of 1

Date(s) Drilled	06/29/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	18.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	356 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.83321, W117.20104		



Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-37a

Sheet 1 of 1

Date(s) Drilled	06/29/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	5.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	373 feet
Water Level Depth	not encountered	Sampling Method(s)	none	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.834441, W117.198367		

Elevation, feet	SAMPLES			MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
	Type	Number	Blows per foot				
0				8" AC over Lindavista Formation (Qln) Dense, moist, brown (7.5YR 4/4), sandy SILT (ML), few very coarse gravel Hard cemented layer/concretion over Moist, brown (7.5YR 4/4), poorly-graded GRAVEL with silt and sand (GP-GM)			
370	1						
5				Bottom of boring at 5 feet below ground surface (refusal on hard materials)			
365							
10							
360							
15							
355							
20							
350							
25							
345							
30							

Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-37b

Sheet 1 of 1

Date(s) Drilled	06/29/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	4.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	373 feet
Water Level Depth	not encountered	Sampling Method(s)	none	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.834445, W117.198321		

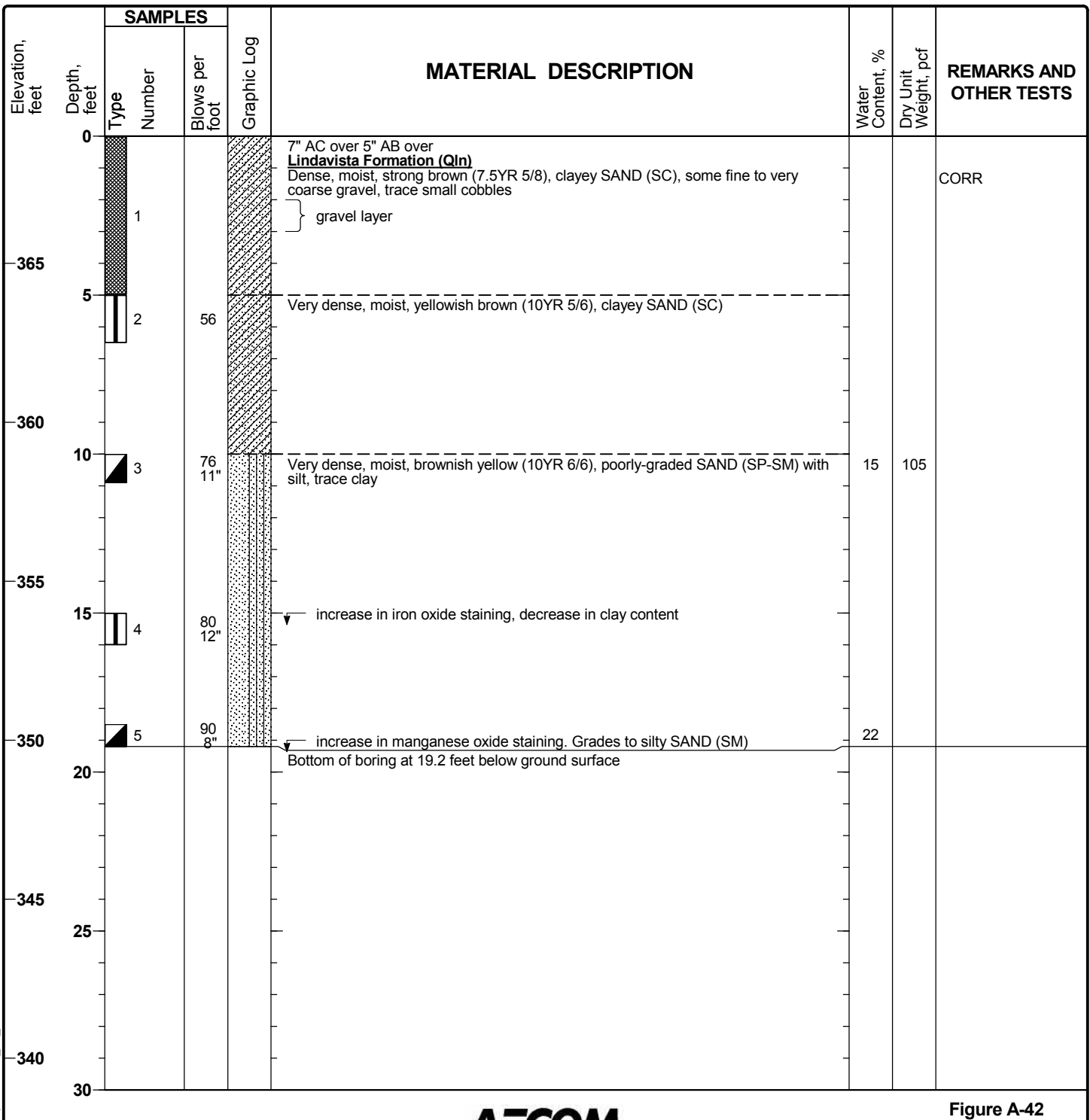
Elevation, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
	Type	Number					
0				8" AC over Lindavista Formation (Qln) Dense, moist, dark yellowish brown, sandy SILT (ML), some fine gravel to small cobbles. Cobble layer at 1'			
370							
5				Bottom of boring at 4 feet below ground surface (refusal on hard materials)			
365							
10							
360							
15							
355							
20							
350							
25							
345							
30							

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-38

Sheet 1 of 1

Date(s) Drilled	06/30/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.2 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	369 feet
Water Level Depth	not encountered	Sampling Method(s)	Bulk / SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.834805, W117.194949		



Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-40

Sheet 1 of 1

Date(s) Drilled	06/30/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	18.9 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	333 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.84014, W117.19674		

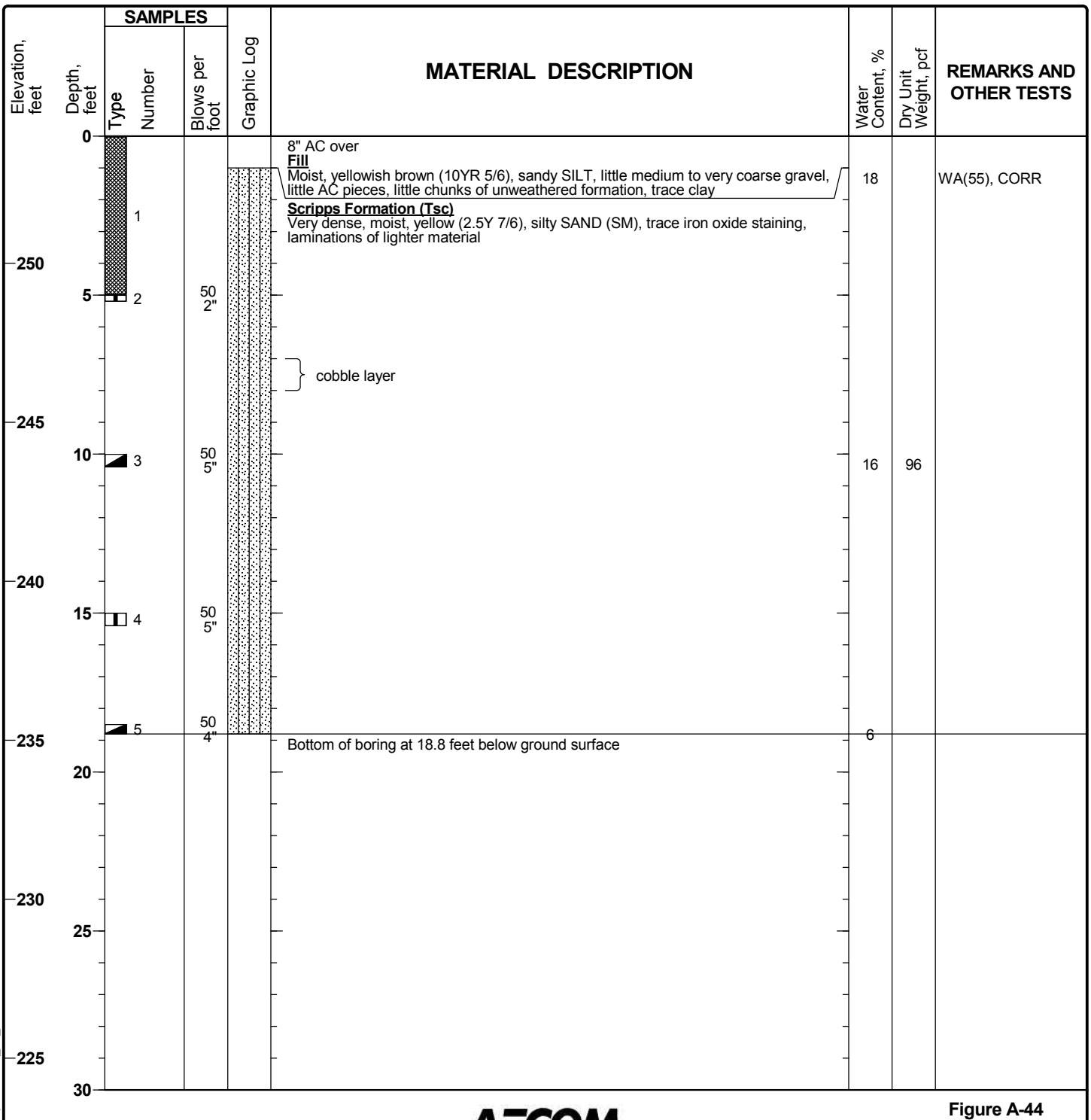
Elevation, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
	Type	Number					
0				7" AC over Fill Very dense, moist, dark yellowish brown (10YR 4/4), silty SAND, some fine to coarse gravel, few small cobbles, fragments of weathered and unweathered formation, trace clay			
330							
5	1	68		increase in iron oxide staining			
325							
10	2	75 12"			14	102	
320							
15	3	76 8"					
315	4	50 5"			13		
20				Bottom of boring at 18.9 feet below ground surface			
310							
25							
305							
30							

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-41

Sheet 1 of 1

Date(s) Drilled	06/30/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	18.8 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	254 feet
Water Level Depth	not encountered	Sampling Method(s)	Bulk / SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.843028, W117.198012		

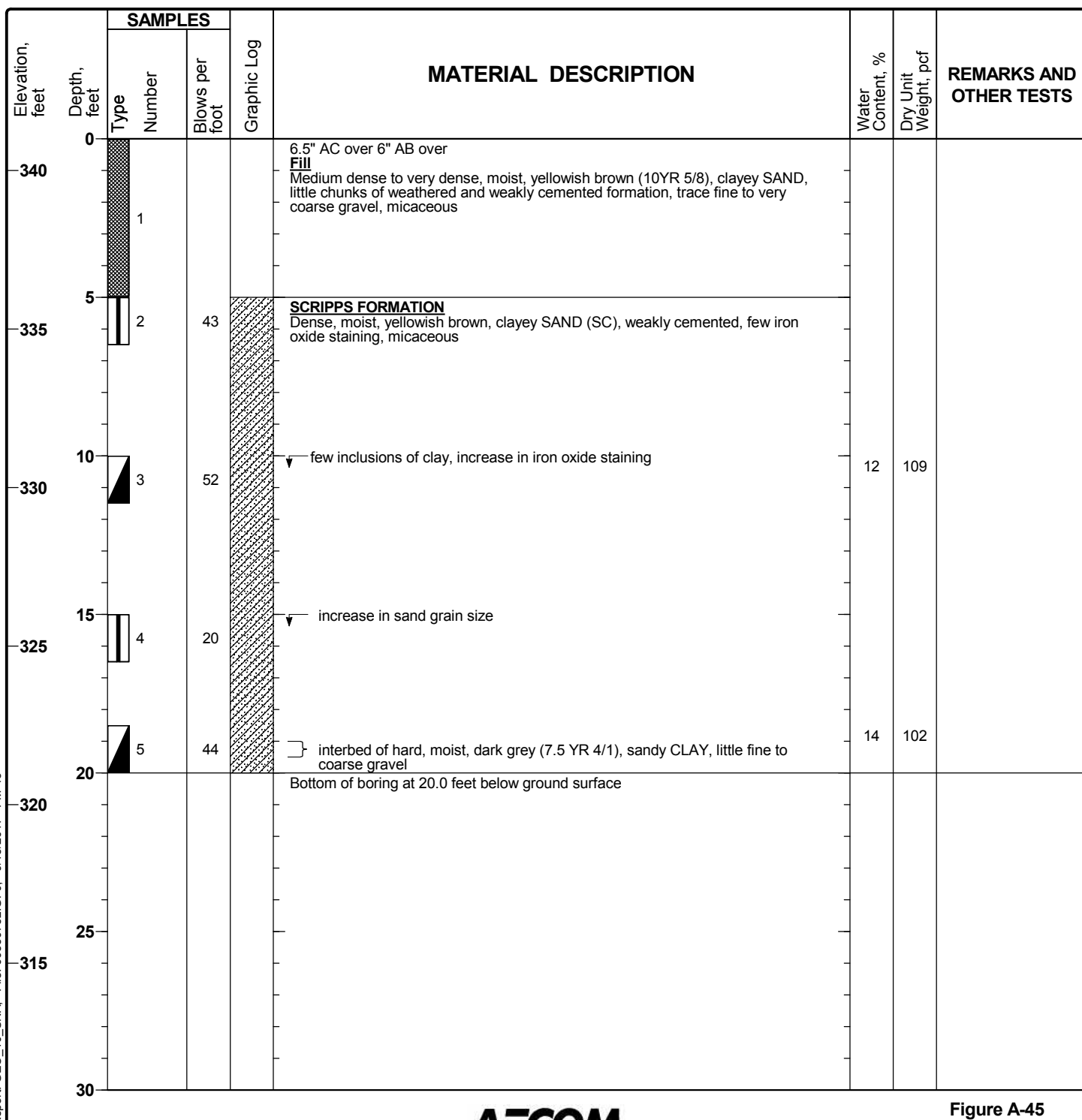


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-43

Sheet 1 of 1

Date(s) Drilled	07/05/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	341 feet
Water Level Depth	not encountered	Sampling Method(s)	Bulk / SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.852015, W117.203768		

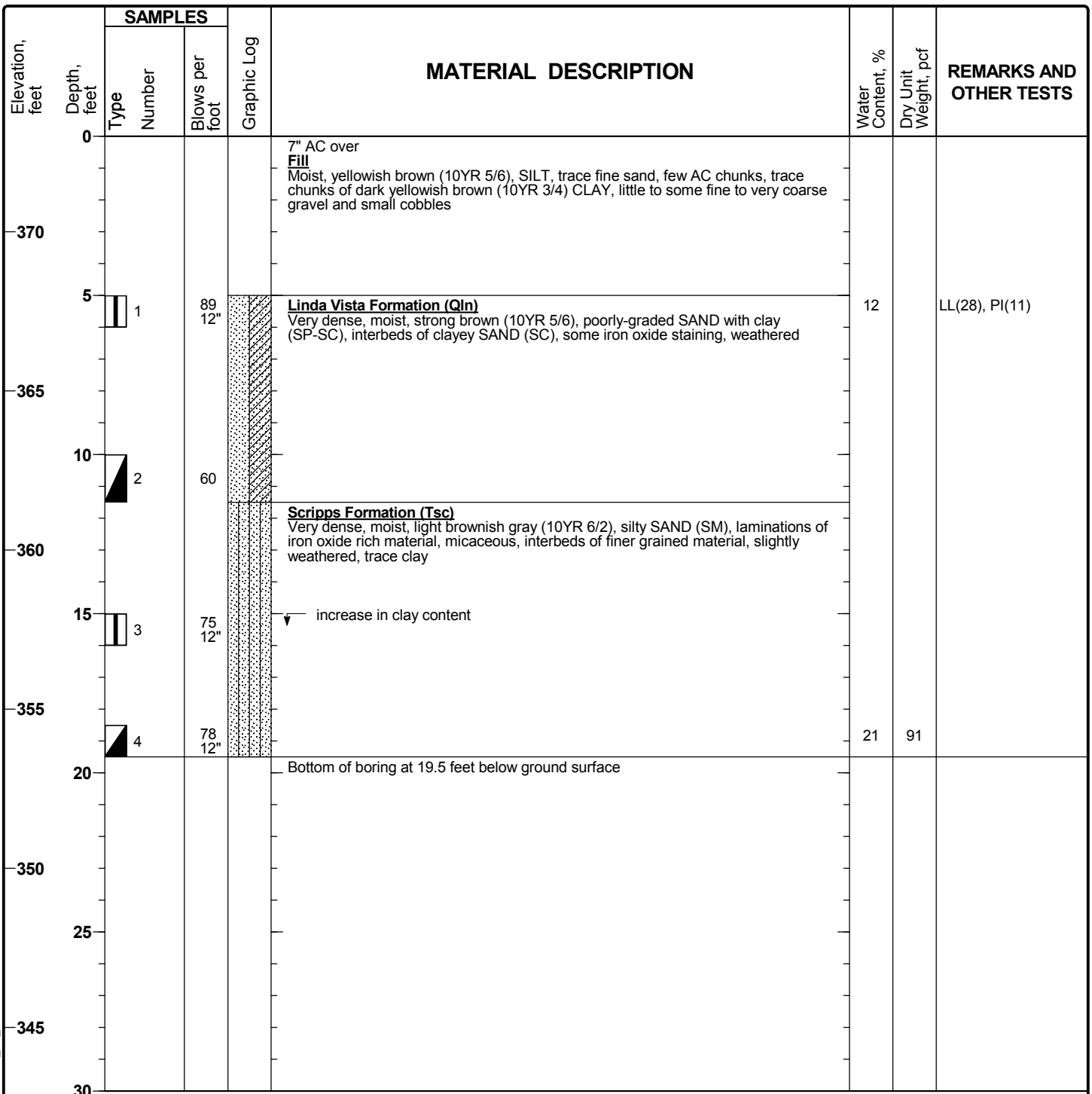


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-44

Sheet 1 of 1

Date(s) Drilled	06/28/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	373 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.854173, W117.204321		

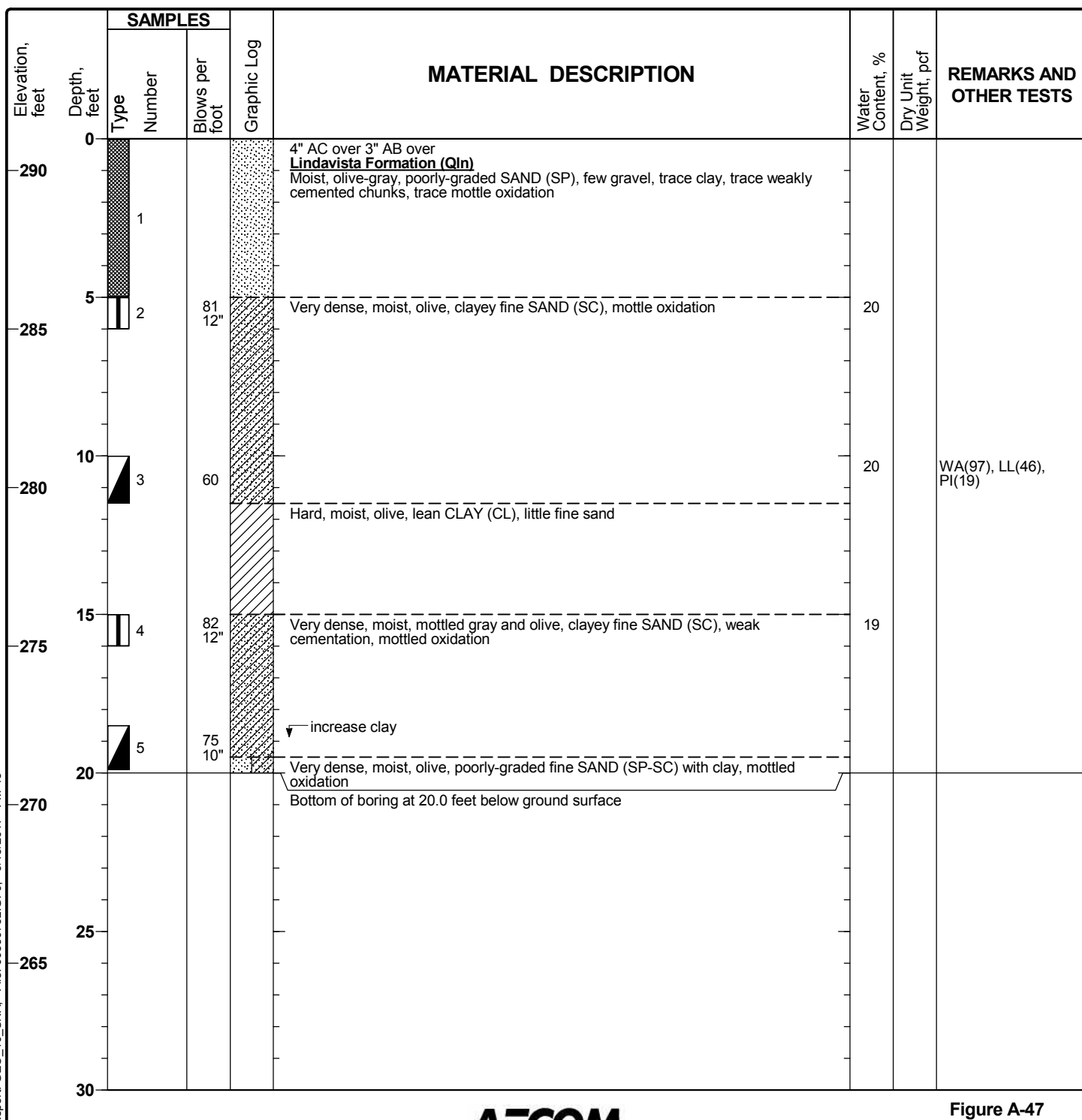


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-45

Sheet 1 of 1

Date(s) Drilled	07/28/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	291 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.858257, W117.205637		

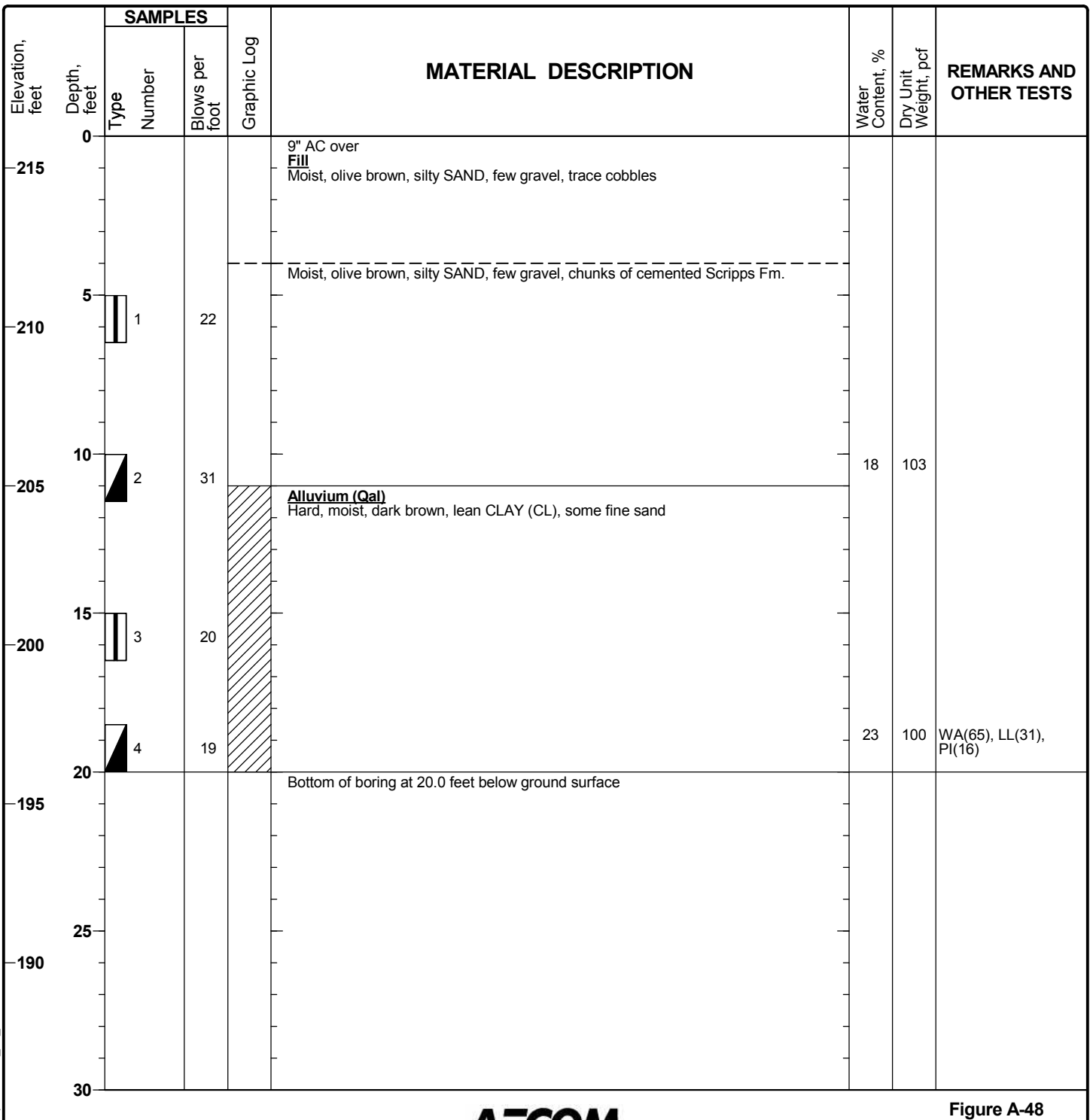


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-46

Sheet 1 of 1

Date(s) Drilled	07/06/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	216 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.860329, W117.208244		

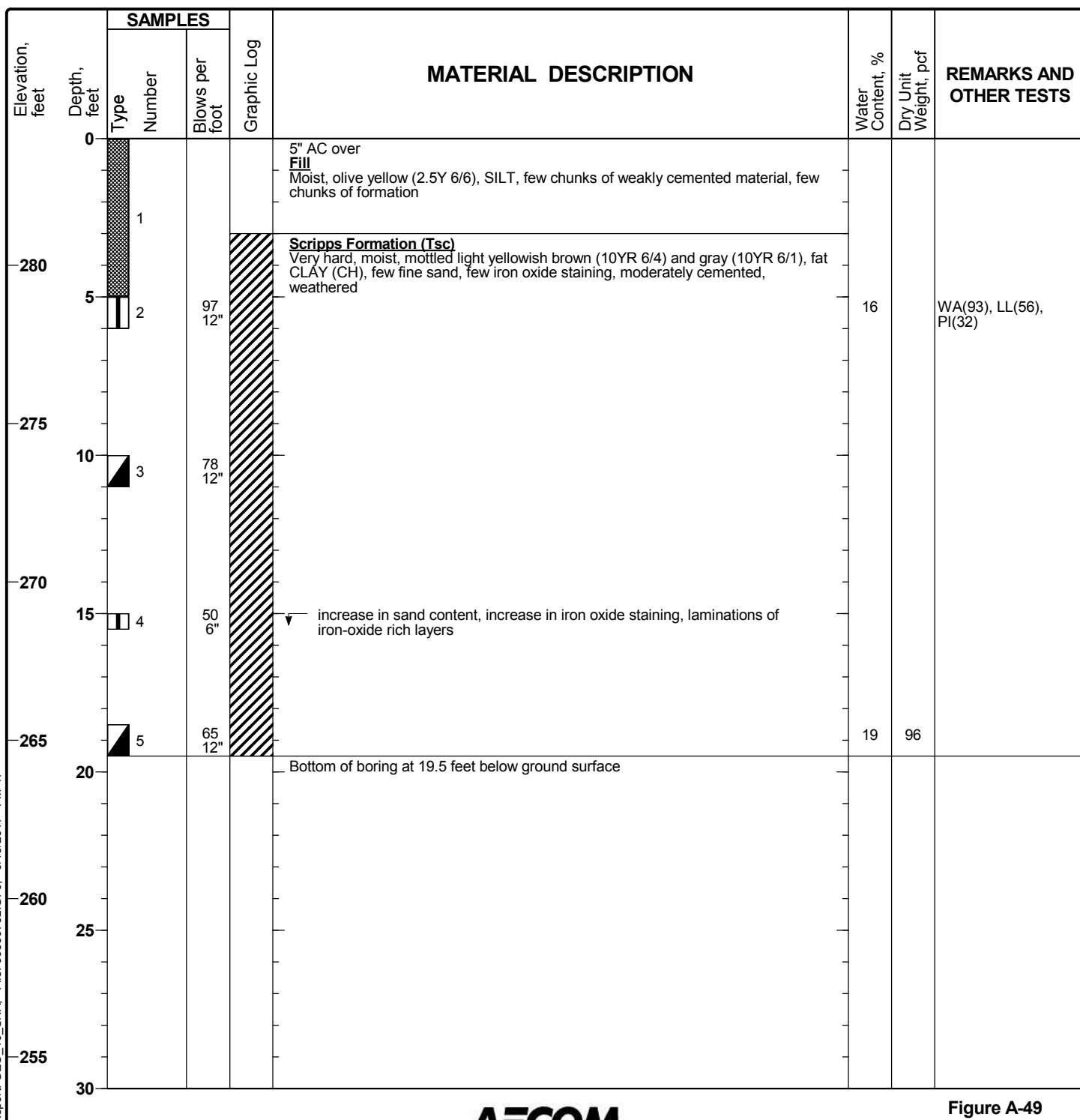


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-47

Sheet 1 of 1

Date(s) Drilled	07/05/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	284 feet
Water Level Depth	not encountered	Sampling Method(s)	Bulk / SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.863786, W117.21234		

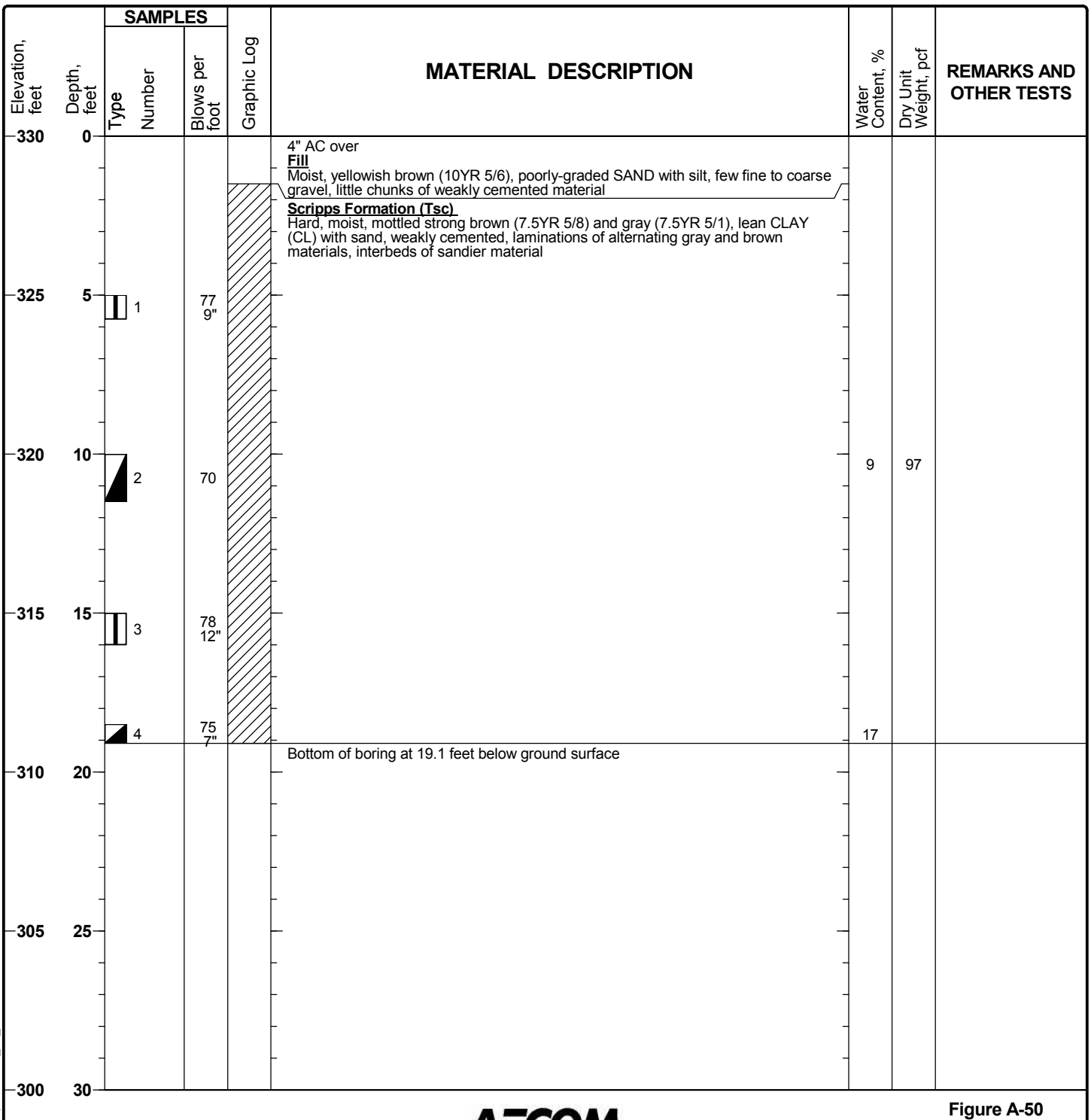


Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-48

Sheet 1 of 1

Date(s) Drilled	07/05/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.1 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	330 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.866619, W117.213338		



Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-49

Sheet 1 of 1

Date(s) Drilled	07/06/2017	Logged By	R. Bourdette	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.8 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	301 feet
Water Level Depth	not encountered	Sampling Method(s)	Bulk / SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.865829, W117.208979		

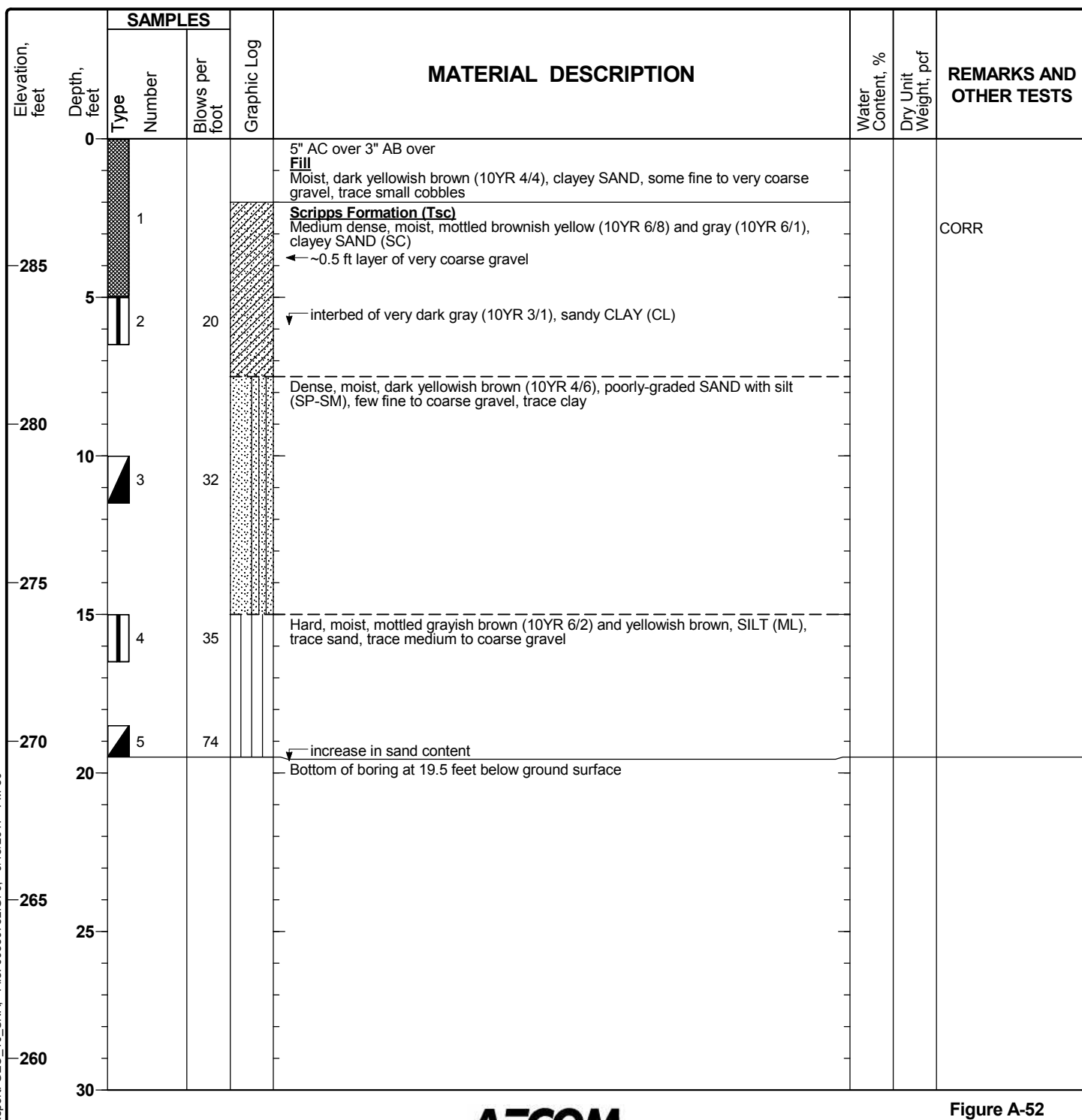
Elevation, feet	SAMPLES			Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
	Type	Number	Blows per foot					
0					5.5" AC over Fill			
300					Moist, yellow brown, silty fine to medium SAND, trace gravel	13		WA(34), CORR
	1				<u>Scripps Formation (Tsc)</u> Very dense, moist, yellow brown, silty fine SAND (SM), trace gravel, some slightly cemented chunks of formation, interbeds of olive-brown silty SAND and oxidized yellow-red poorly-graded SAND			
5								
295	2		78 12"		Very dense, moist, pale gray brown, poorly-graded SAND (SP-SM) with silt, trace oxidation, interbeds of very oxidized, yellow-red layers	18		
10	3		75 12"			14	98	
290					Very dense, moist, light gray, poorly graded SAND (SP)			
15	4		84 12"		↙ increase in oxidation	7		
285								
	5		90 9"		↙ increase in SILT content (SP-SM) ↙ becomes yellow brown			
20					Bottom of boring at 19.8 feet below ground surface			
280								
25								
275								
30								

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-50

Sheet 1 of 1

Date(s) Drilled	7/24/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.5 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	289 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.86825, W117.207153		



Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-51a

Sheet 1 of 1

Date(s) Drilled	7/24/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	4.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	379 feet
Water Level Depth	not encountered	Sampling Method(s)	none	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.872422, W117.20358		

Elevation, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
	Type	Number					
0				6" AC over 2" AB over Fill Moist, yellowish brown, clayey SAND, little coarse gravel and large cobbles, few chunks of strongly cemented formation ----- Moist, dark yellowish brown, CLAY			
375				Bottom of boring at 4 feet below ground surface, refusal on possible slurry			
5							
370							
10							
365							
15							
360							
20							
355							
25							
350							
30							

Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-51b

Sheet 1 of 1

Date(s) Drilled	7/24/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	3.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	379 feet
Water Level Depth	not encountered	Sampling Method(s)	none	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.872397, W117.207355		

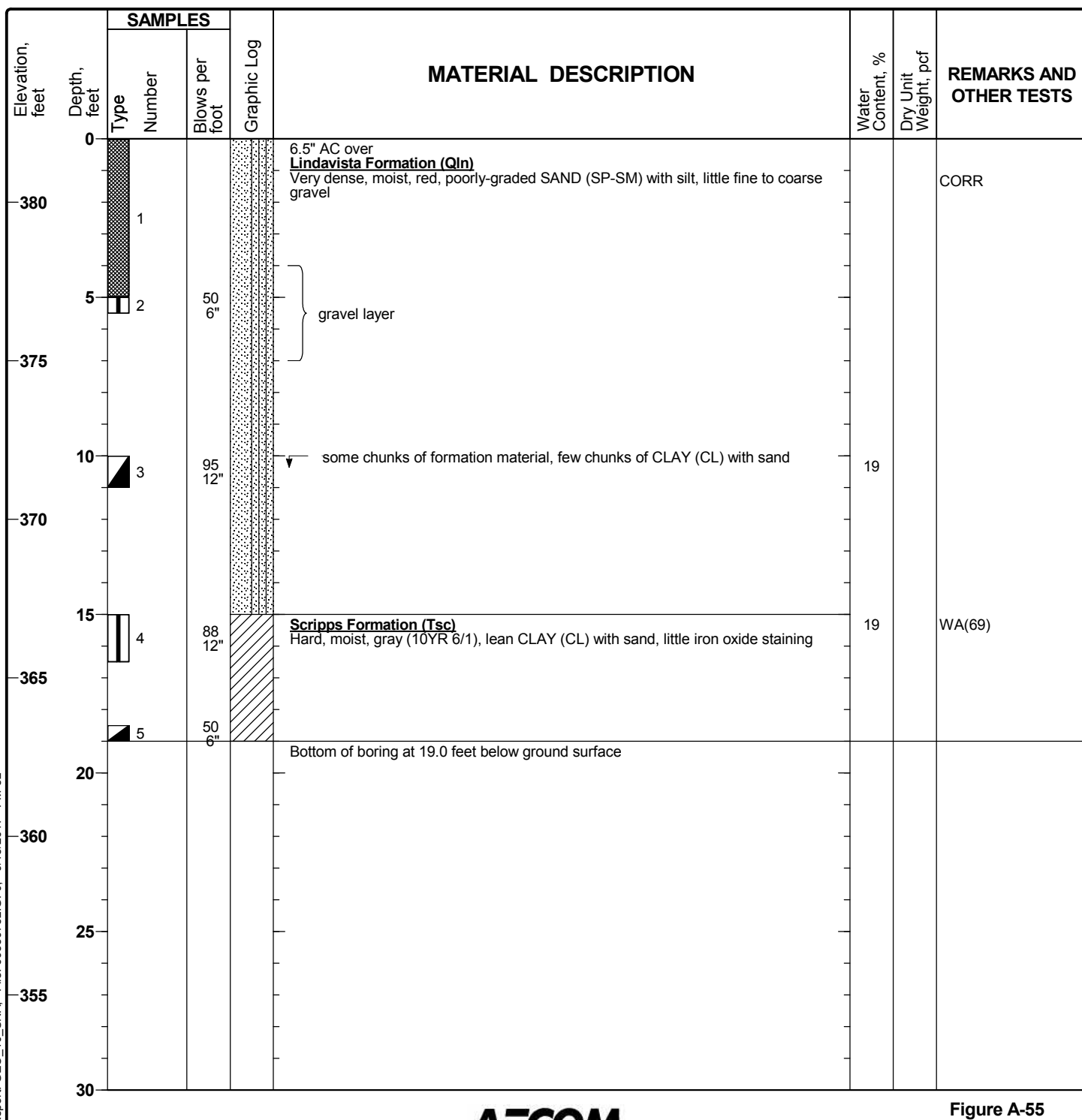
Elevation, feet	Depth, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
		Type	Number					
0					6" AC over 2" AB over Fill Moist, olive-brown, clayey SAND, few fine to medium gravel cobble/sandier layer becomes sandy CLAY with an odor, possibly contaminated			
375					Bottom of boring at 3 feet below ground surface, refusal on possible concrete			
5								
370								
10								
365								
15								
360								
20								
355								
25								
350								
30								

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring FM-52

Sheet 1 of 1

Date(s) Drilled	07/07/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	19.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	382 feet
Water Level Depth	not encountered	Sampling Method(s)	Bulk / SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.876399, W117.207639		



Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-53a

Sheet 1 of 1

Date(s) Drilled	07/07/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	2.0 feet
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	359 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.876687, W117.205001		

Elevation, feet	Depth, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
		Type	Number					
0					6" AC over 8" gravelly slurry over 6" loose gravelly sand over 6" gravelly slurry			
					Bottom of boring at 2 feet below ground surface			
355	5							
350	10							
345	15							
340	20							
335	25							
330	30							

Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring FM-53b

Sheet 1 of 1

Date(s) Drilled	07/07/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	1.0 foot
Drill Rig Type	CME 85	Drilling Contractor	Cascade Drilling	Approximate Surface Elevation	359 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.876684, W117.20493		

Elevation, feet	Depth, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
		Type	Number					
0					6" AC over gravelly slurry			
					Bottom of boring at 1.0 feet below ground surface			
355	5							
350	10							
345	15							
340	20							
335	25							
330	30							

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring PS-1

Sheet 1 of 2

Date(s) Drilled	03/27/2017	Logged By	A . Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	61.5 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	13.5 feet
Water Level Depth	9.0	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cement/Bentonite Grout	Location	N32.76323, W117.1995		

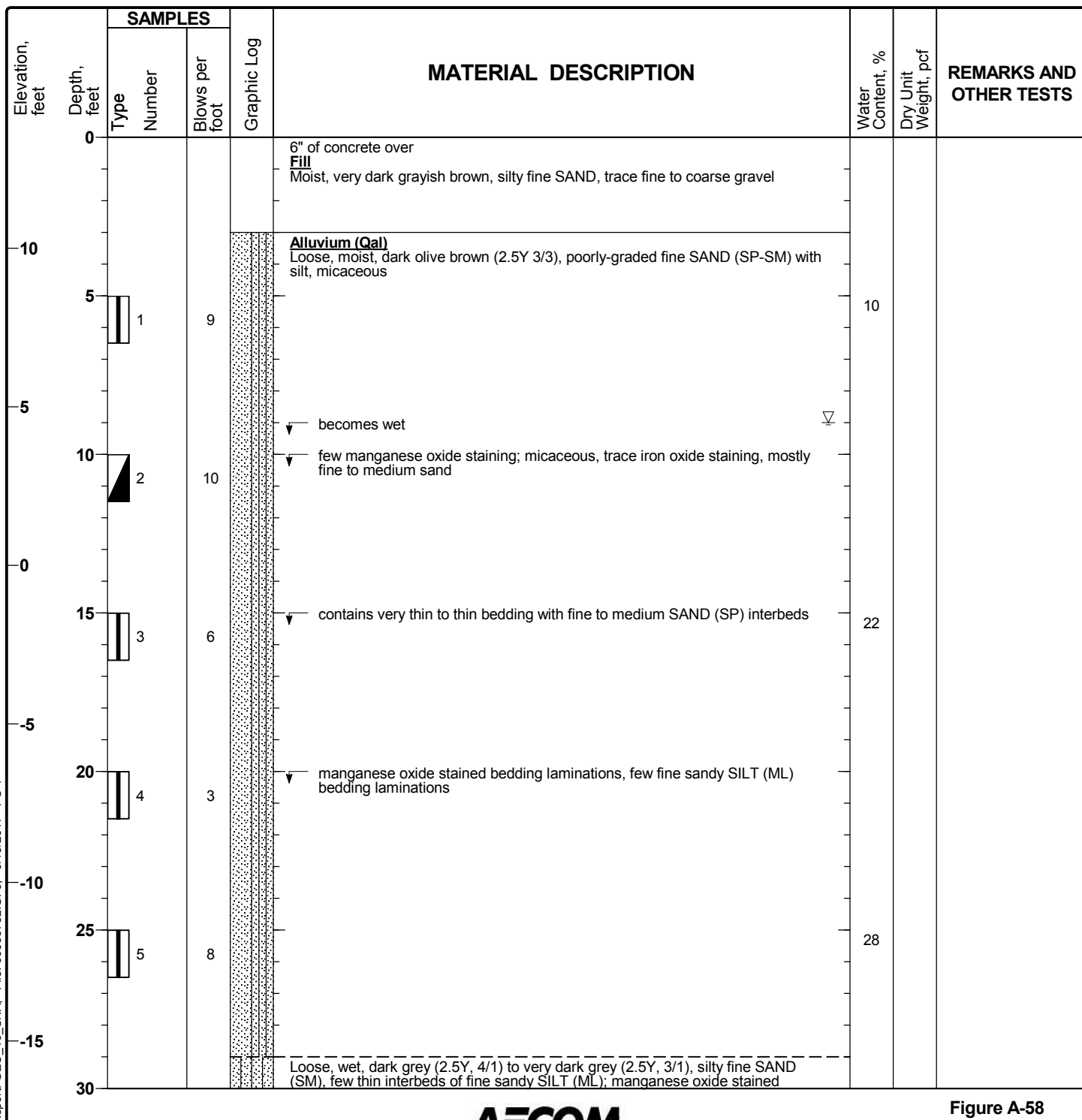
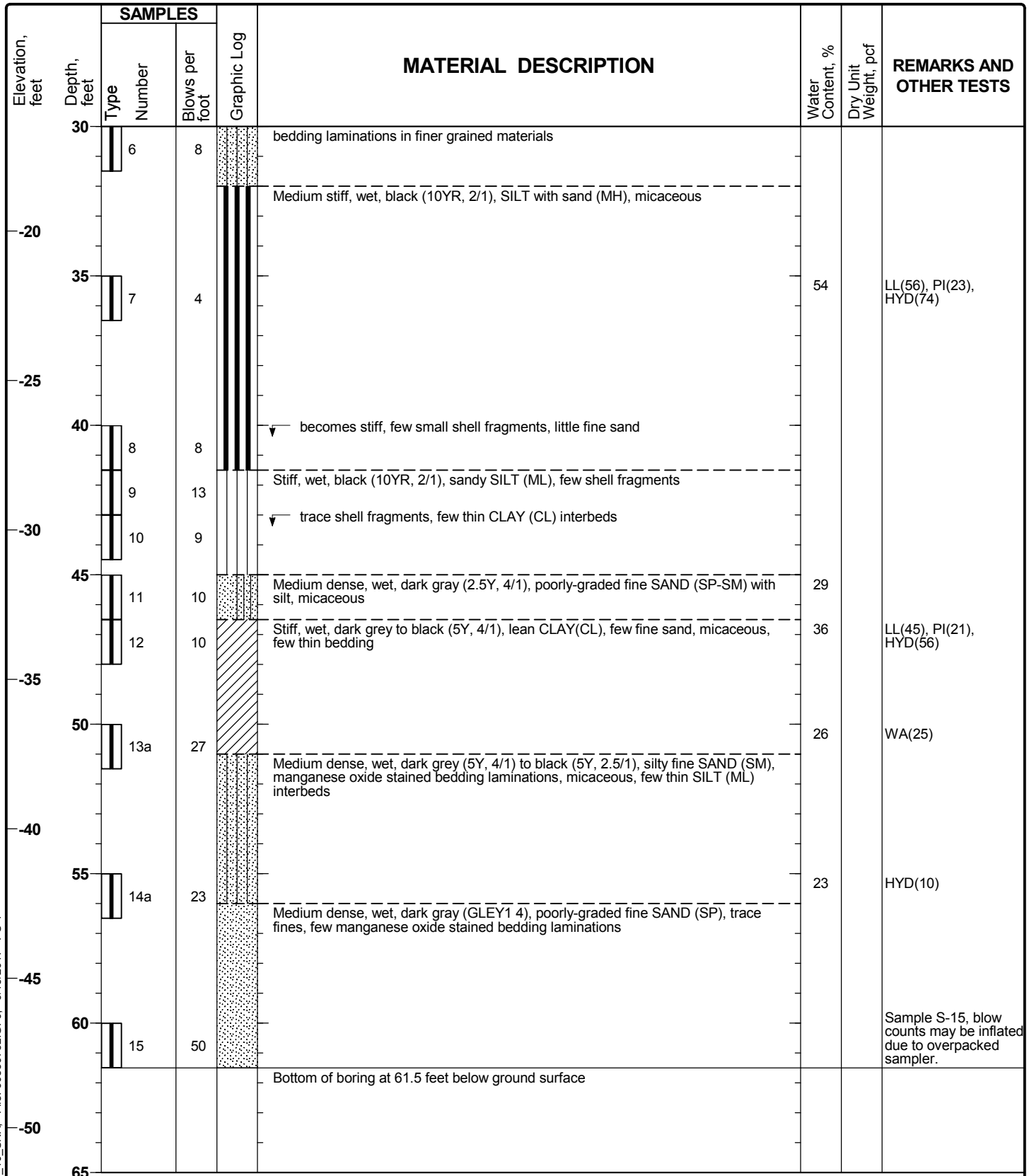


Figure A-58

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring PS-1

Sheet 2 of 2



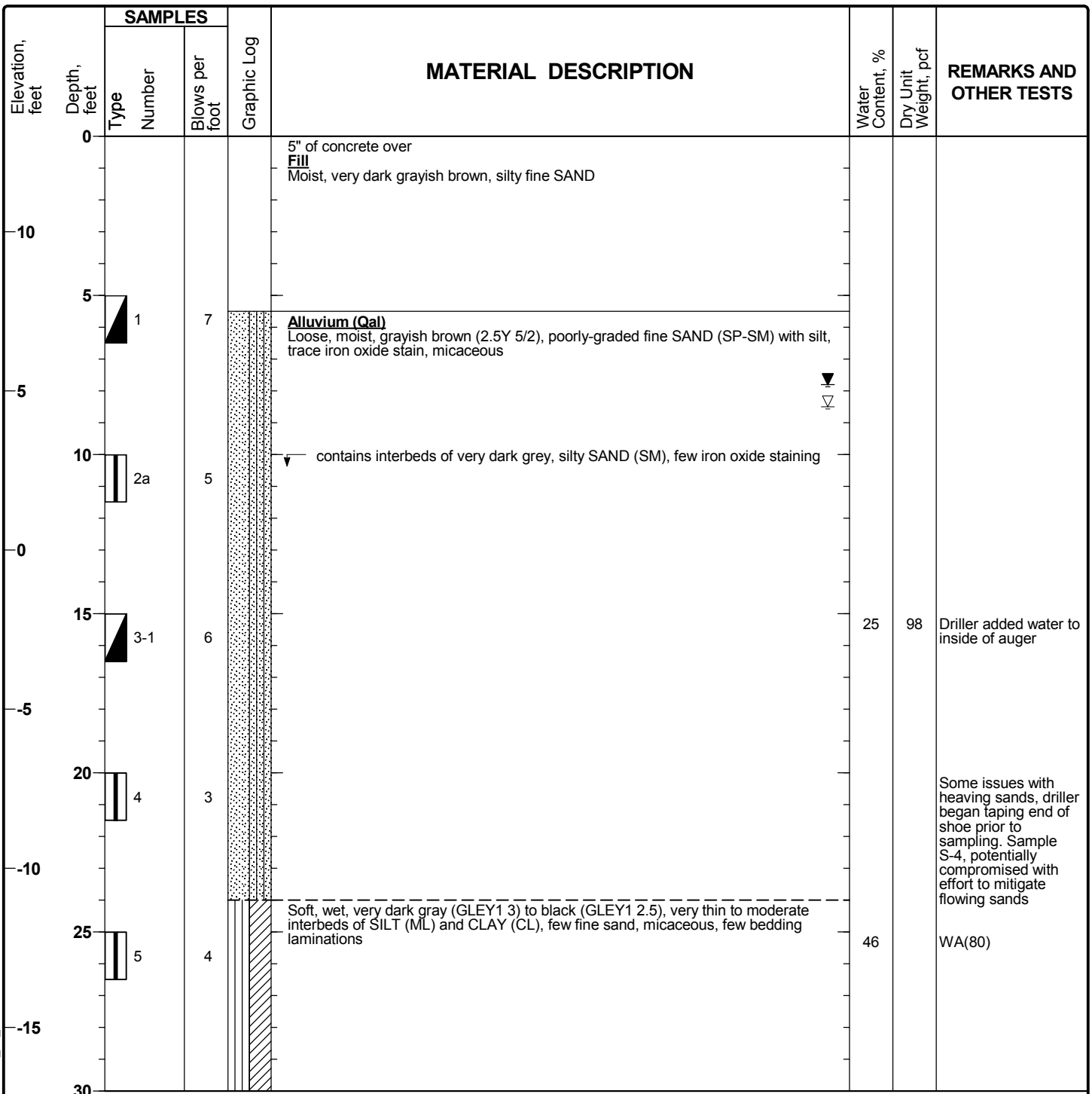
Report: GEO_10_SNA: File: 60530732.GPJ: 9/15/2017 PS-1

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring PS-2

Sheet 1 of 2

Date(s) Drilled	03/28/2017	Logged By	A . Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	65.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	13.0 feet
Water Level Depth	7.8 ft	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Well Construction Per County Well Permit Guidelines		Location	N32.76364, W117.20007	

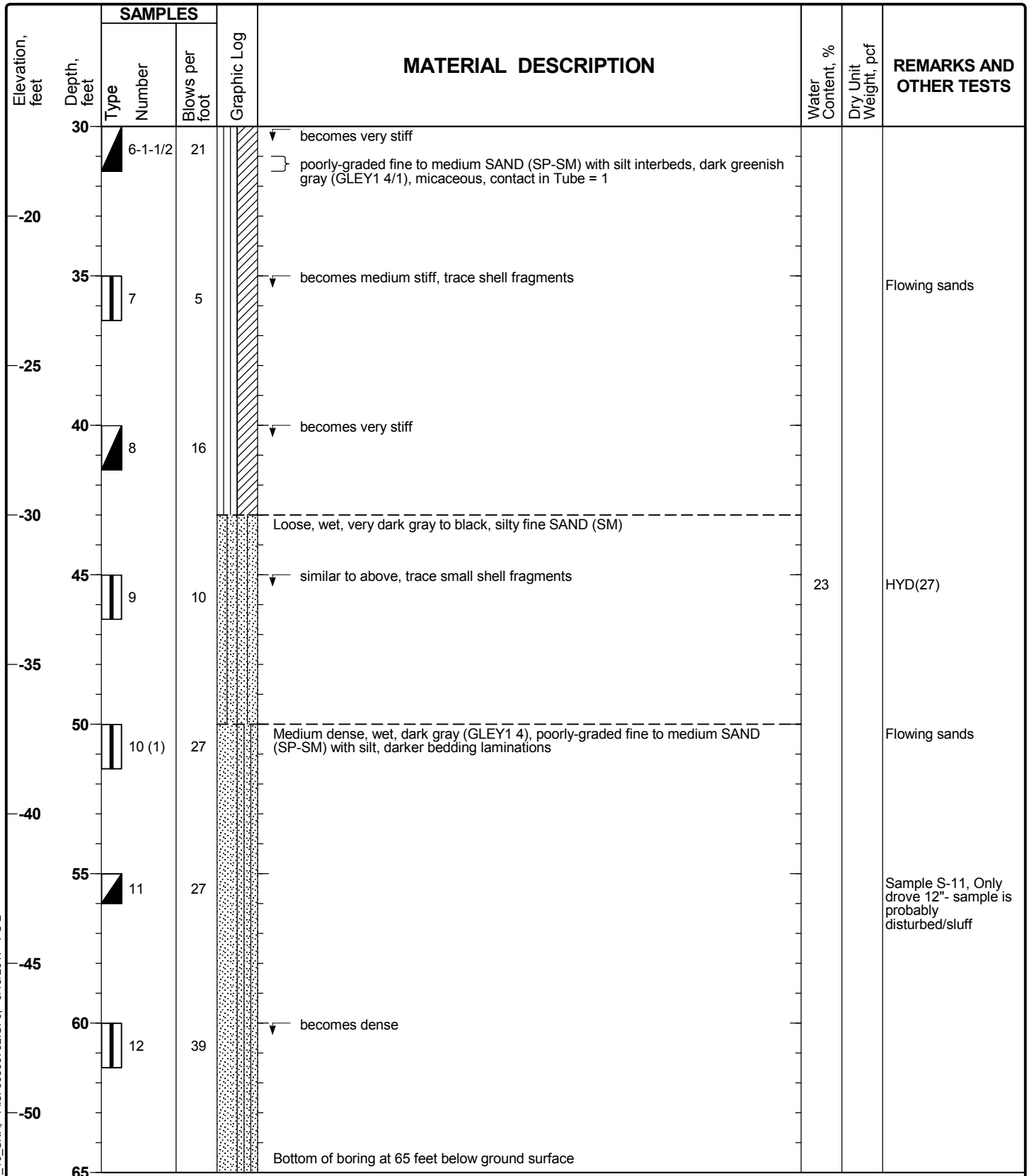


Report: GEO_10_SNA- File: 60530732.GPJ- 9/15/2017 PS-2

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring PS-2

Sheet 2 of 2



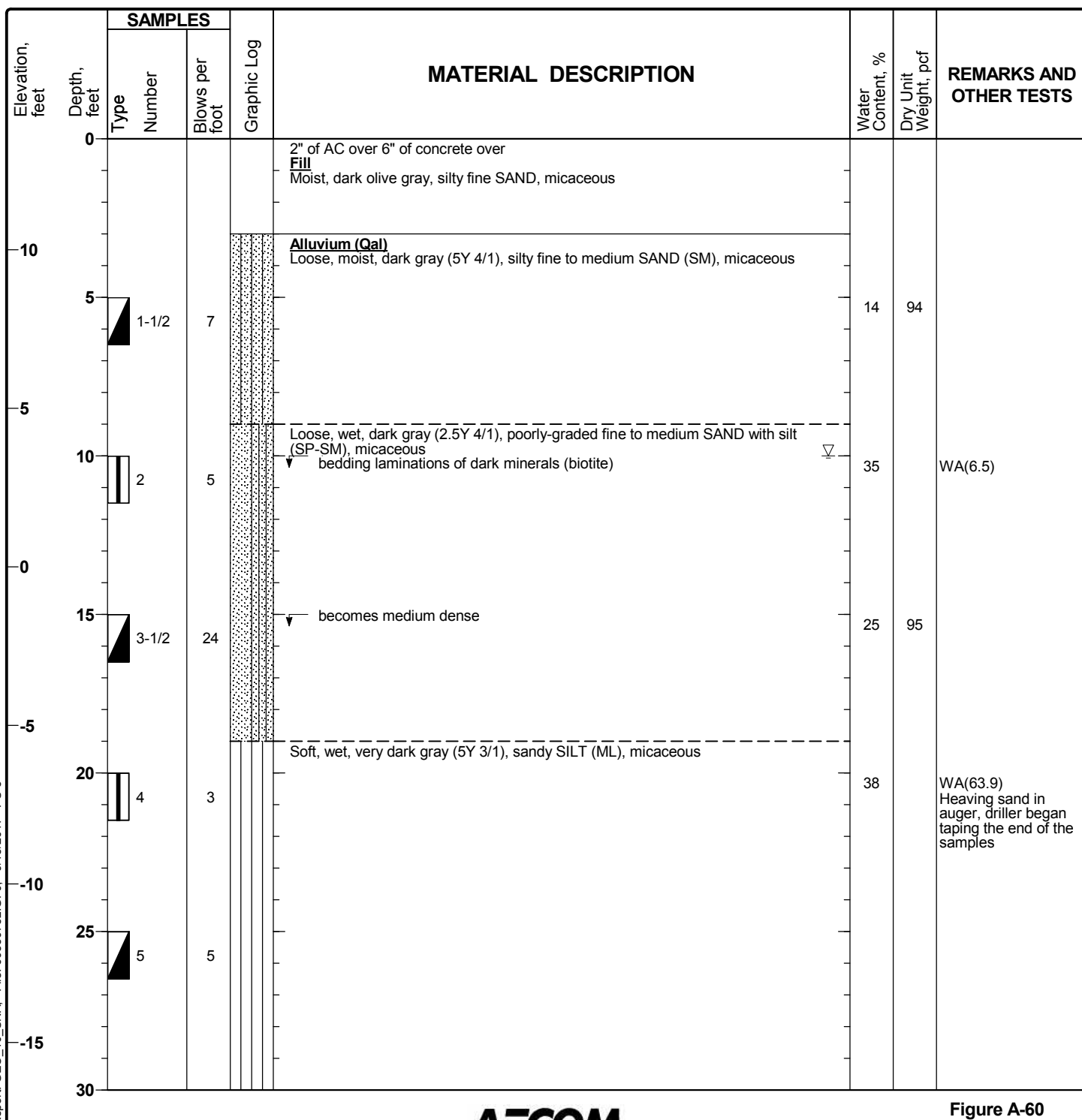
Report: GEO_10_SNA: File: 60530732.GPJ: 9/15/2017 PS-2

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring PS-3

Sheet 1 of 3

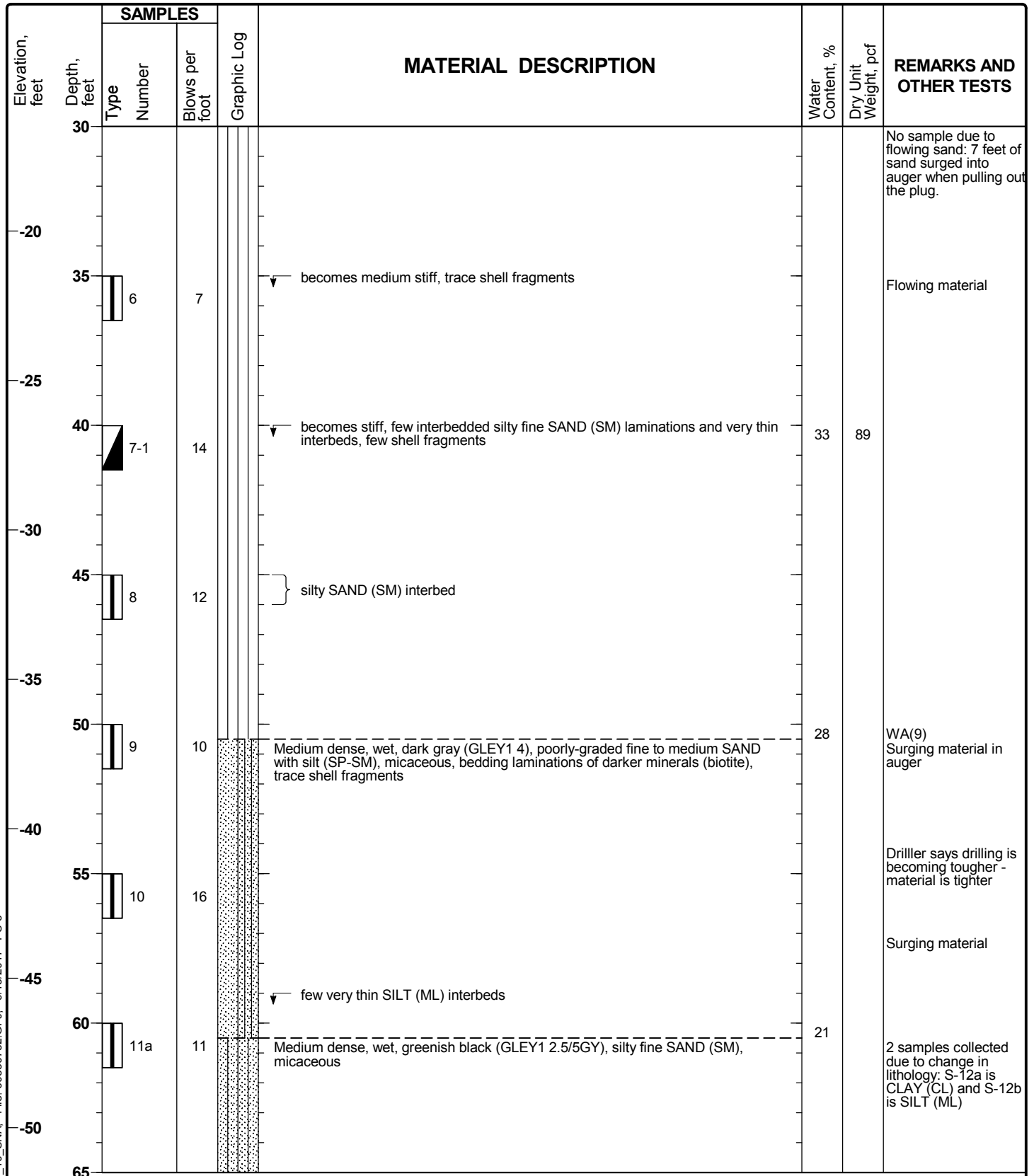
Date(s) Drilled	04/03/2017	Logged By	D. Rector	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	8-inch	Total Depth of Borehole	81.0 feet
Drill Rig Type	Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	13.5 feet
Water Level Depth	10.0	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cement/Bentonite Grout	Location	N32.76344, W117.20058		



Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring PS-3

Sheet 2 of 3

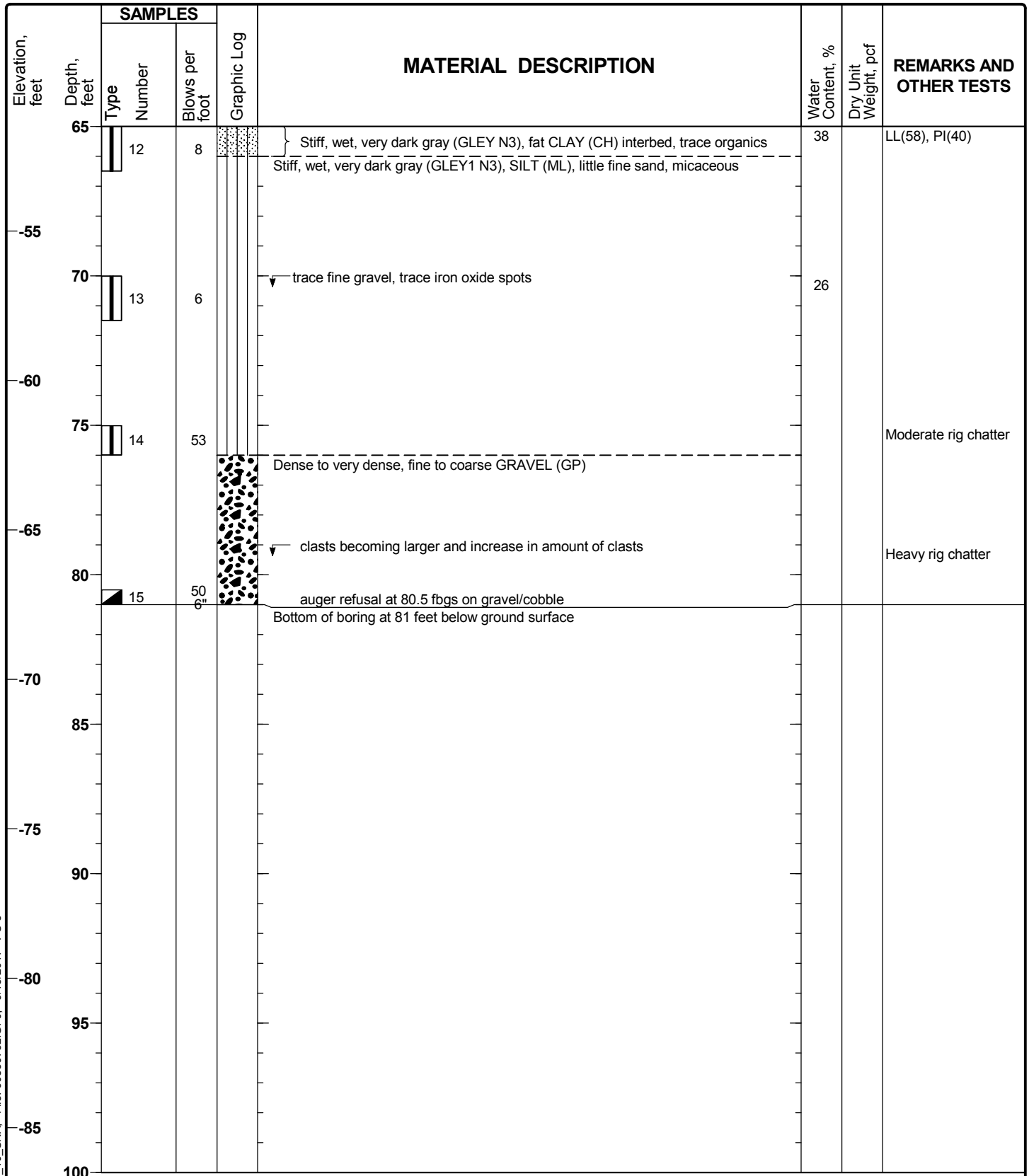


Report: GEO_10_SNA: File: 60530732.GPJ: 9/15/2017 PS-3

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring PS-3

Sheet 3 of 3

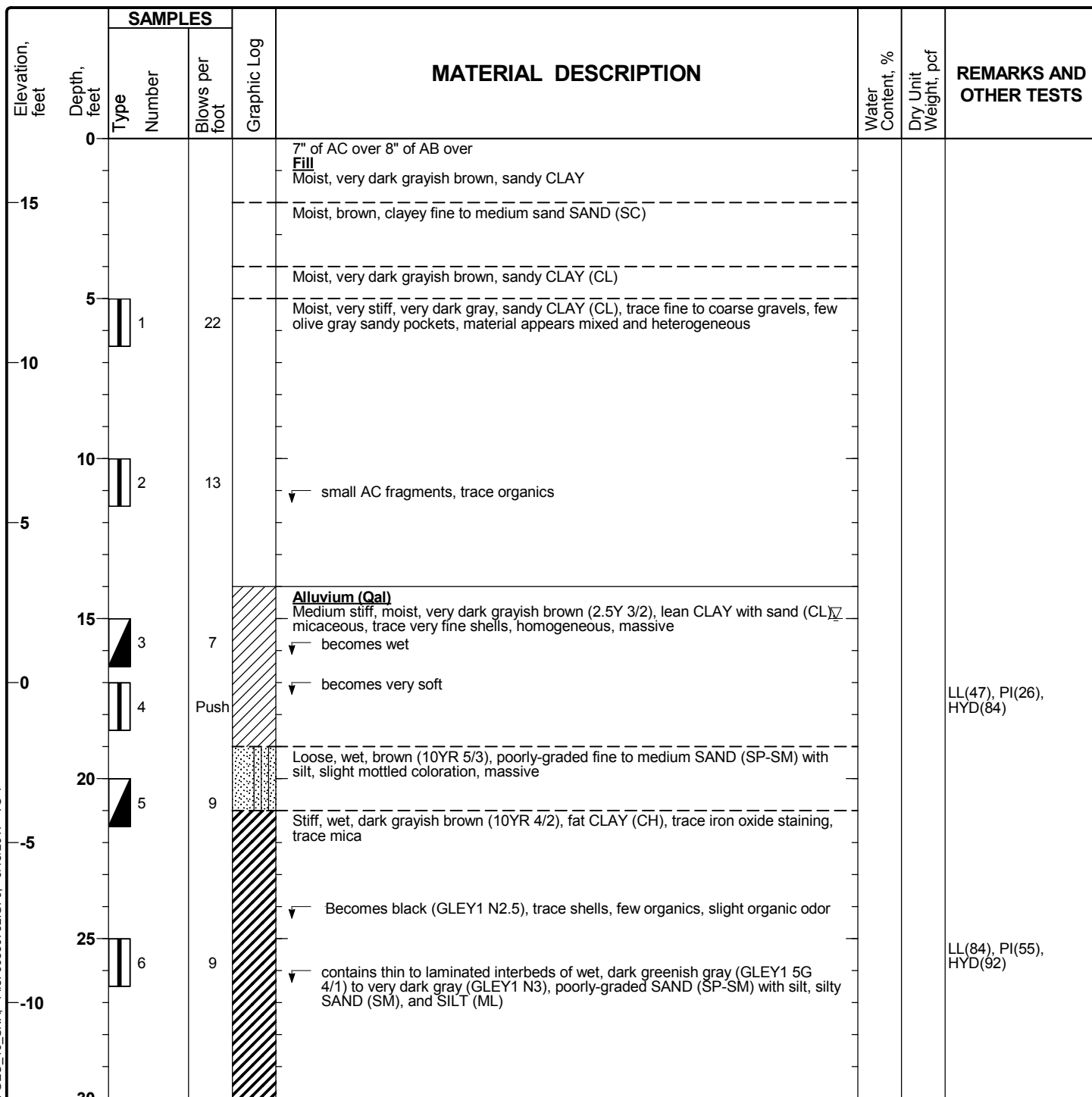


Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring TC-1

Sheet 1 of 3

Date(s) Drilled	04/06/17, 04/07/17	Logged By	A . Avakian	Checked By	S. Fitzwilliam
Drilling Method	HSA / Mud Rotary	Drill Bit Size/Type	8-inch / 4" drag bit and tri-cone bit	Total Depth of Borehole	96.5 feet
Drill Rig Type	Diedrich D-50 Turbo	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	17 feet
Water Level Depth	15.0 ft	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cement/Bentonite Grout	Location	N32.77068, W117.20457		

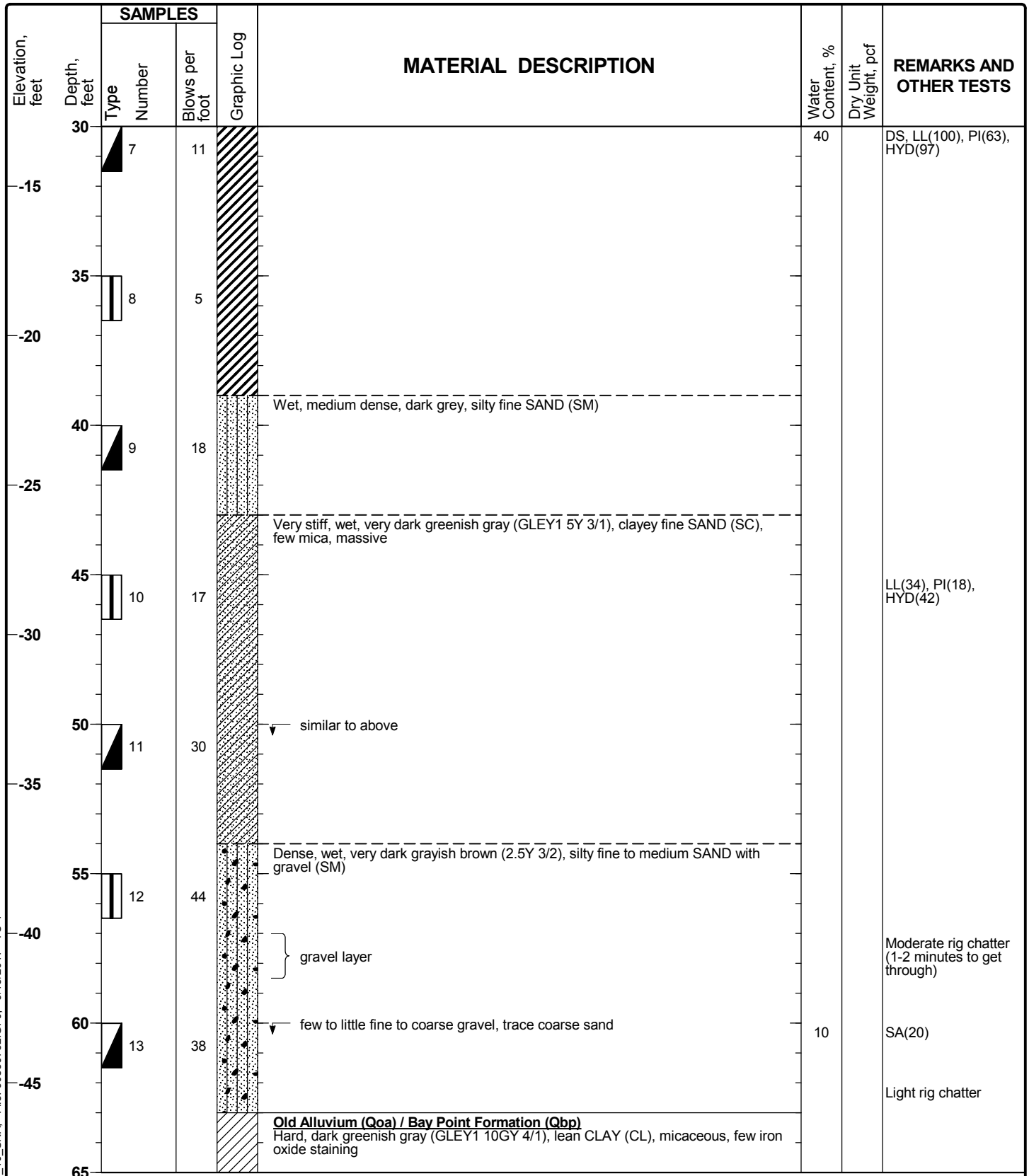


Report: GEO_10_SNA: File: 60530732.GPJ: 9/15/2017 TC-1

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring TC-1

Sheet 2 of 3

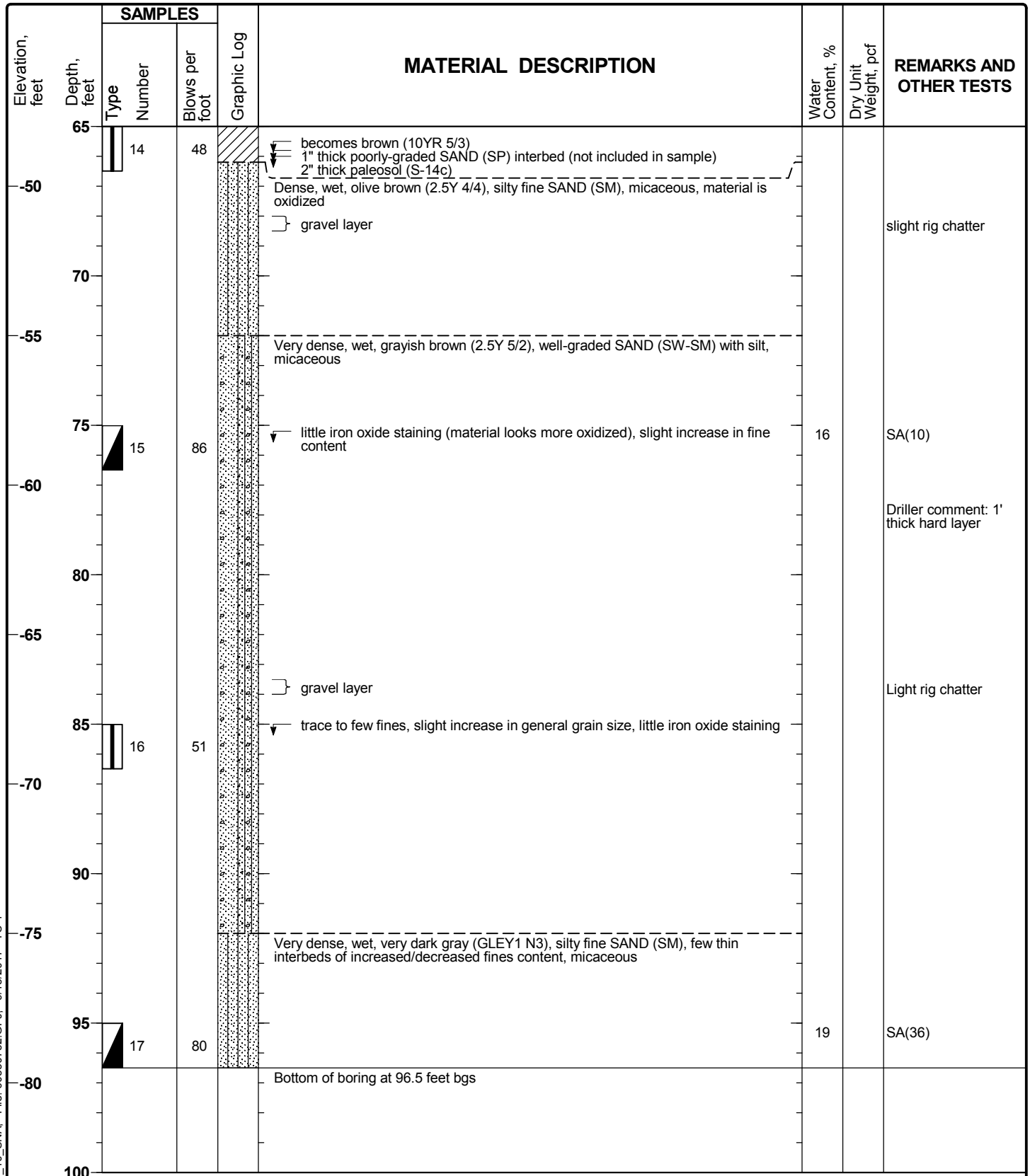


Report: GEO_10_SNA: File: 60530732.GPJ: 9/15/2017 TC-1

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring TC-1

Sheet 3 of 3



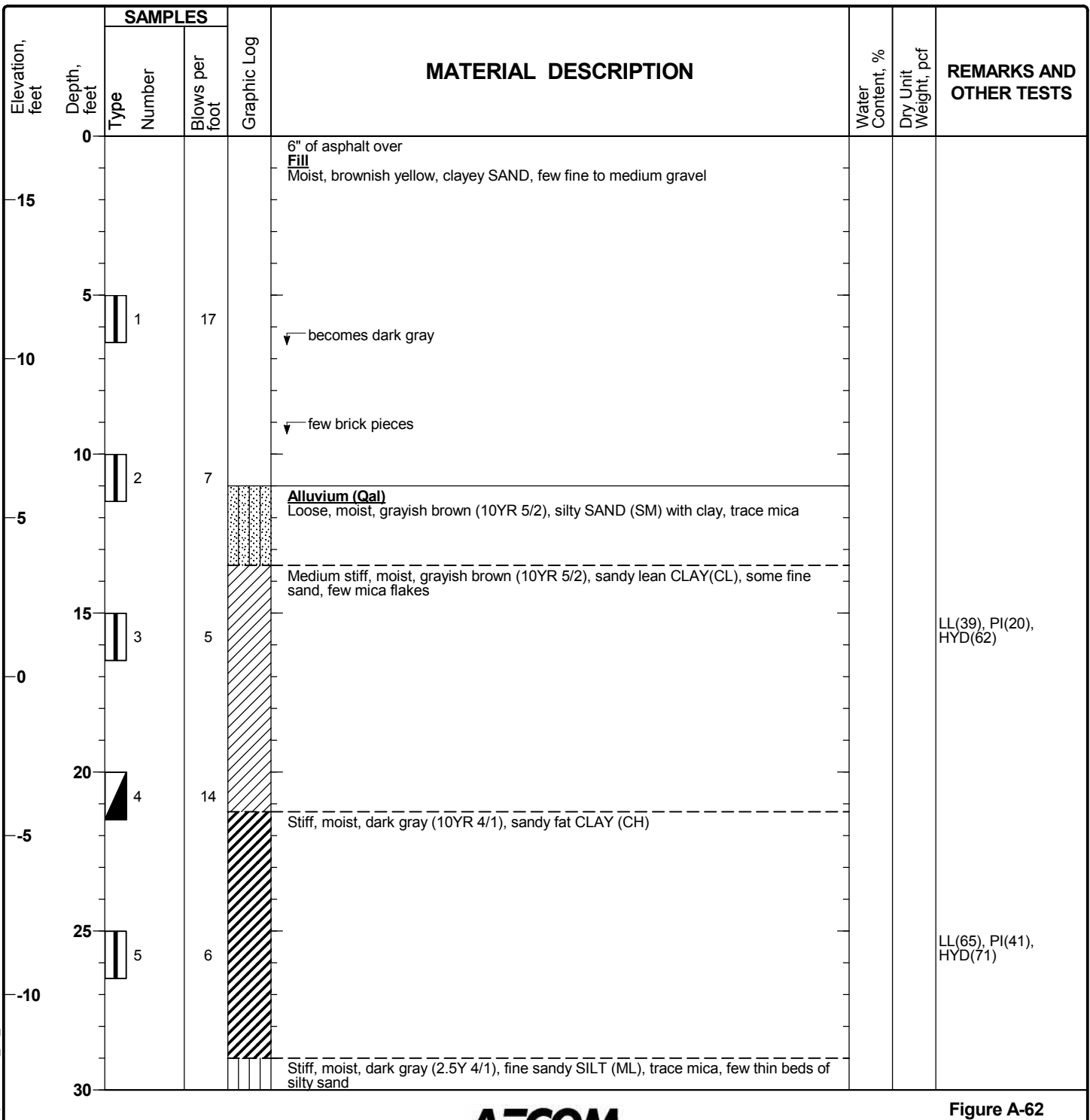
Report: GEO_10_SNA- File: 60530732.GPJ- 9/15/2017 TC-1

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring TC-2

Sheet 1 of 3

Date(s) Drilled	04/10/17 - 04/11/17	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	HSA / Mud Rotary	Drill Bit Size/Type	8-inch / 4" drag bit and tri-cone bit	Total Depth of Borehole	96.5 feet
Drill Rig Type	Diedrich D-50 Turbo	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	17 feet
Water Level Depth	not measured	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cement/Bentonite Grout	Location	N32.77146, W117.20521		

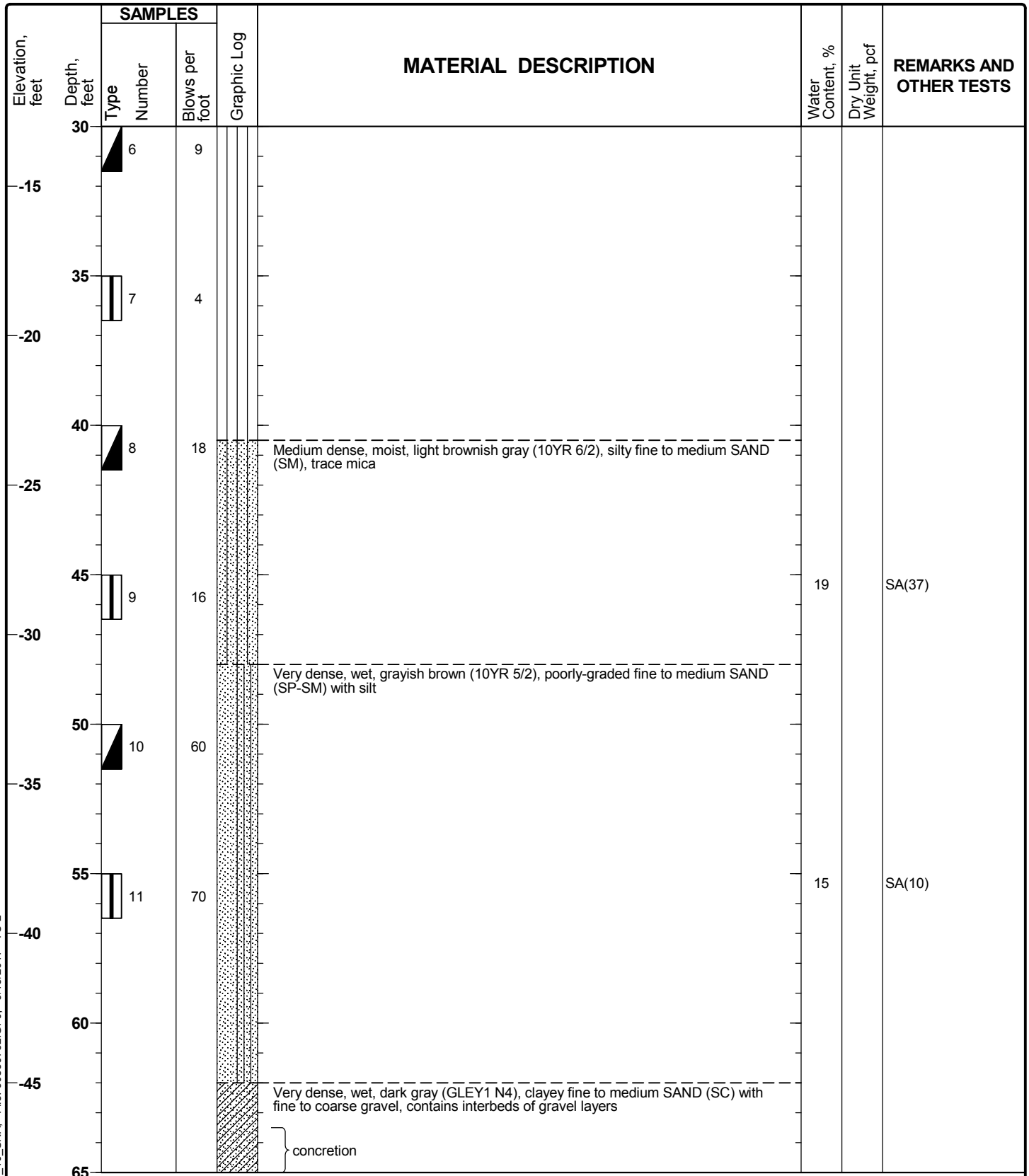


Report: GEO_10_SNA- File: 60530732.GPJ- 9/15/2017 TC-2

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring TC-2

Sheet 2 of 3

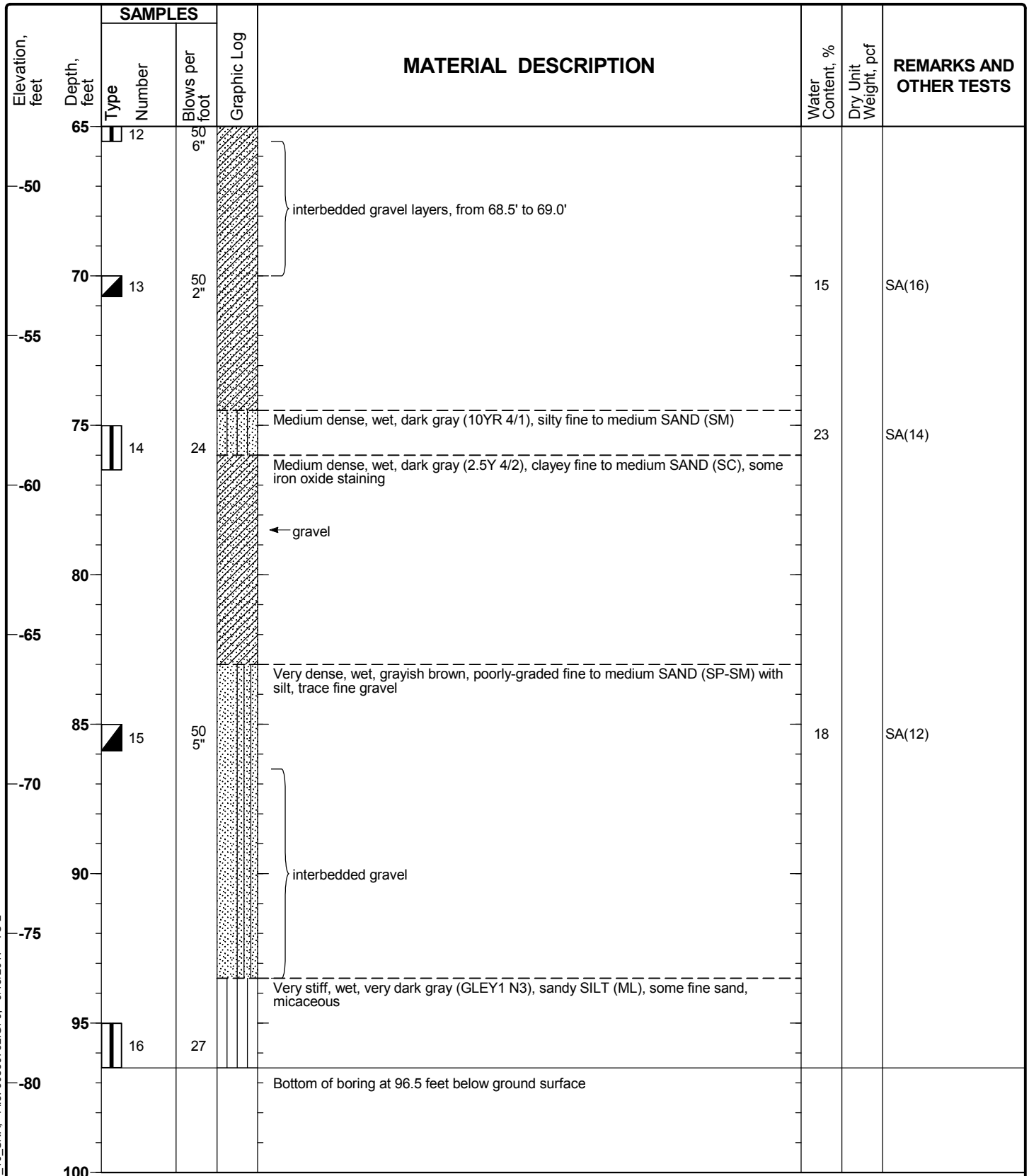


Report: GEO_10_SNA: File: 60530732.GPJ: 9/15/2017 TC-2

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring TC-2

Sheet 3 of 3



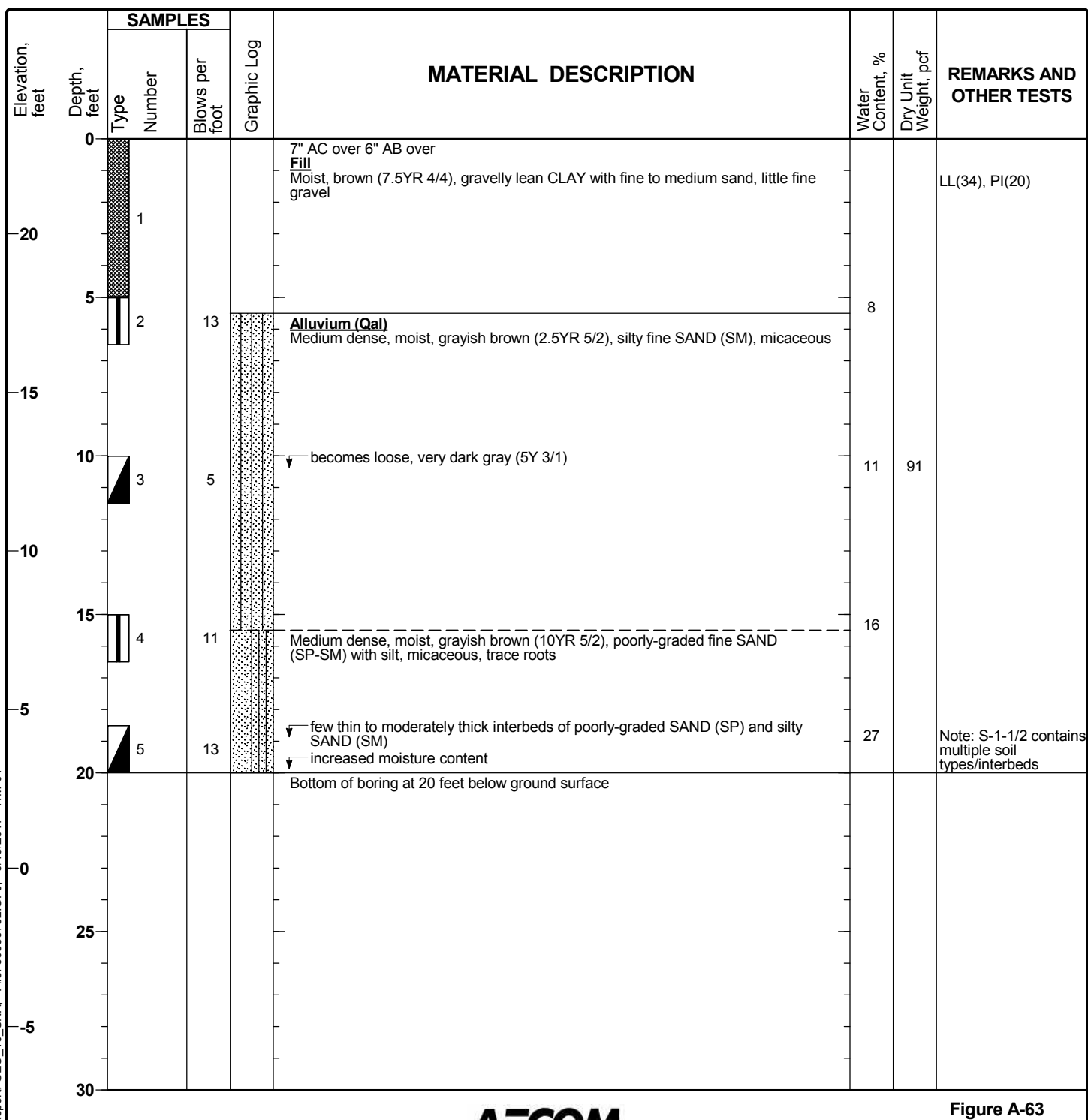
Report: GEO_10_SNA- File: 60530732.GPJ- 9/15/2017 TC-2

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-01

Sheet 1 of 1

Date(s) Drilled	06/08/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	23 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.76391, W117.1953		

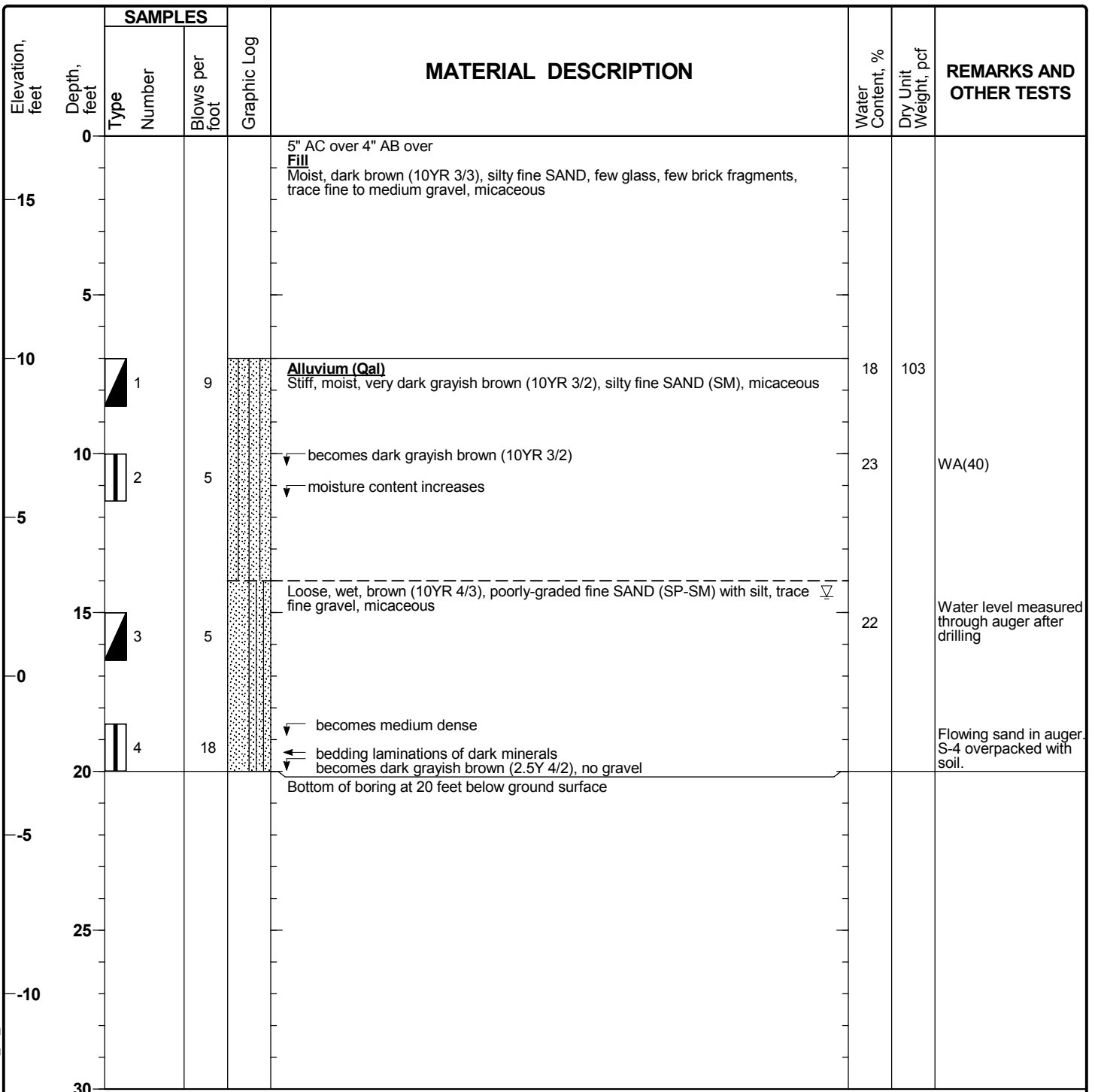


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-02

Sheet 1 of 1

Date(s) Drilled	06/08/2017	Logged By	A. Avakian / E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	17 feet
Water Level Depth	14.5 ft	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.76464, W117.19582		

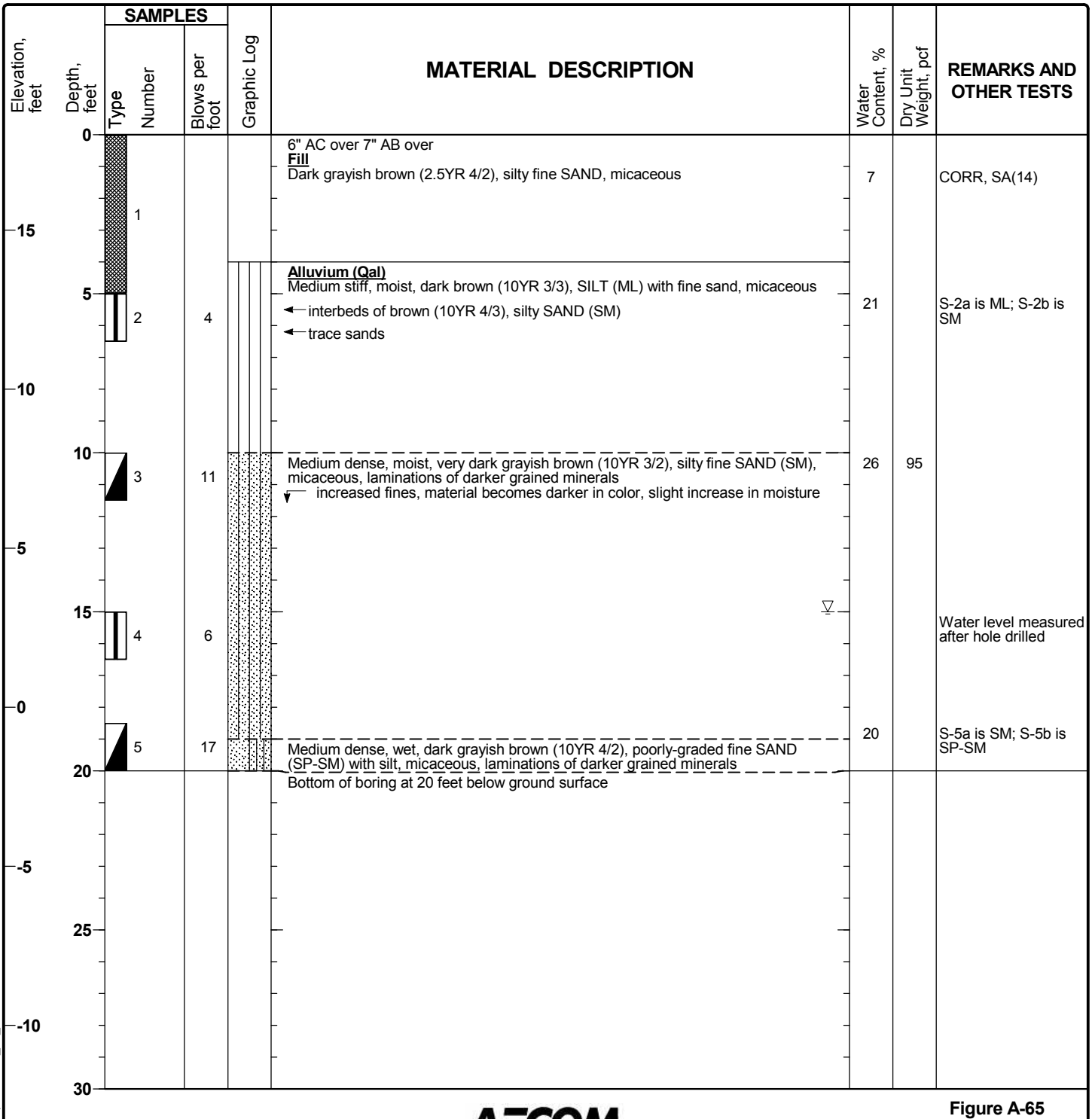


Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring WM-03

Sheet 1 of 1

Date(s) Drilled	06/08/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	18 feet
Water Level Depth	15 ft	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.765073, W117.196454		



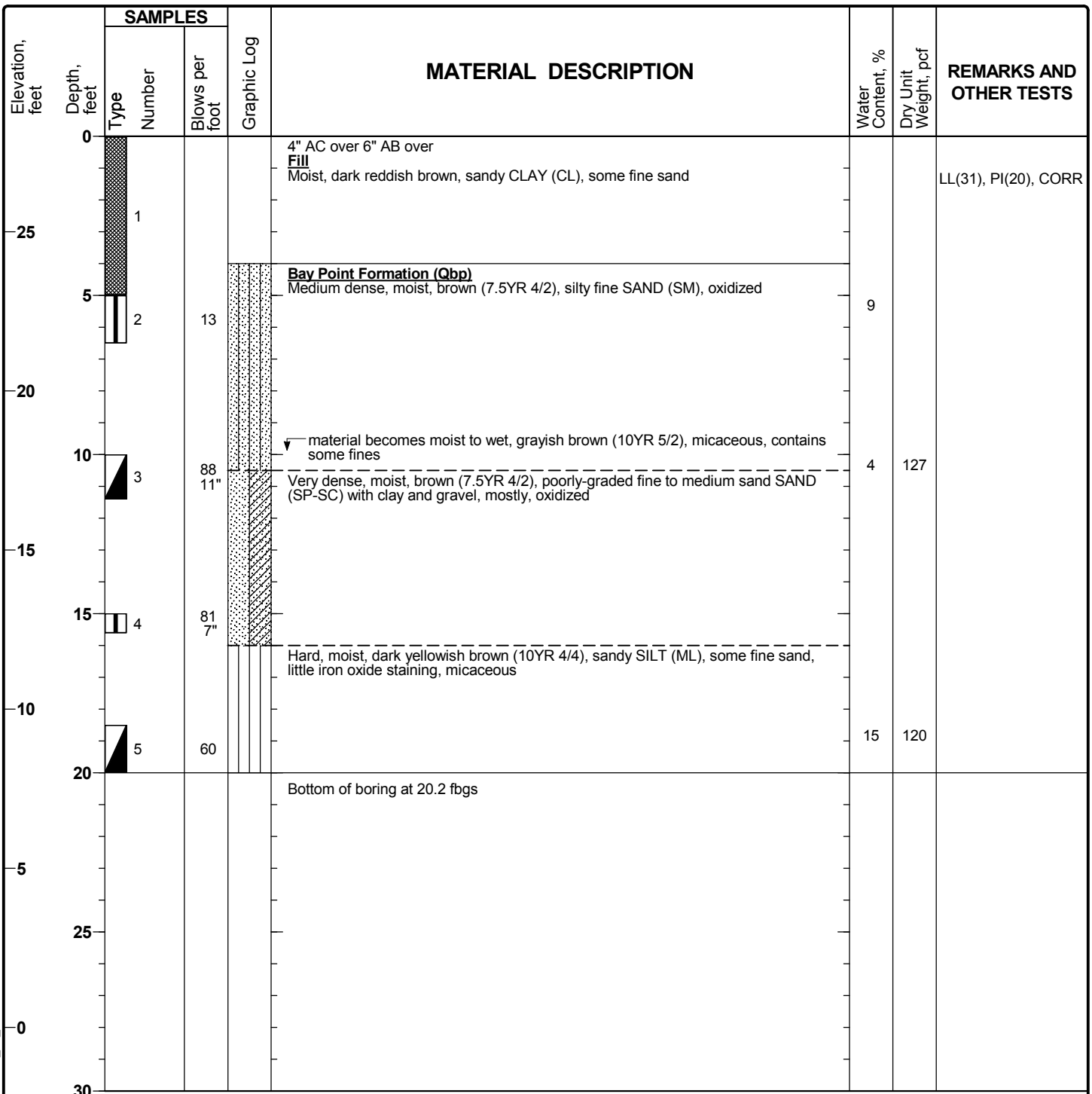
Report: GEO_10_SNA- File: 60530732.GPJ- 9/15/2017 WM-03

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-06

Sheet 1 of 1

Date(s) Drilled	05/18/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	28 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.76883, W117.19991		

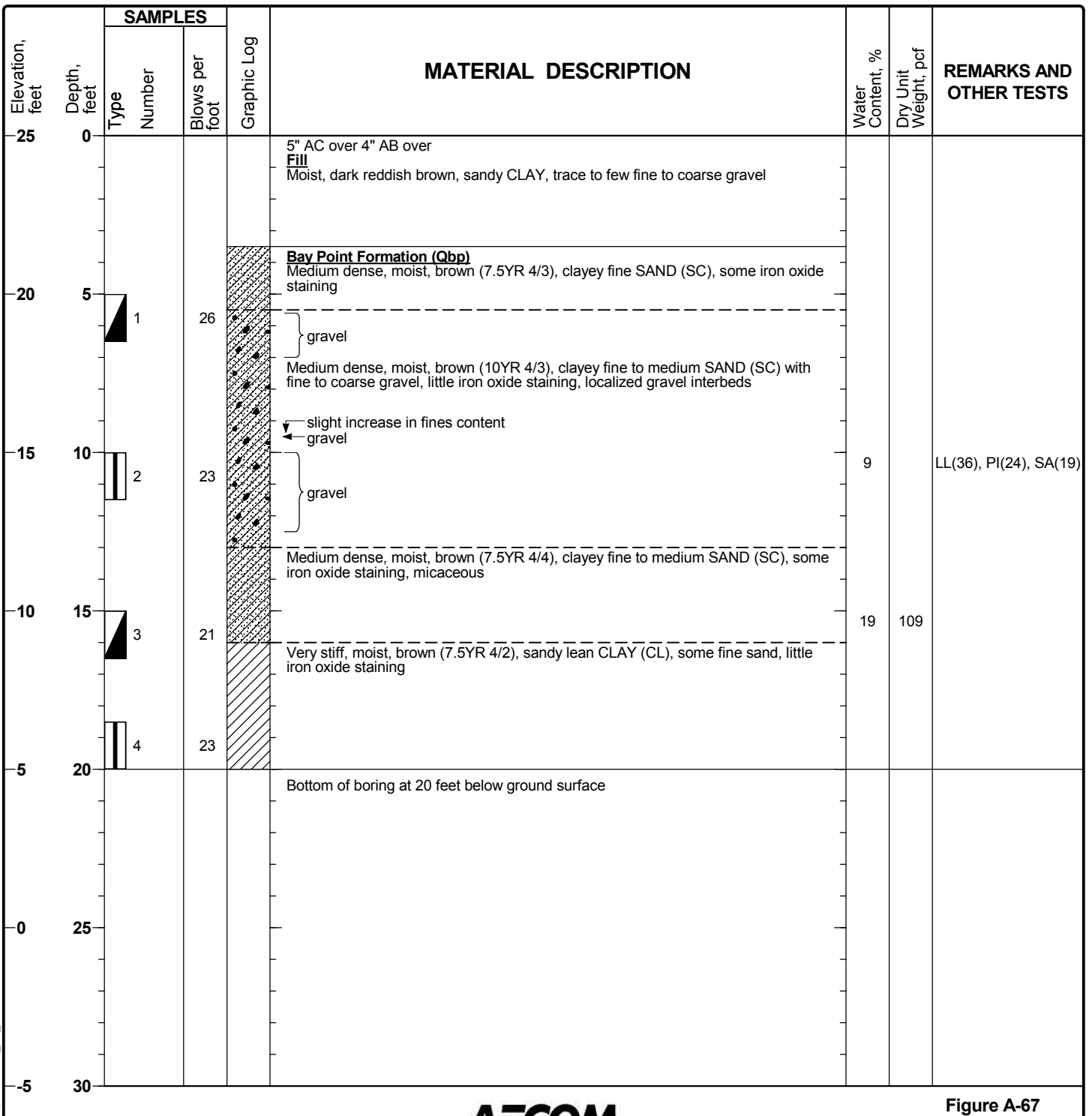


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-07

Sheet 1 of 1

Date(s) Drilled	05/17/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	25 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.76951, W117.20103		



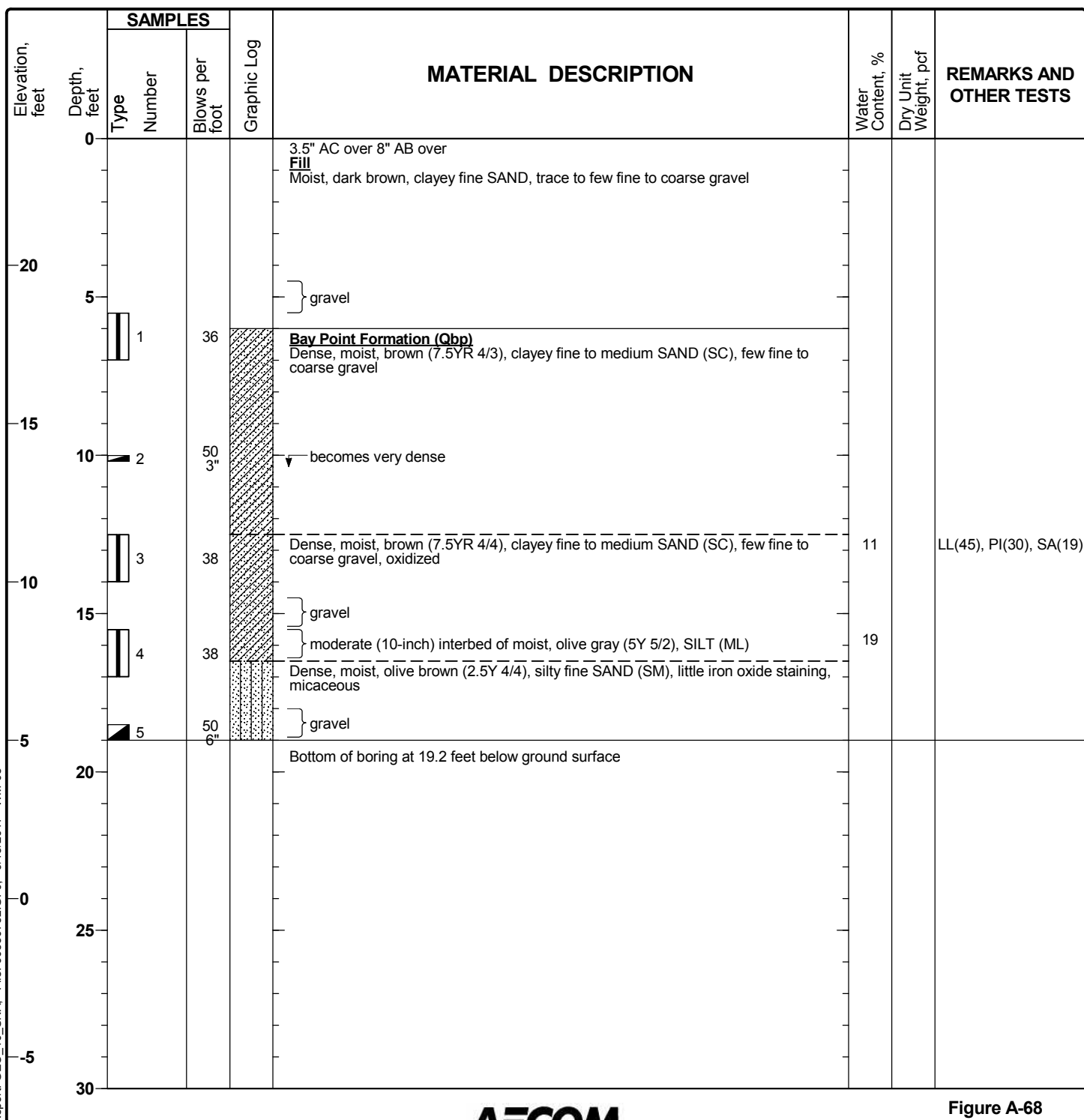
Report: GEO_10_SNA- File: 60530732.GPJ- 9/15/2017 WM-07

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-08

Sheet 1 of 1

Date(s) Drilled	05/18/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	19.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	24 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.77003, W117.20155		

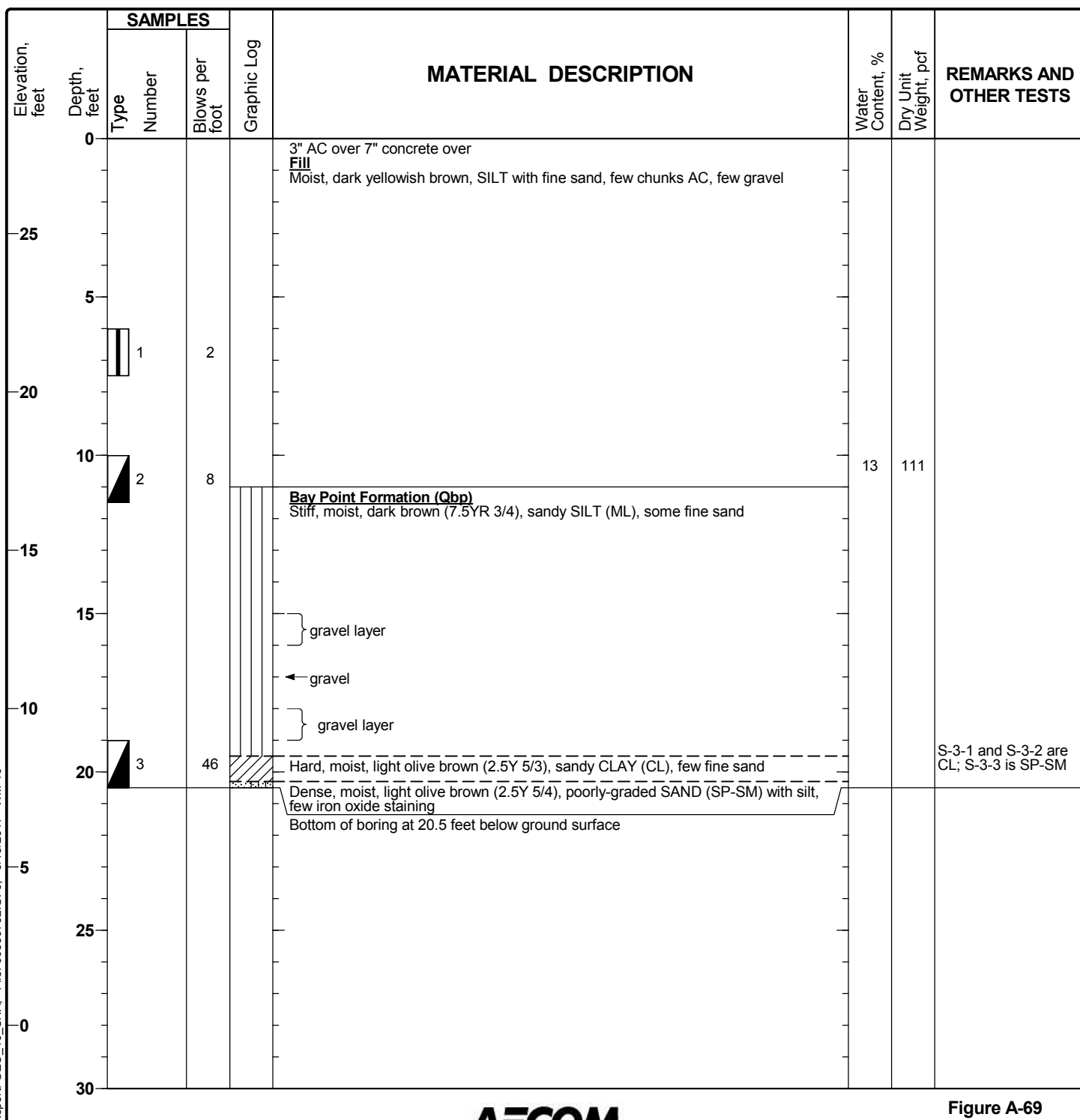


Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring WM-10

Sheet 1 of 1

Date(s) Drilled	06/09/2017	Logged By	A. Avakian / E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.5 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	28 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.77214, W117.20211		

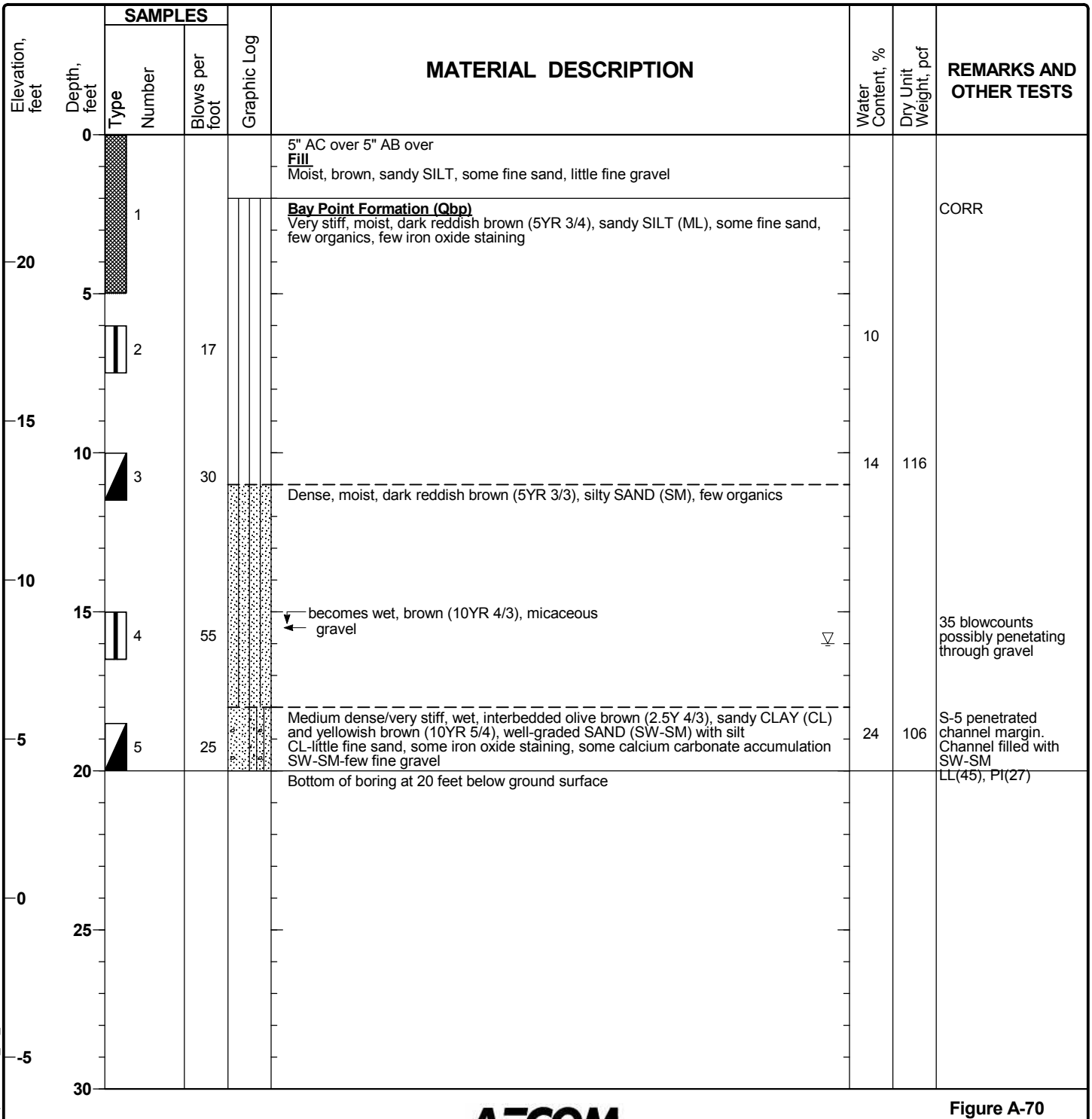


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-11

Sheet 1 of 1

Date(s) Drilled	06/09/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	24 feet
Water Level Depth	16 ft	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap		Location	N32.77253, W117.20245	



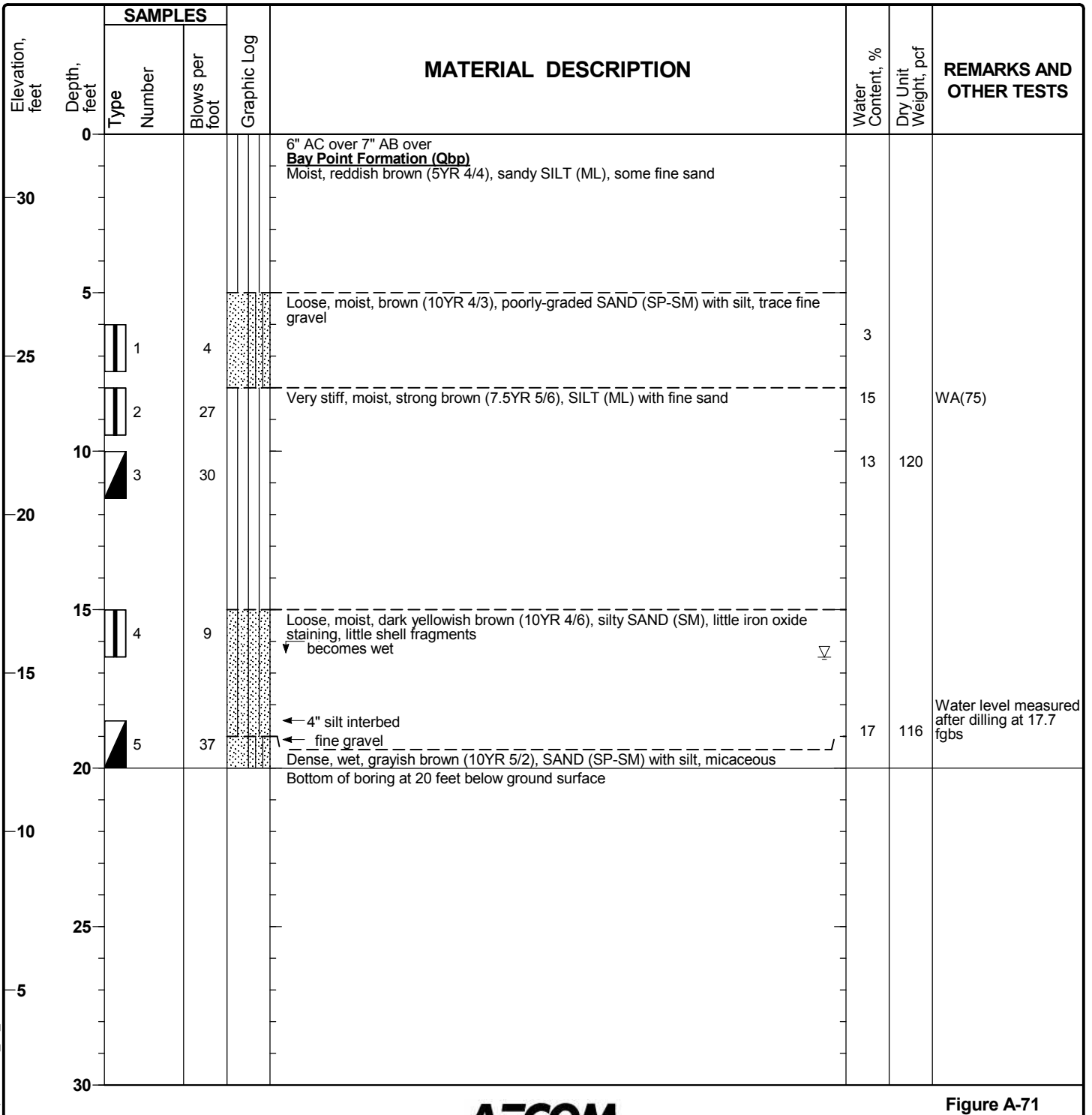
Report: GEO_10_SNA- File: 60530732.GPJ; 9/15/2017 WM-11

Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-16

Sheet 1 of 1

Date(s) Drilled	06/09/2017	Logged By	E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	32 feet
Water Level Depth	16.5 ft	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Bentonite and cuttings with concrete cap	Location	N32.79163, W117.20609		

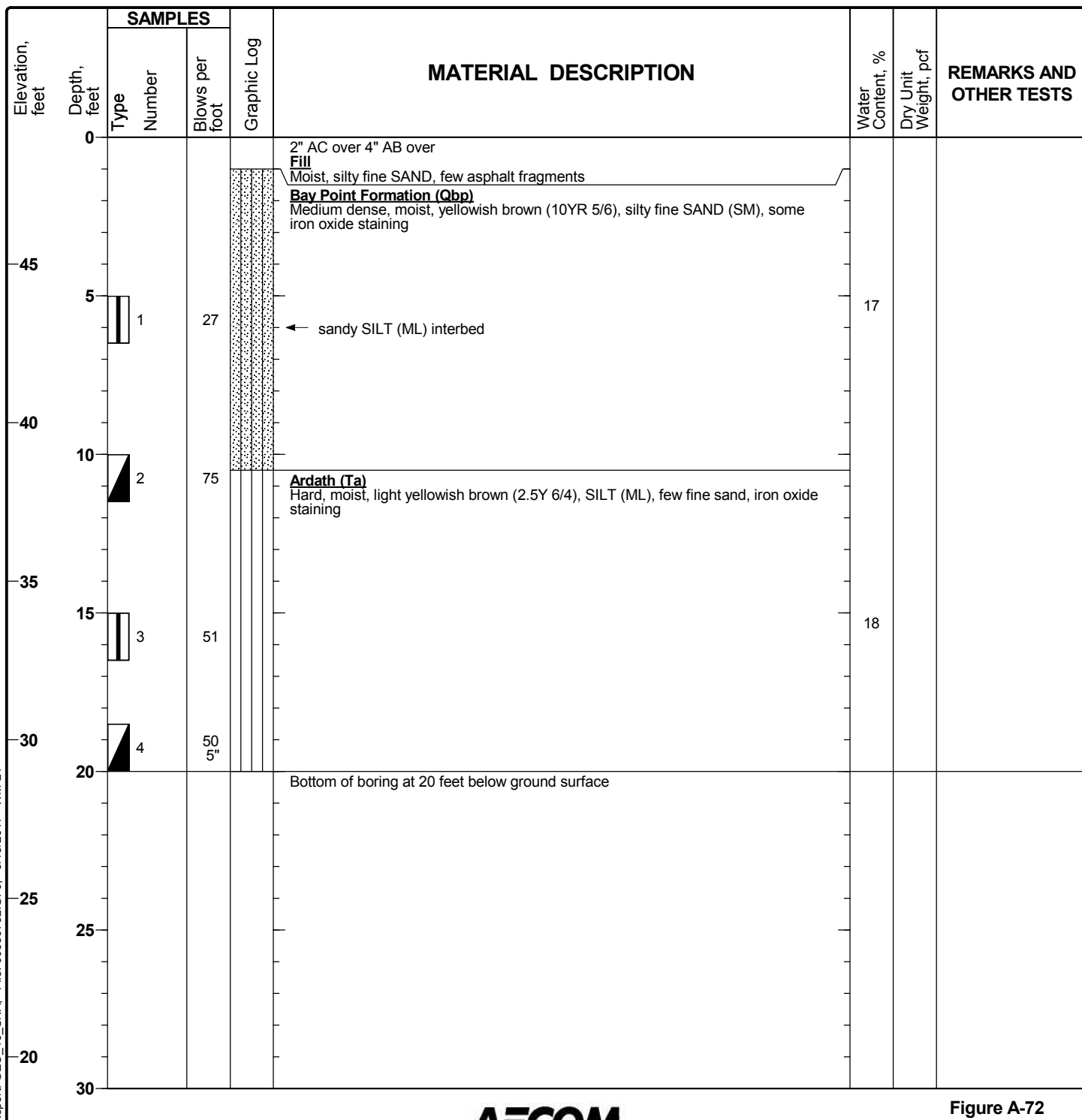


Project: Pure Water, City of San Diego
Project Location: San Diego, California
Project Number: 60530732

Log of Boring WM-21

Sheet 1 of 1

Date(s) Drilled	06/14/2017	Logged By	A. Avakian / E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Diedrich D-50 Turbo	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	49 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.799195, W117.20976		

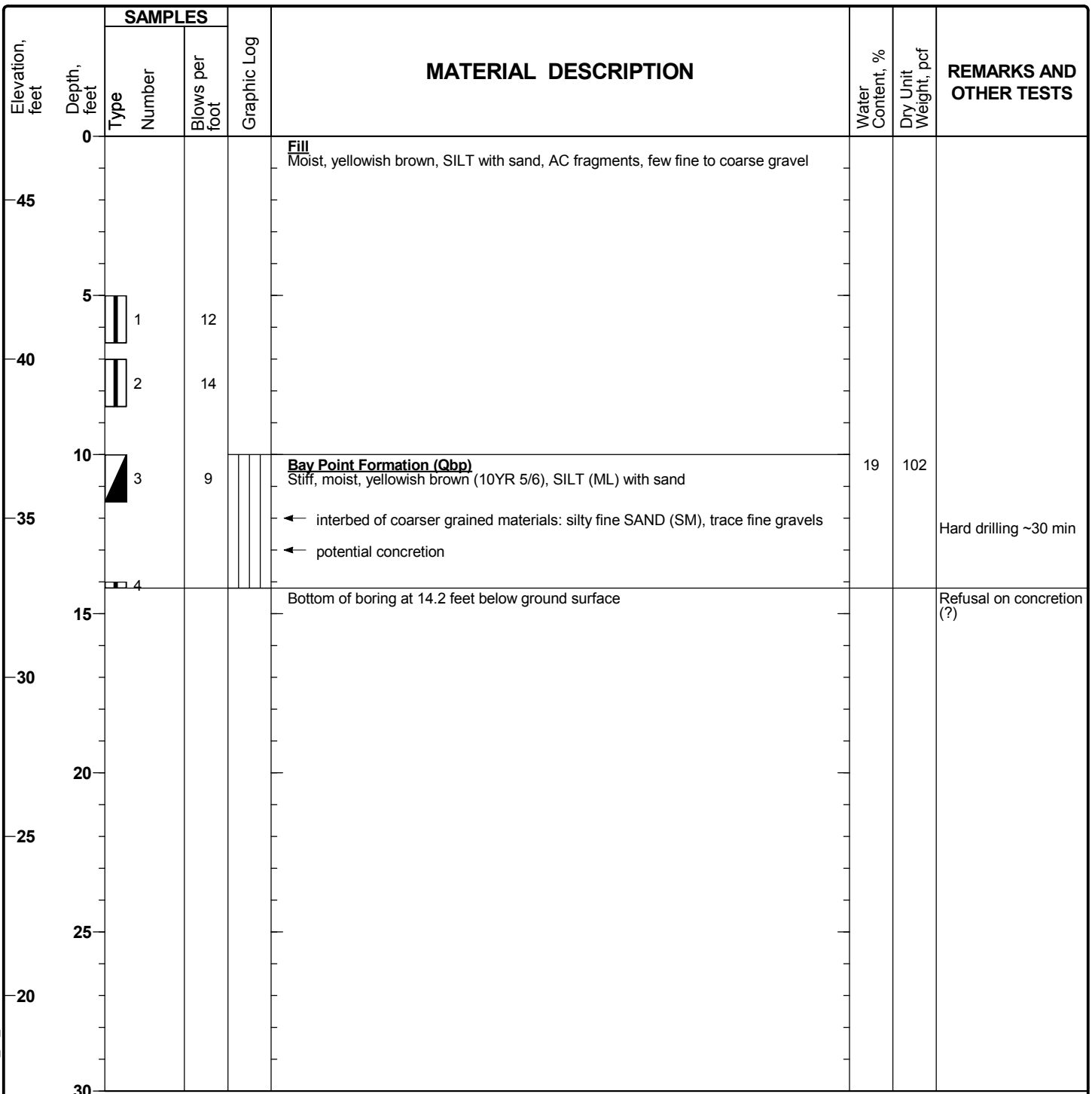


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-22

Sheet 1 of 1

Date(s) Drilled	06/14/2017	Logged By	A. Avakian / E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	14.2 feet
Drill Rig Type	Diedrich D-50 Turbo	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	47 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.80042, W117.21051		

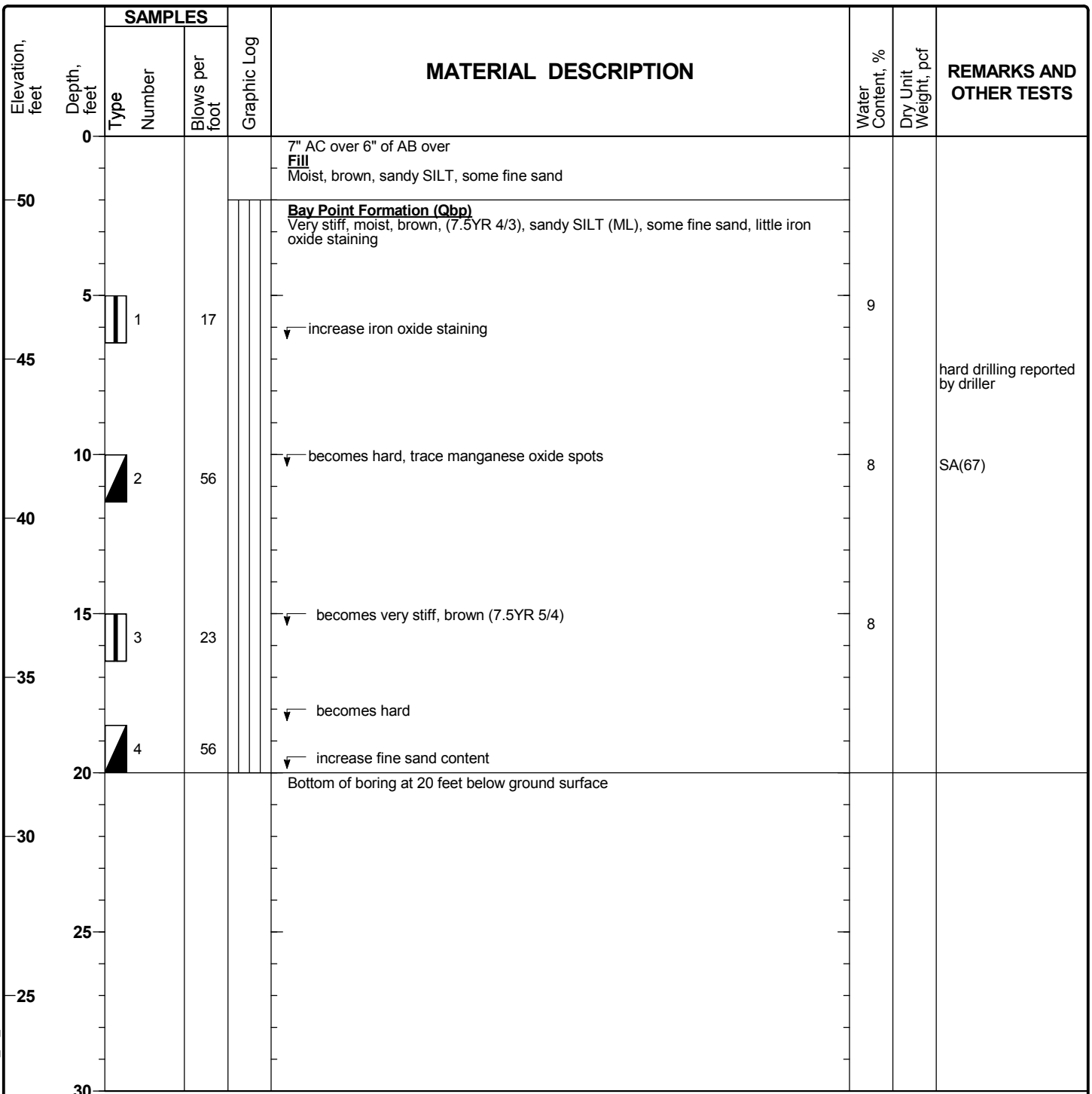


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-23

Sheet 1 of 1

Date(s) Drilled	05/18/2017	Logged By	A. Avakian	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Unimog Marl M5	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	52 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.802118, W117.211547		

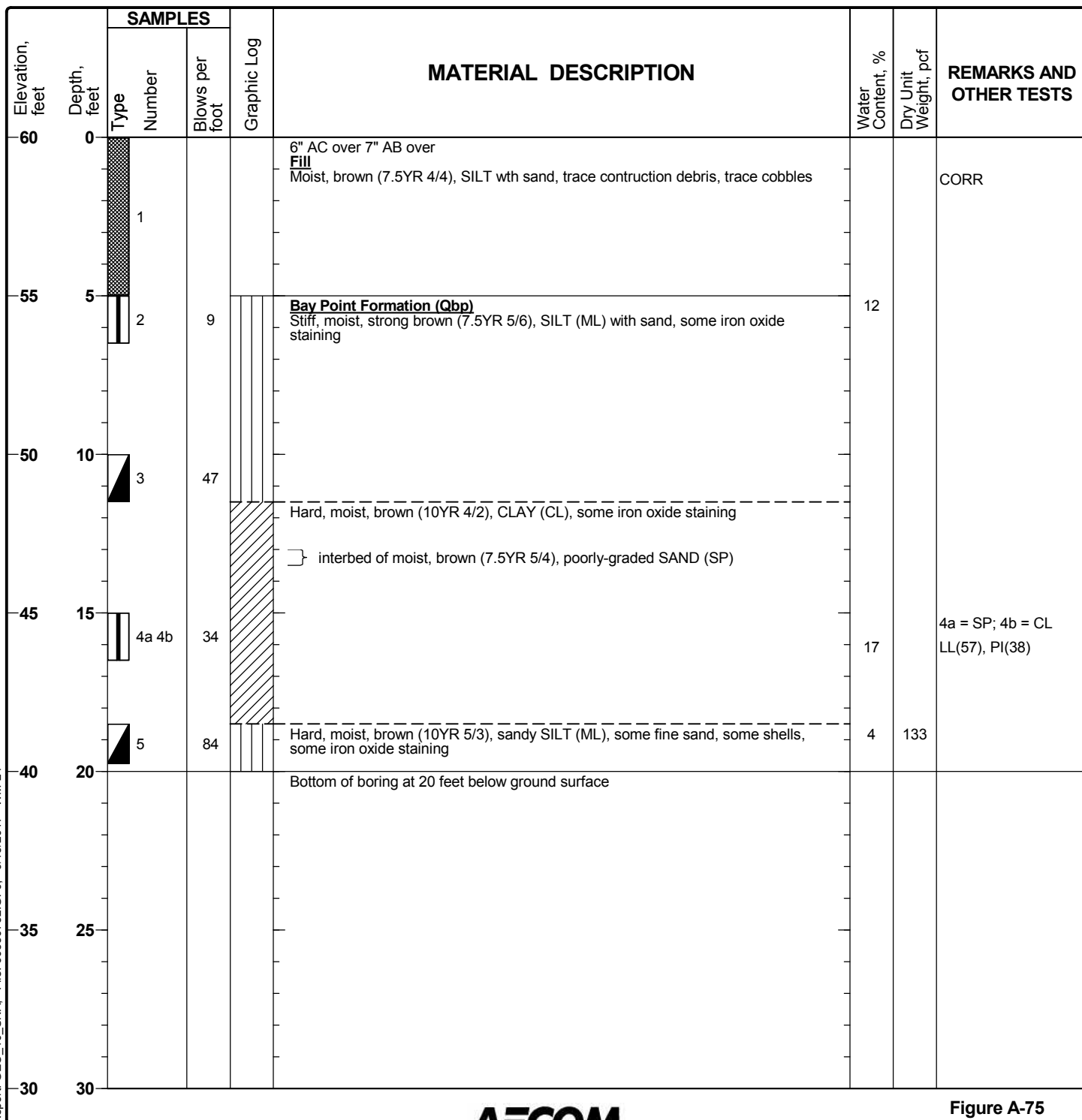


Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-24

Sheet 1 of 1

Date(s) Drilled	06/14/2017	Logged By	A. Avakian / E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	20.0 feet
Drill Rig Type	Diedrich D-50 Turbo	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	60 feet
Water Level Depth	not encountered	Sampling Method(s)	SPT / 2.5" ID / Bulk	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.803758, W117.212571		



Project: Pure Water, City of San Diego
 Project Location: San Diego, California
 Project Number: 60530732

Log of Boring WM-25

Sheet 1 of 1

Date(s) Drilled	06/14/2017	Logged By	A. Avakian / E. Marquez	Checked By	S. Fitzwilliam
Drilling Method	Hollow Stem Auger	Drill Bit Size/Type	6-inch	Total Depth of Borehole	6.5 feet
Drill Rig Type	Diedrich D-50 Turbo	Drilling Contractor	Pacific Drilling	Approximate Surface Elevation	69 feet
Water Level Depth	not encountered	Sampling Method(s)	none	Hammer Data	140 lbs / 30-inch
Borehole Completion	Cuttings with concrete cap	Location	N32.80525, W117.21318		

Elevation, feet	Depth, feet	SAMPLES		Graphic Log	MATERIAL DESCRIPTION	Water Content, %	Dry Unit Weight, pcf	REMARKS AND OTHER TESTS
		Type	Number					
0	0				6" AC over 7" AB over San Diego Formation (Tsd) Moist, light yellowish brown (10YR 6/4), poorly-graded gravel (GP-GM) with silt and fine sand			
65	5							Hard drilling ~ 40 min Skipped 5' sample due to dense cobbles and gravels @6' Driller broke shear pin on drill, 15 minutes to fix
60	10				Bottom of boring at 6.5 feet below ground surface			Refusal on cobbles/gravels
55	15							
50	20							
45	25							
40	30							

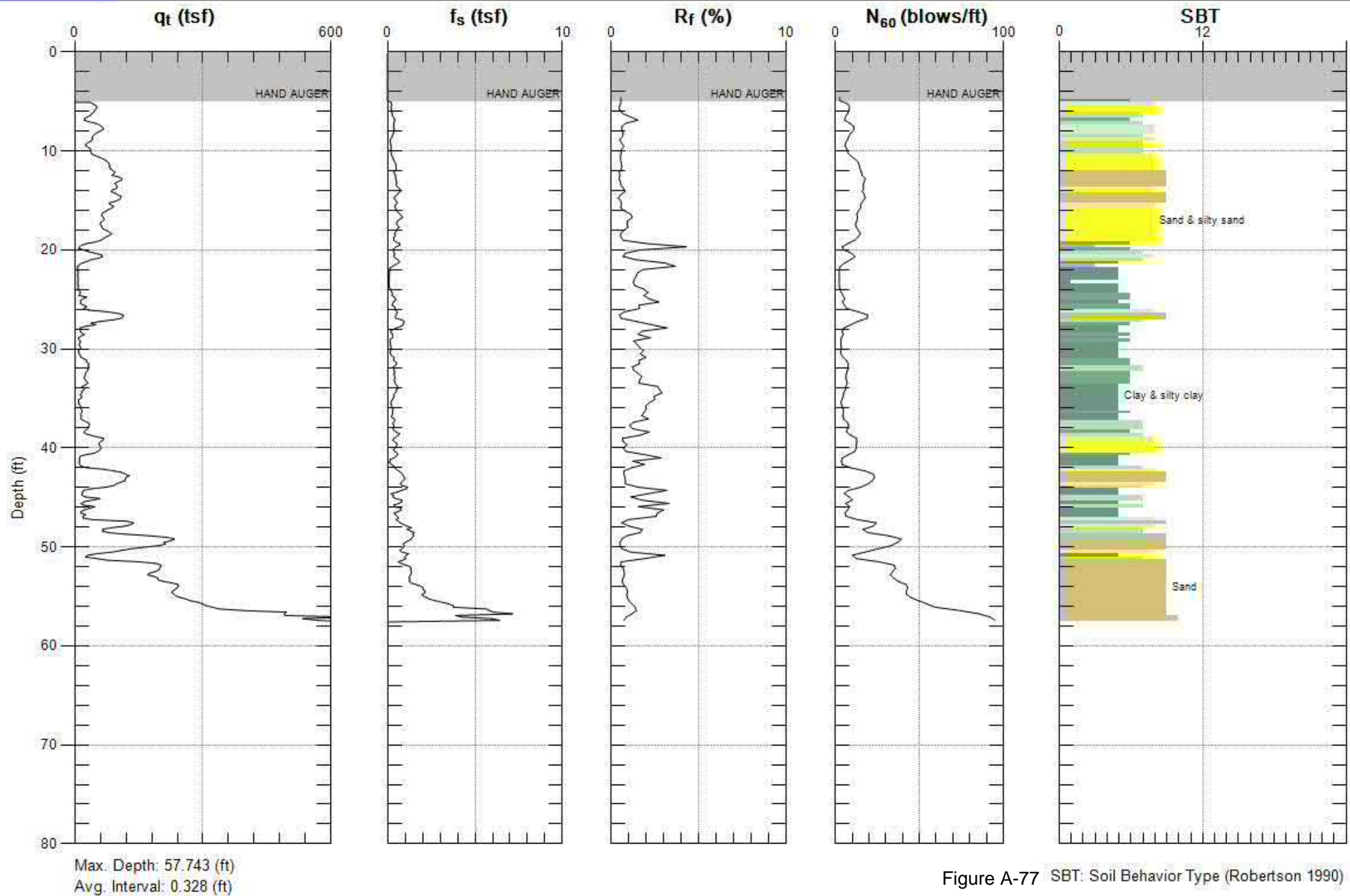


Figure A-77 SBT: Soil Behavior Type (Robertson 1990)

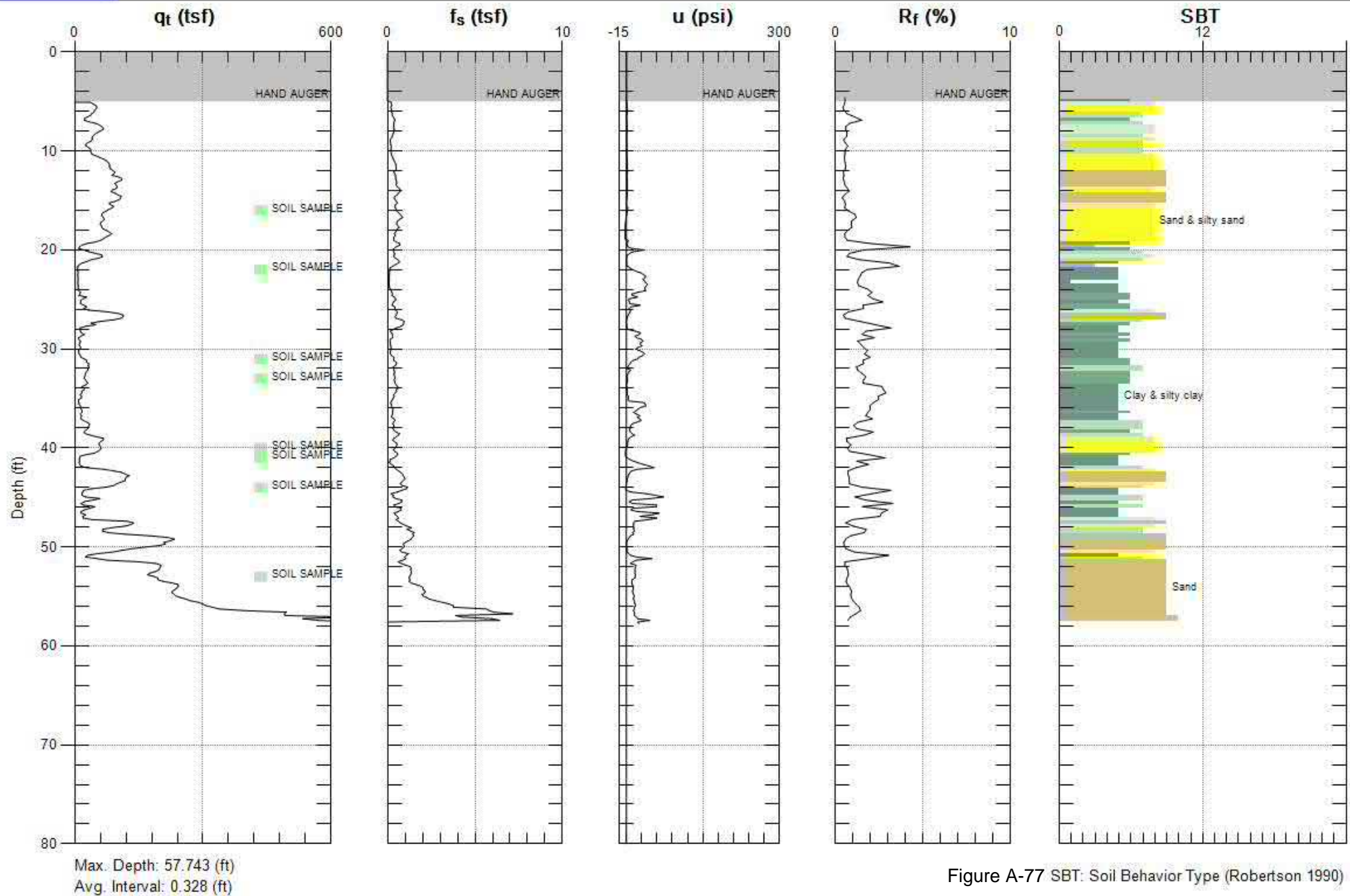


Figure A-77 SBT: Soil Behavior Type (Robertson 1990)

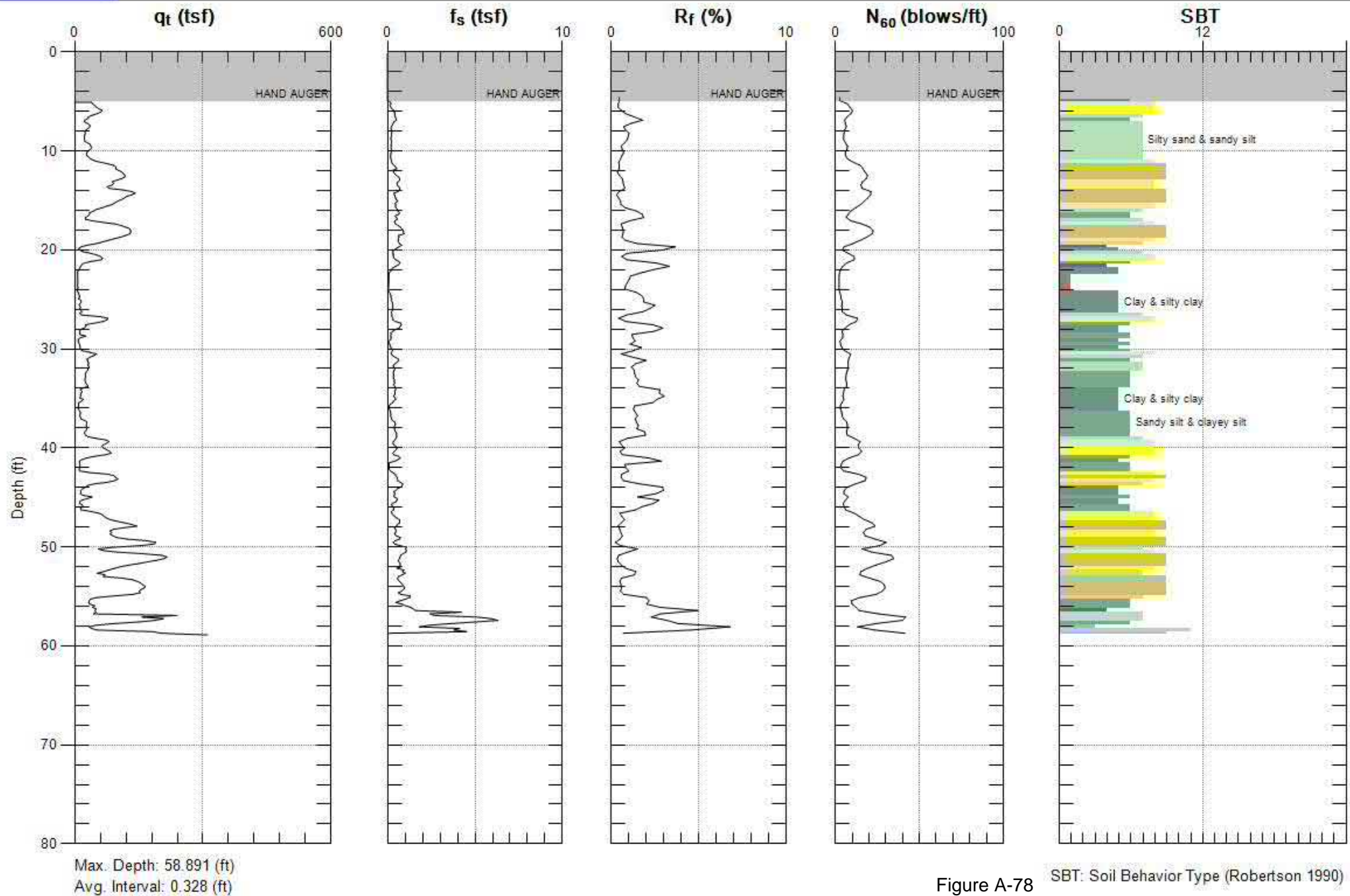


Figure A-78 SBT: Soil Behavior Type (Robertson 1990)

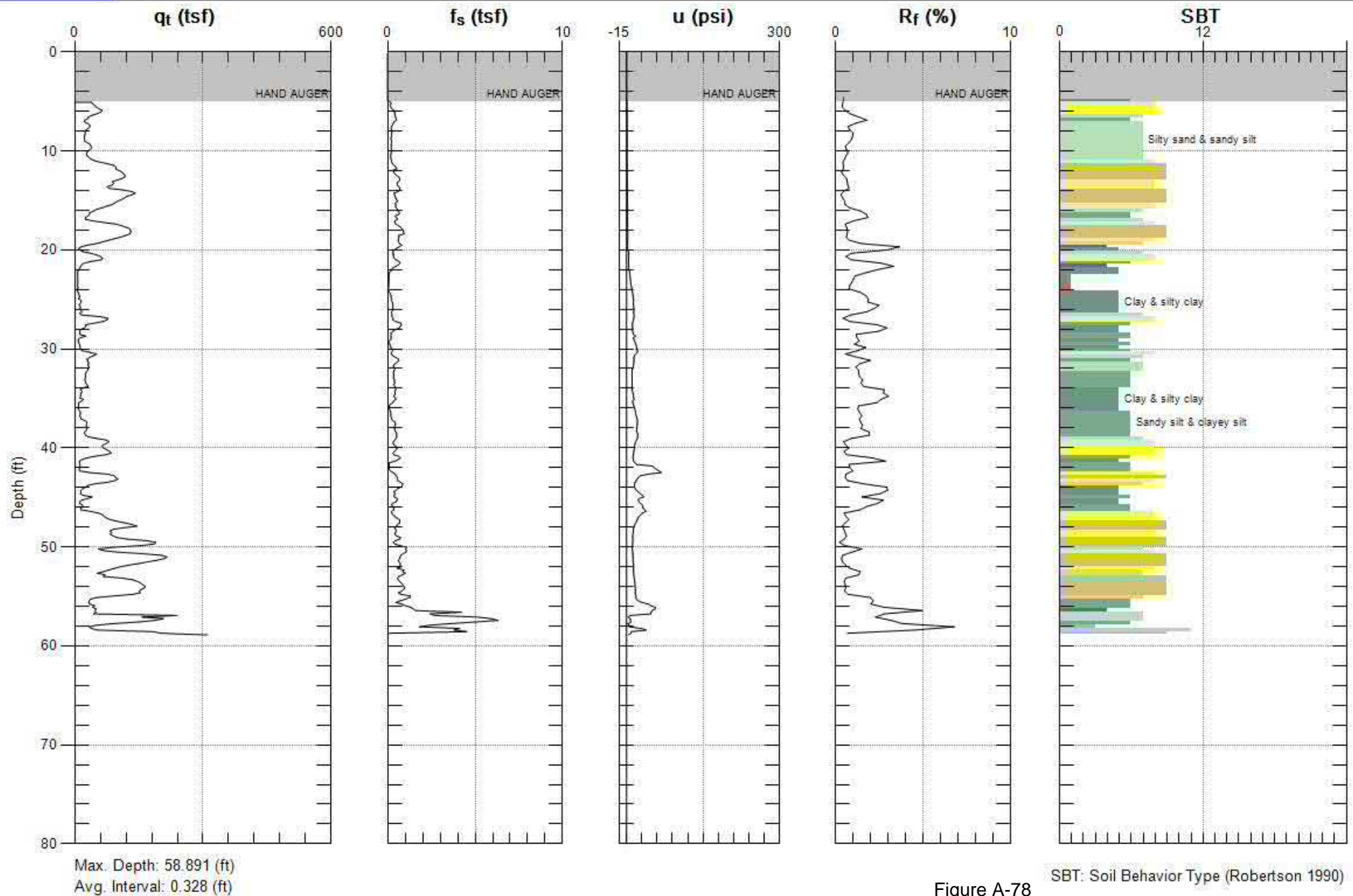


Figure A-78

SBT: Soil Behavior Type (Robertson 1990)

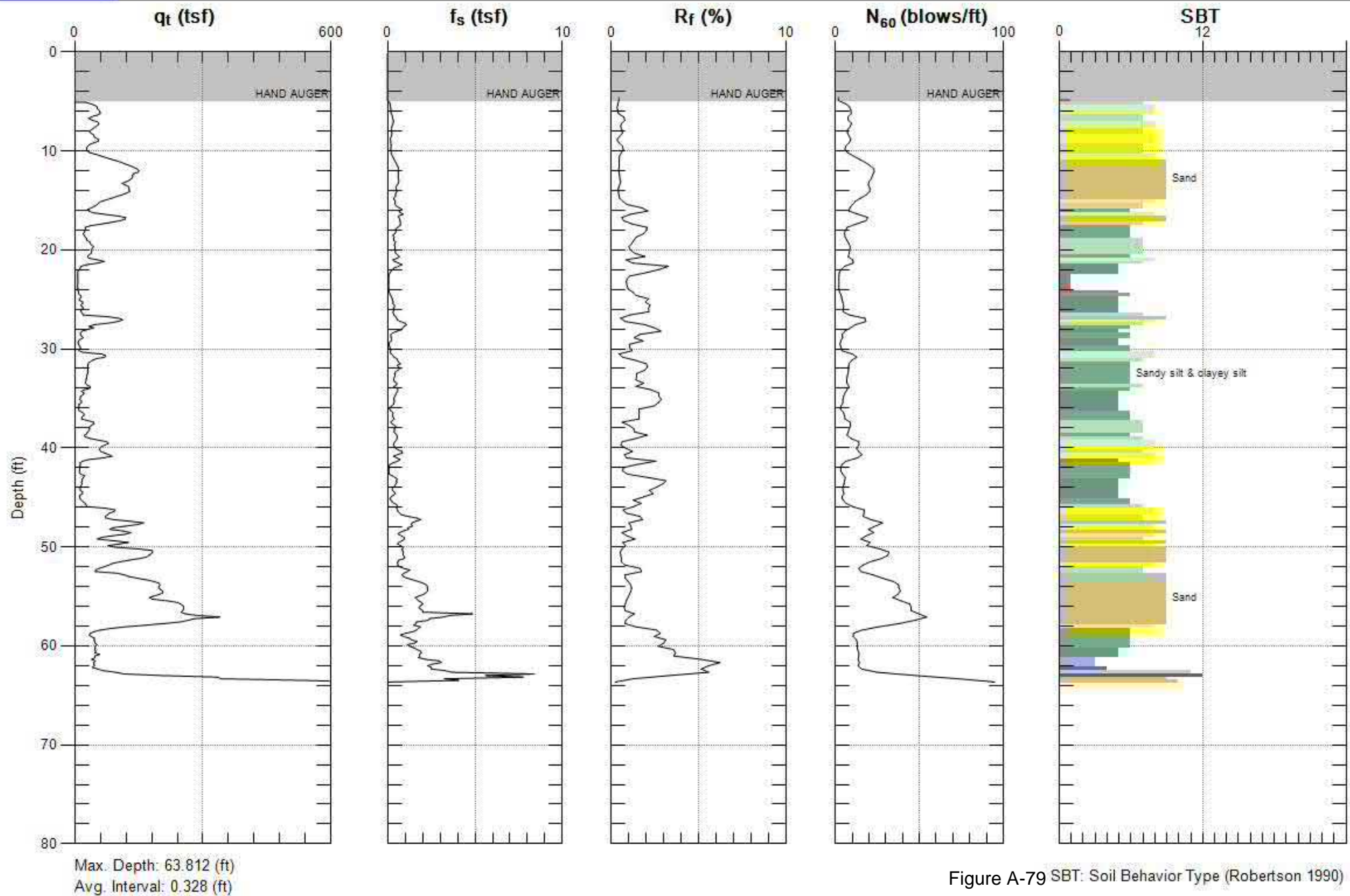


Figure A-79 SBT: Soil Behavior Type (Robertson 1990)

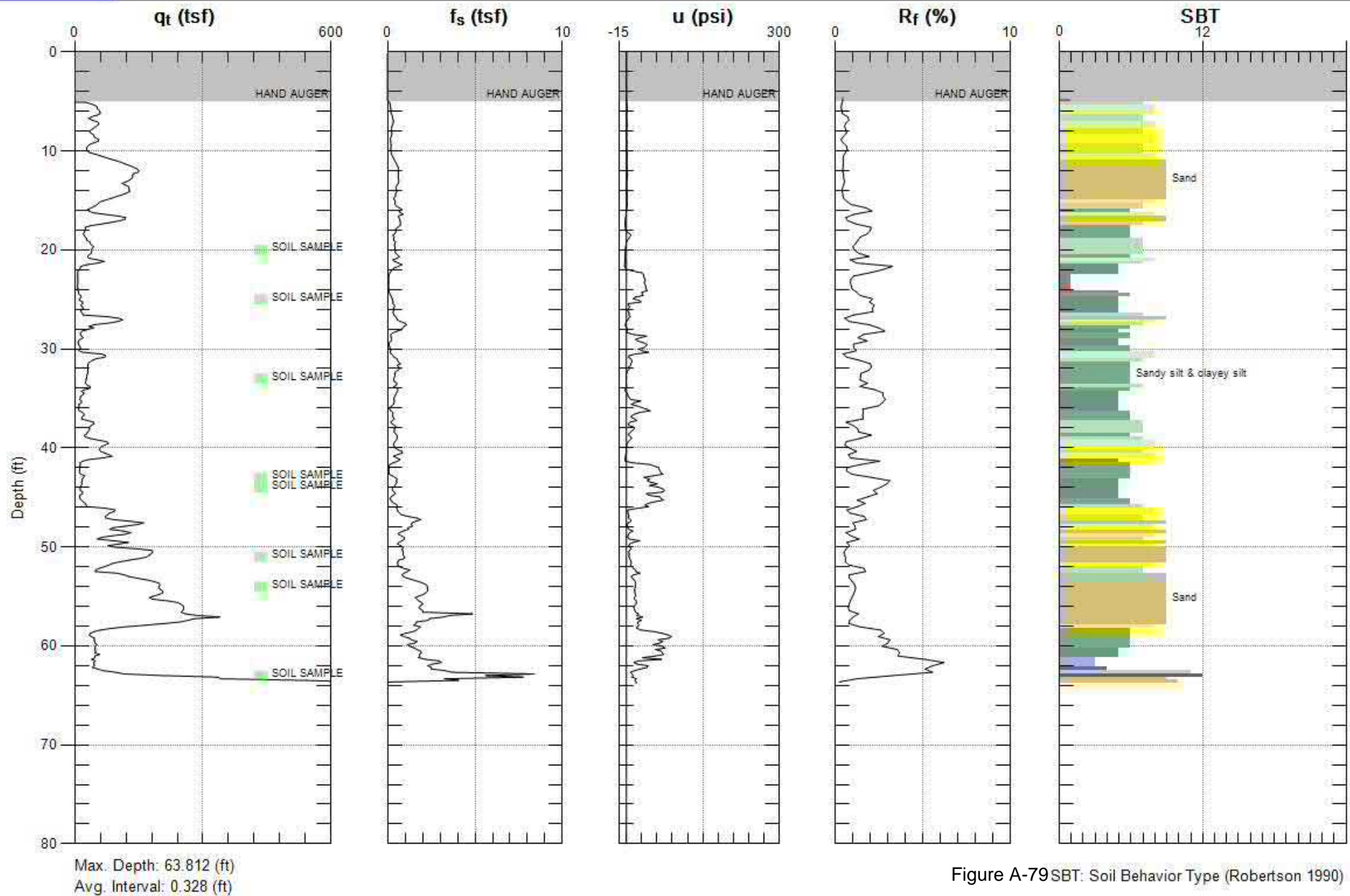


Figure A-79 SBT: Soil Behavior Type (Robertson 1990)

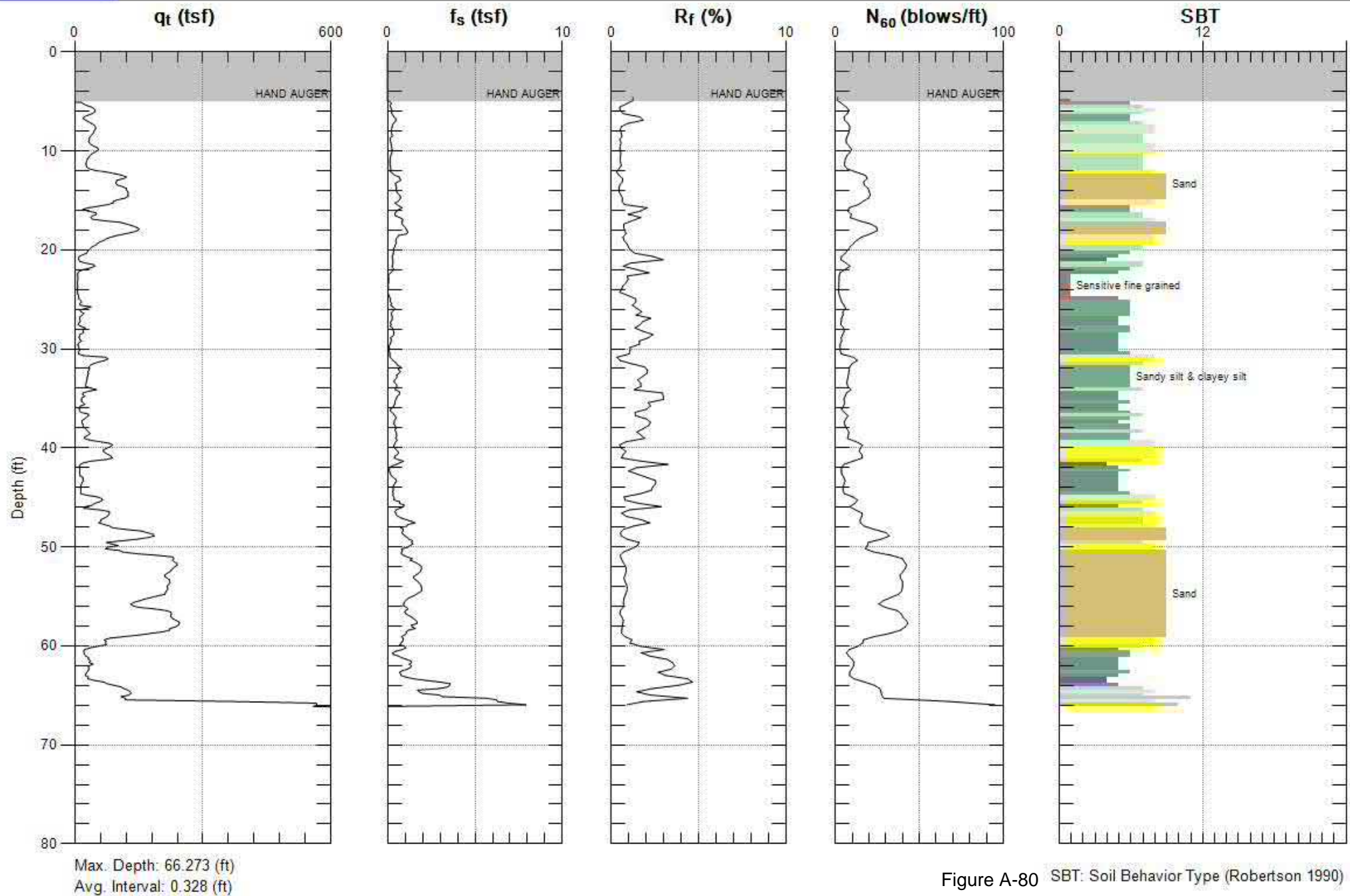


Figure A-80 SBT: Soil Behavior Type (Robertson 1990)

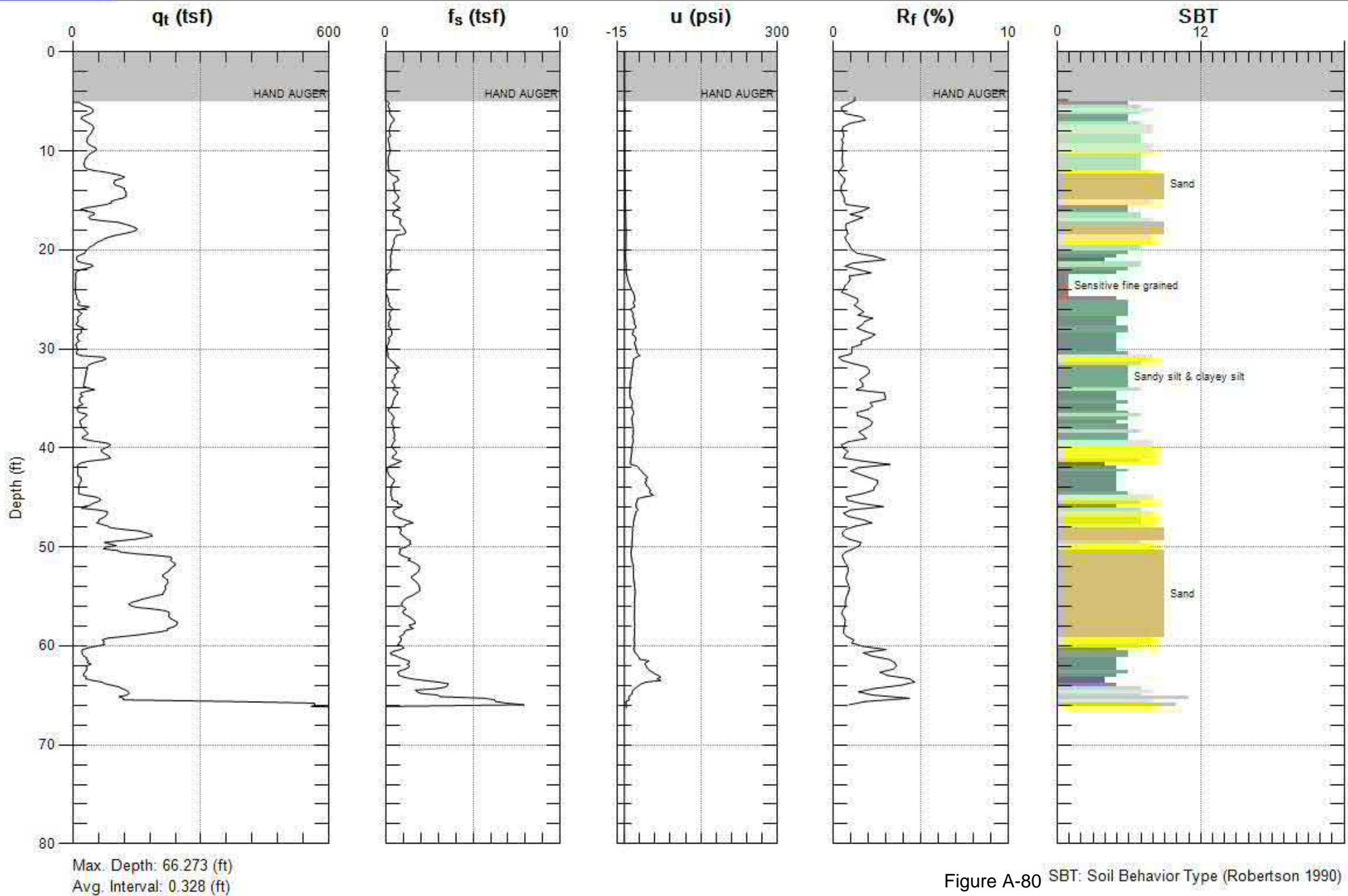


Figure A-80 SBT: Soil Behavior Type (Robertson 1990)

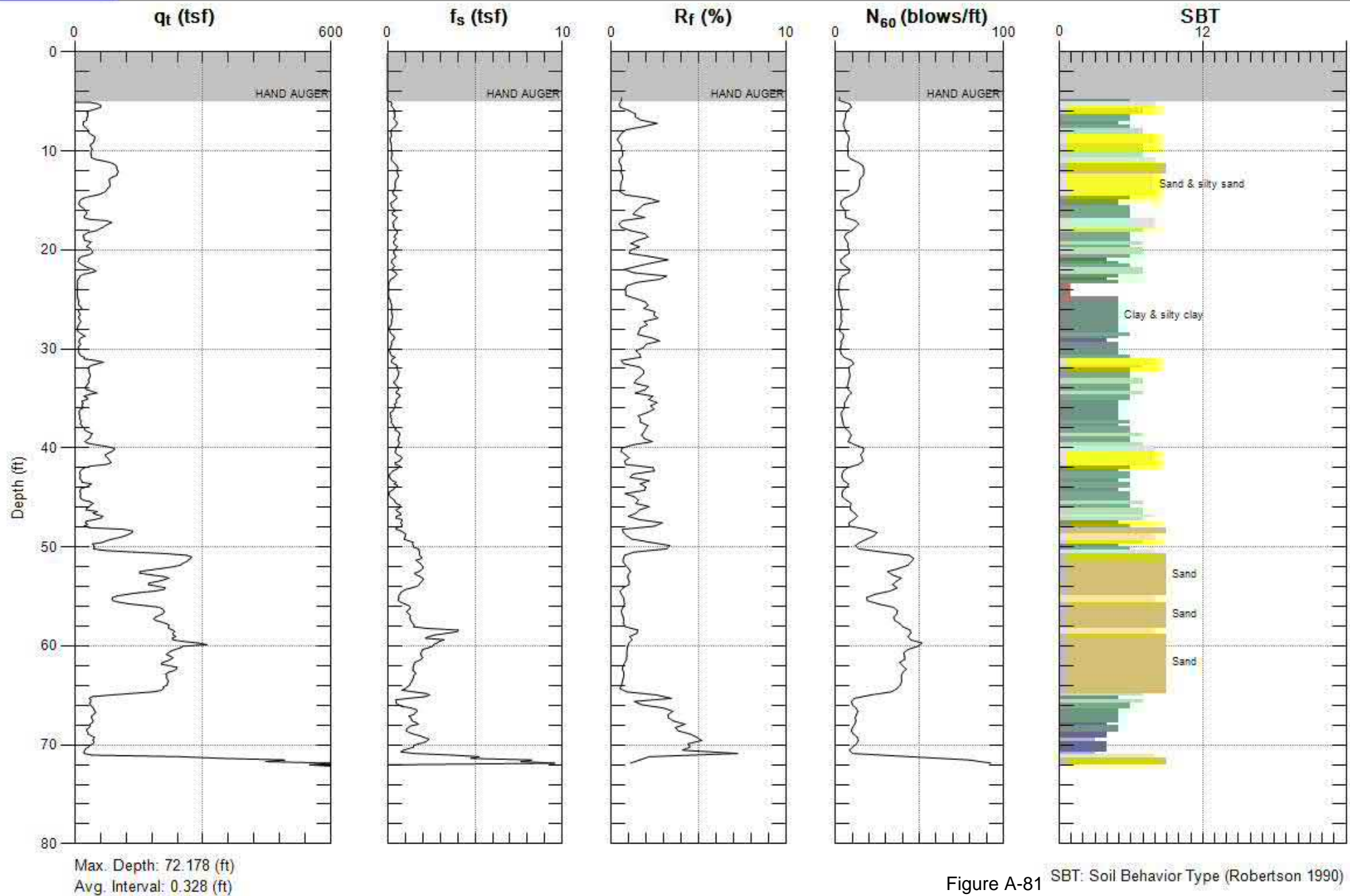


Figure A-81 SBT: Soil Behavior Type (Robertson 1990)

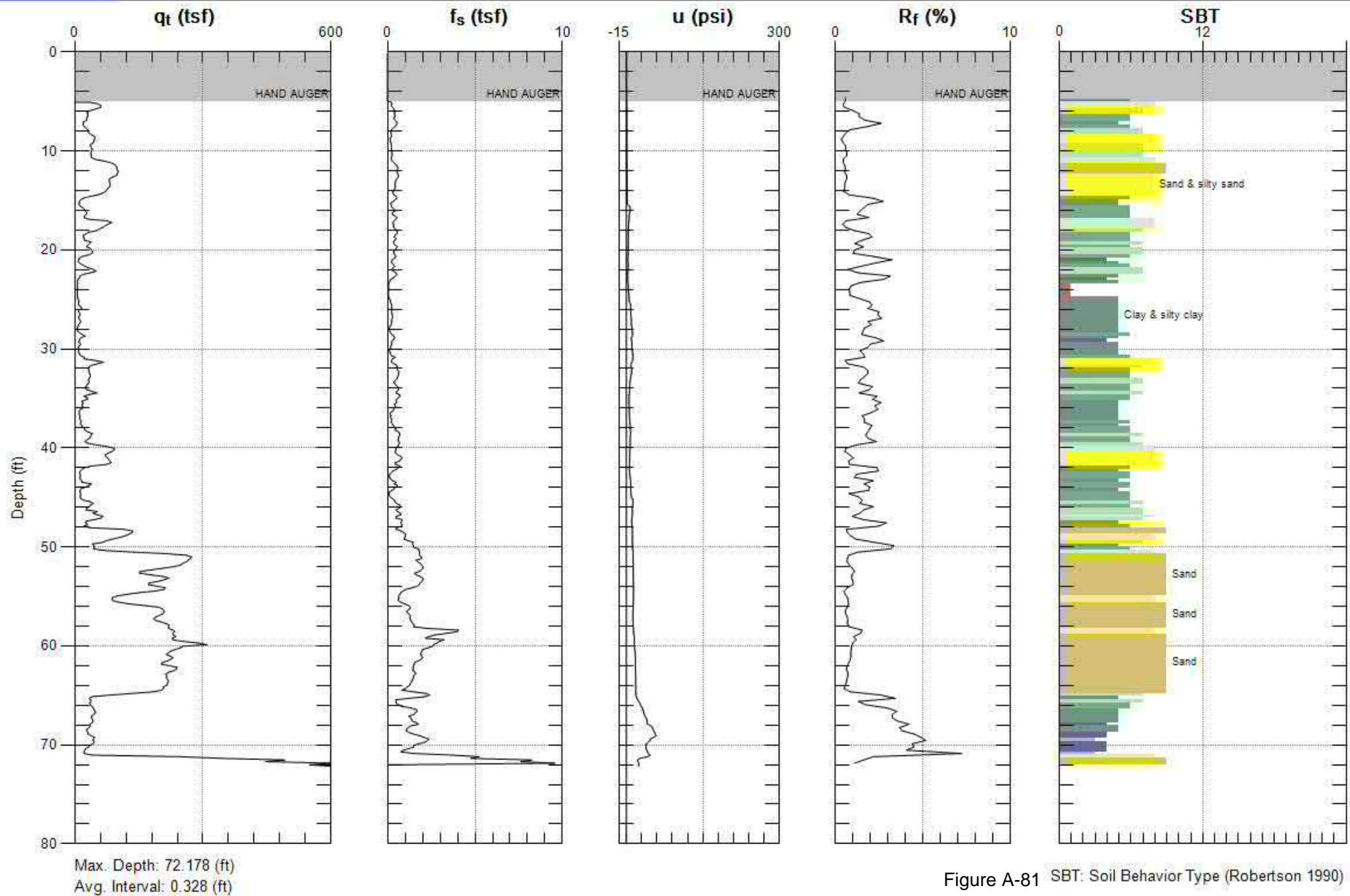


Figure A-81 SBT: Soil Behavior Type (Robertson 1990)

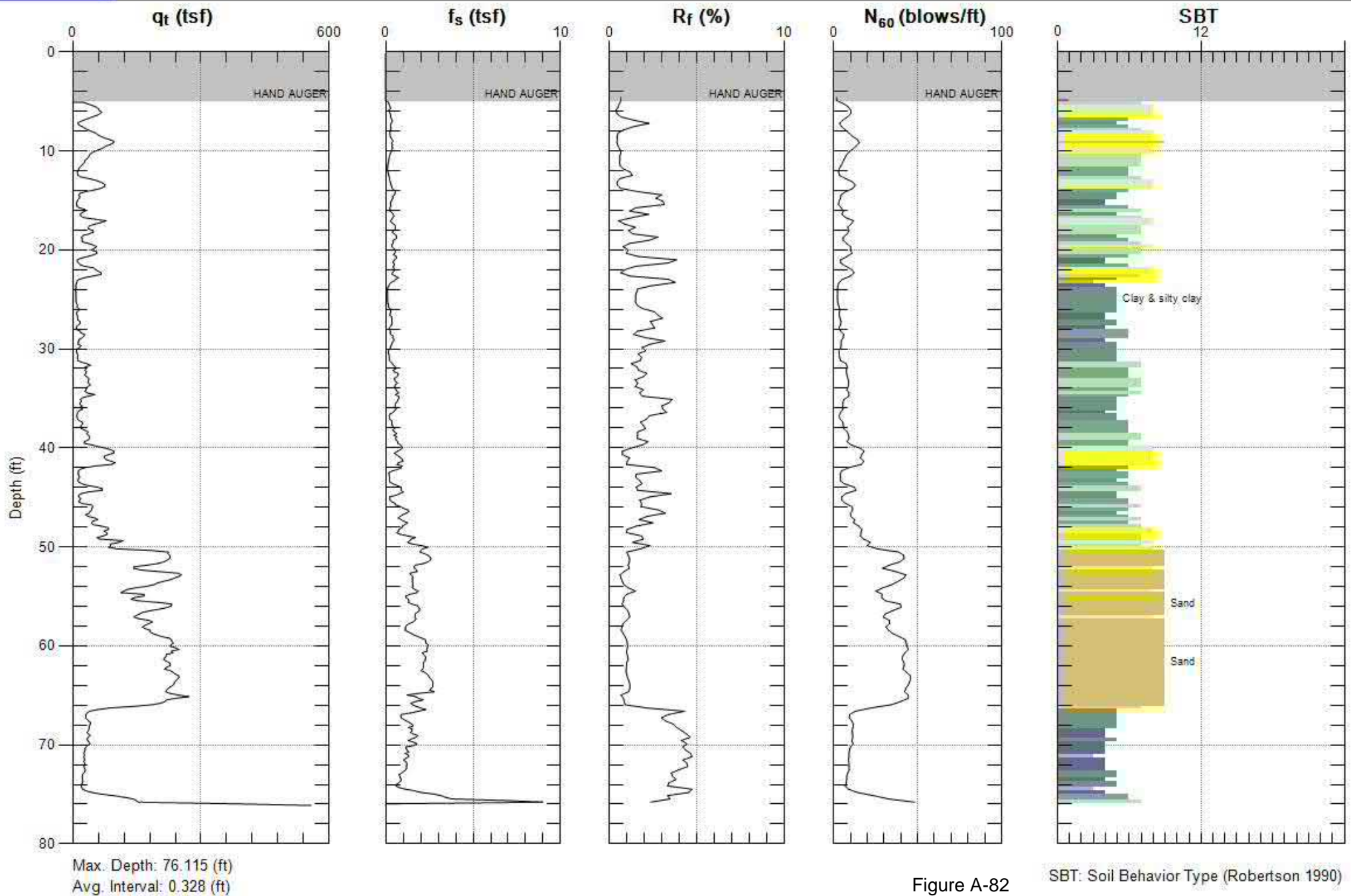


Figure A-82

SBT: Soil Behavior Type (Robertson 1990)

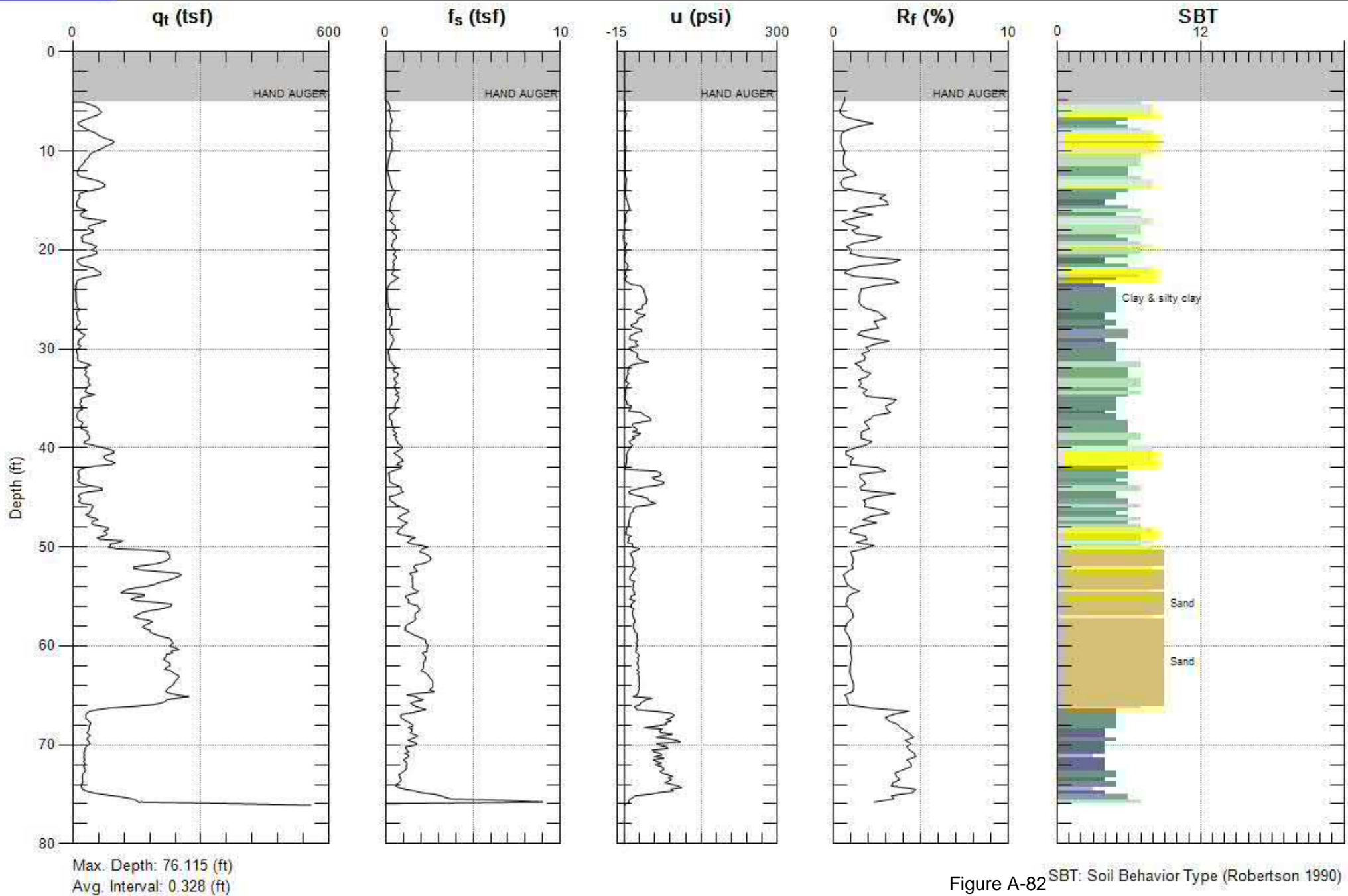


Figure A-82 SBT: Soil Behavior Type (Robertson 1990)

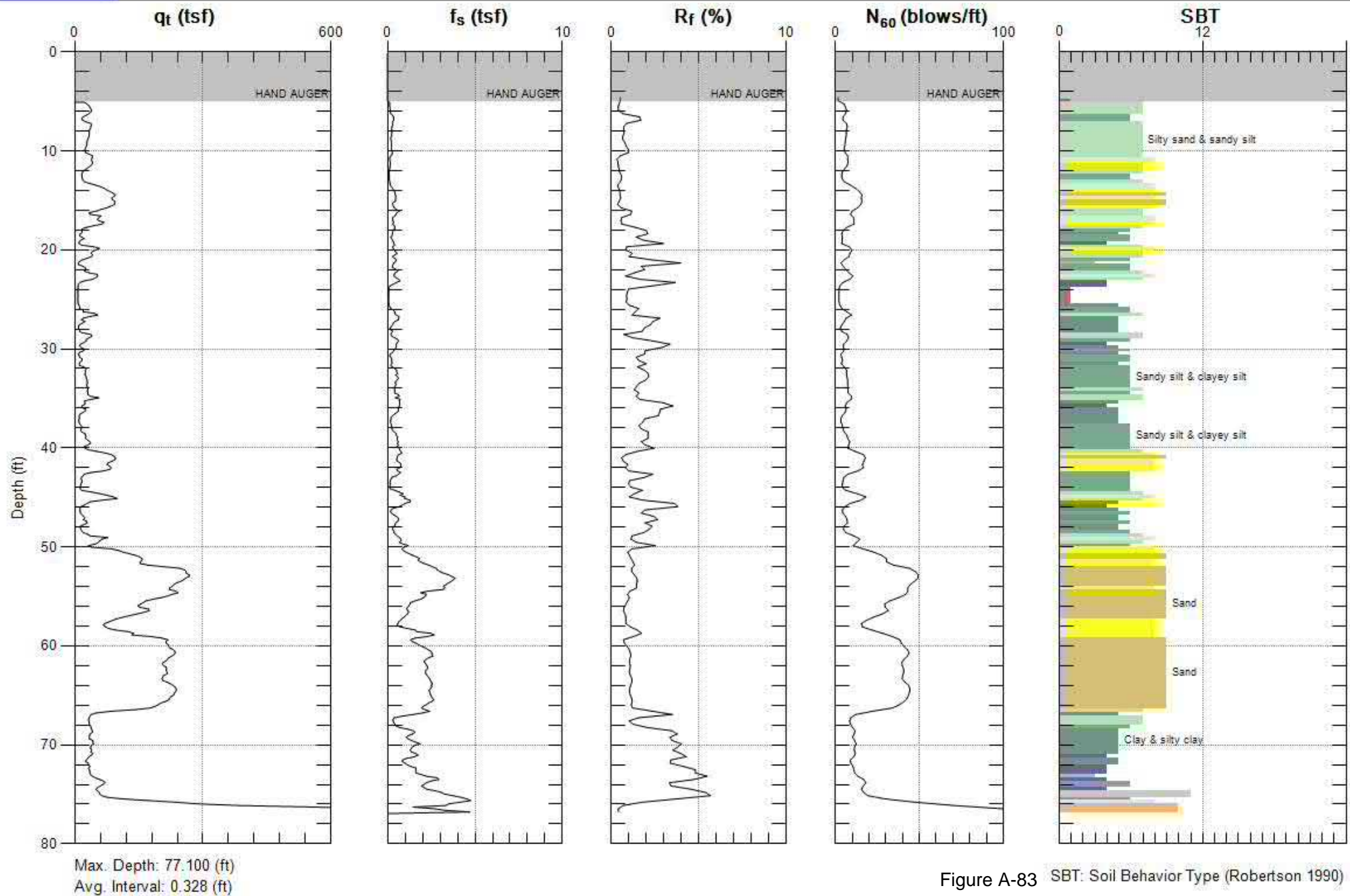


Figure A-83 SBT: Soil Behavior Type (Robertson 1990)

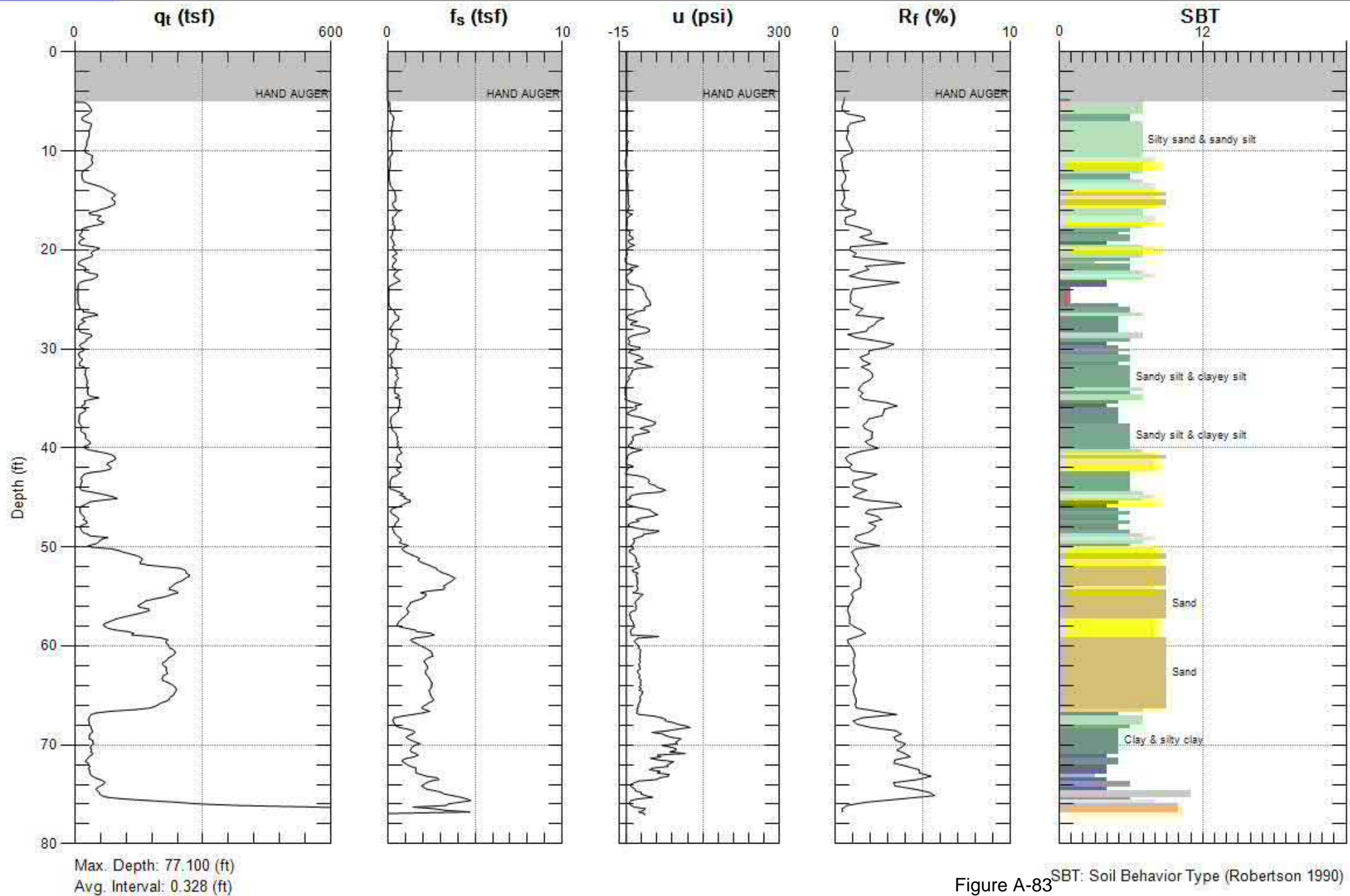
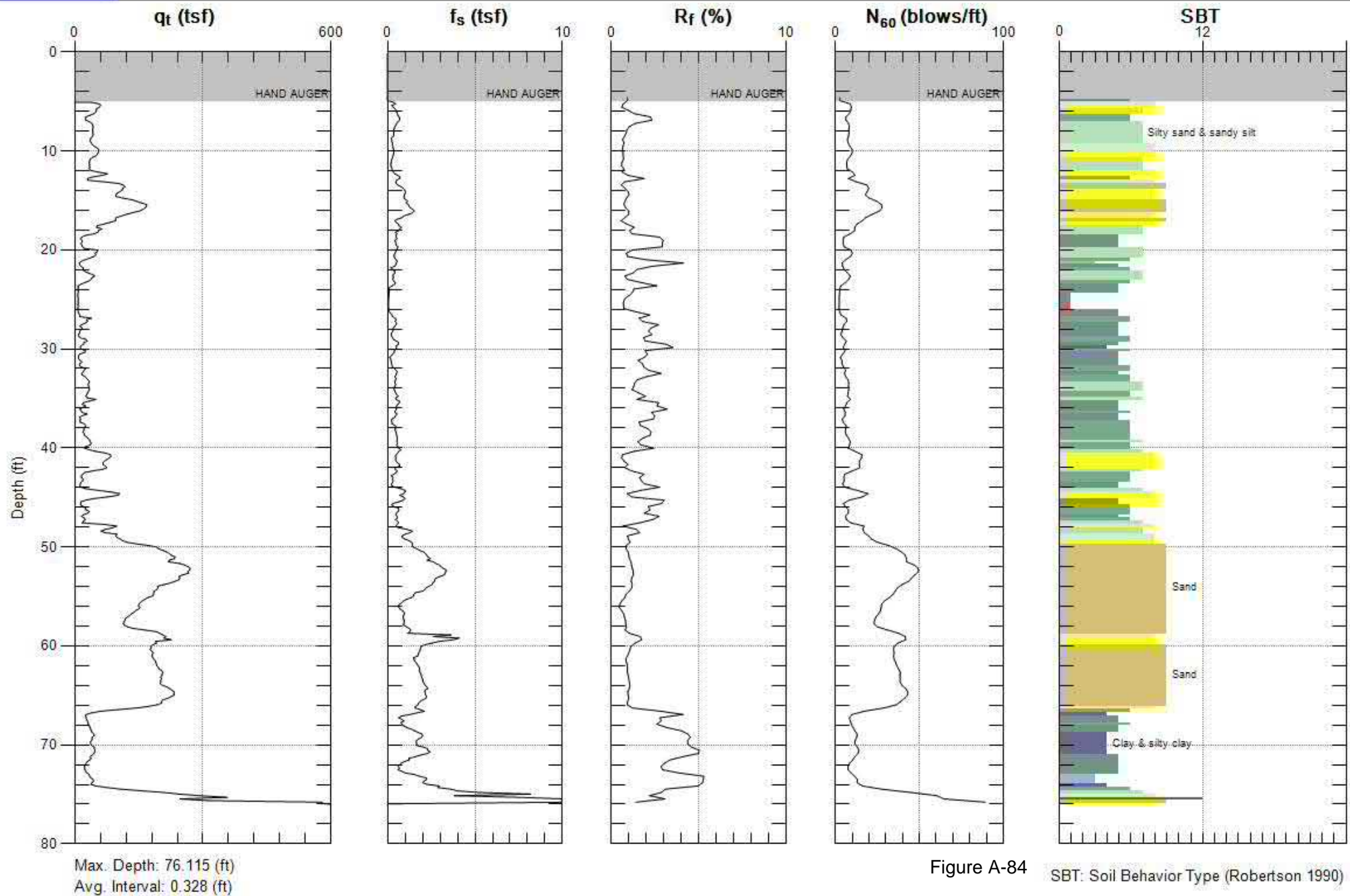


Figure A-83 SBT: Soil Behavior Type (Robertson 1990)



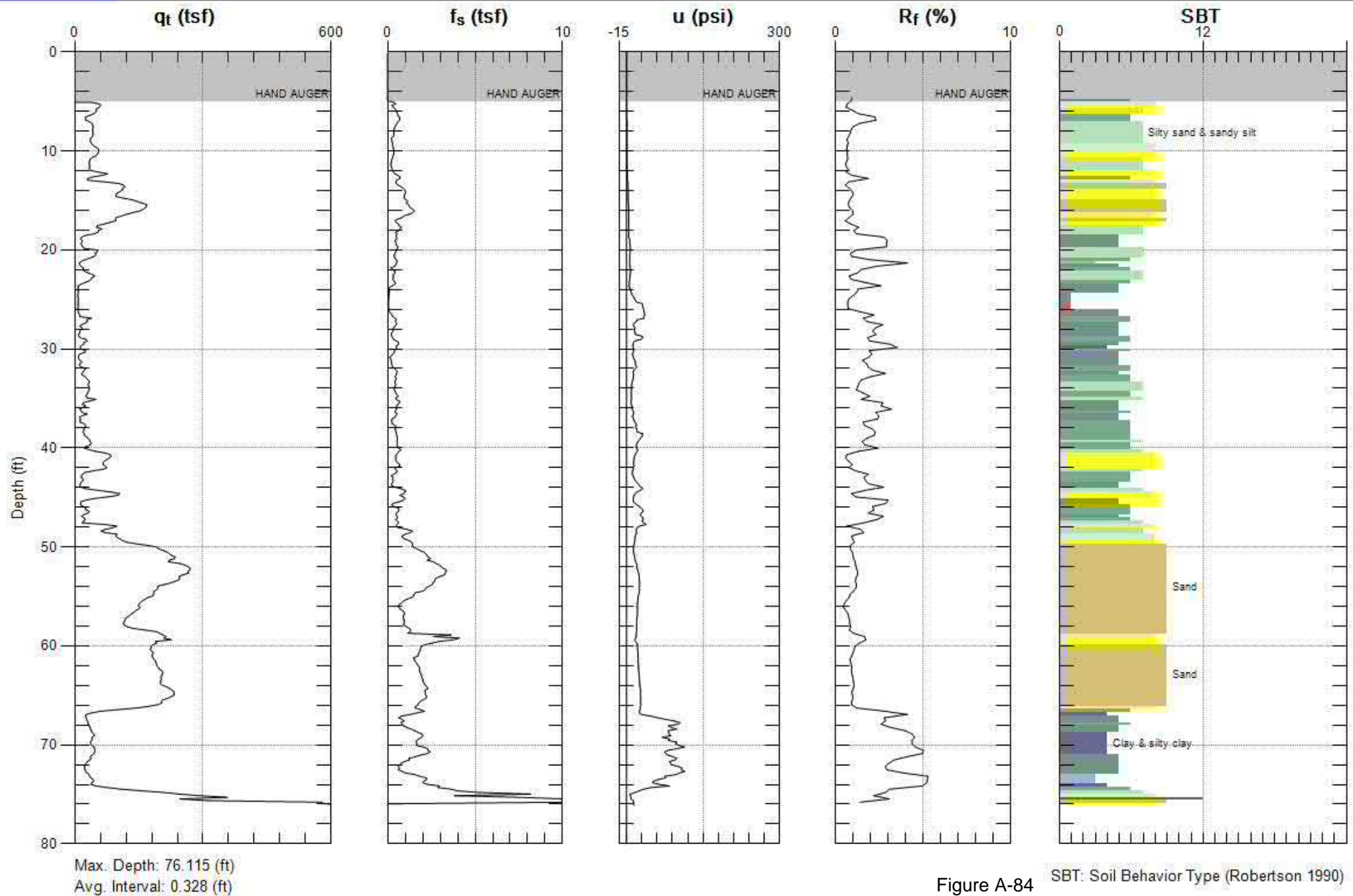
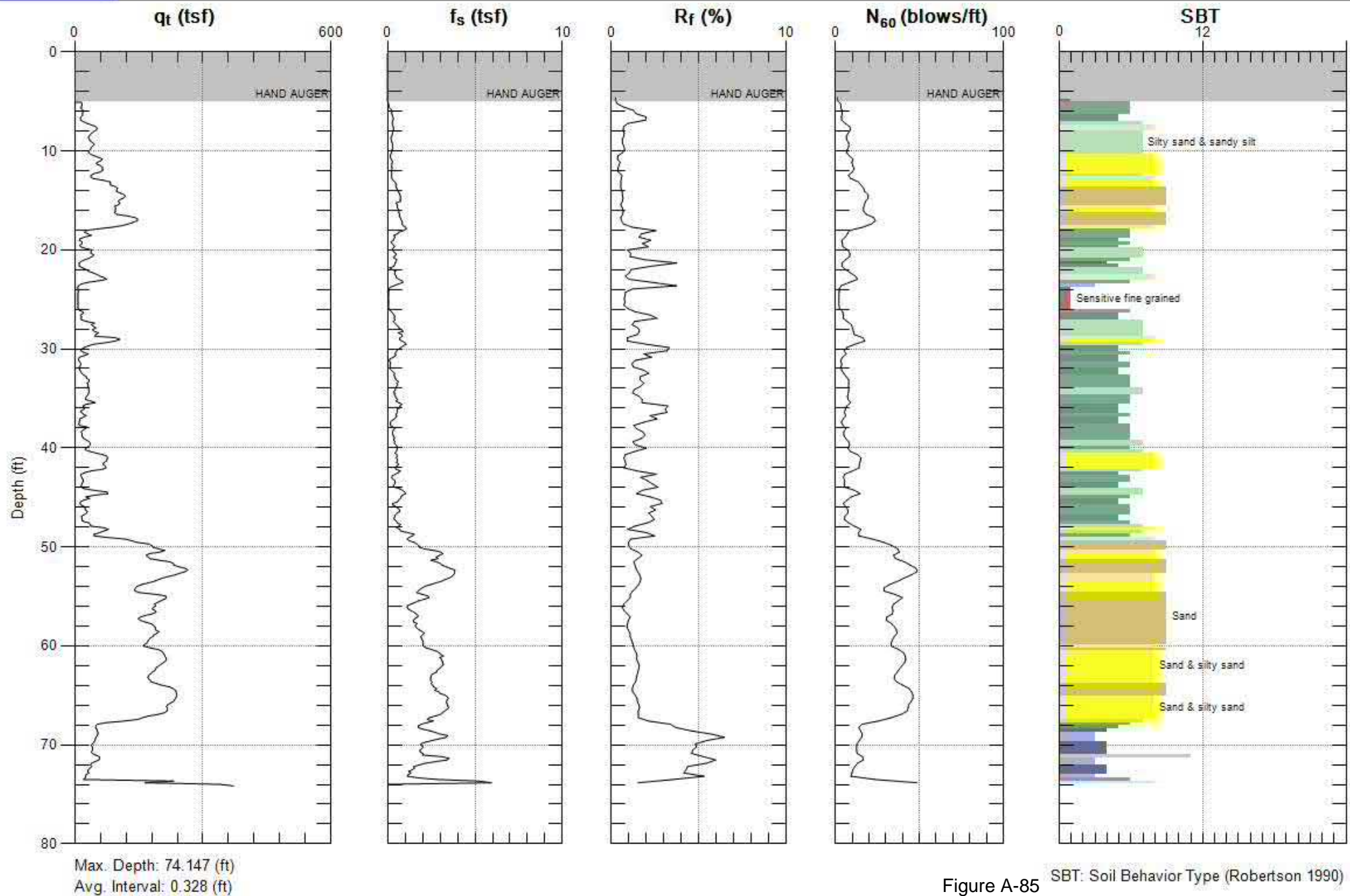


Figure A-84 SBT: Soil Behavior Type (Robertson 1990)



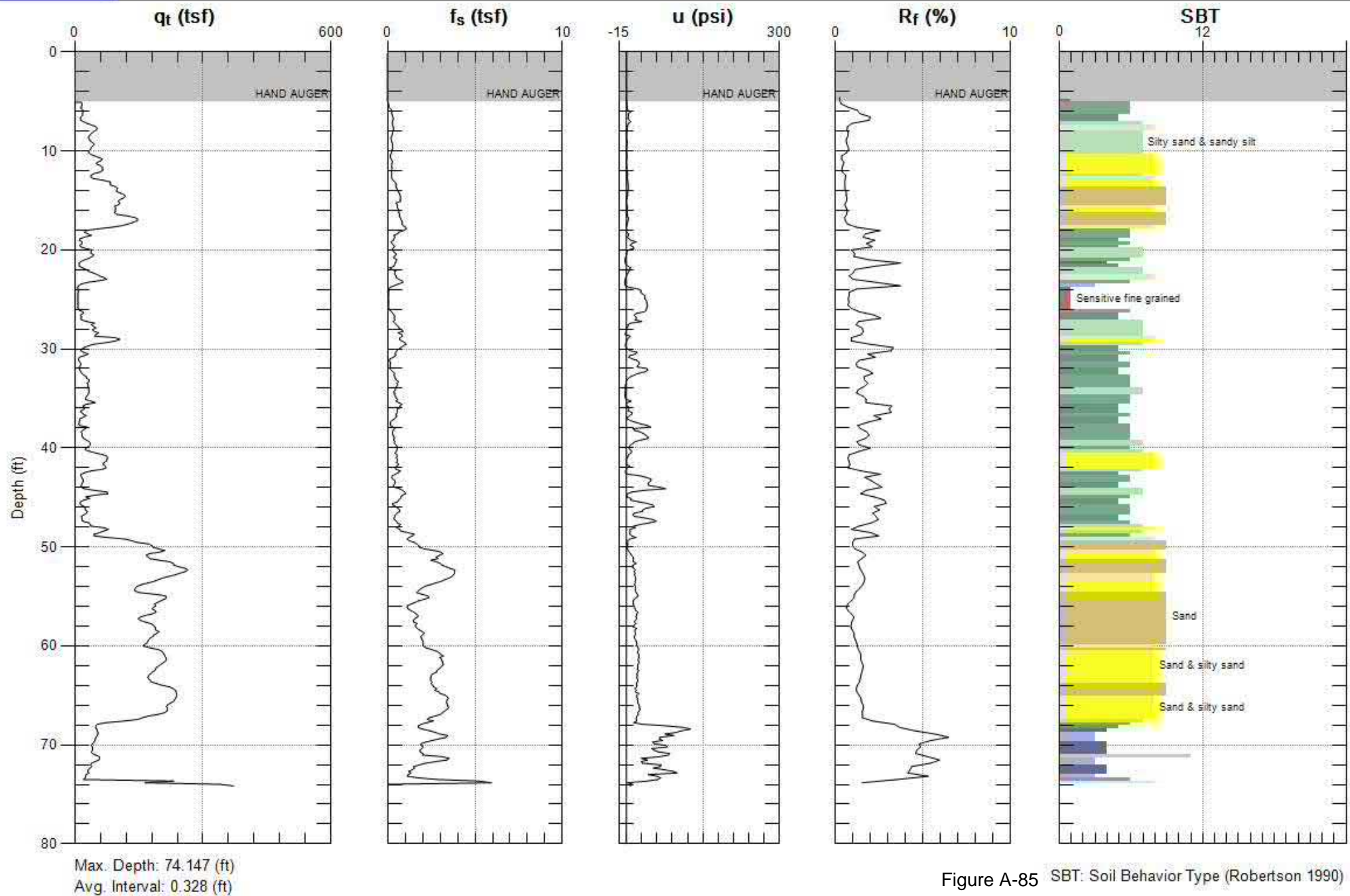
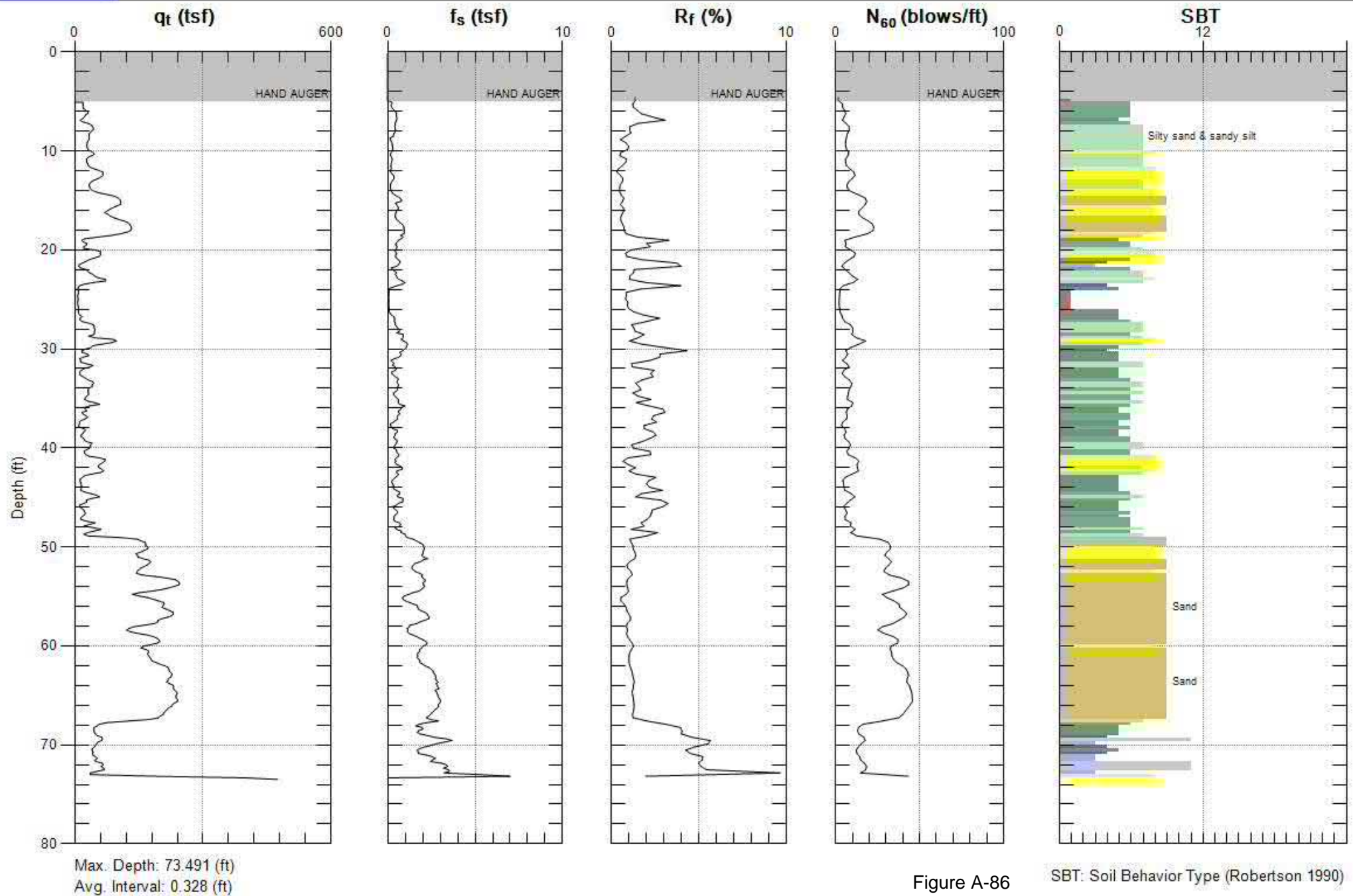


Figure A-85 SBT: Soil Behavior Type (Robertson 1990)



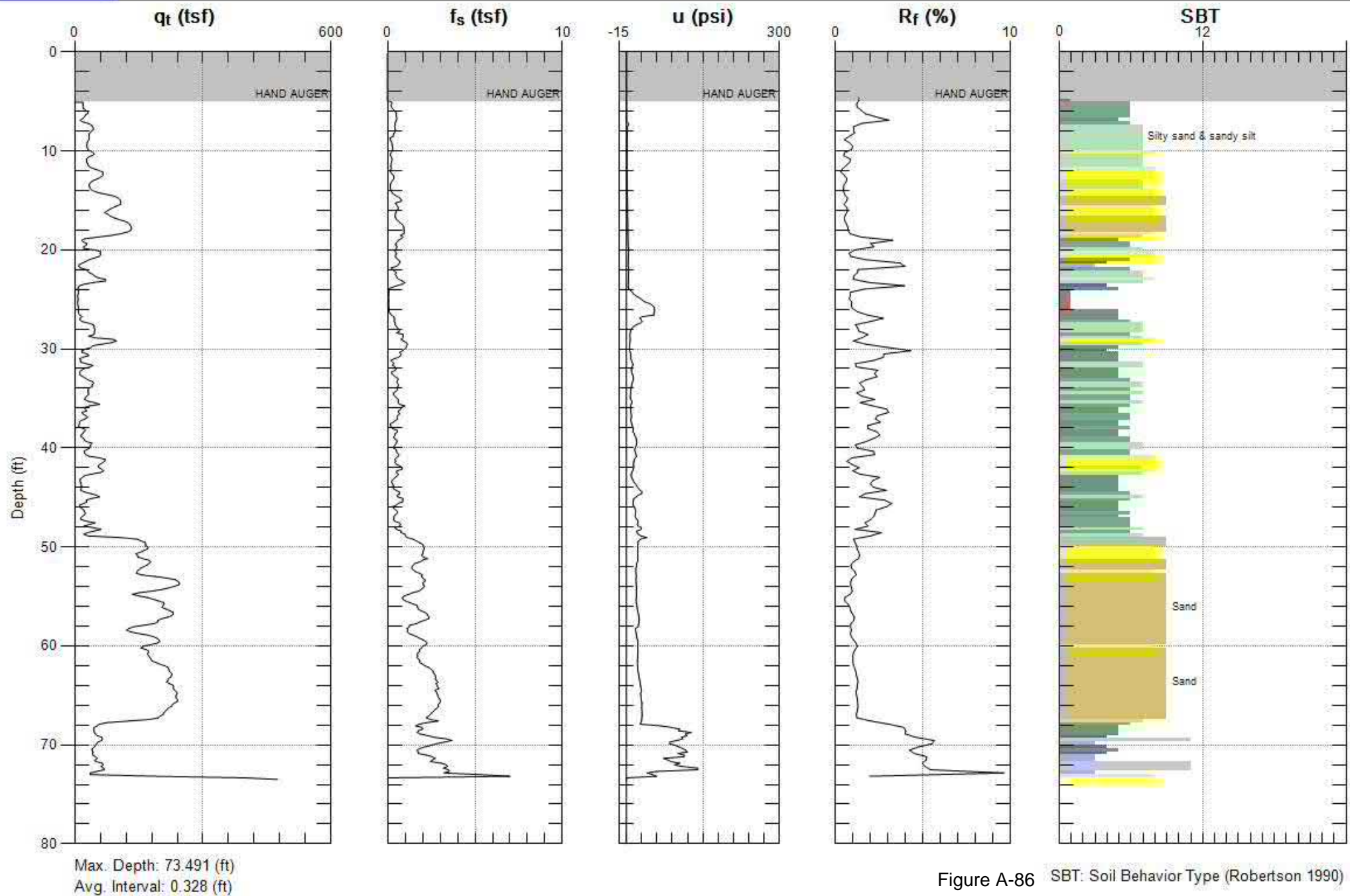


Figure A-86 SBT: Soil Behavior Type (Robertson 1990)

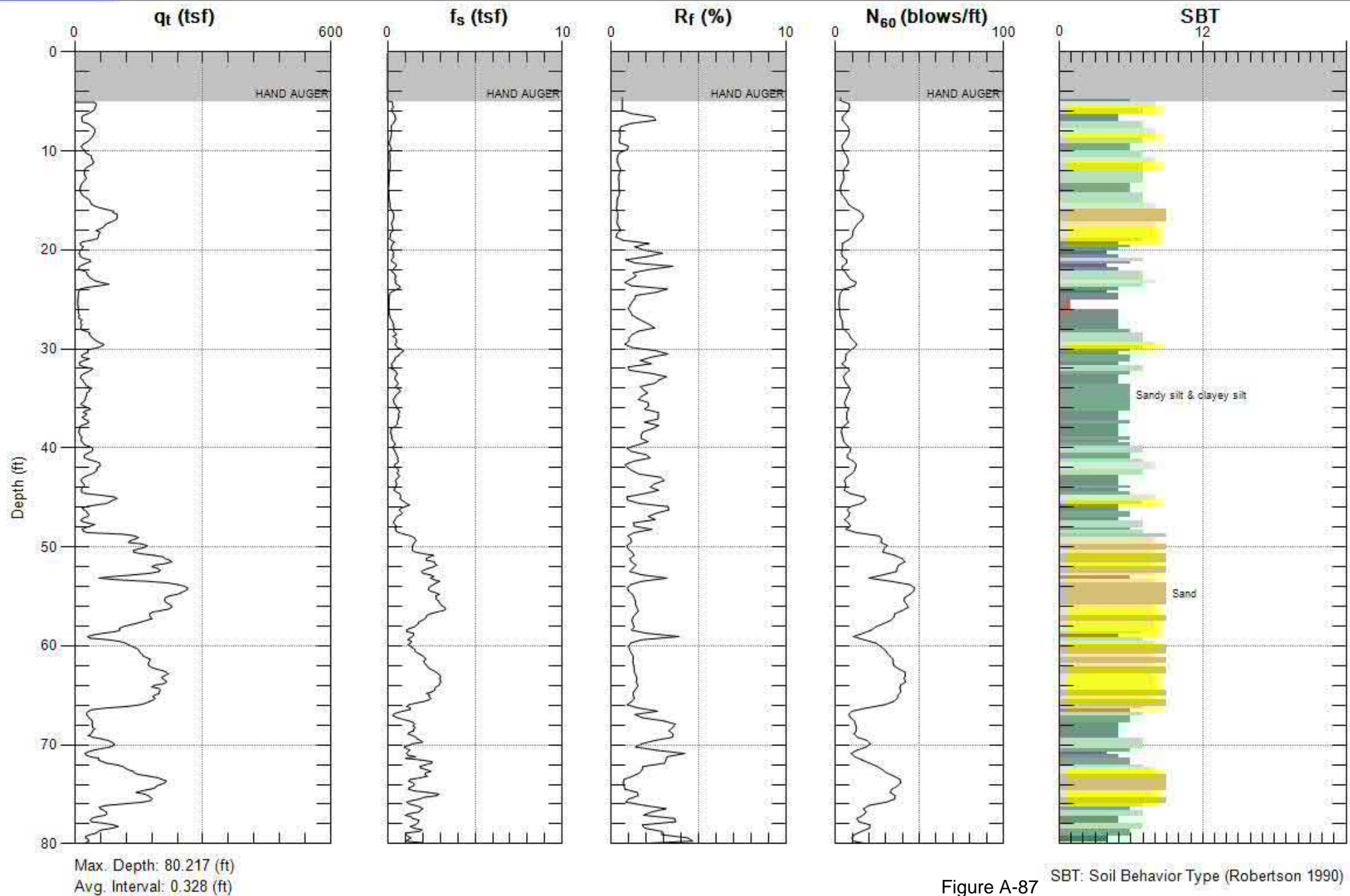
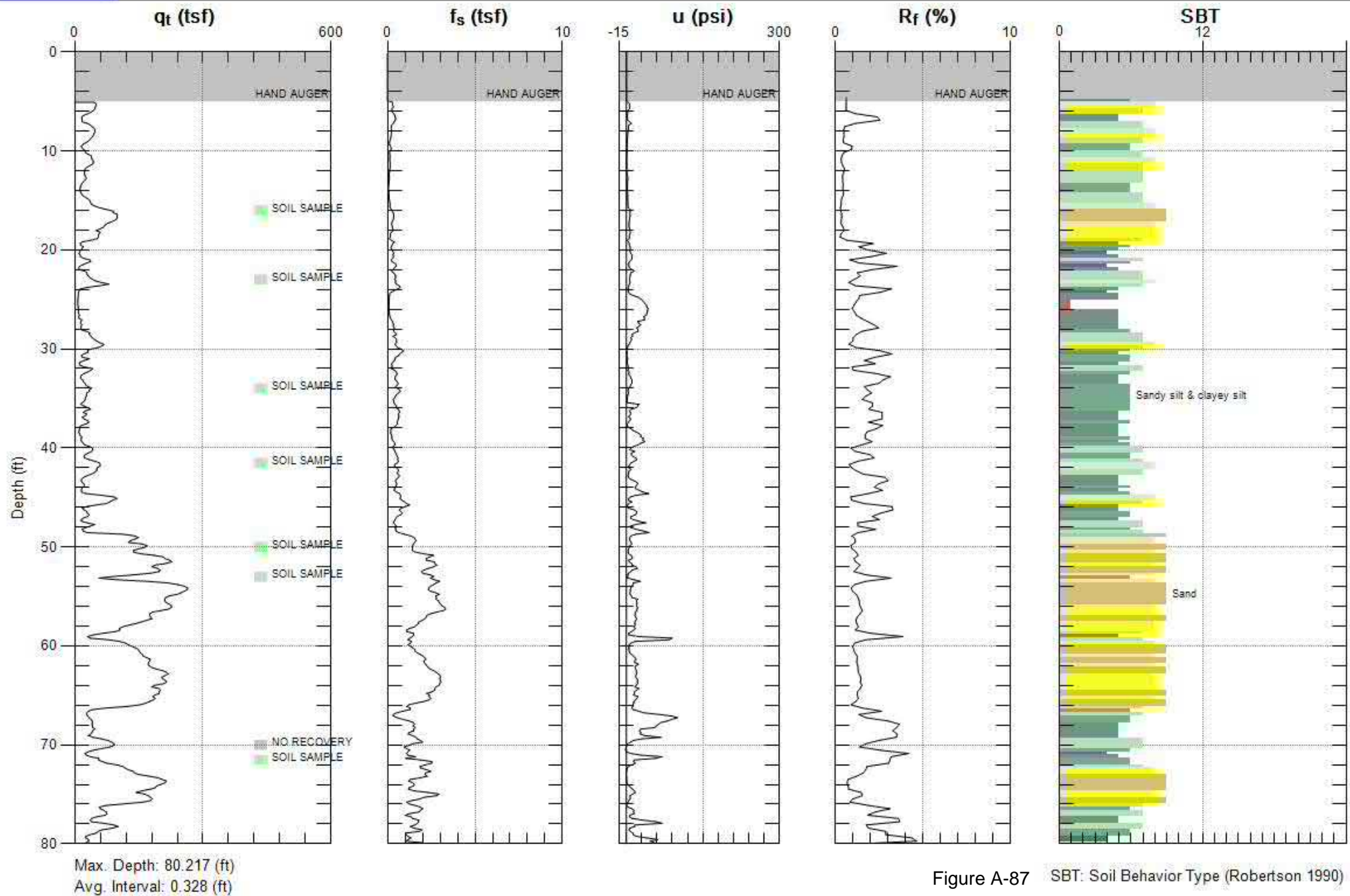


Figure A-87 SBT: Soil Behavior Type (Robertson 1990)



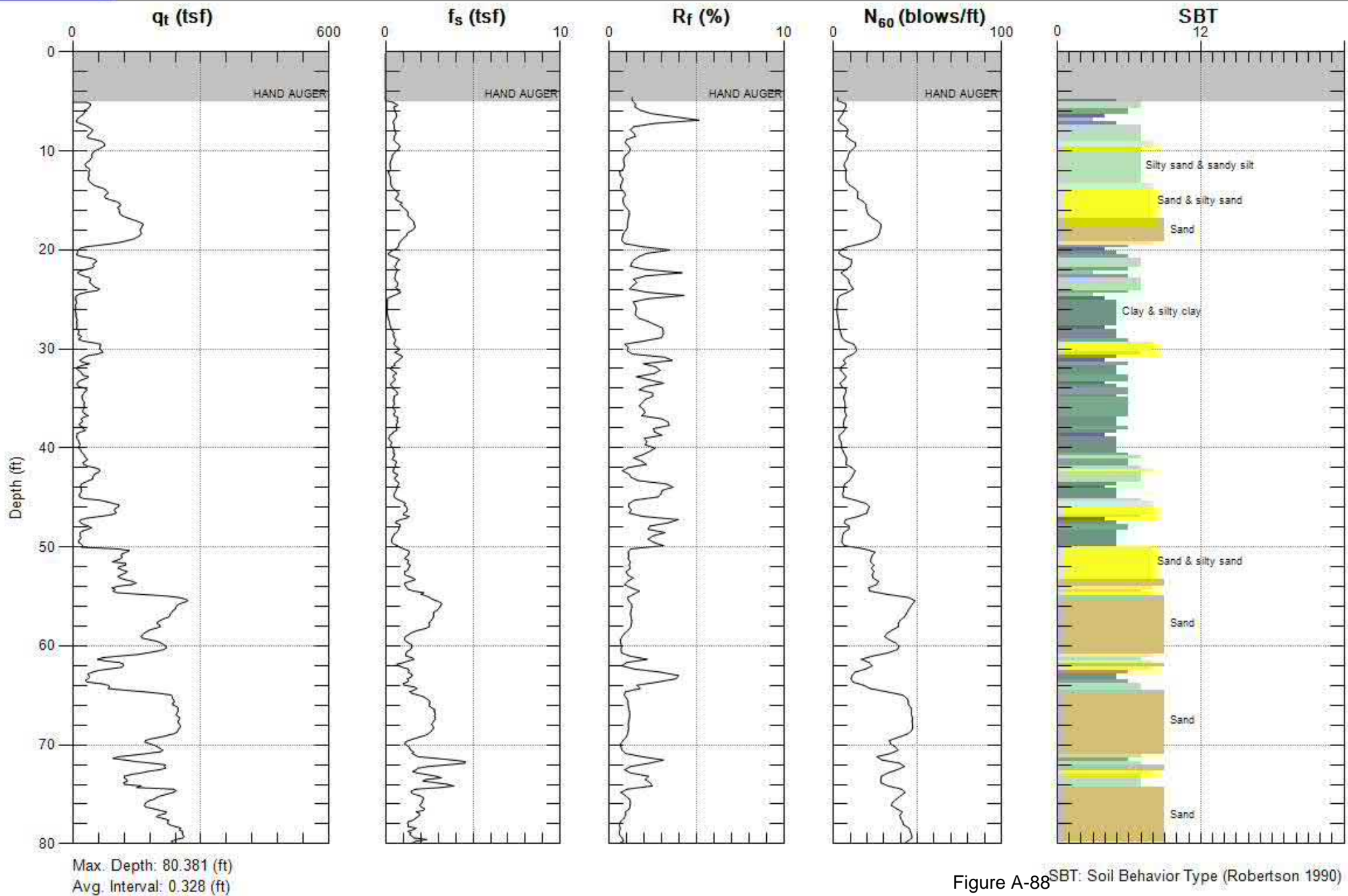
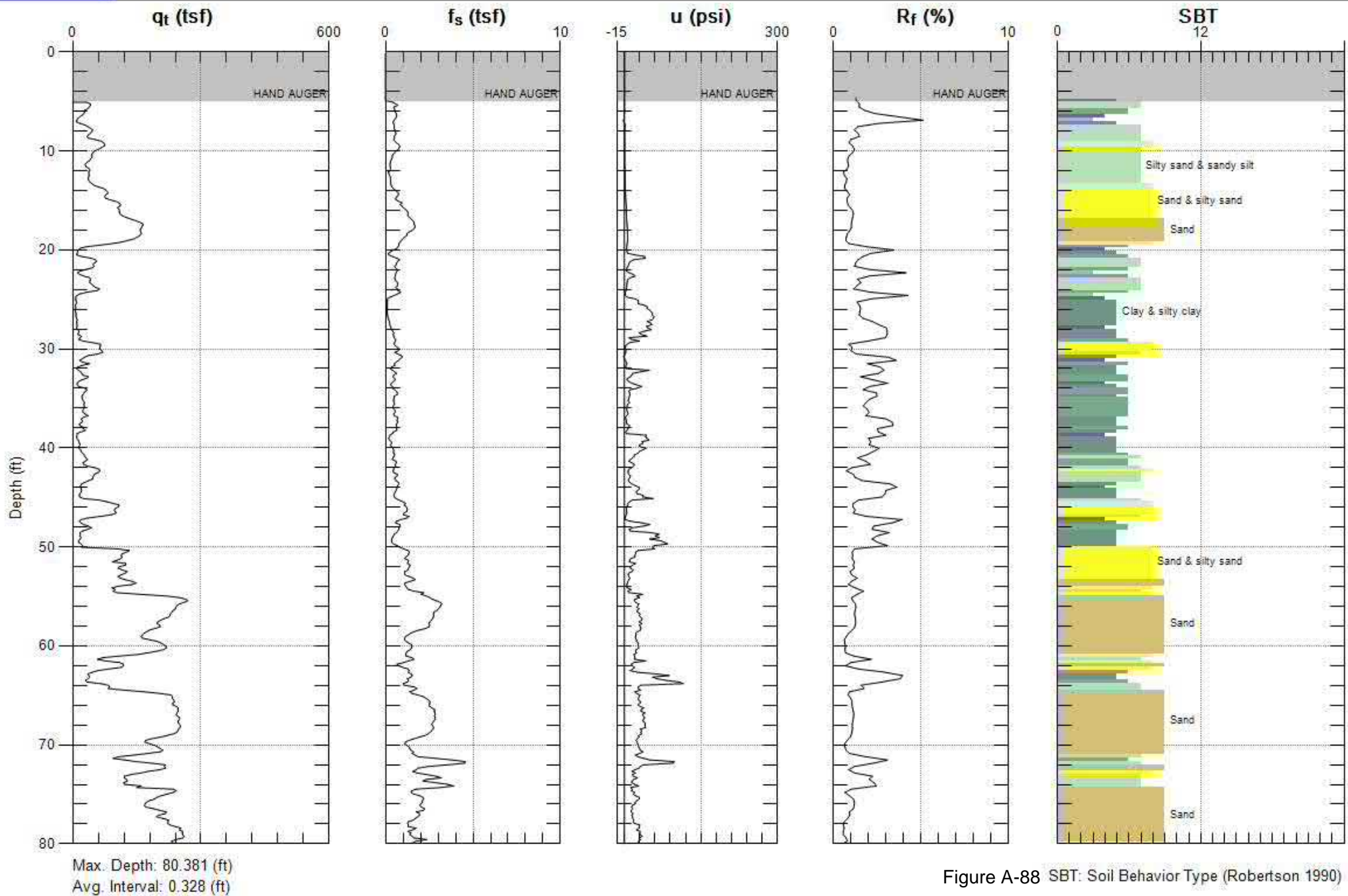
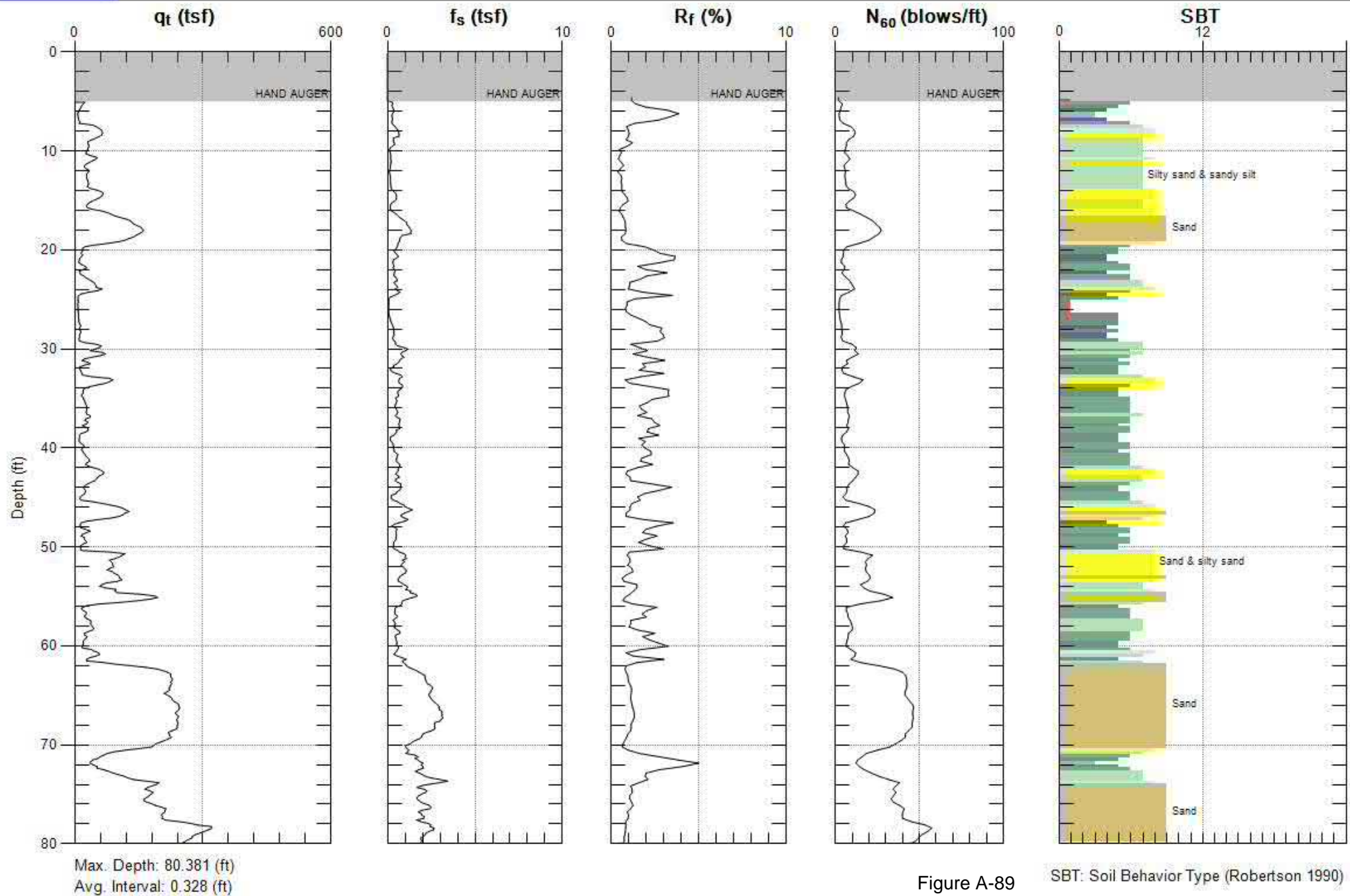


Figure A-88 SBT: Soil Behavior Type (Robertson 1990)





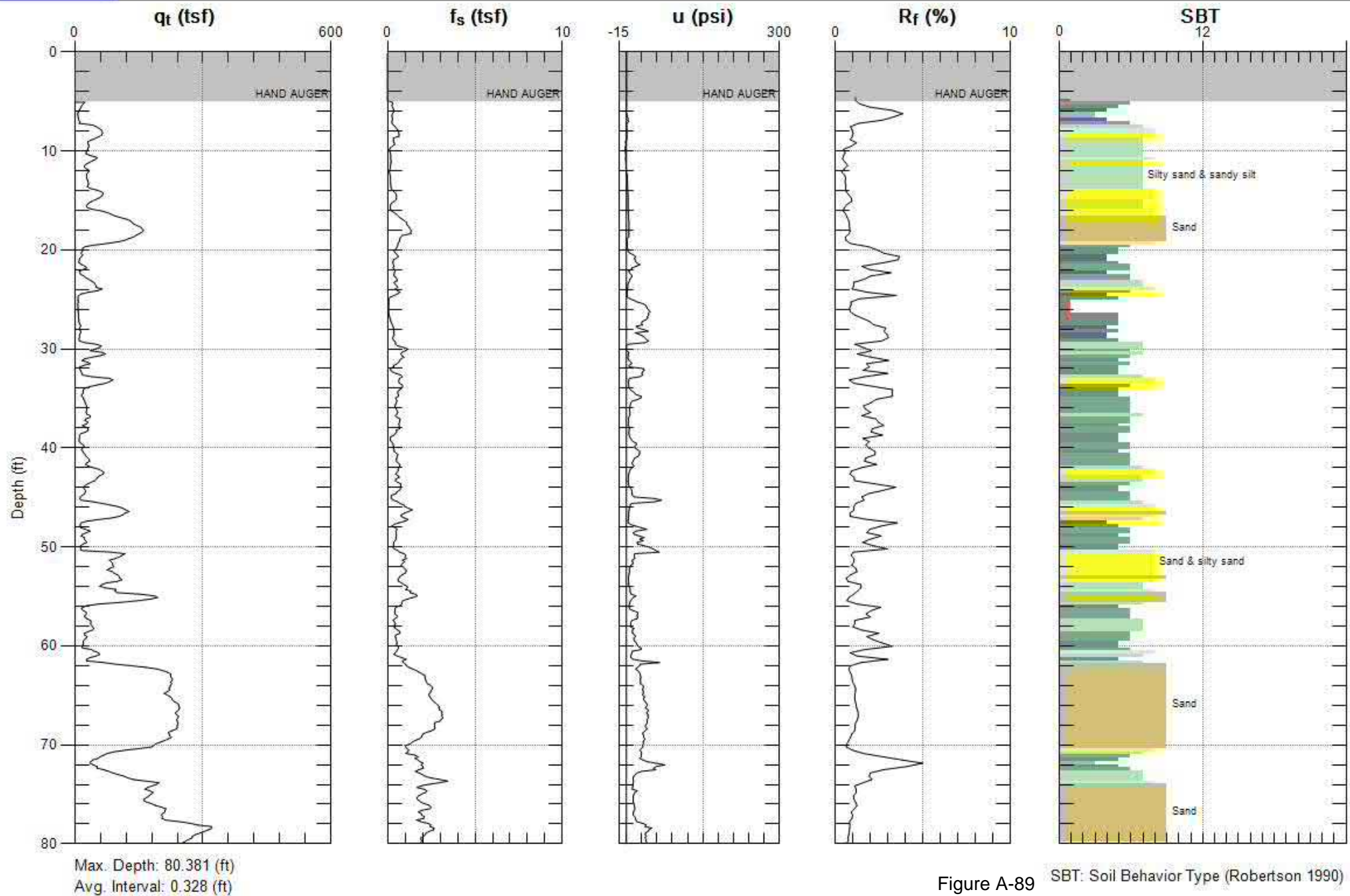


Figure A-89 SBT: Soil Behavior Type (Robertson 1990)

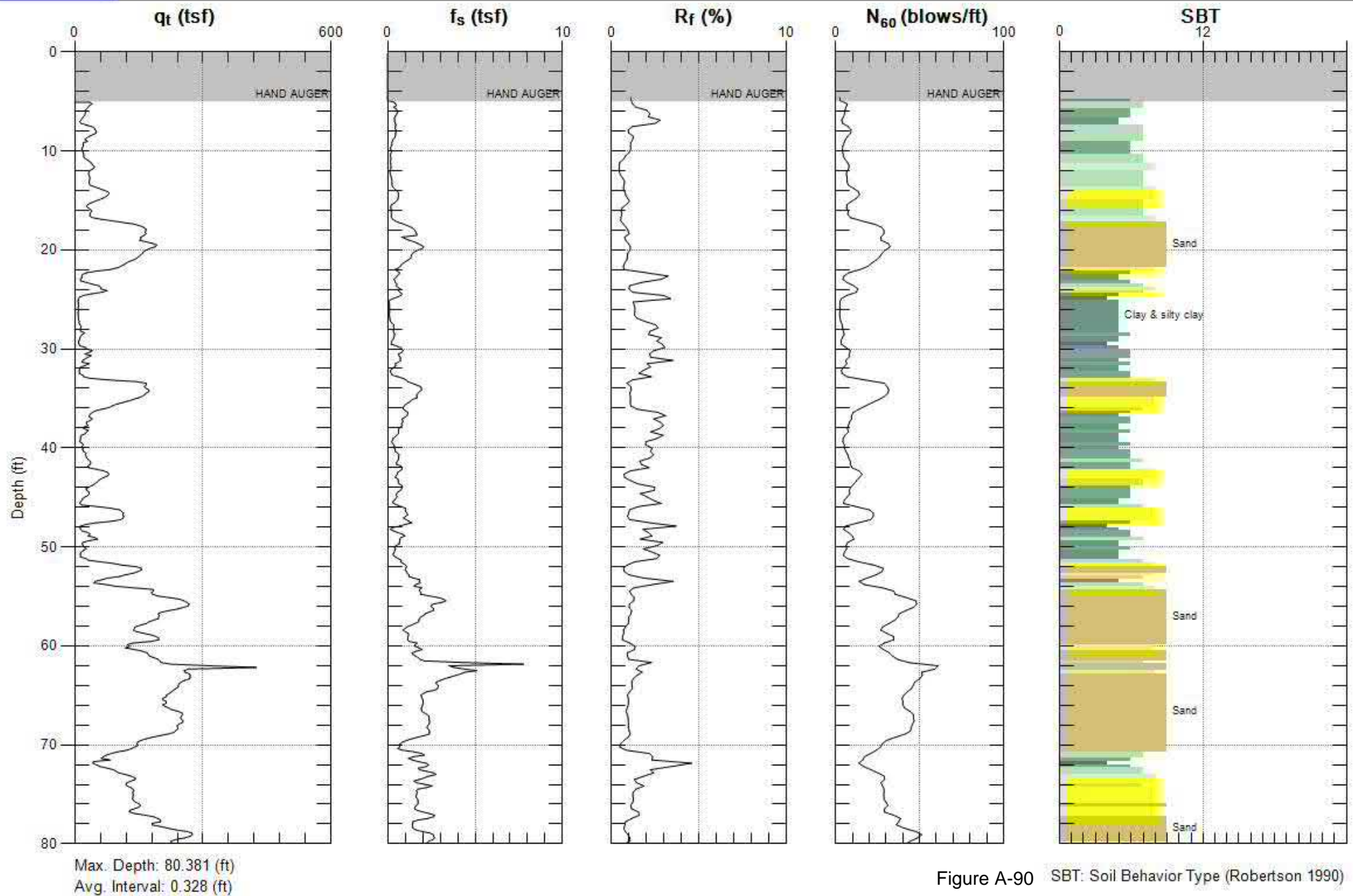


Figure A-90 SBT: Soil Behavior Type (Robertson 1990)

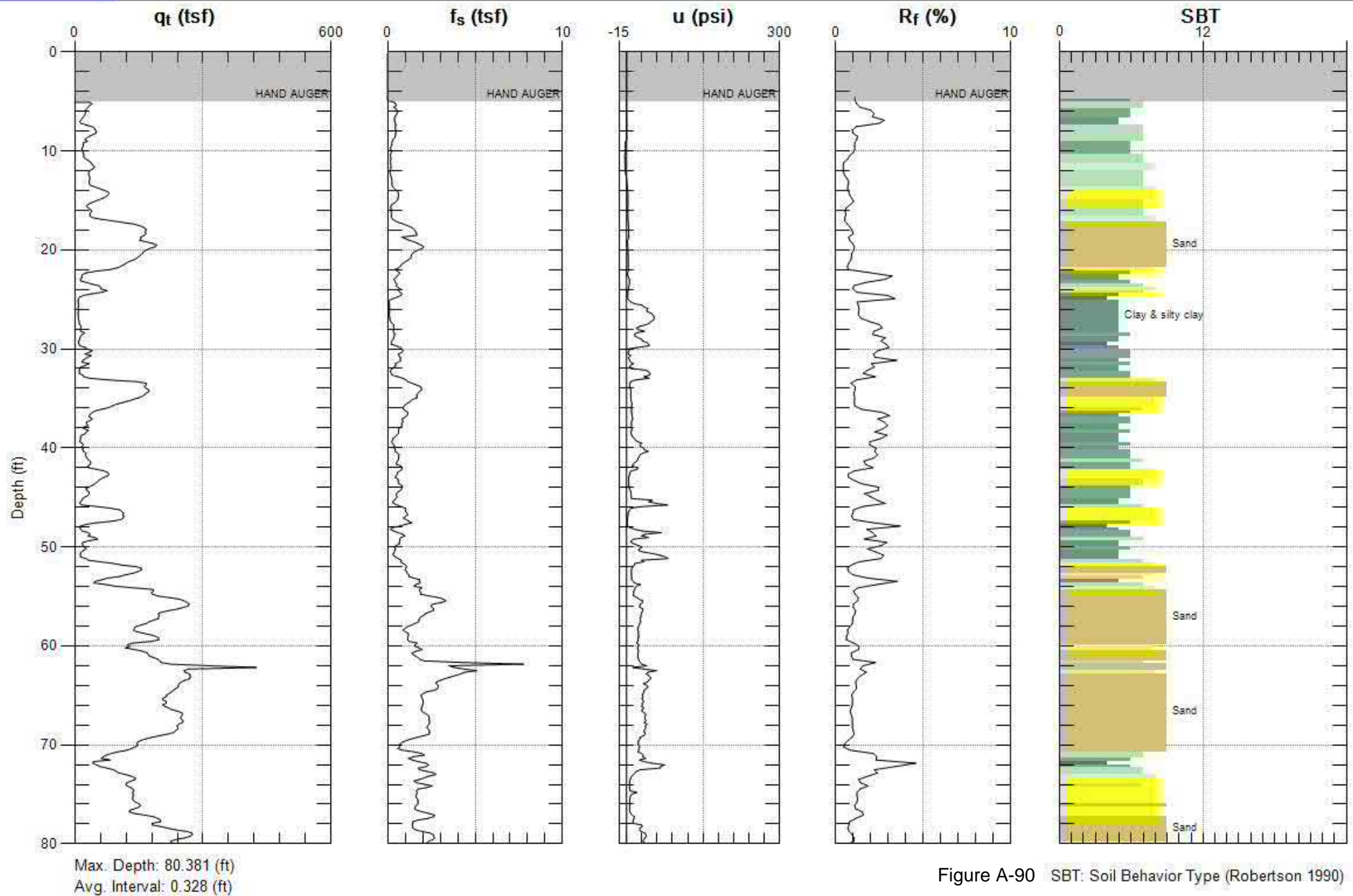


Figure A-90 SBT: Soil Behavior Type (Robertson 1990)

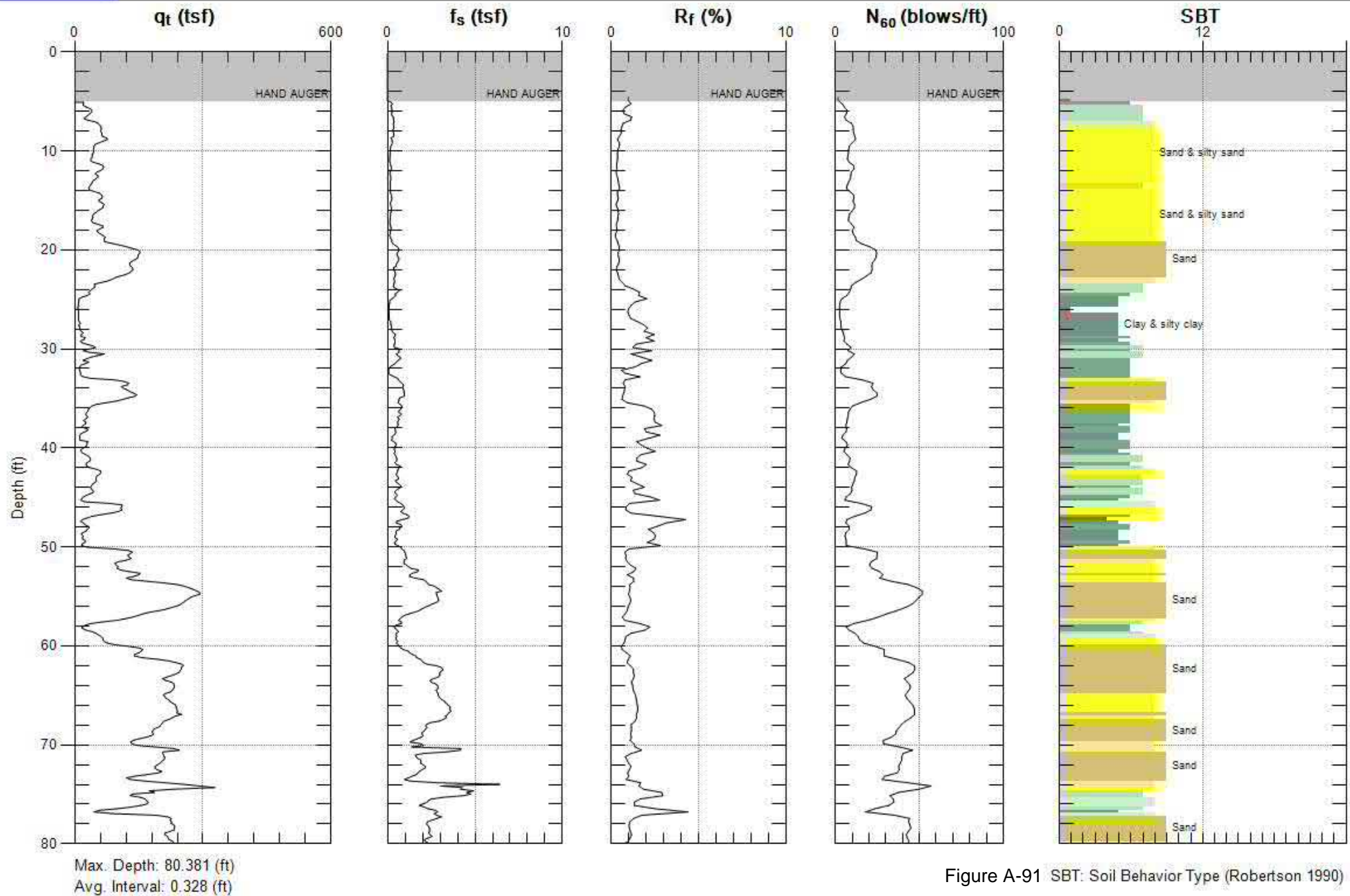


Figure A-91 SBT: Soil Behavior Type (Robertson 1990)

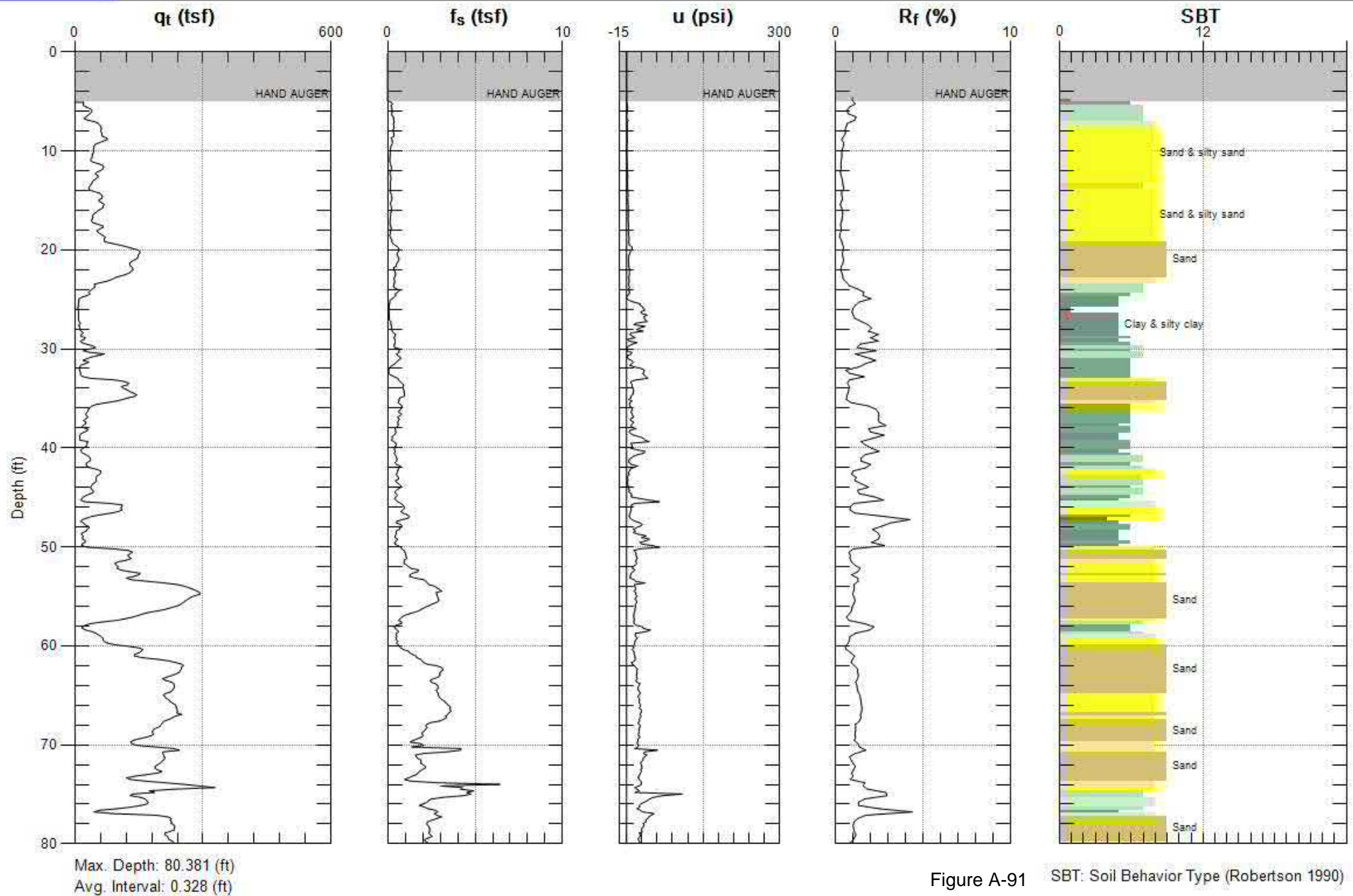


Figure A-91 SBT: Soil Behavior Type (Robertson 1990)

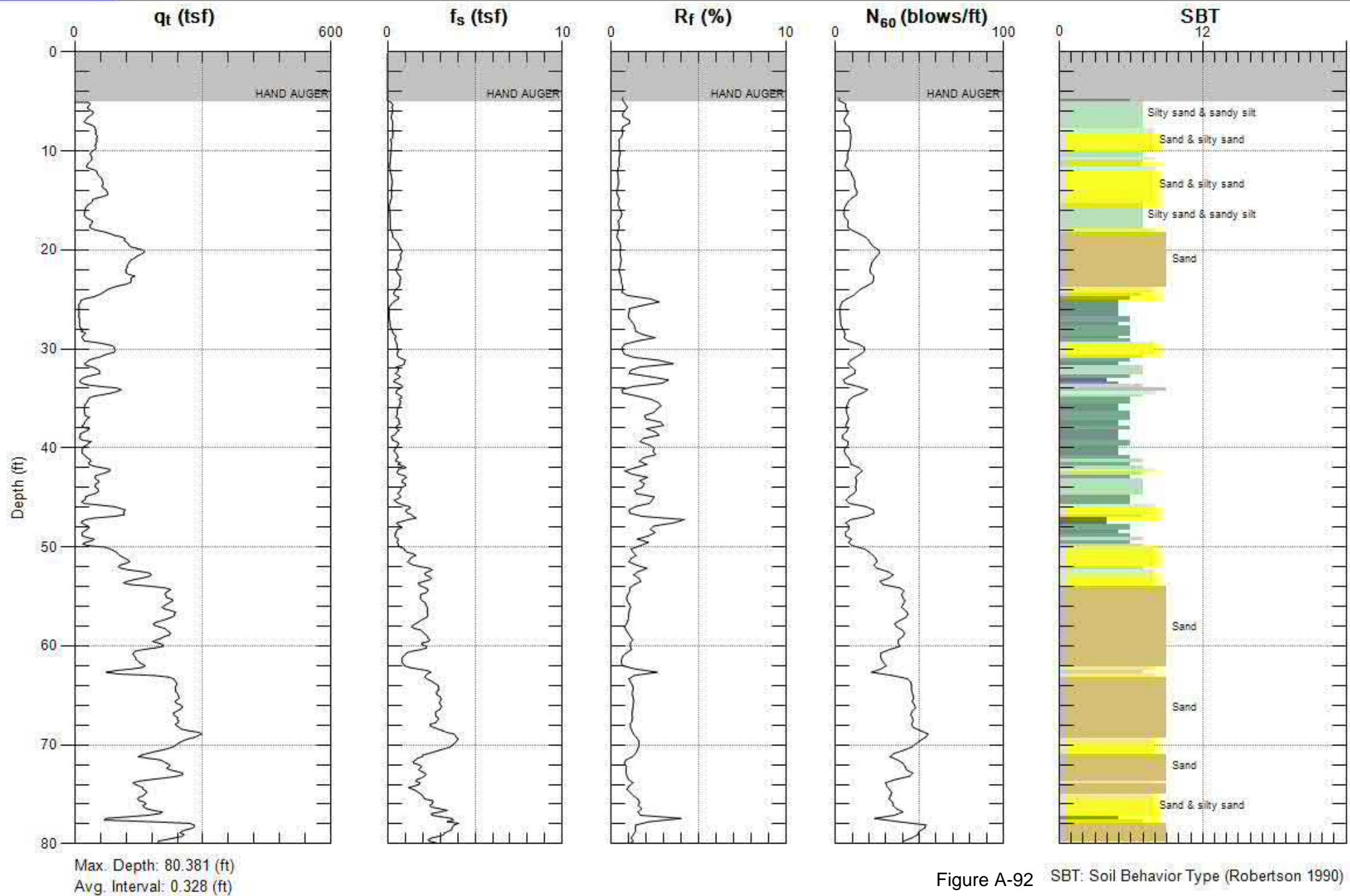


Figure A-92 SBT: Soil Behavior Type (Robertson 1990)

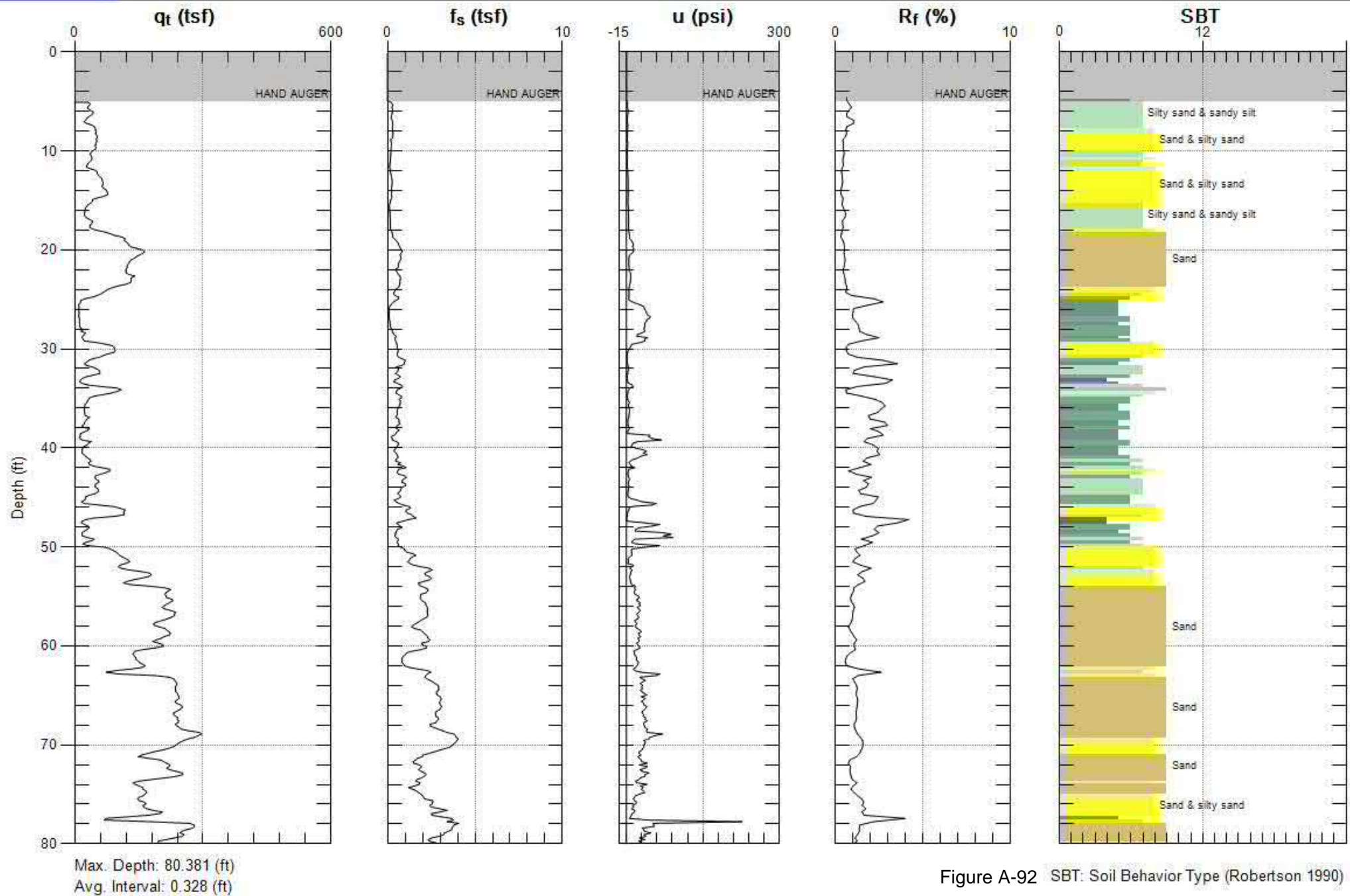


Figure A-92 SBT: Soil Behavior Type (Robertson 1990)

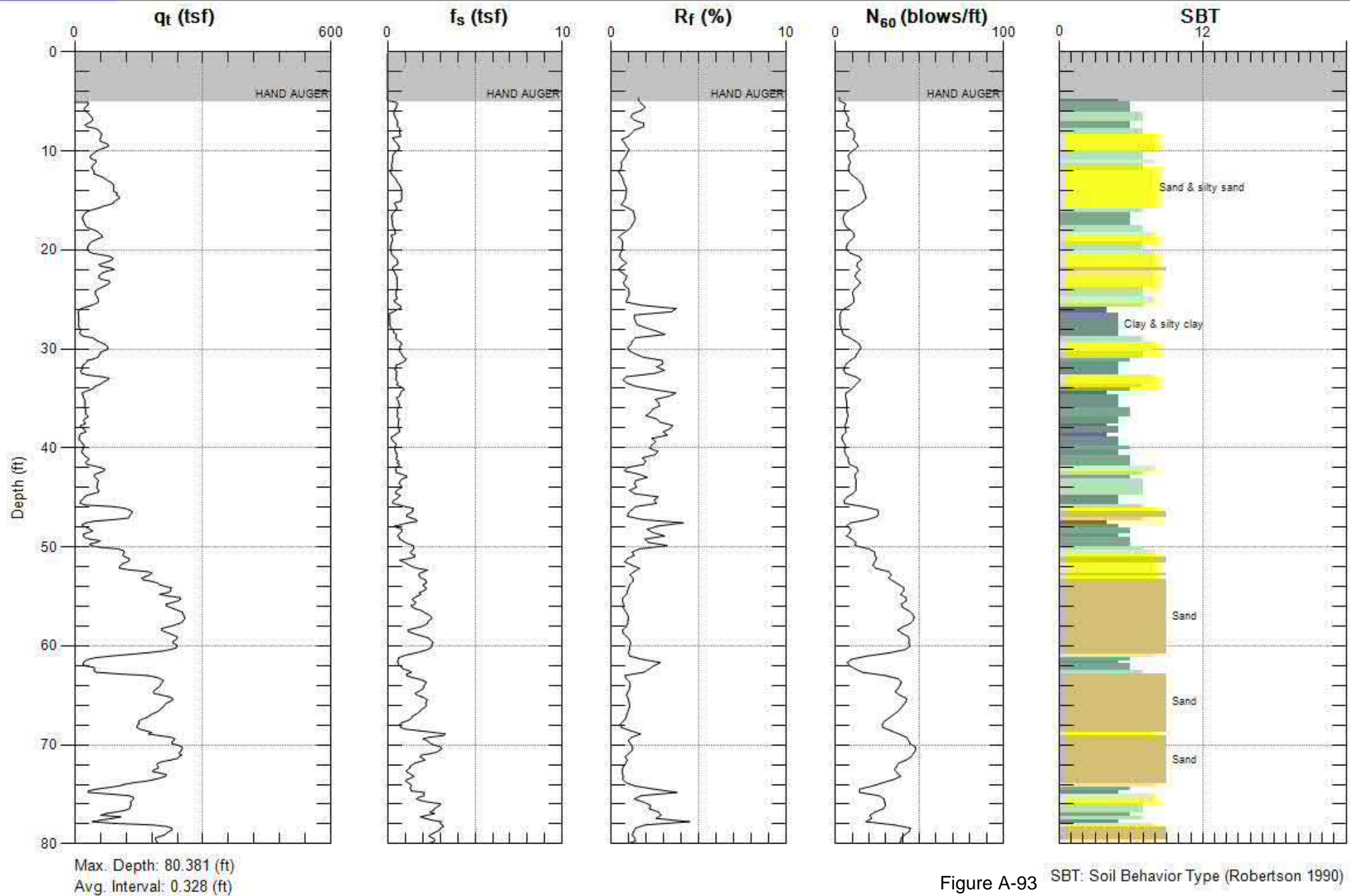


Figure A-93 SBT: Soil Behavior Type (Robertson 1990)

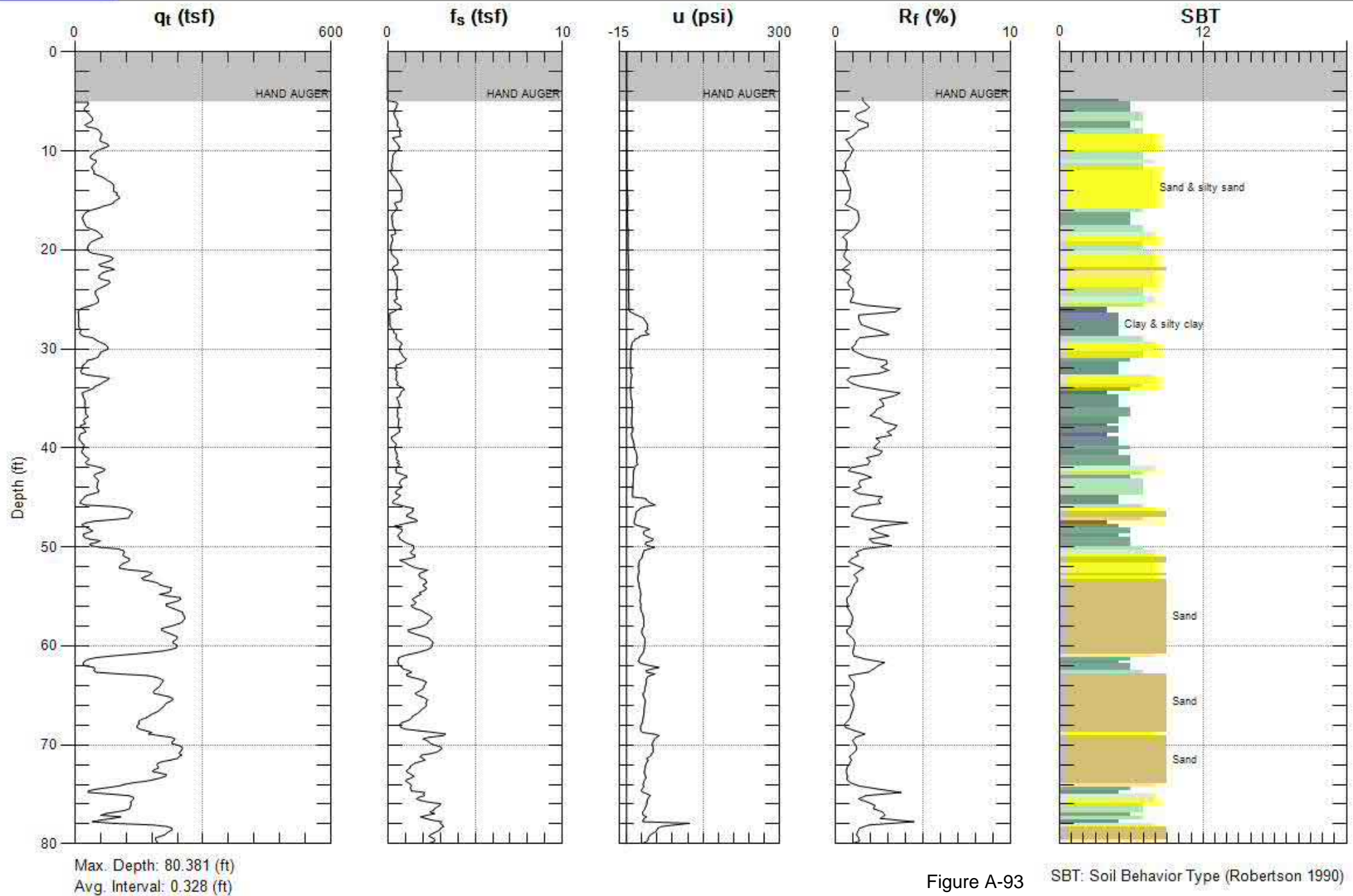
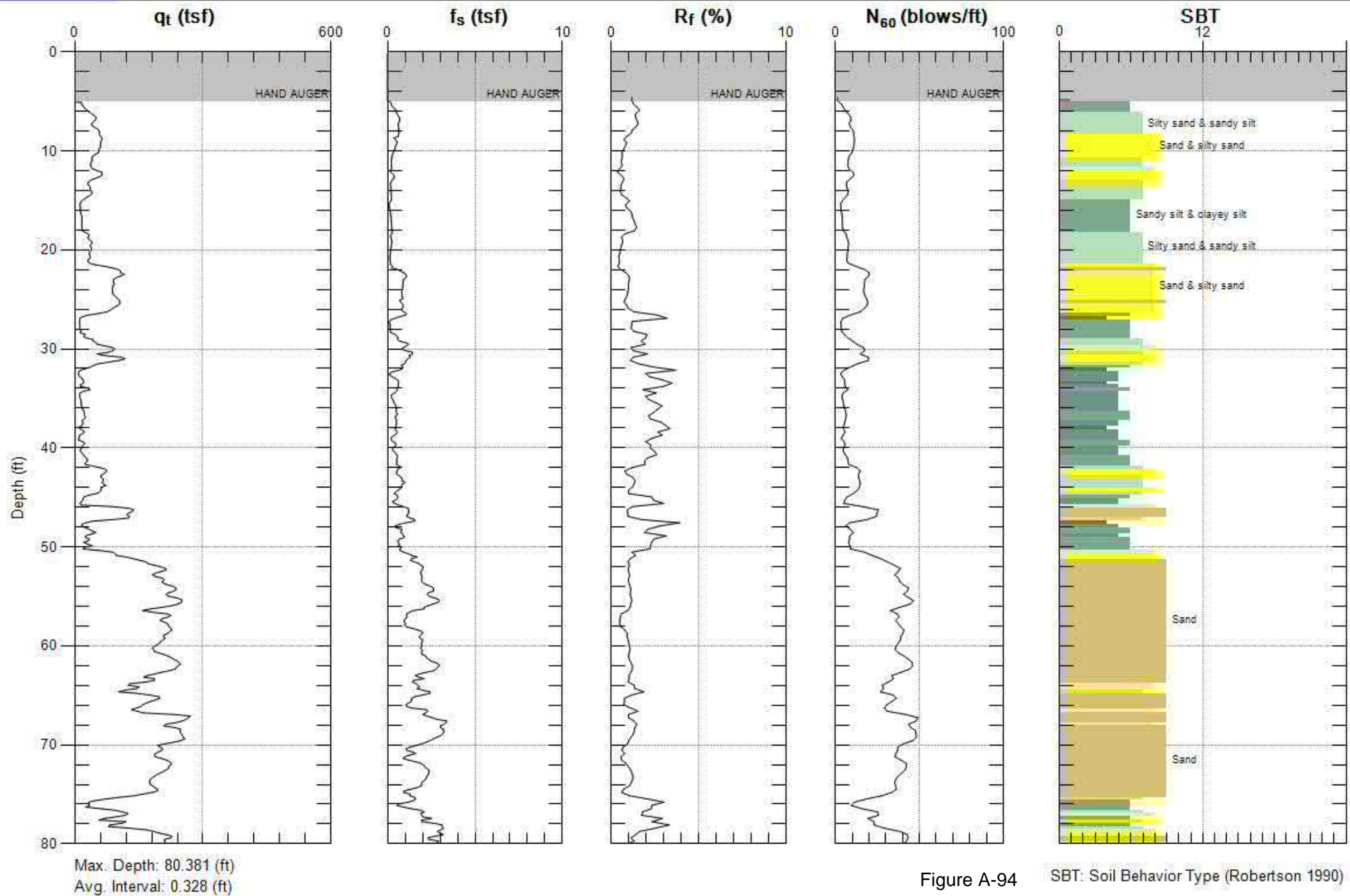


Figure A-93

SBT: Soil Behavior Type (Robertson 1990)



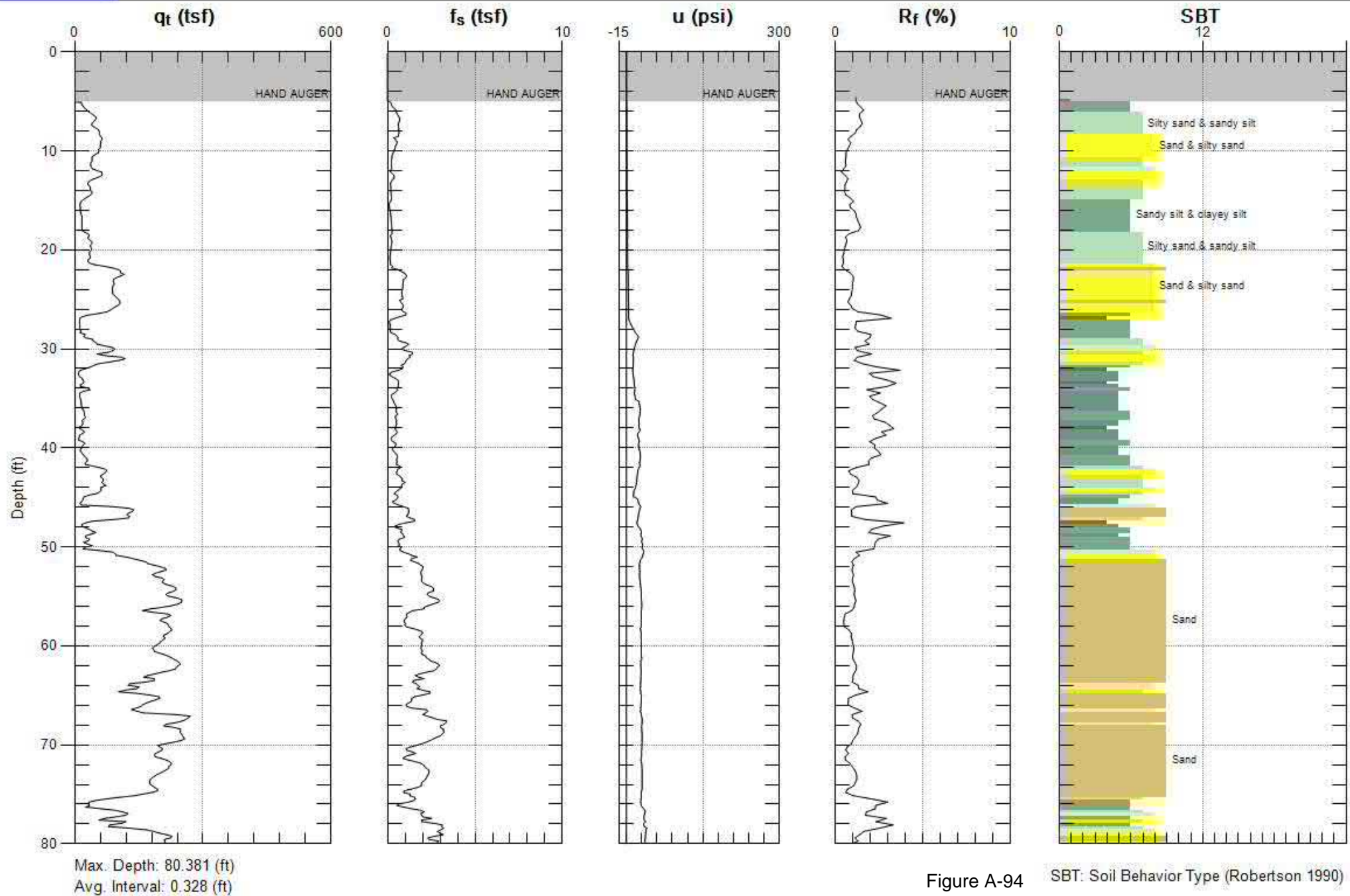


Figure A-94

SBT: Soil Behavior Type (Robertson 1990)

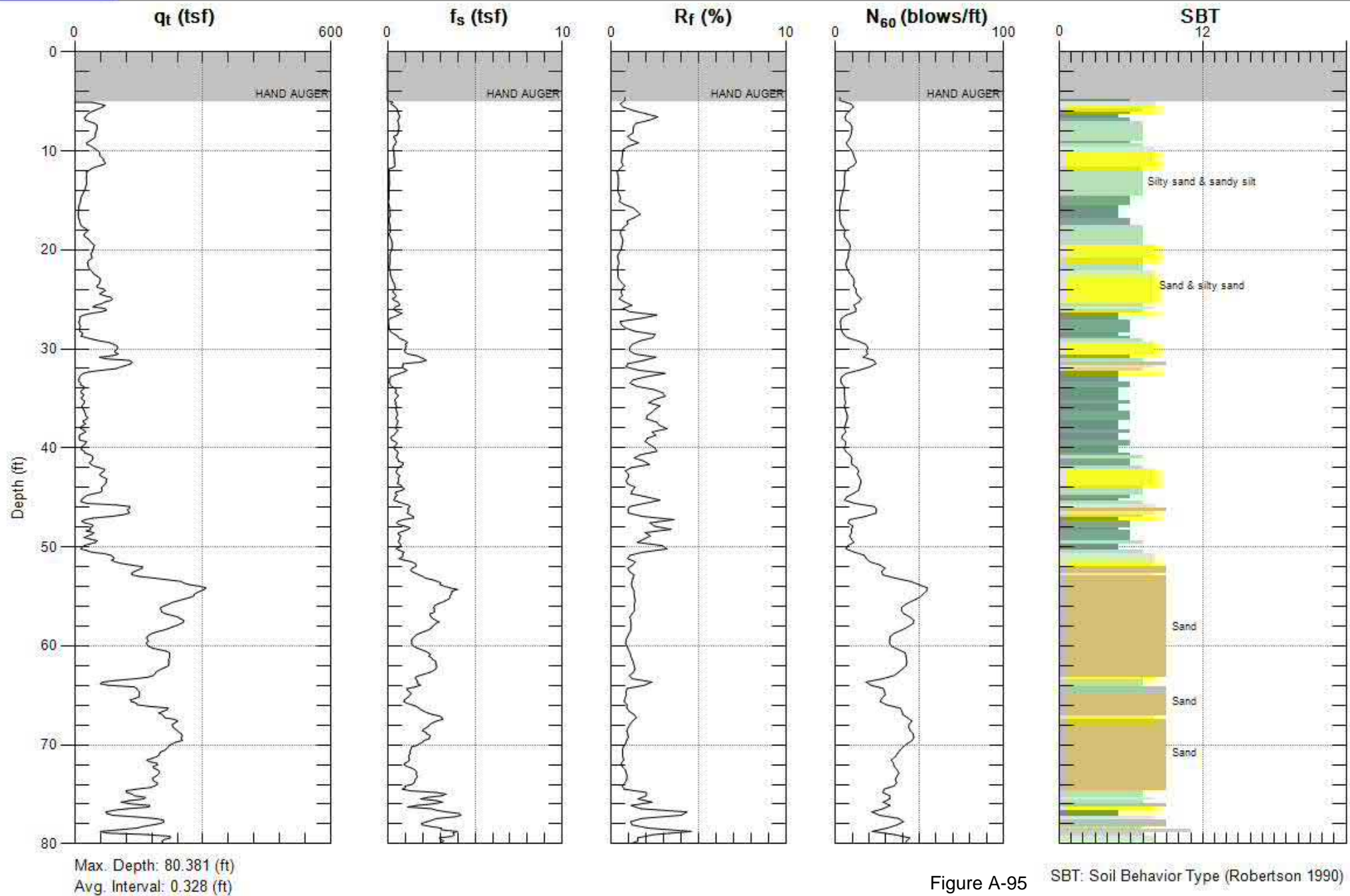


Figure A-95

SBT: Soil Behavior Type (Robertson 1990)

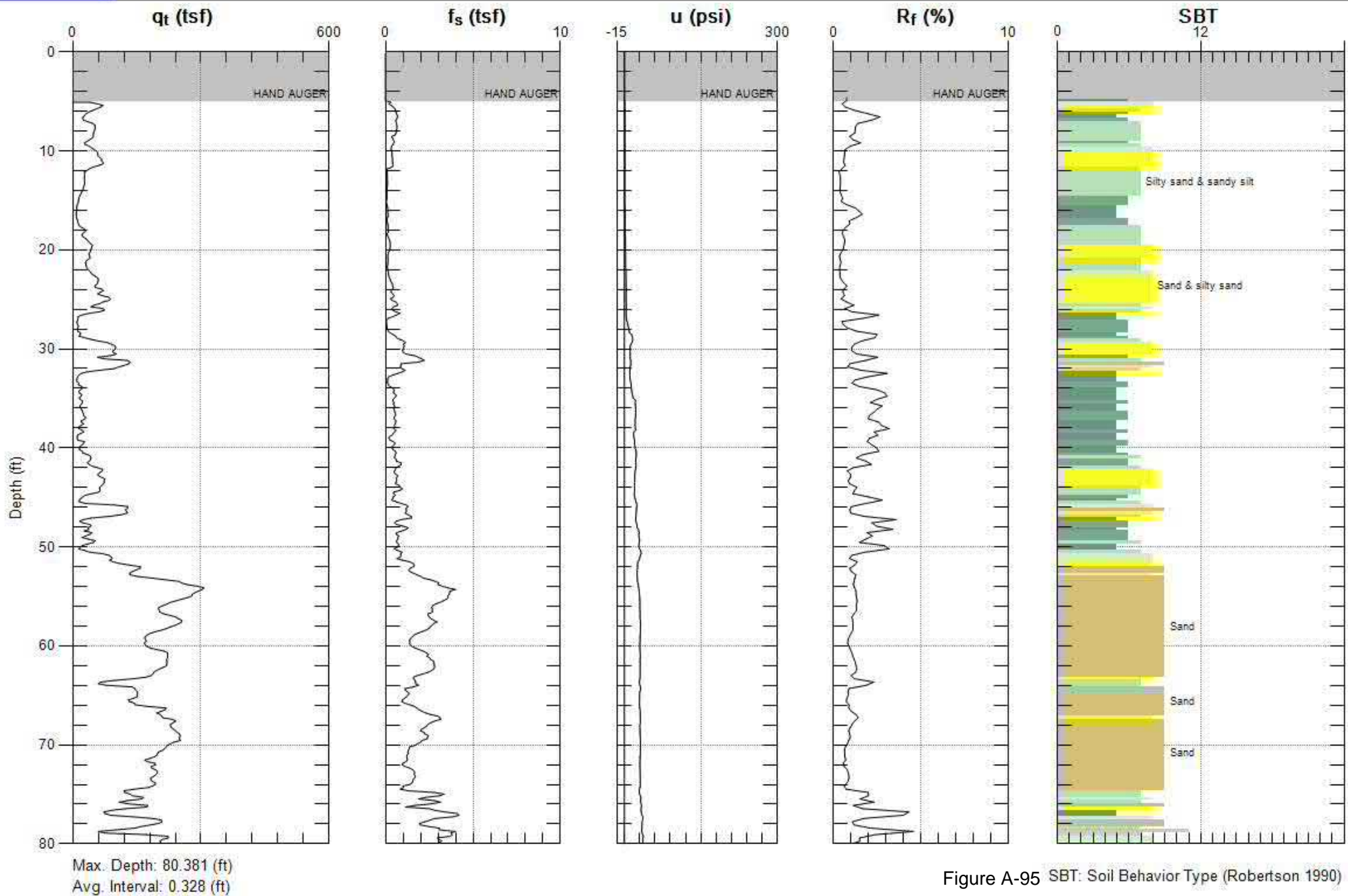


Figure A-95 SBT: Soil Behavior Type (Robertson 1990)

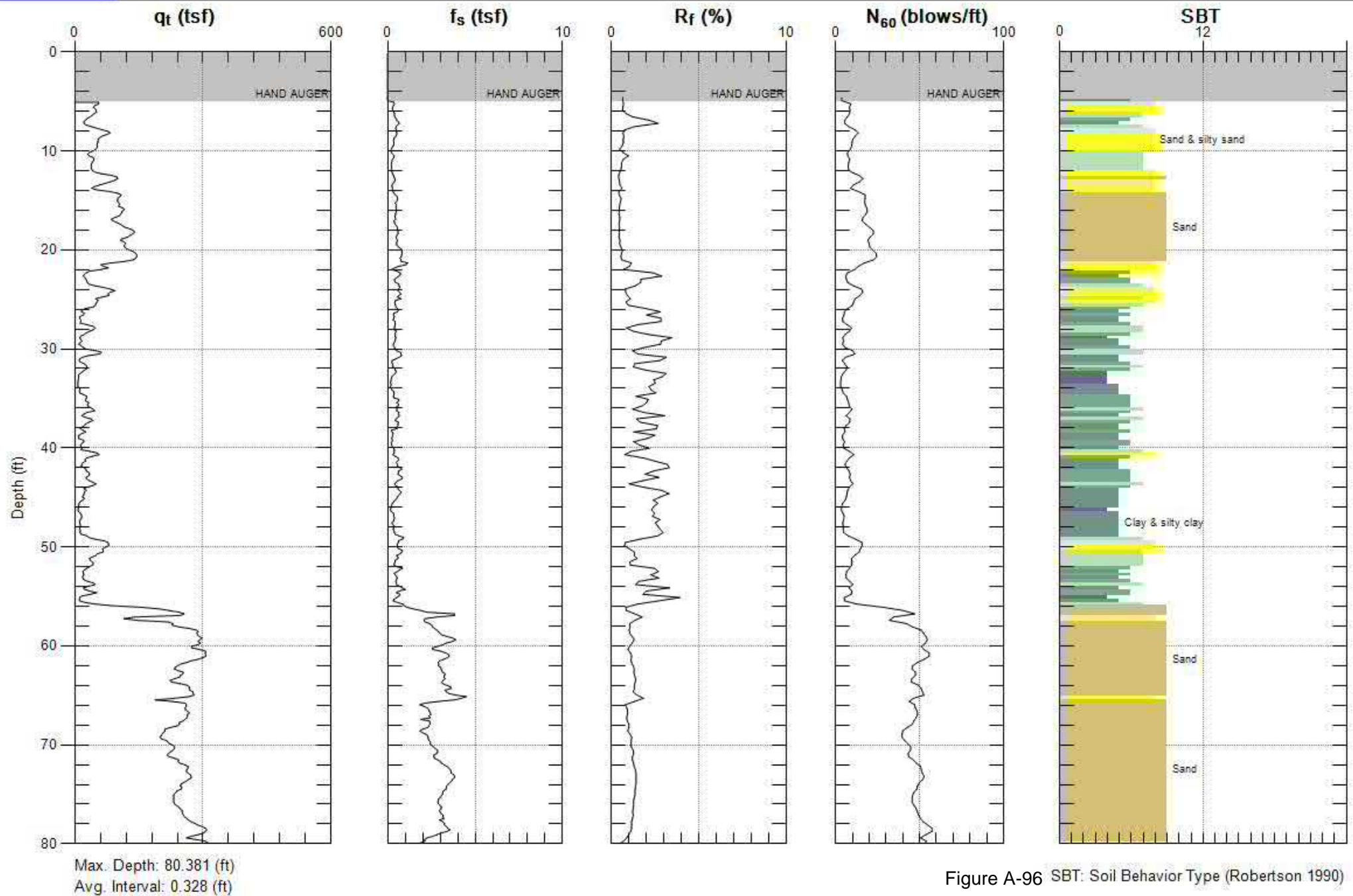


Figure A-96 SBT: Soil Behavior Type (Robertson 1990)

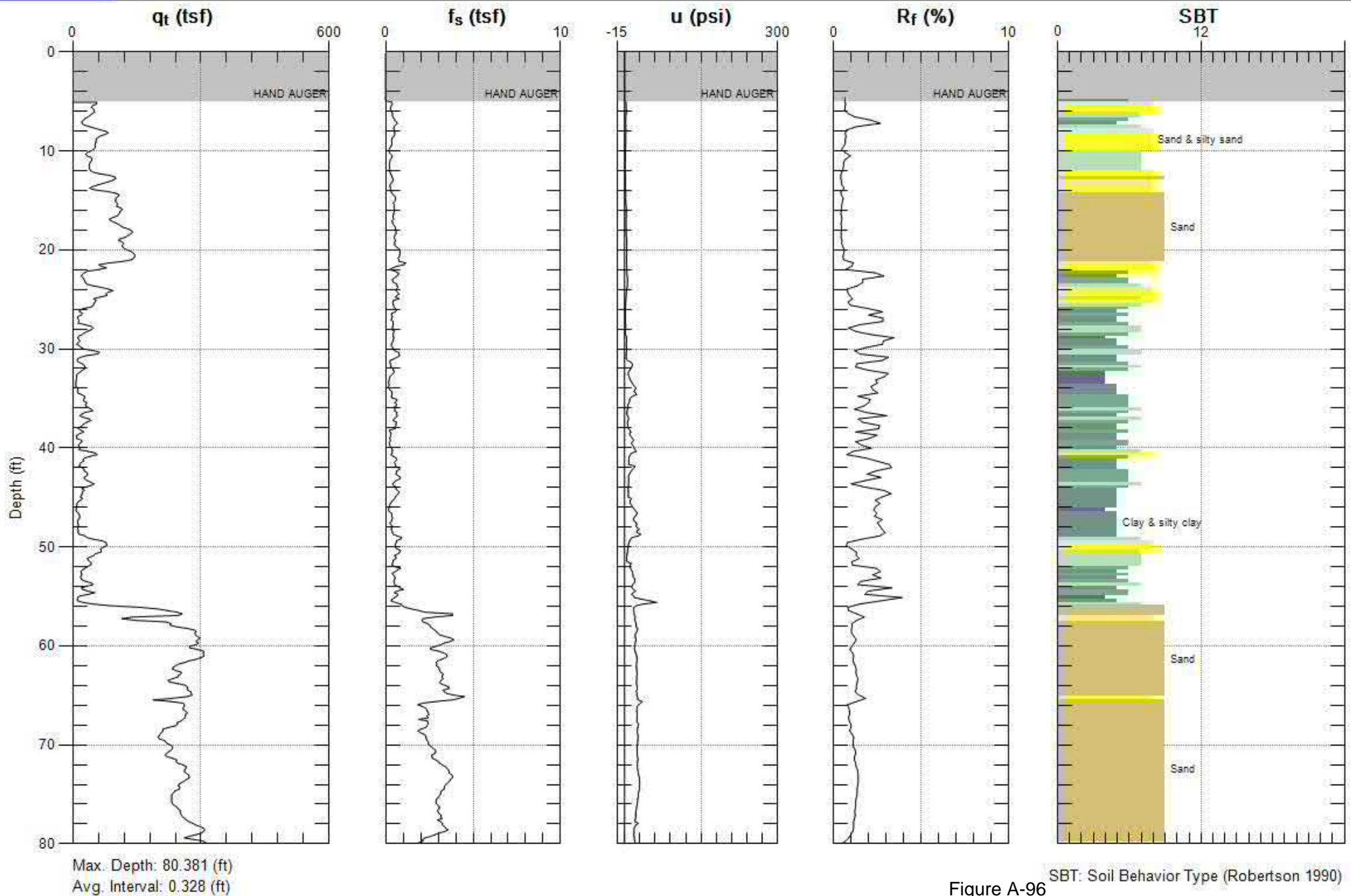


Figure A-96 SBT: Soil Behavior Type (Robertson 1990)

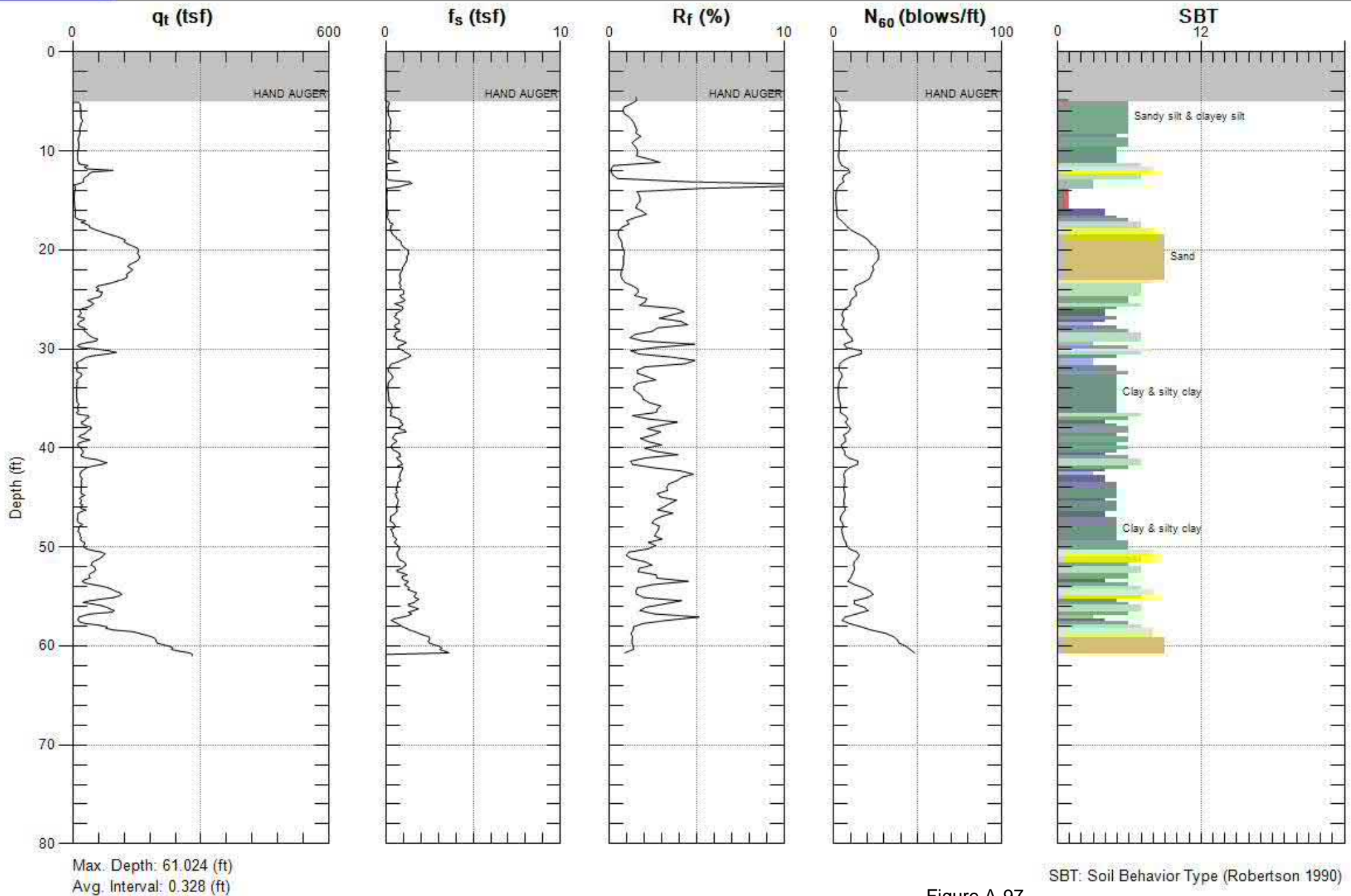
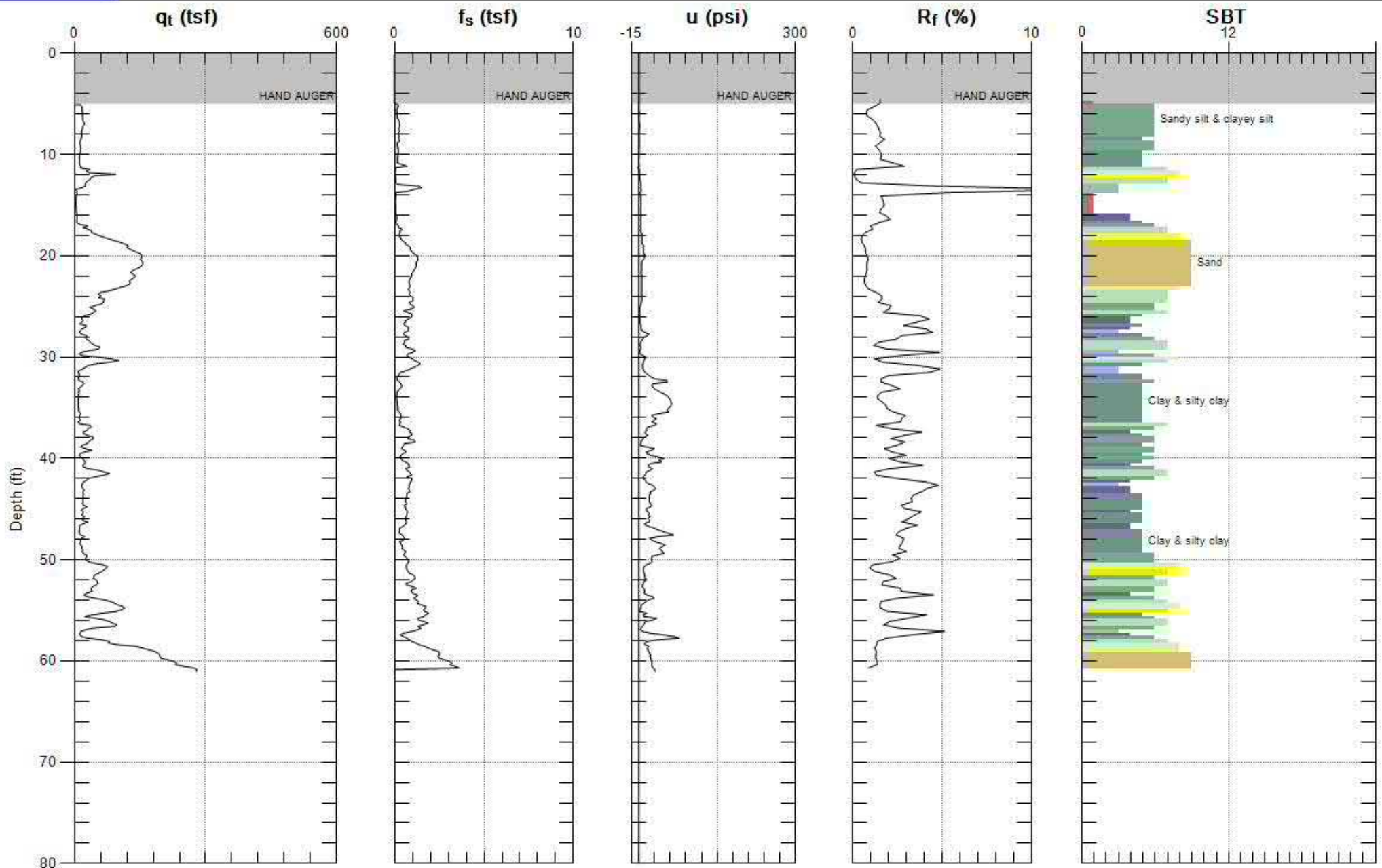


Figure A-97



Max. Depth: 61.024 (ft)
Avg. Interval: 0.328 (ft)

Figure A-97 SBT: Soil Behavior Type (Robertson 1990)

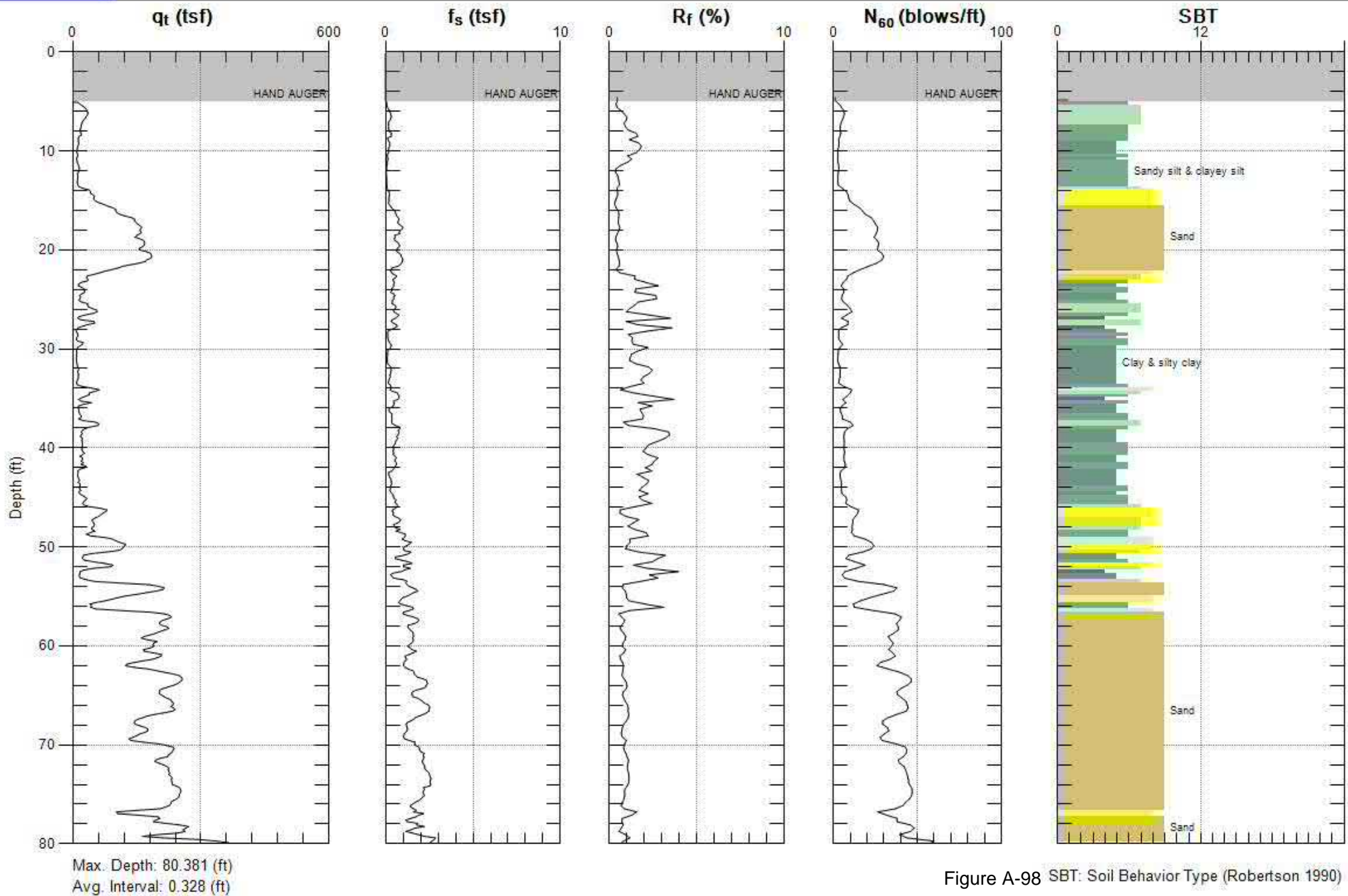


Figure A-98 SBT: Soil Behavior Type (Robertson 1990)

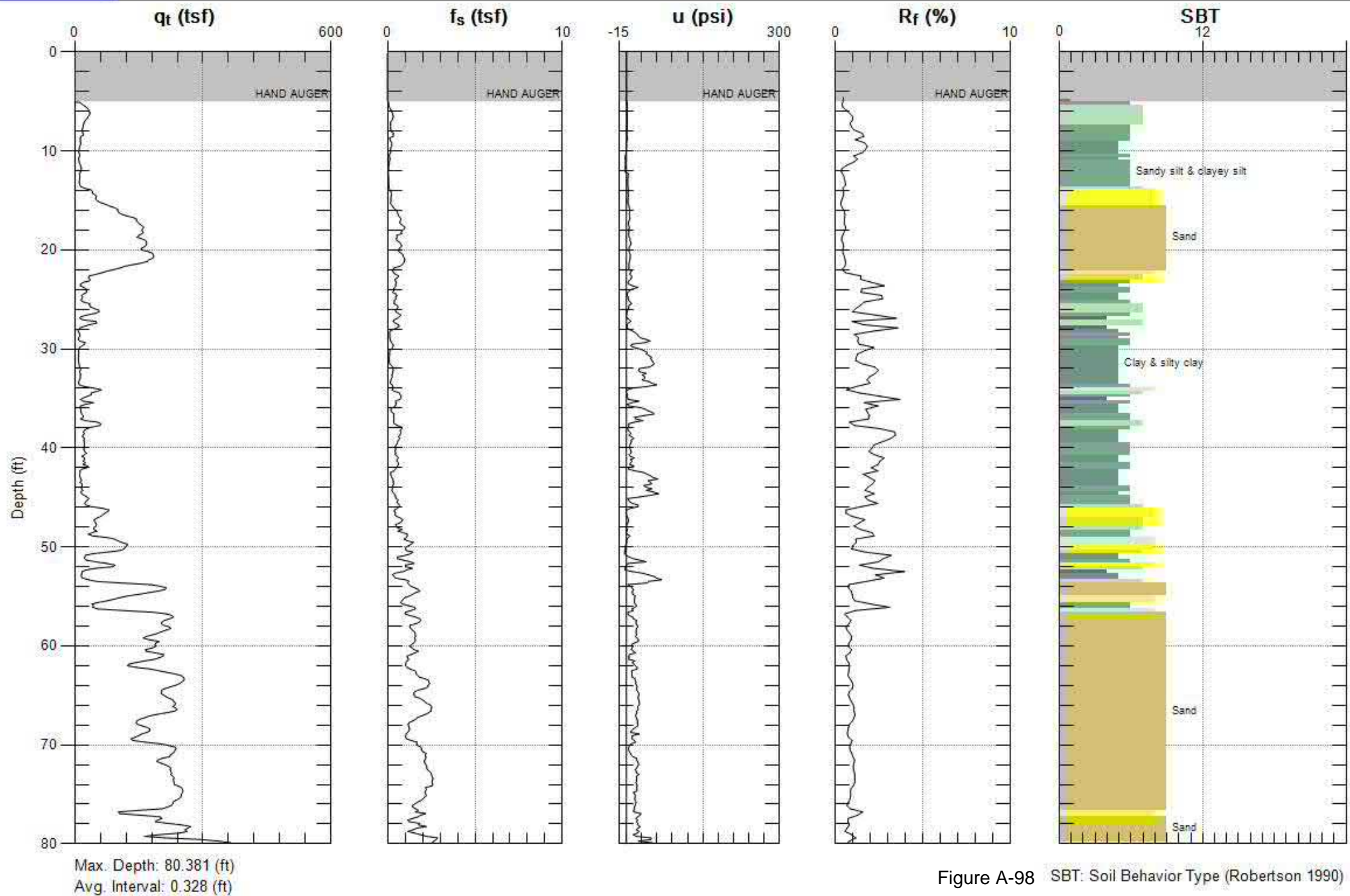
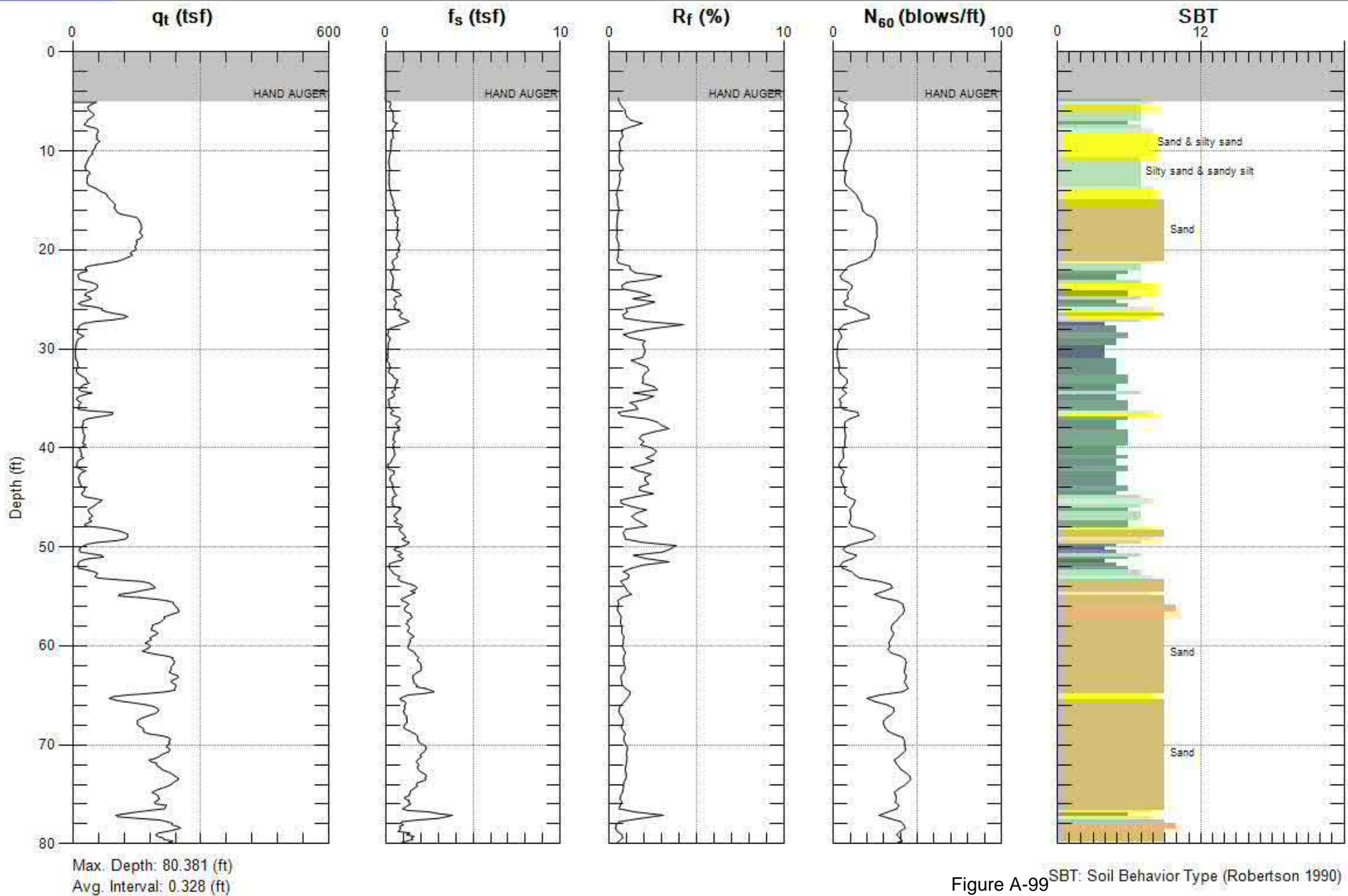


Figure A-98 SBT: Soil Behavior Type (Robertson 1990)



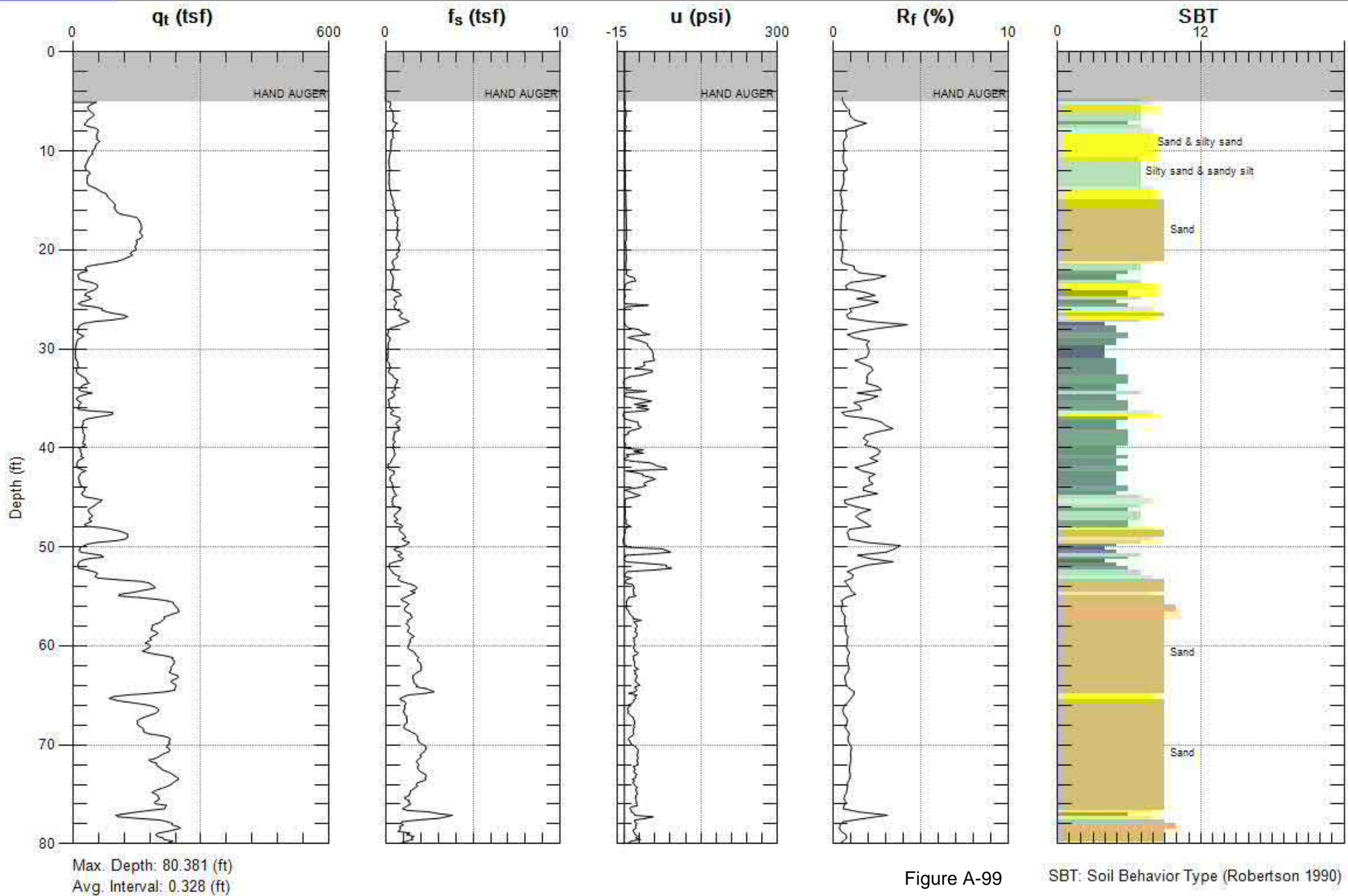


Figure A-99

SBT: Soil Behavior Type (Robertson 1990)

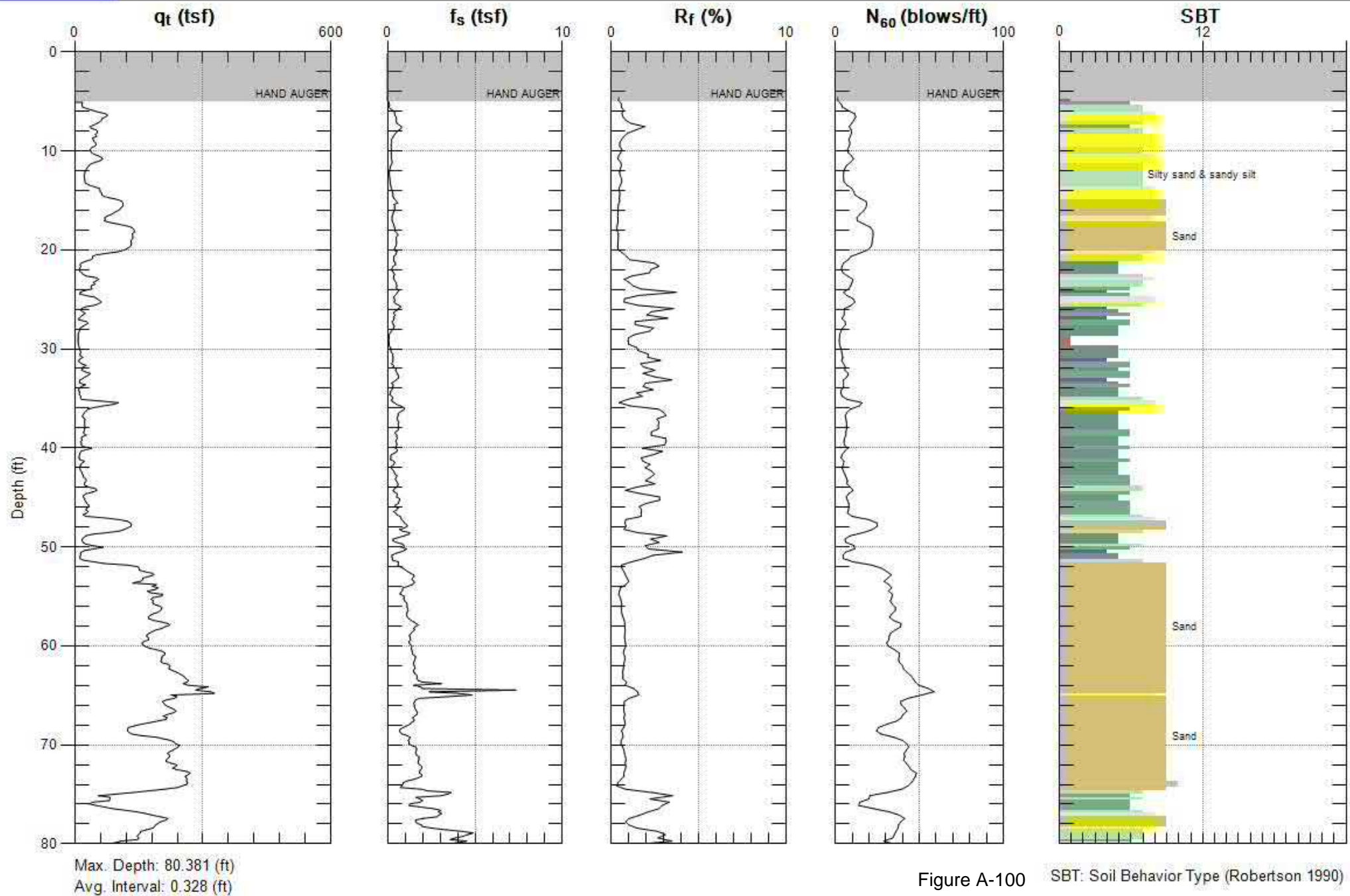


Figure A-100

SBT: Soil Behavior Type (Robertson 1990)

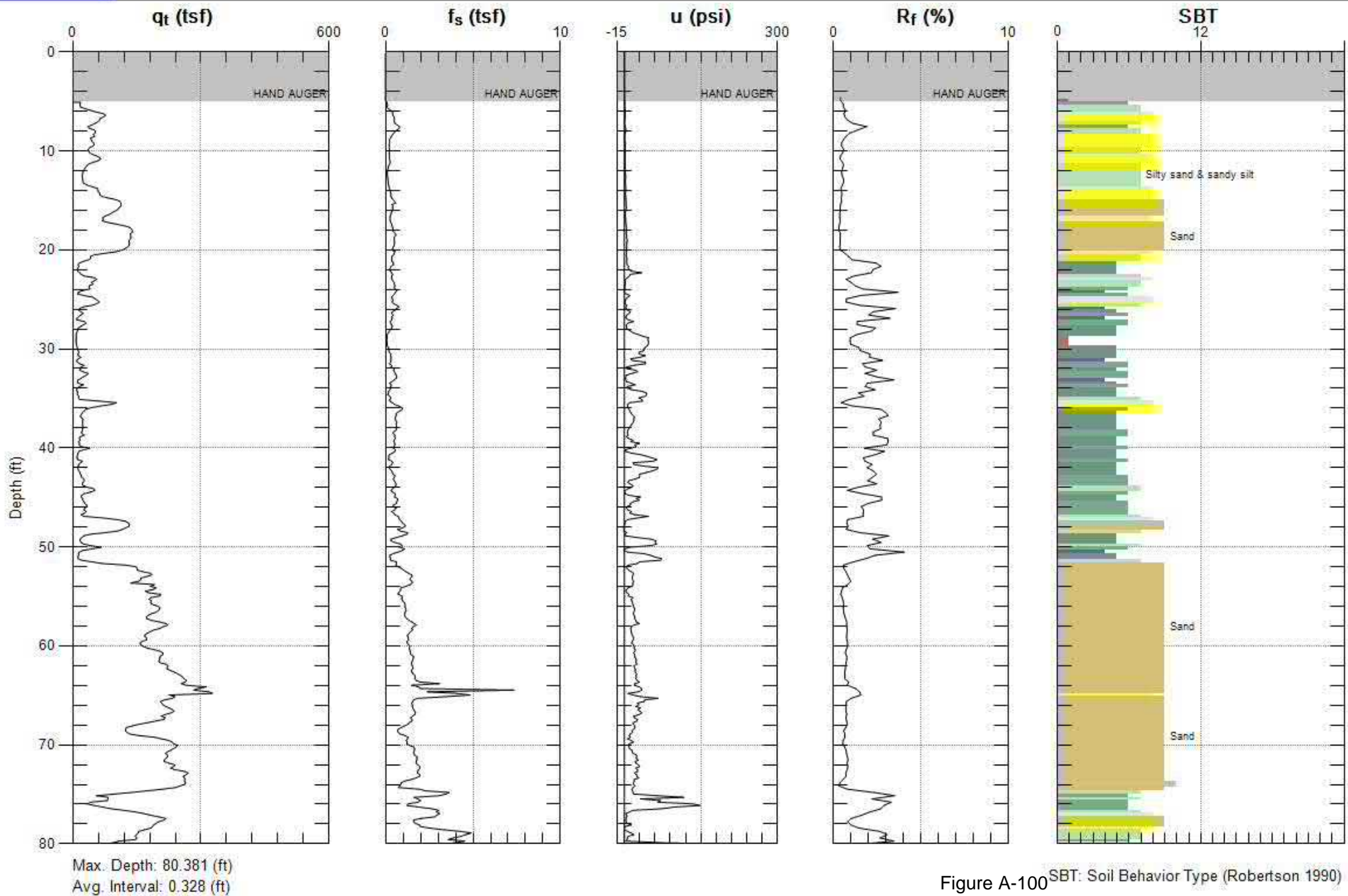


Figure A-100 SBT: Soil Behavior Type (Robertson 1990)

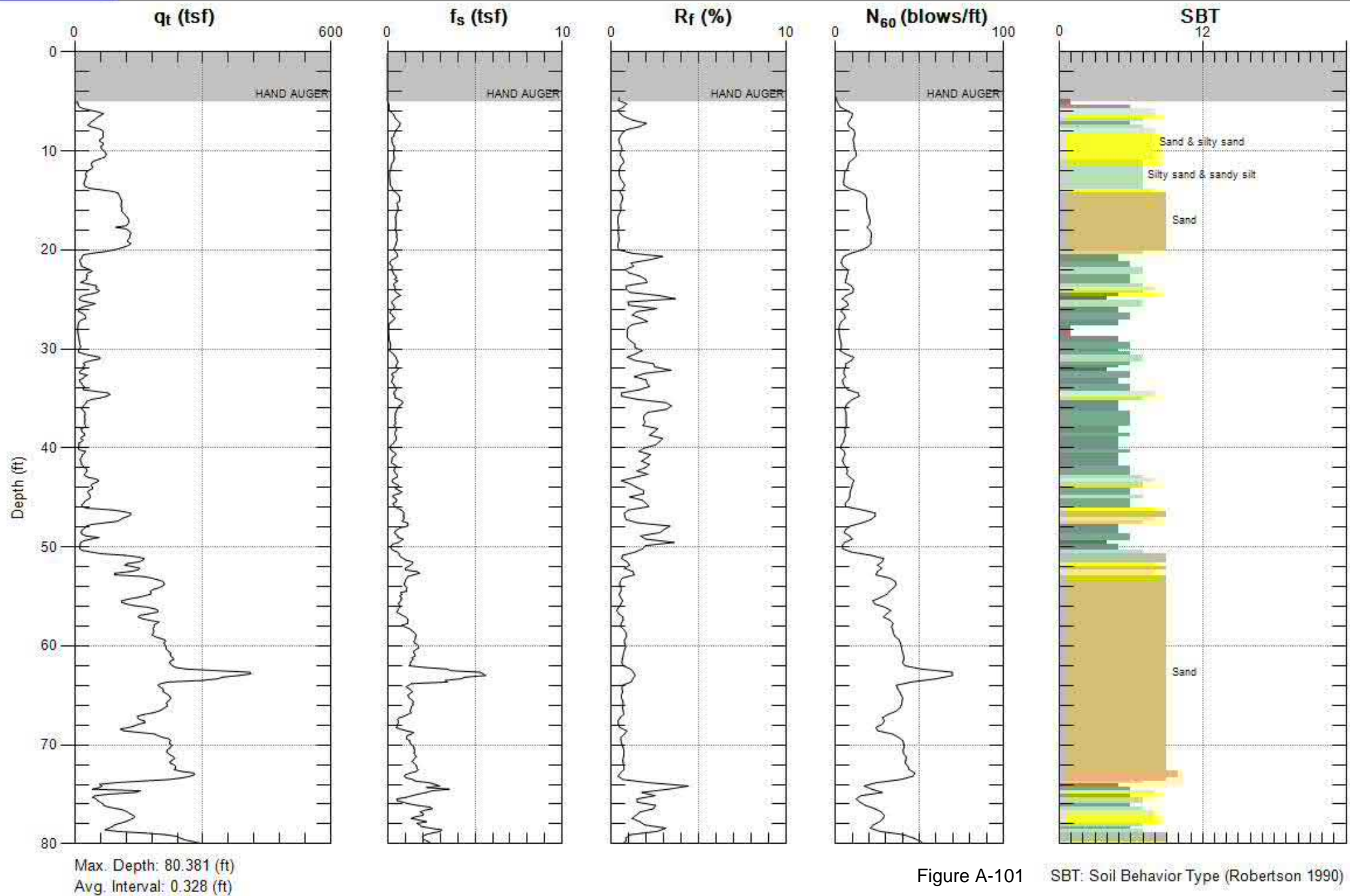


Figure A-101

SBT: Soil Behavior Type (Robertson 1990)

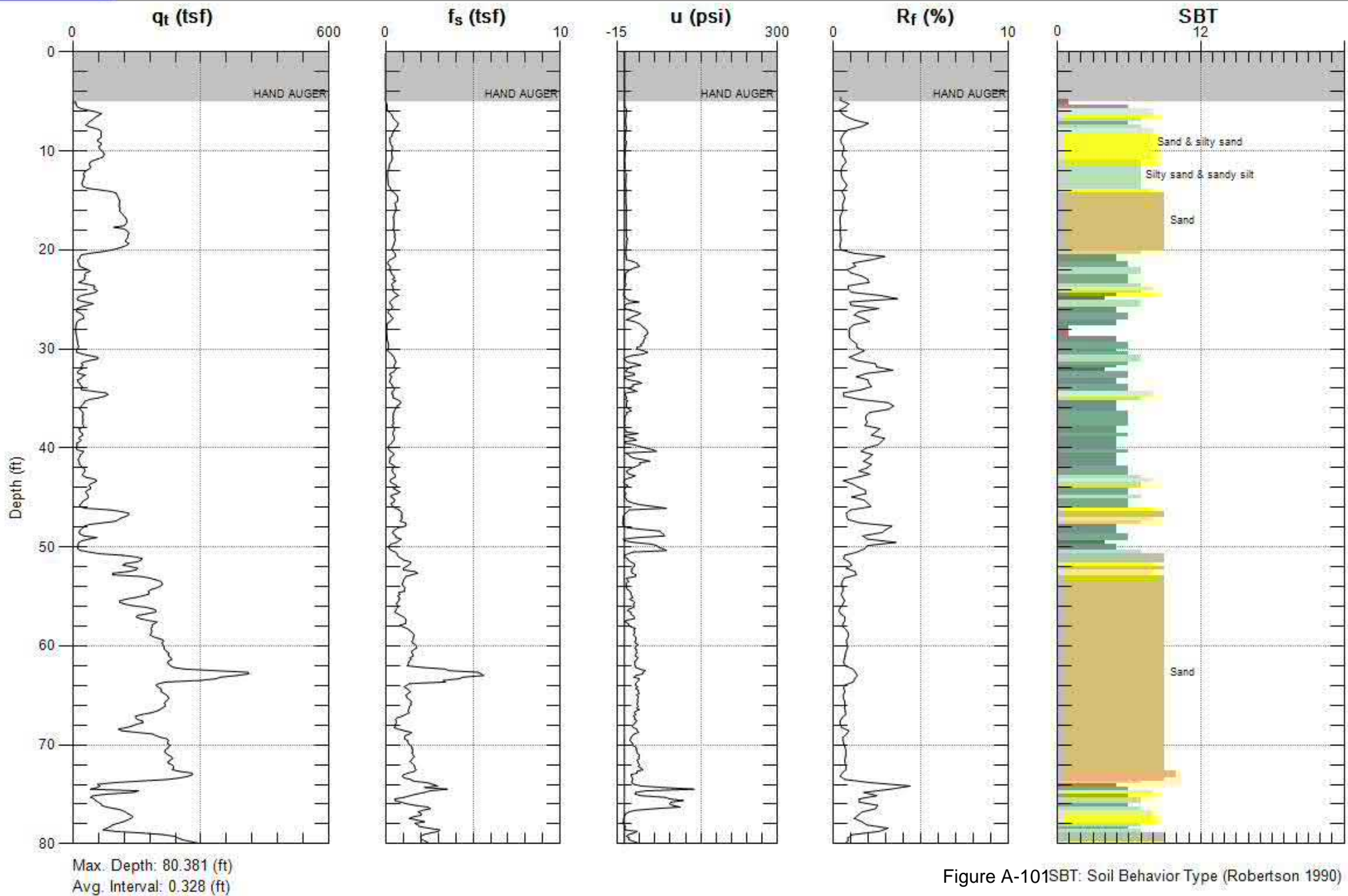
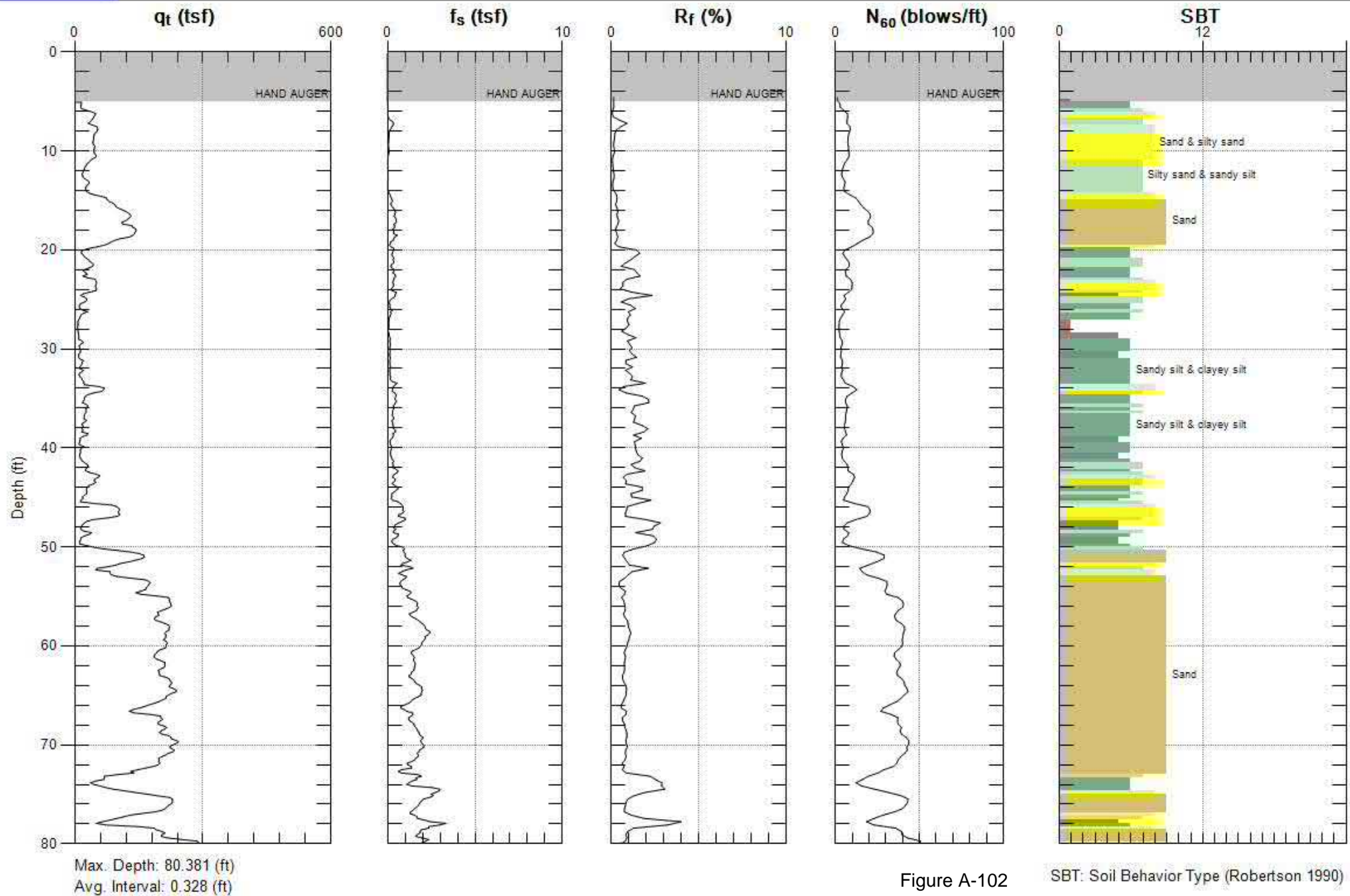


Figure A-101SBT: Soil Behavior Type (Robertson 1990)



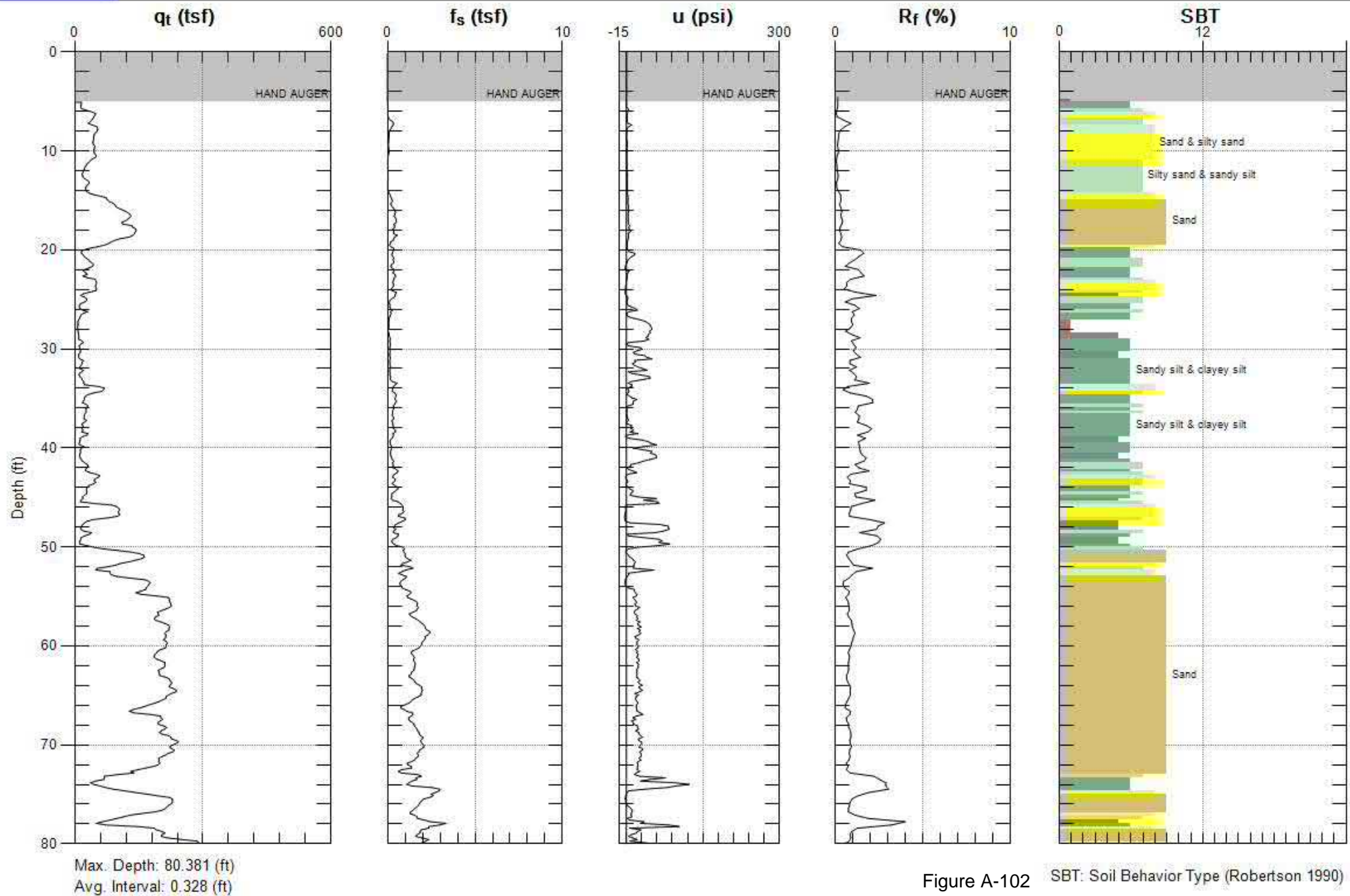
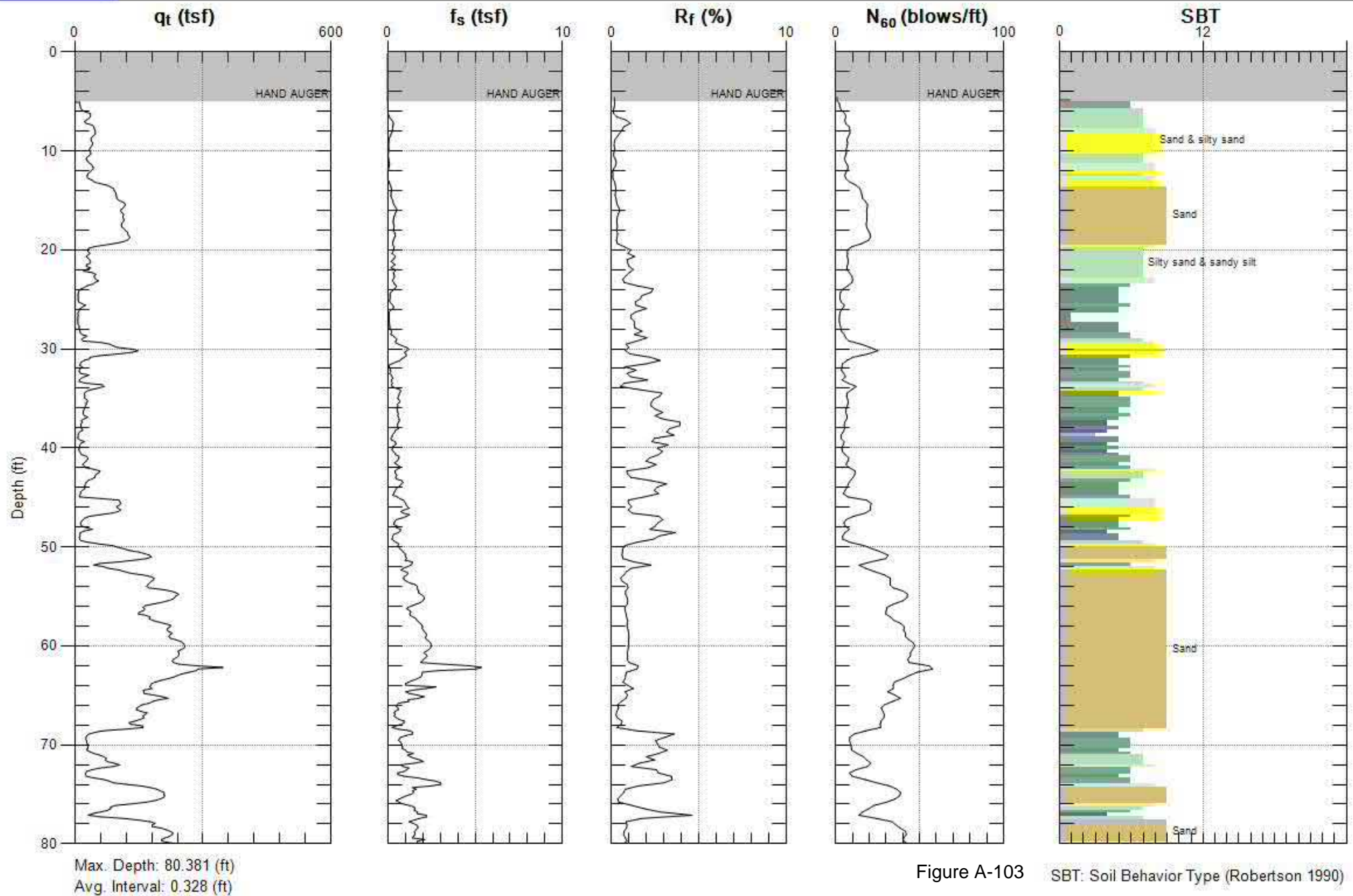


Figure A-102 SBT: Soil Behavior Type (Robertson 1990)



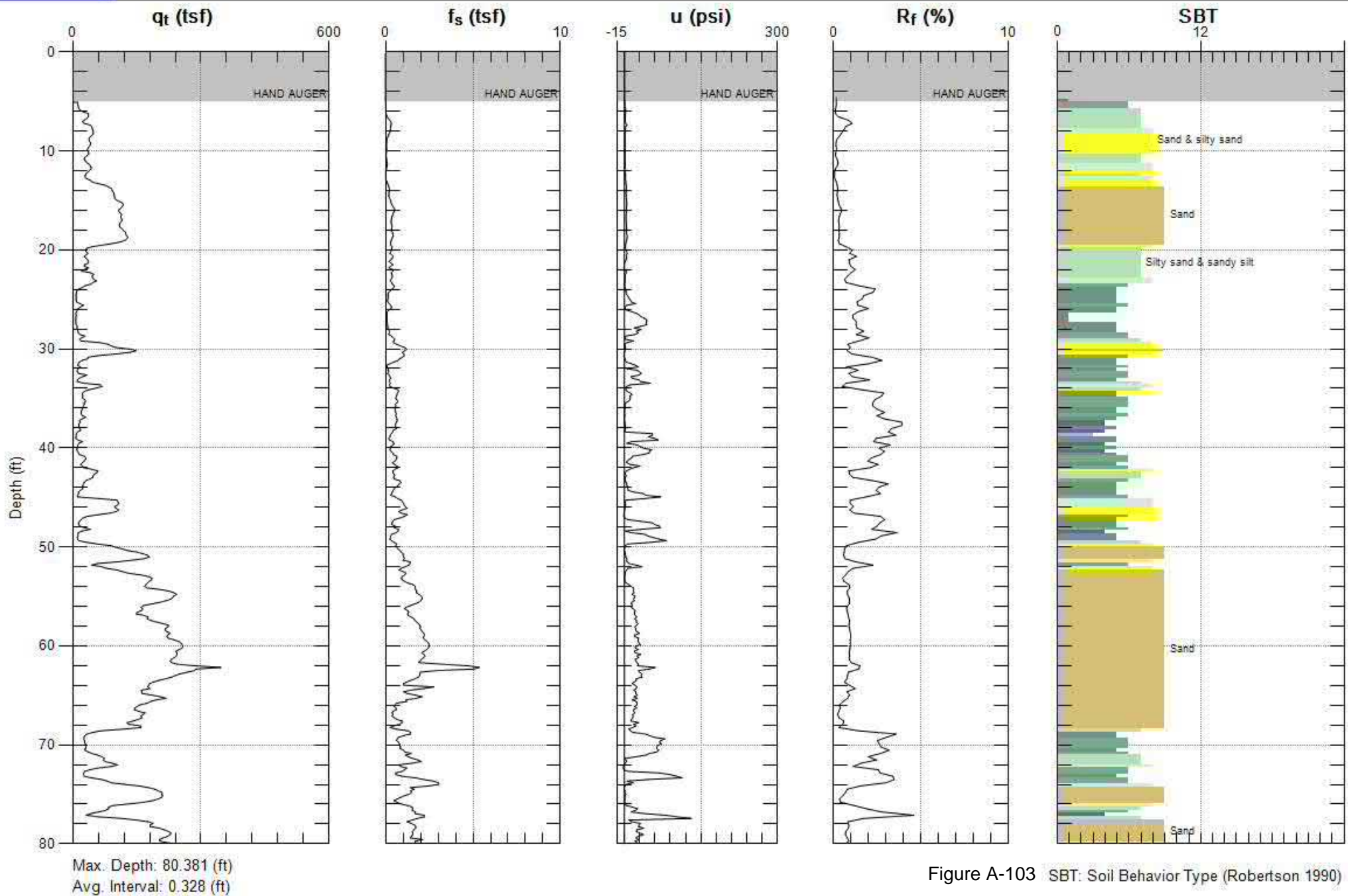


Figure A-103 SBT: Soil Behavior Type (Robertson 1990)

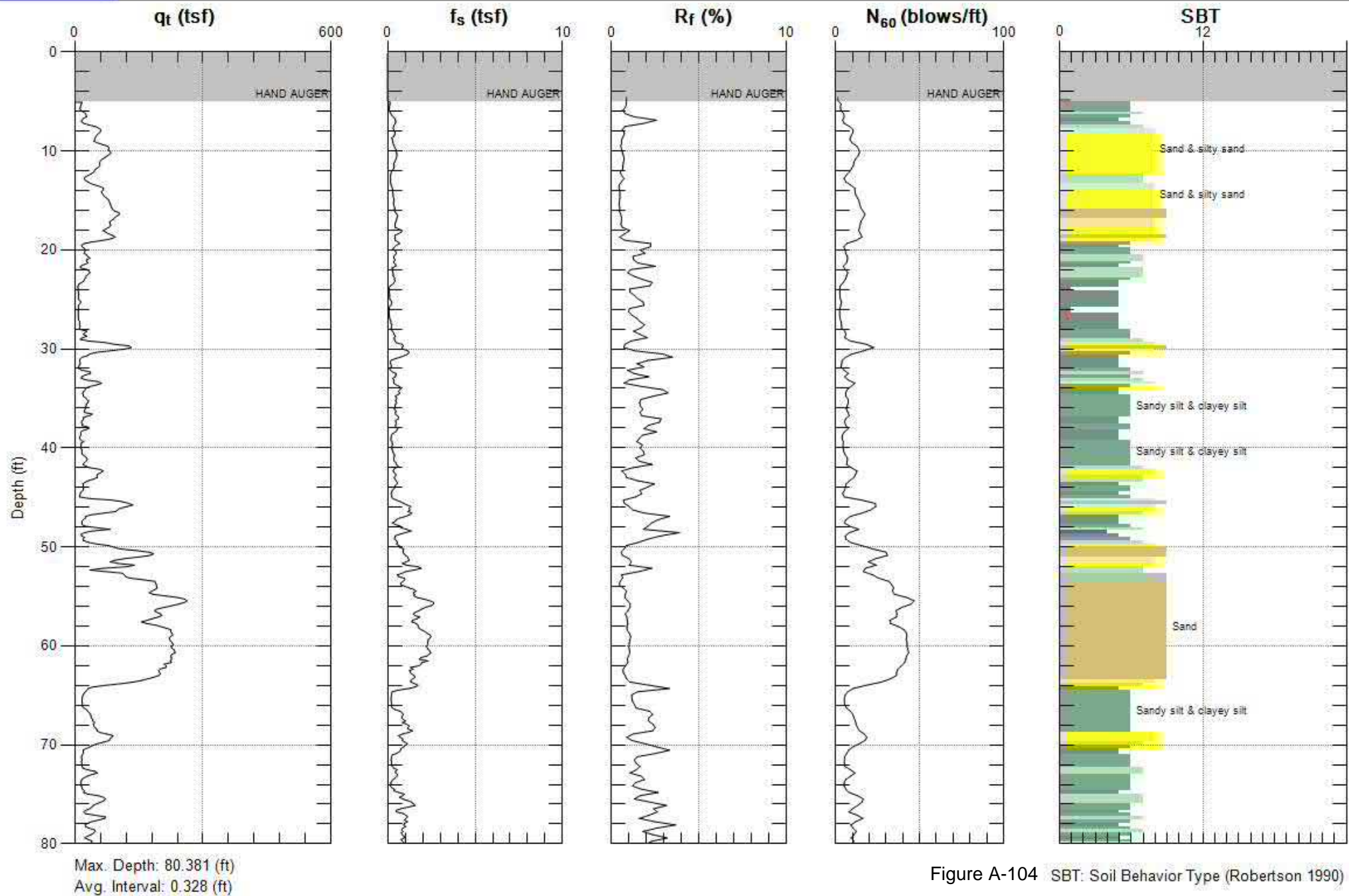


Figure A-104 SBT: Soil Behavior Type (Robertson 1990)

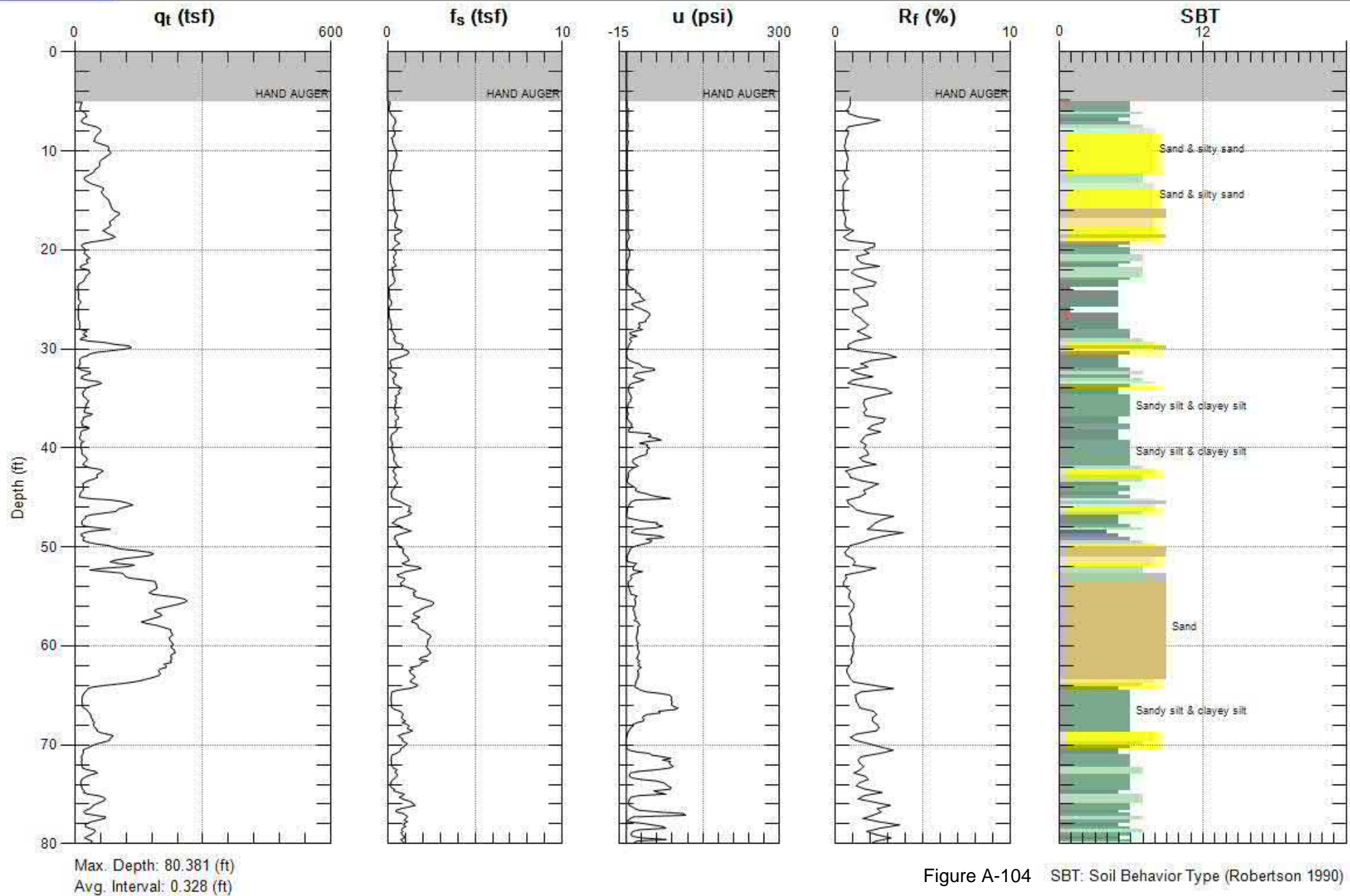


Figure A-104 SBT: Soil Behavior Type (Robertson 1990)

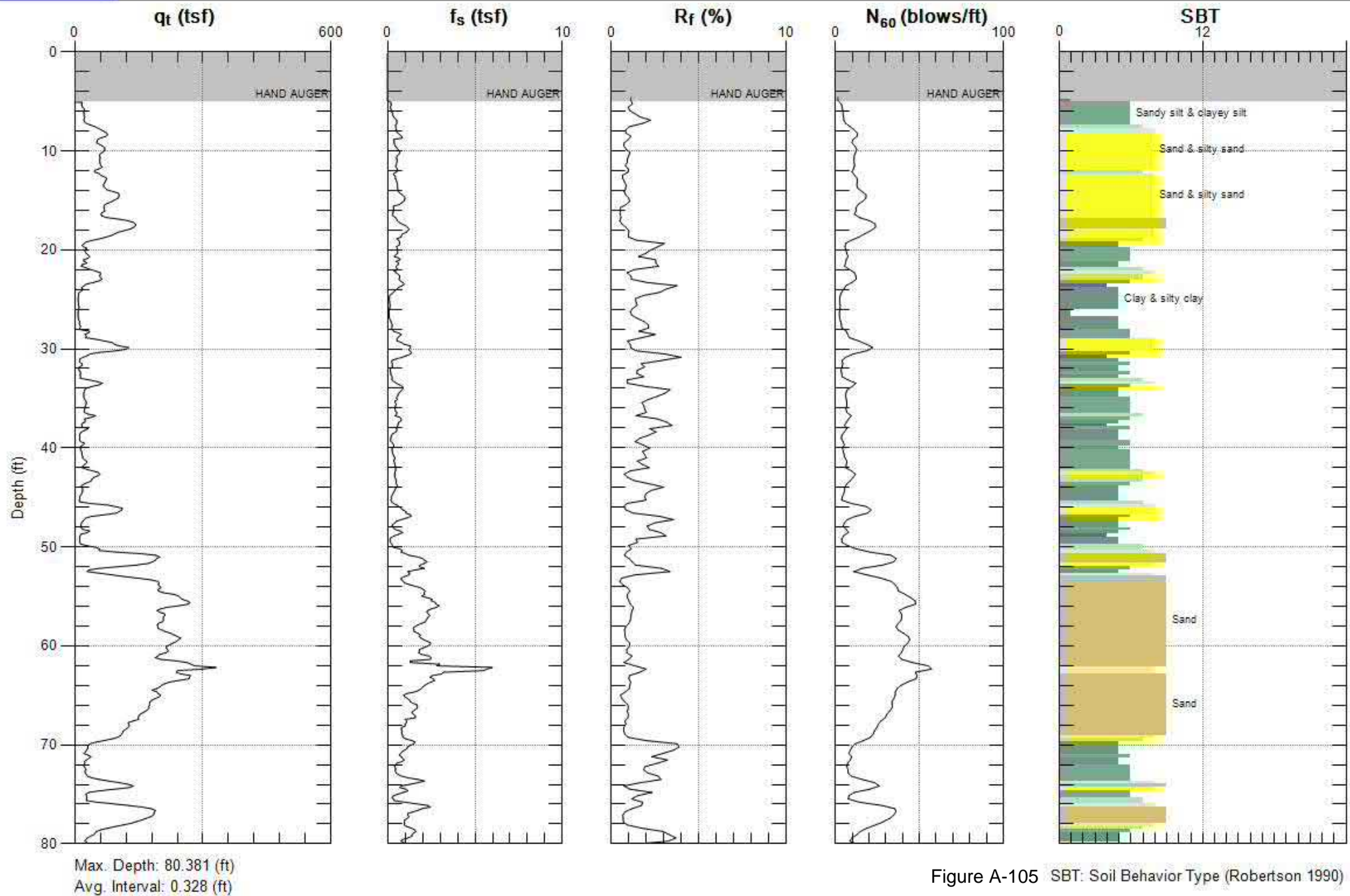


Figure A-105 SBT: Soil Behavior Type (Robertson 1990)

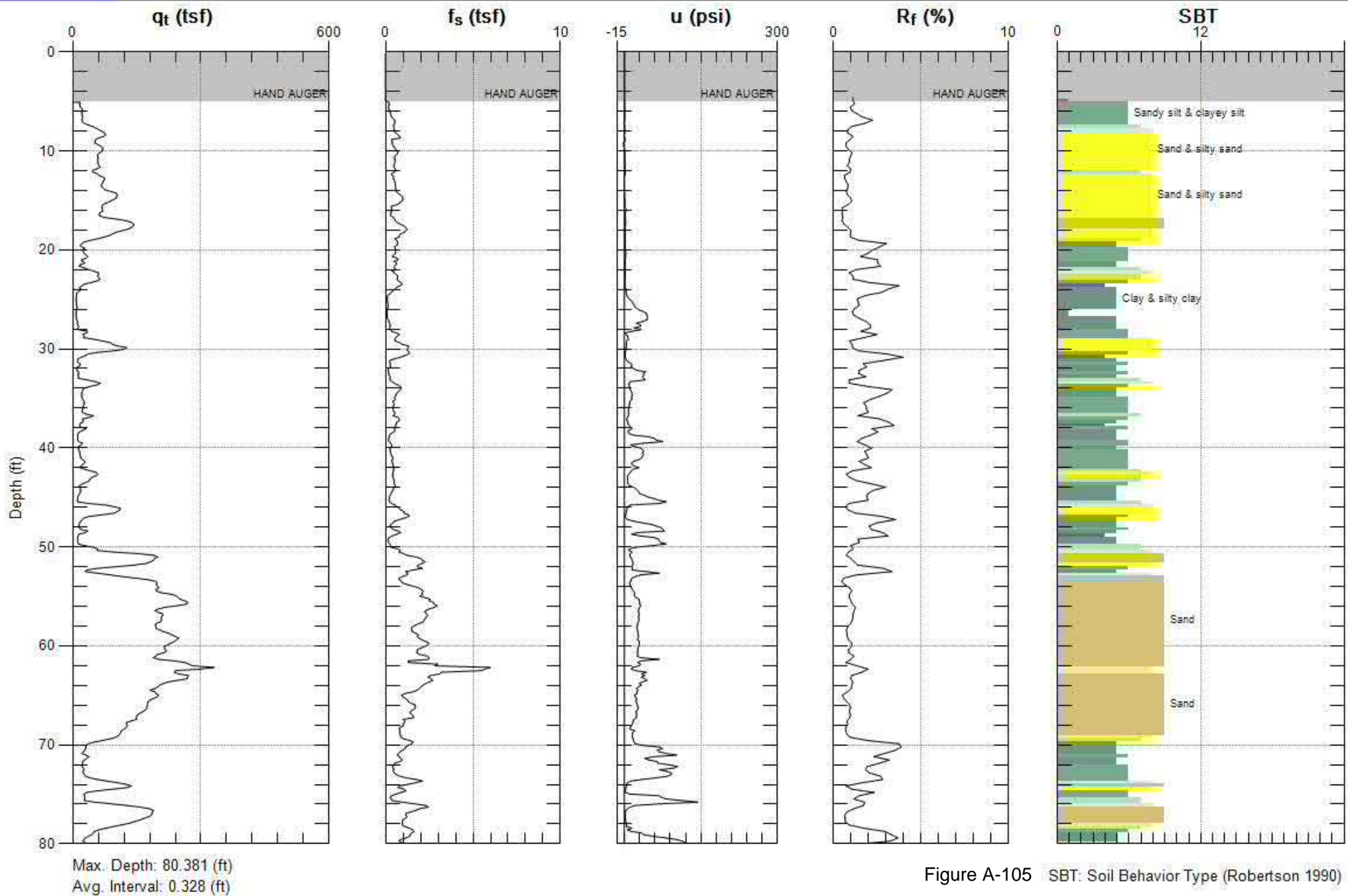
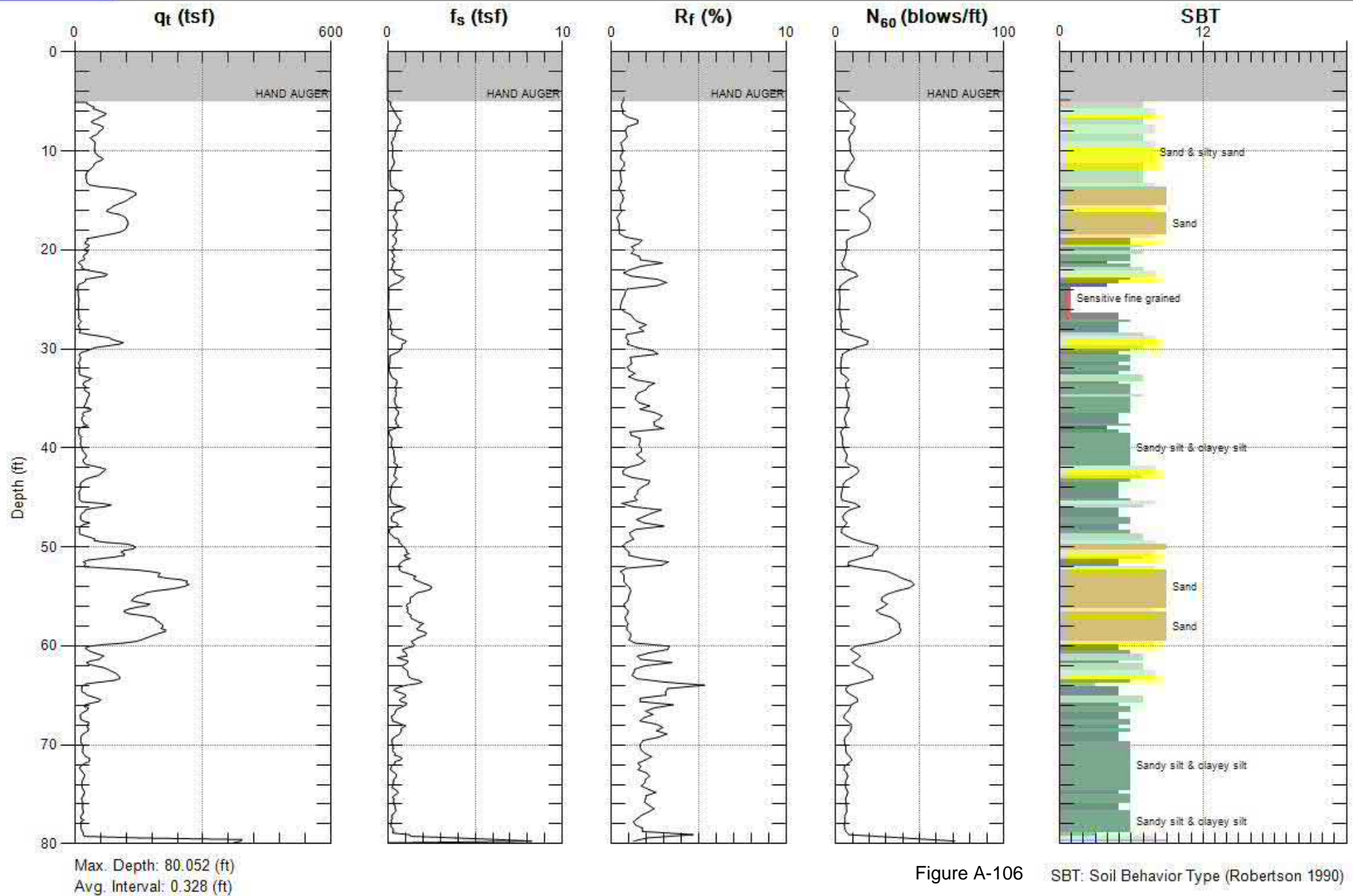


Figure A-105 SBT: Soil Behavior Type (Robertson 1990)



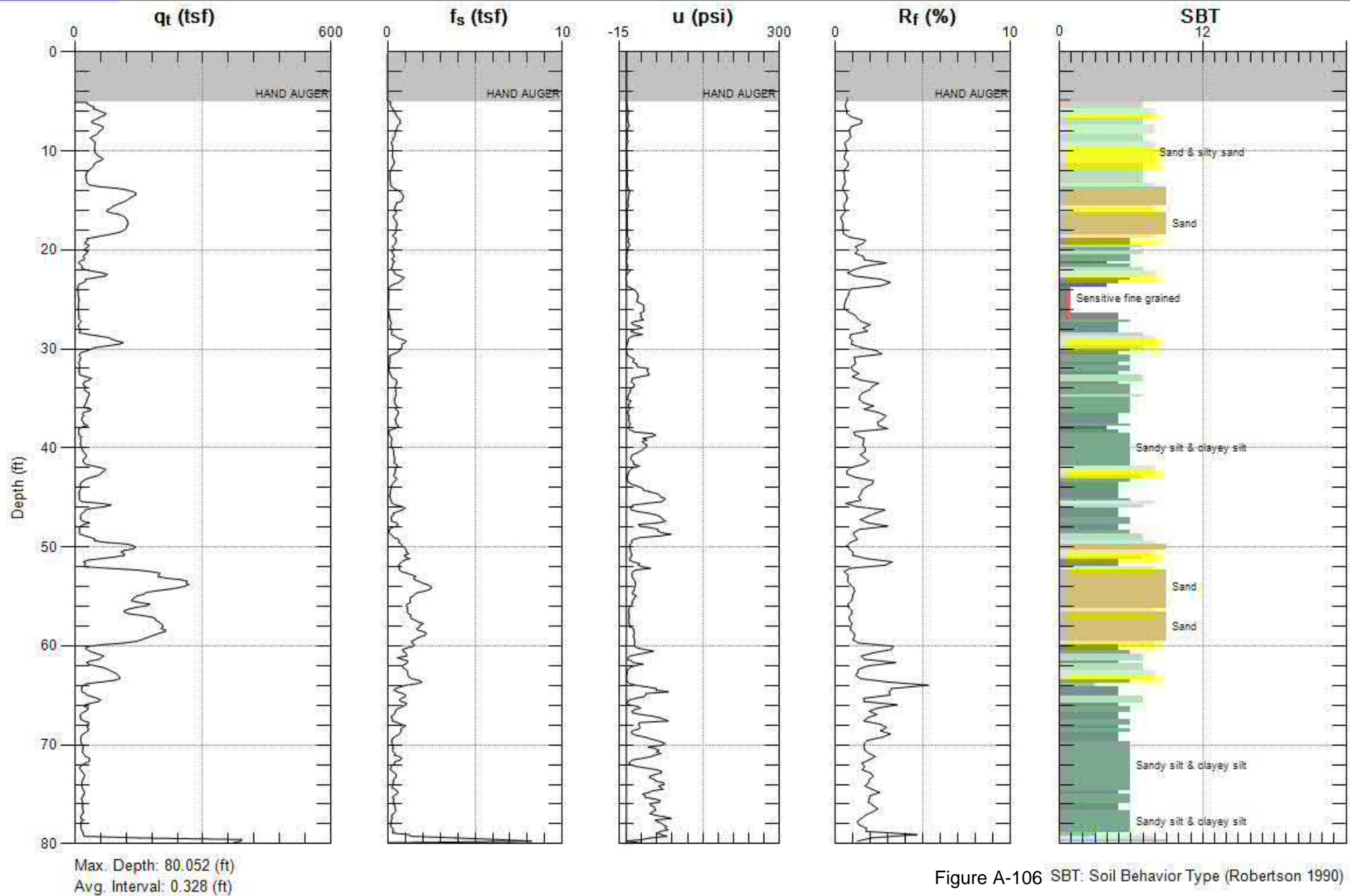


Figure A-106 SBT: Soil Behavior Type (Robertson 1990)

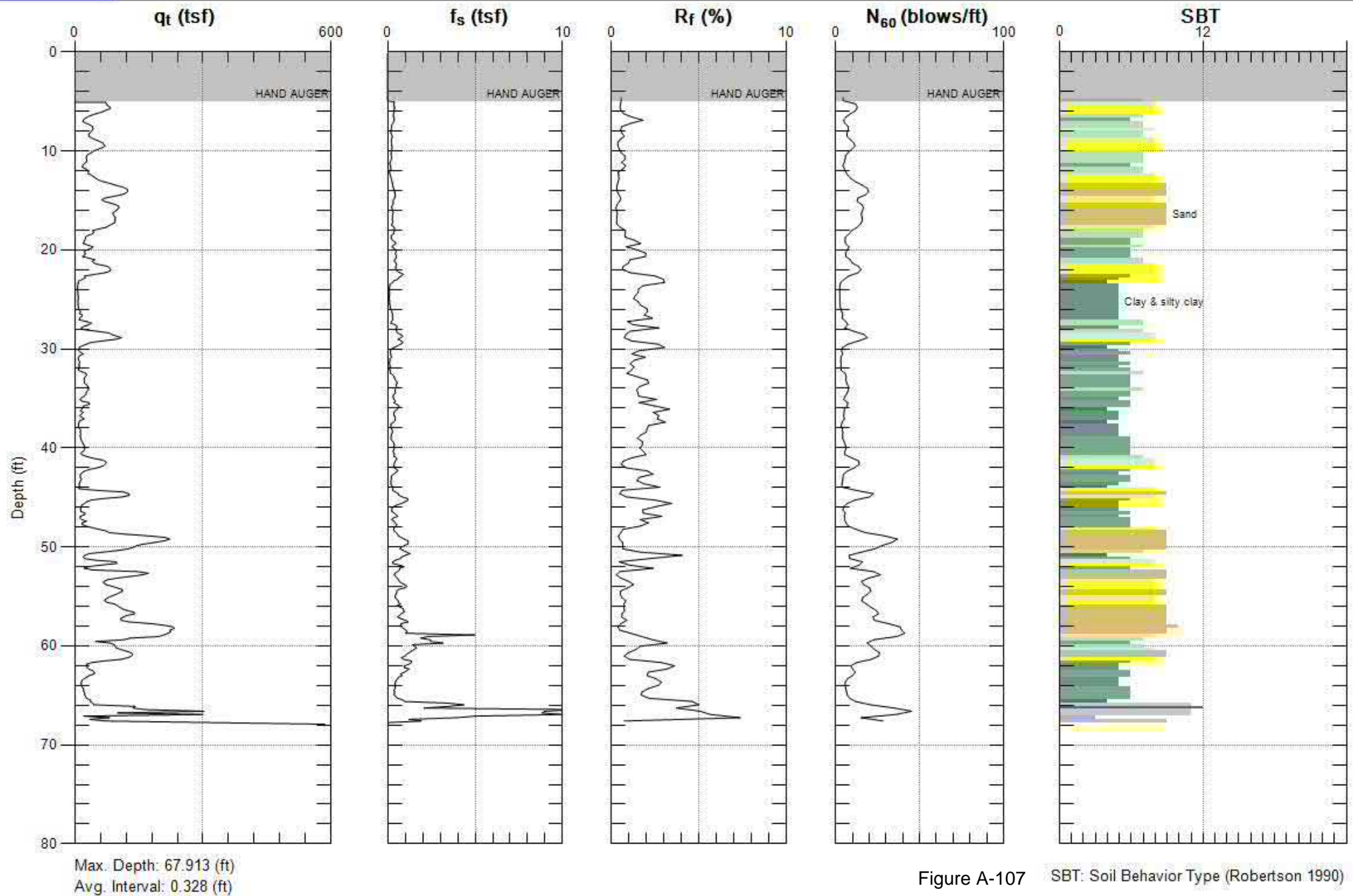


Figure A-107 SBT: Soil Behavior Type (Robertson 1990)

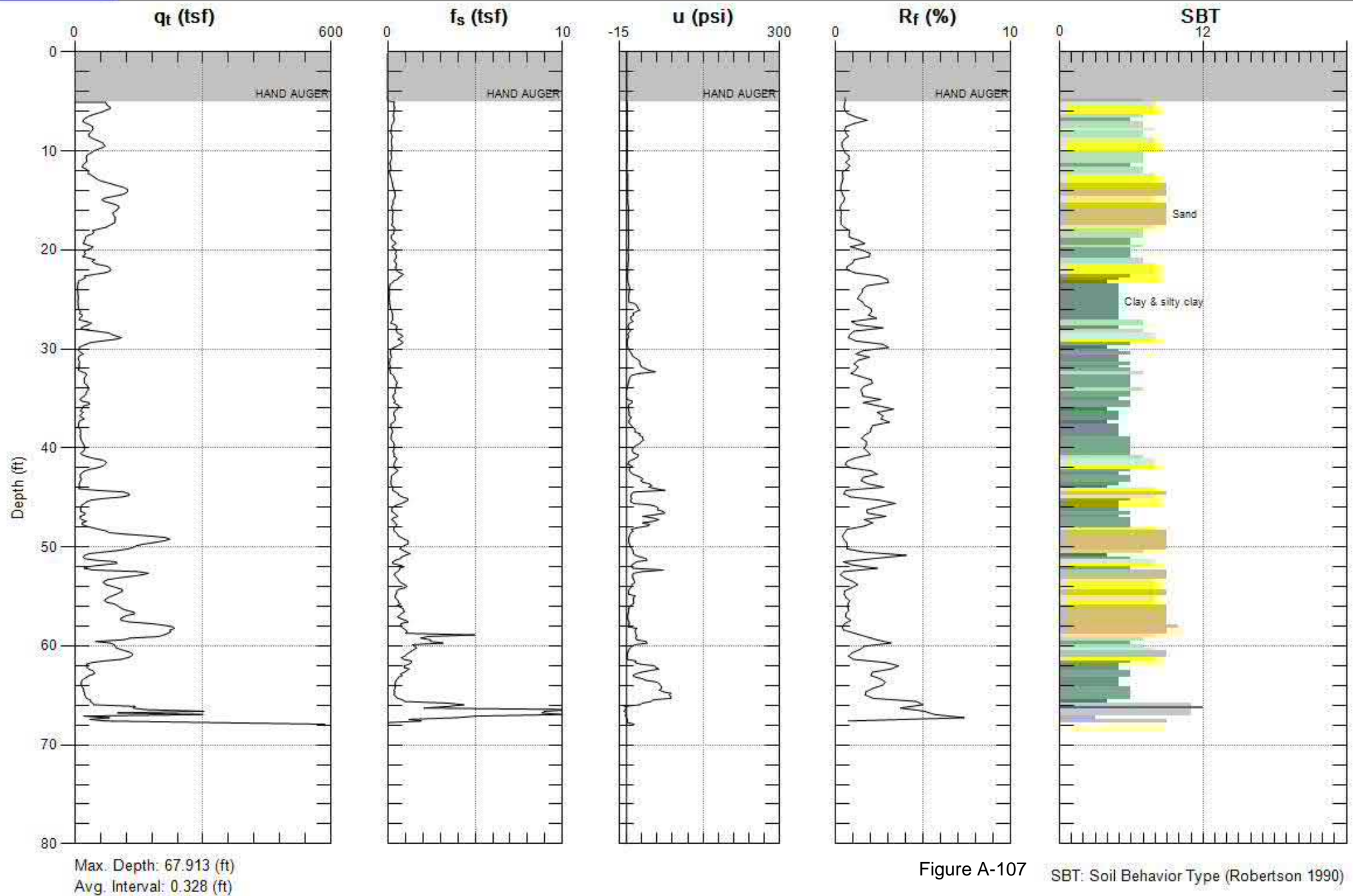


Figure A-107

SBT: Soil Behavior Type (Robertson 1990)

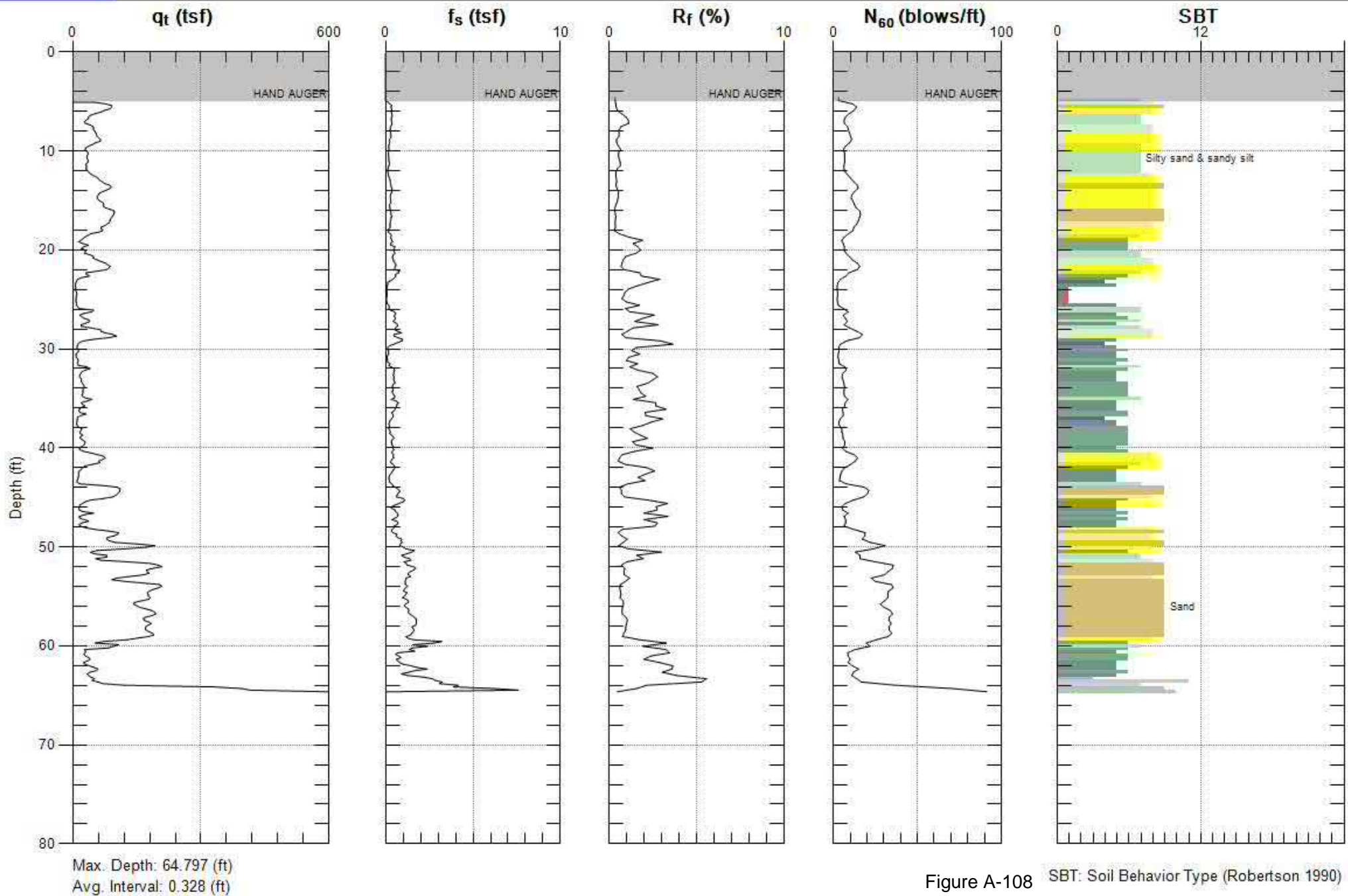


Figure A-108 SBT: Soil Behavior Type (Robertson 1990)

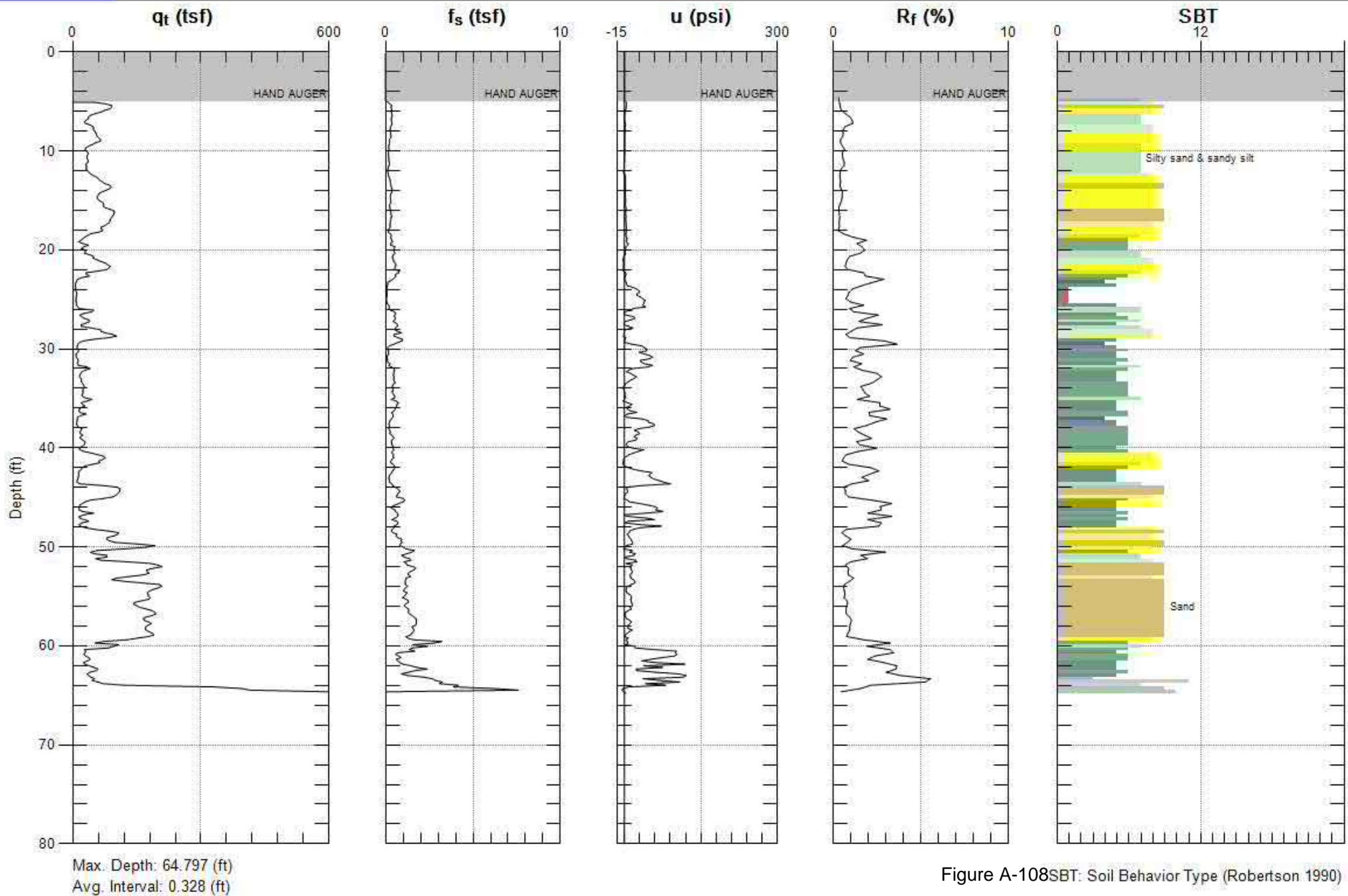


Figure A-108SBT: Soil Behavior Type (Robertson 1990)

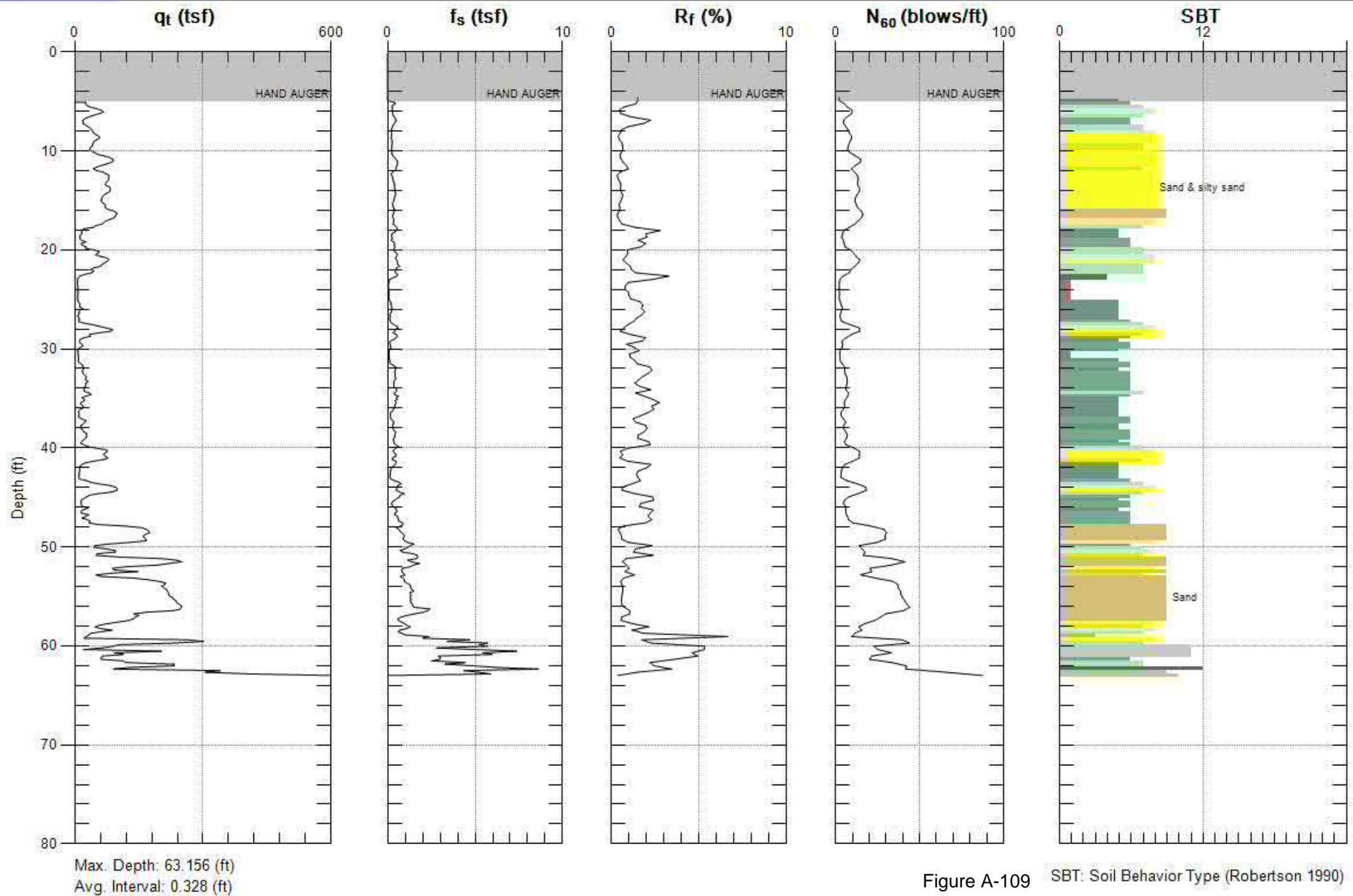


Figure A-109 SBT: Soil Behavior Type (Robertson 1990)

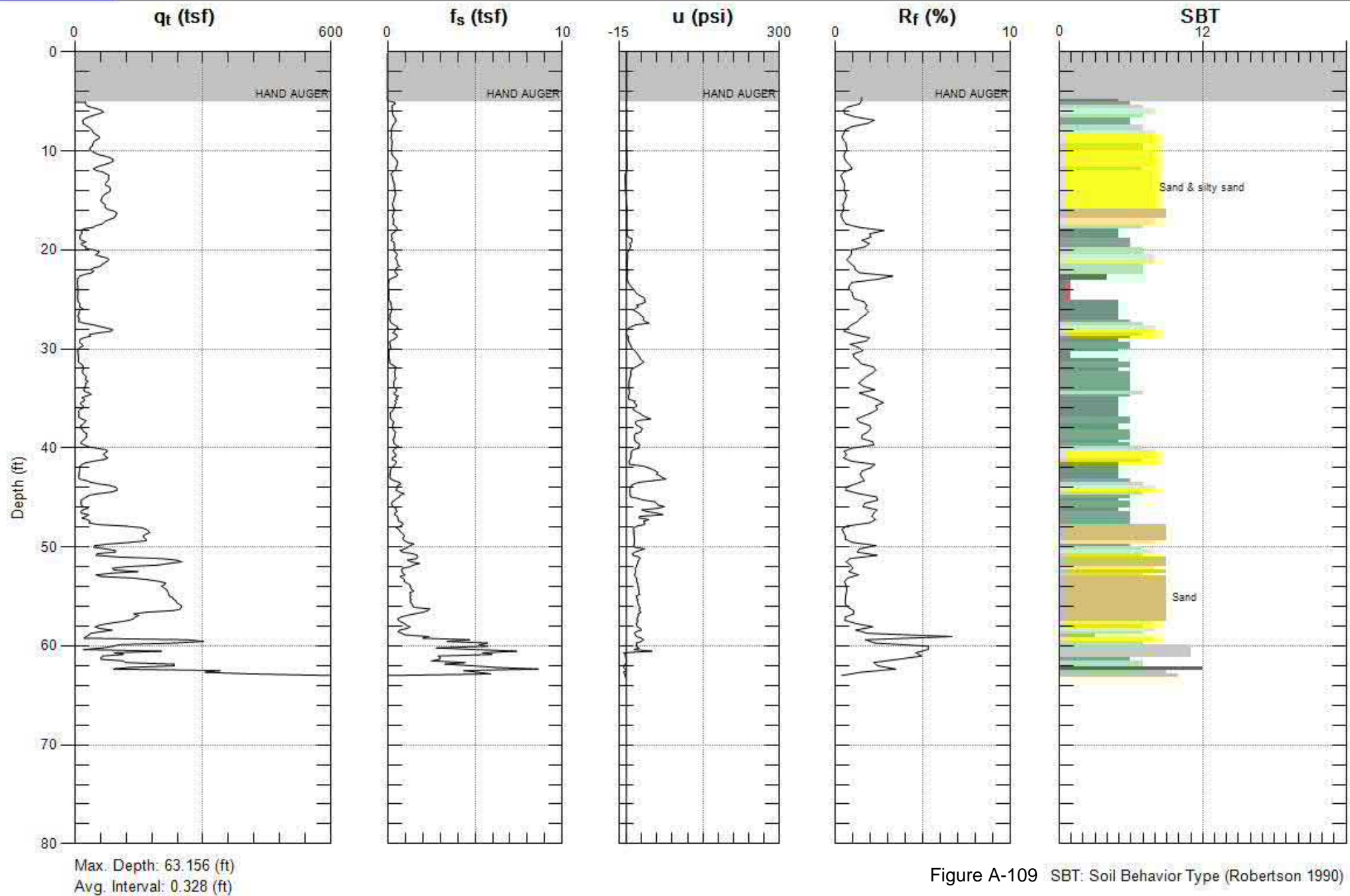


Figure A-109 SBT: Soil Behavior Type (Robertson 1990)

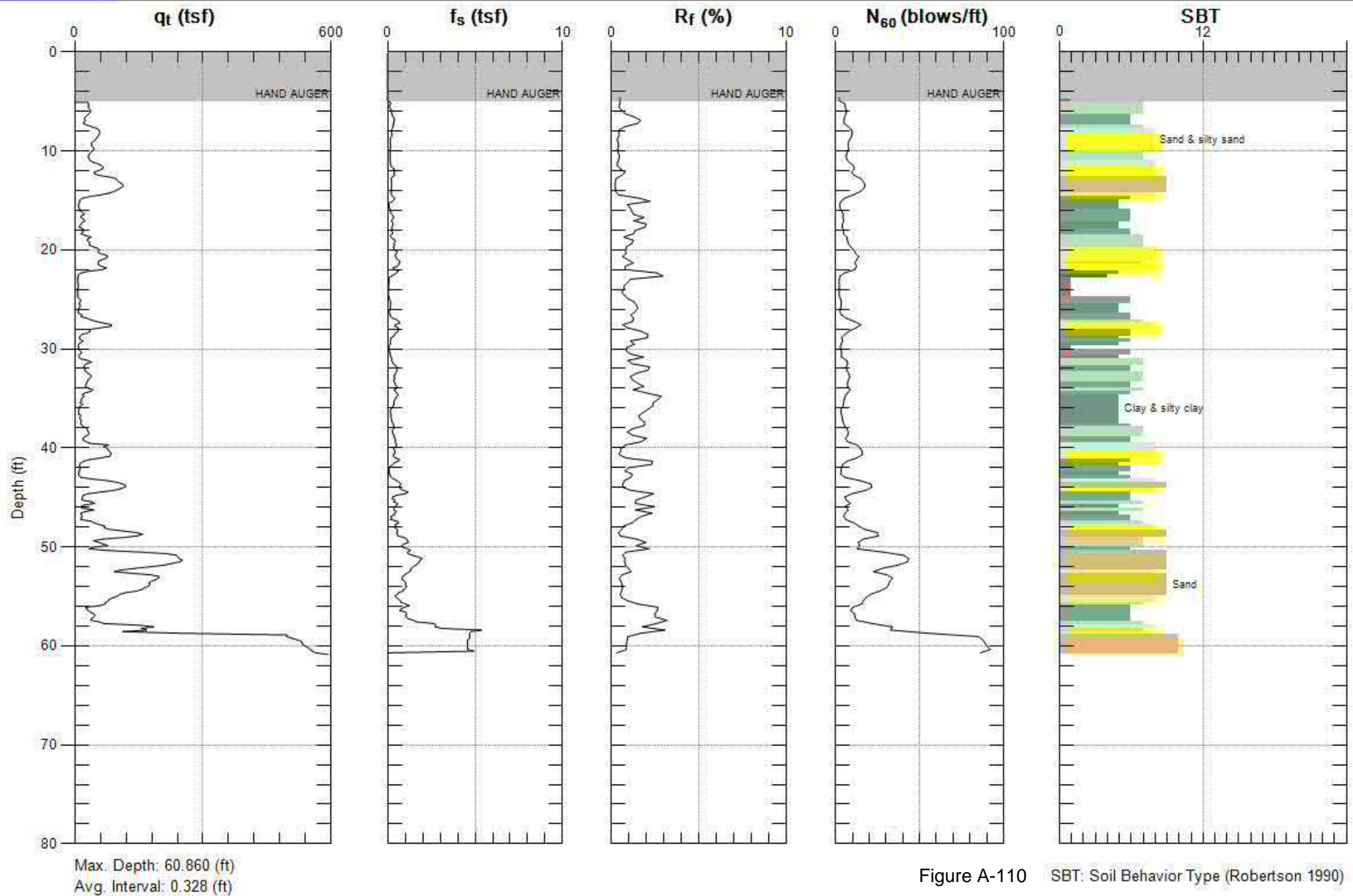


Figure A-110 SBT: Soil Behavior Type (Robertson 1990)

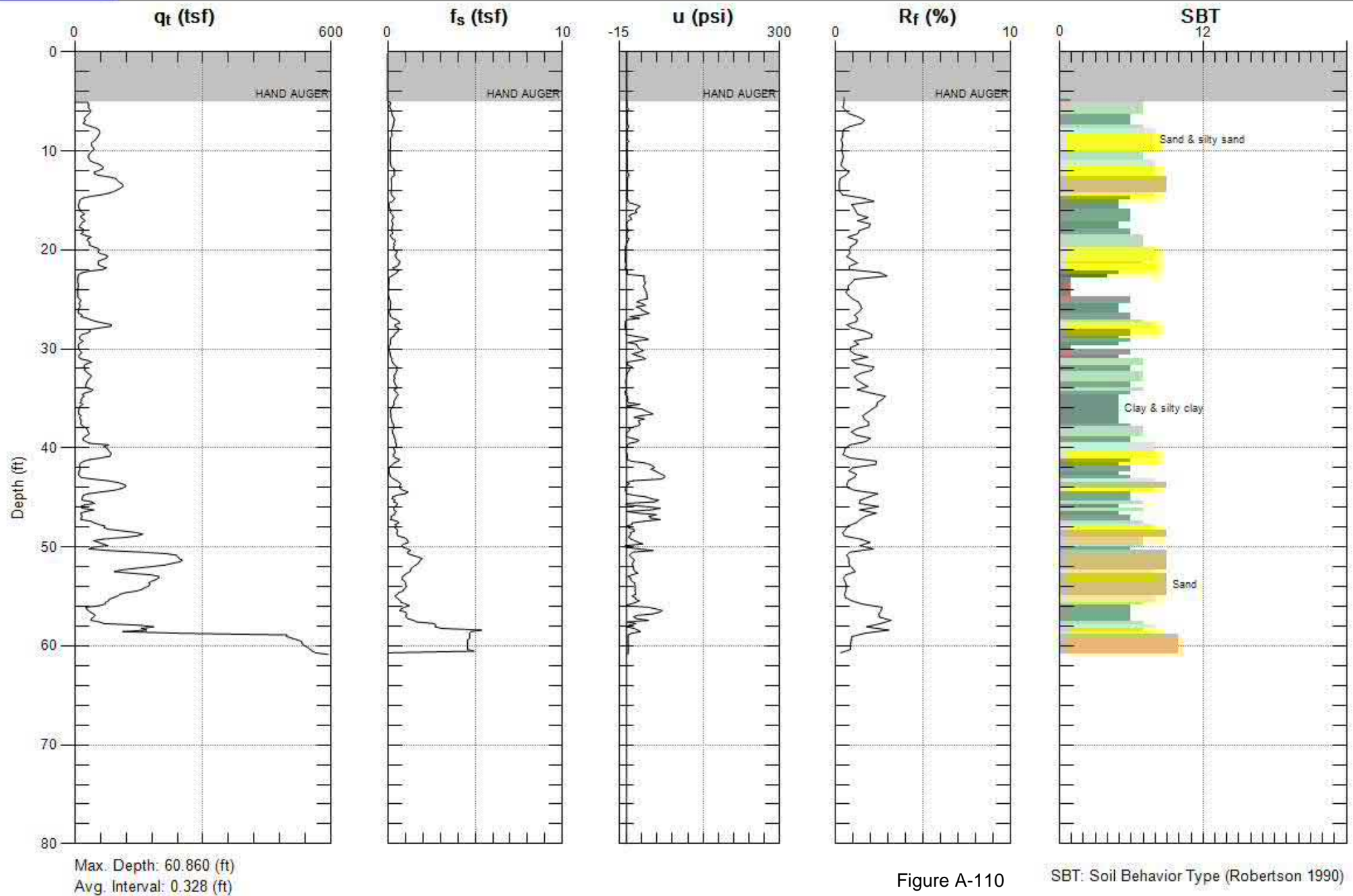


Figure A-110

SBT: Soil Behavior Type (Robertson 1990)

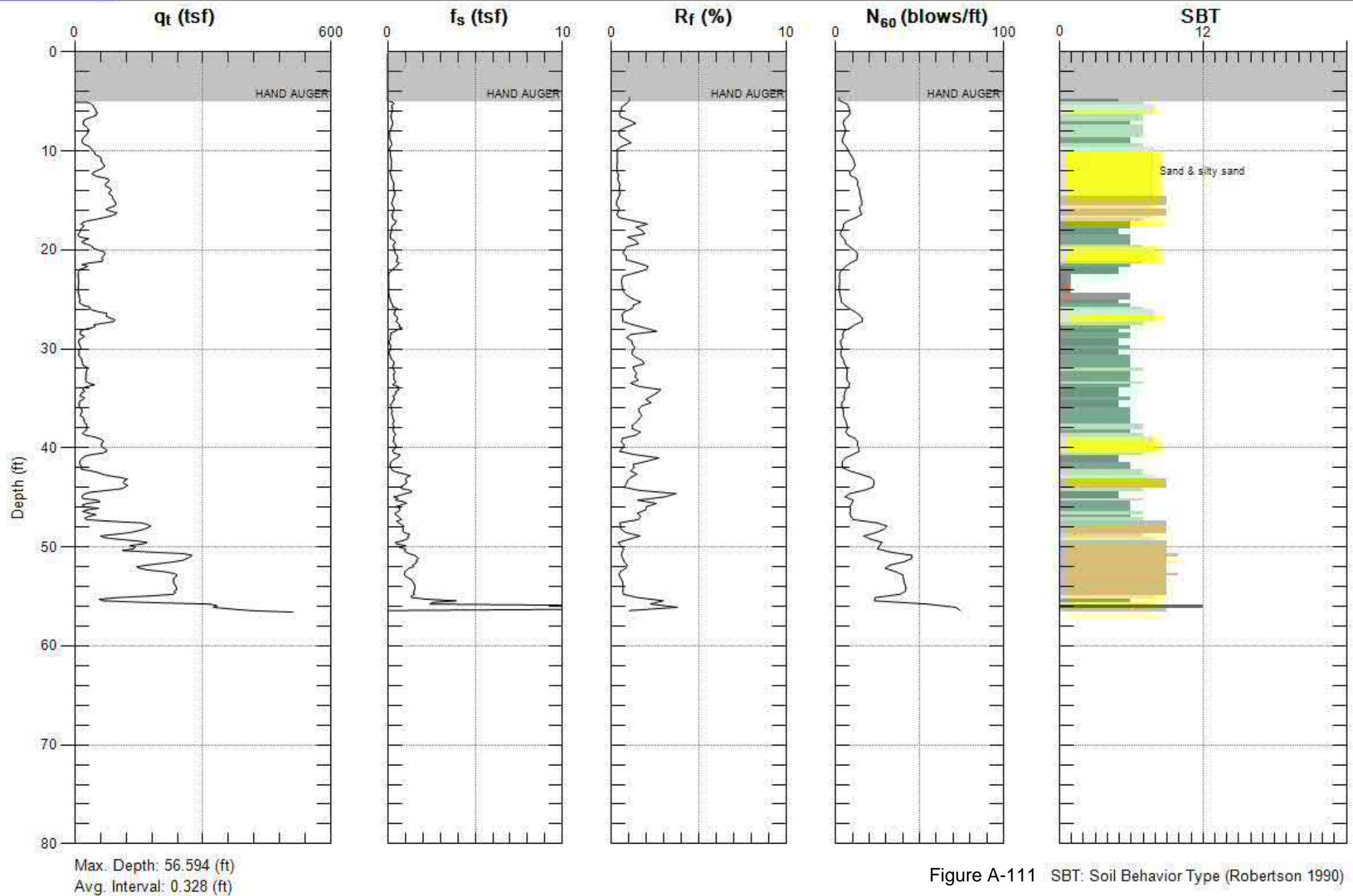


Figure A-111 SBT: Soil Behavior Type (Robertson 1990)

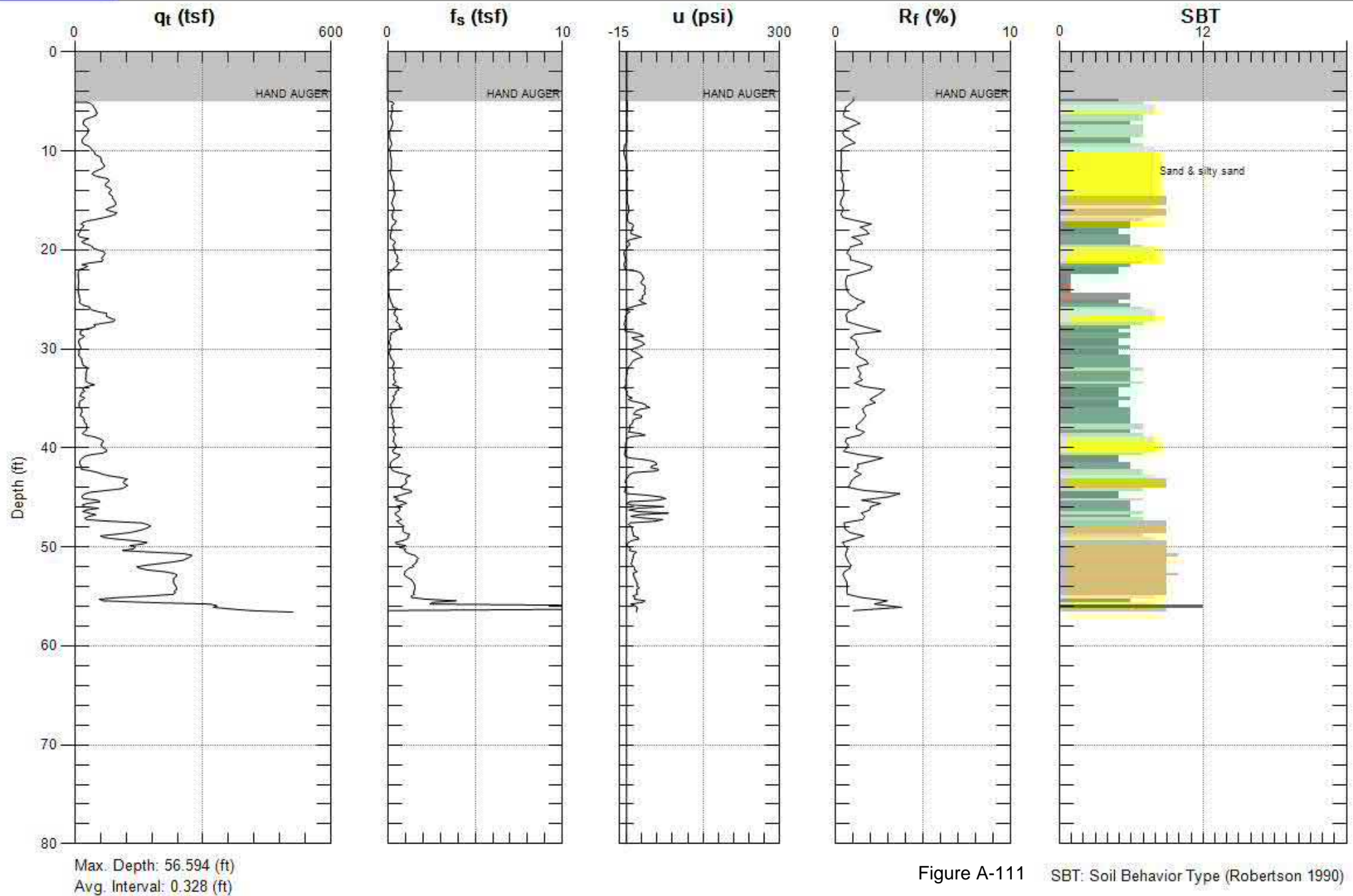


Figure A-111

SBT: Soil Behavior Type (Robertson 1990)

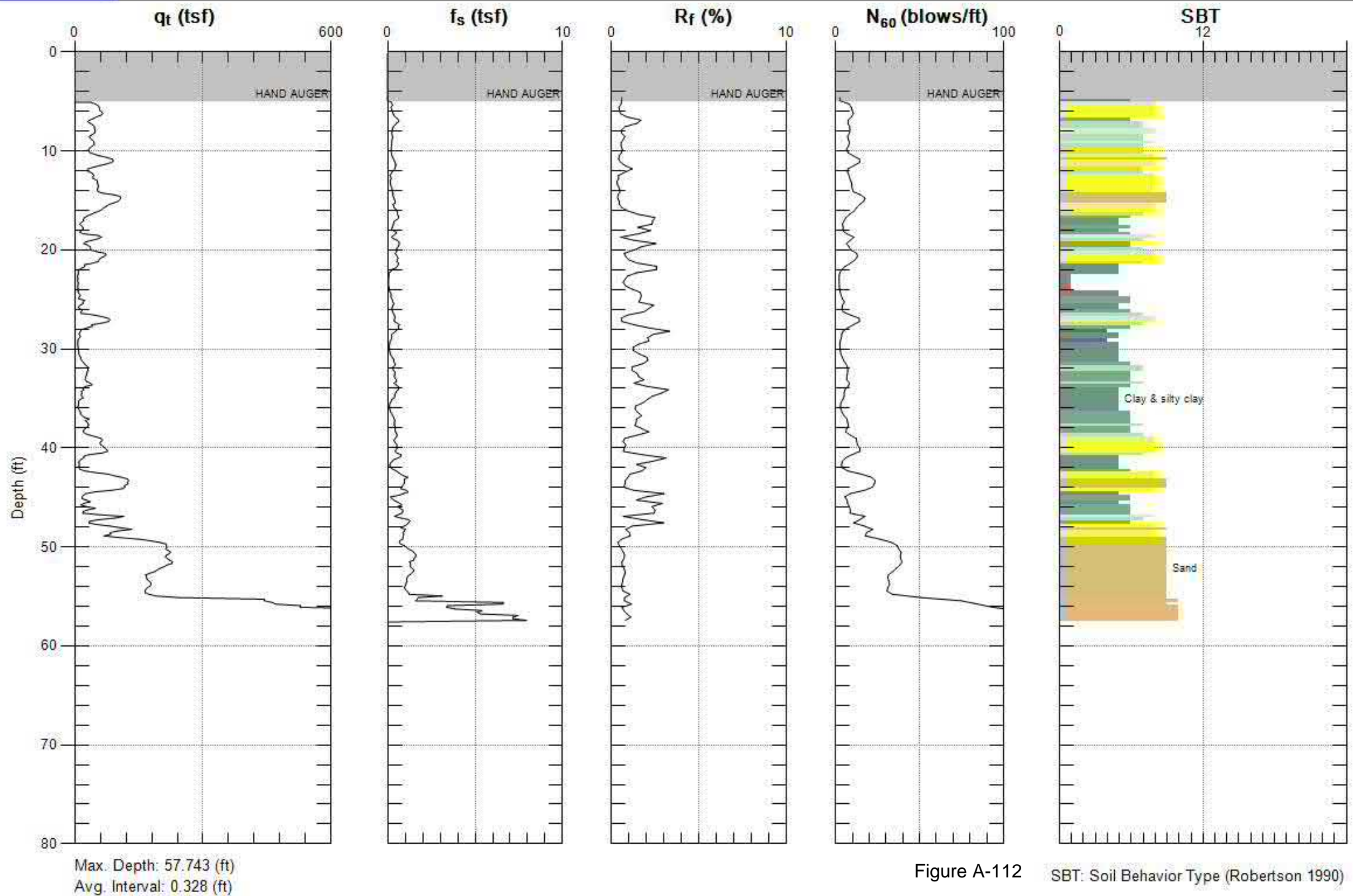


Figure A-112

SBT: Soil Behavior Type (Robertson 1990)

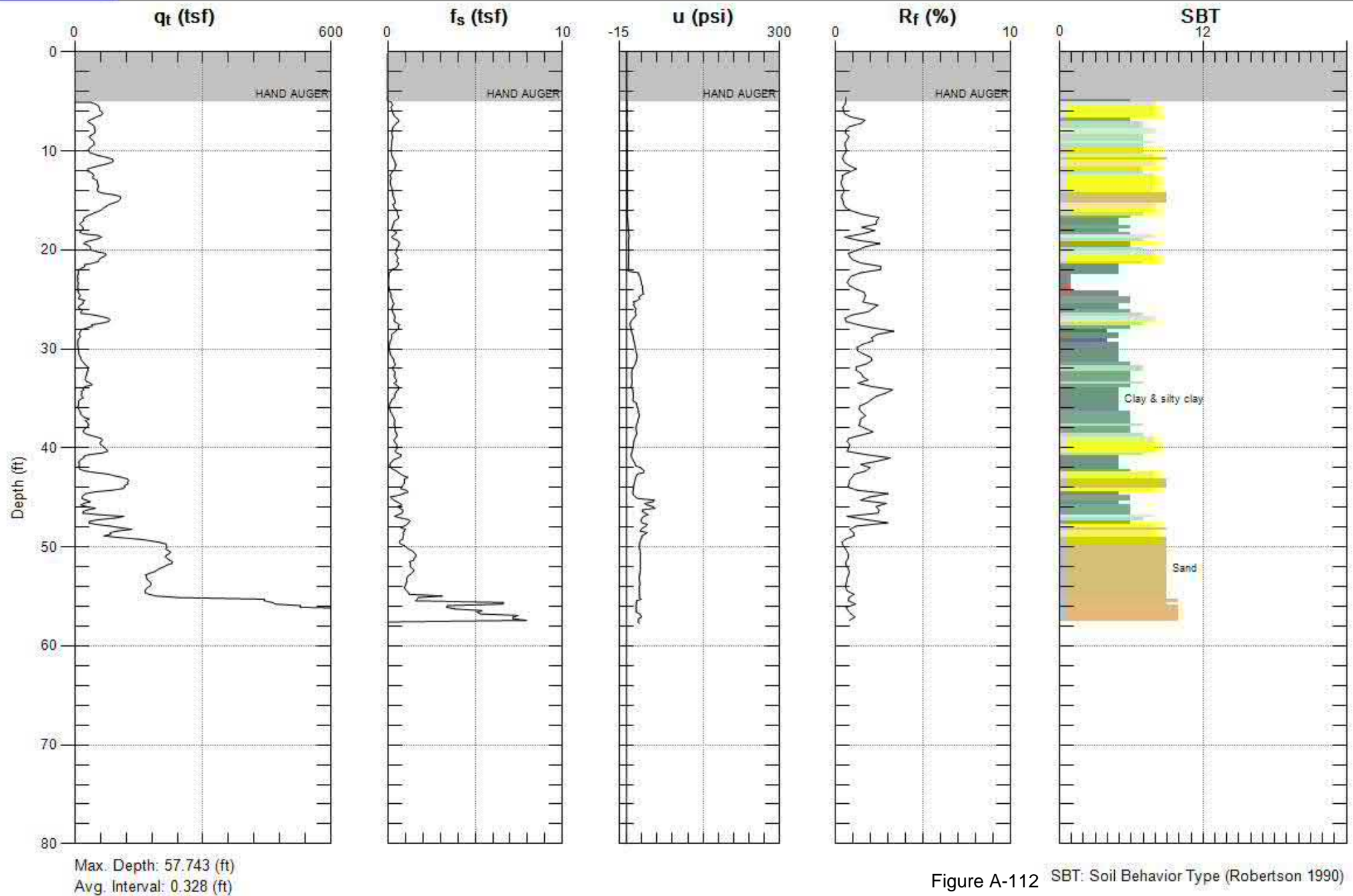


Figure A-112 SBT: Soil Behavior Type (Robertson 1990)

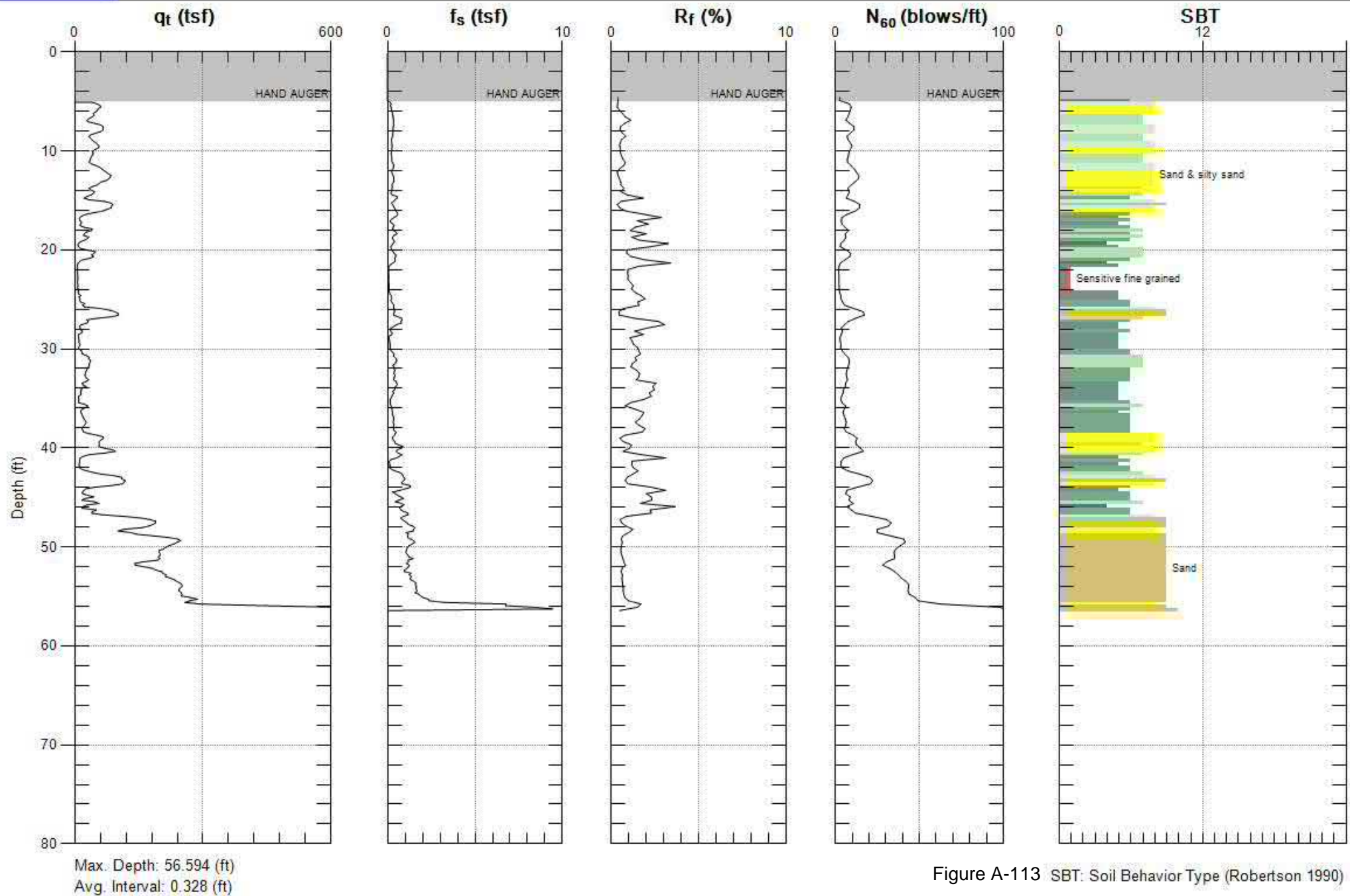


Figure A-113 SBT: Soil Behavior Type (Robertson 1990)

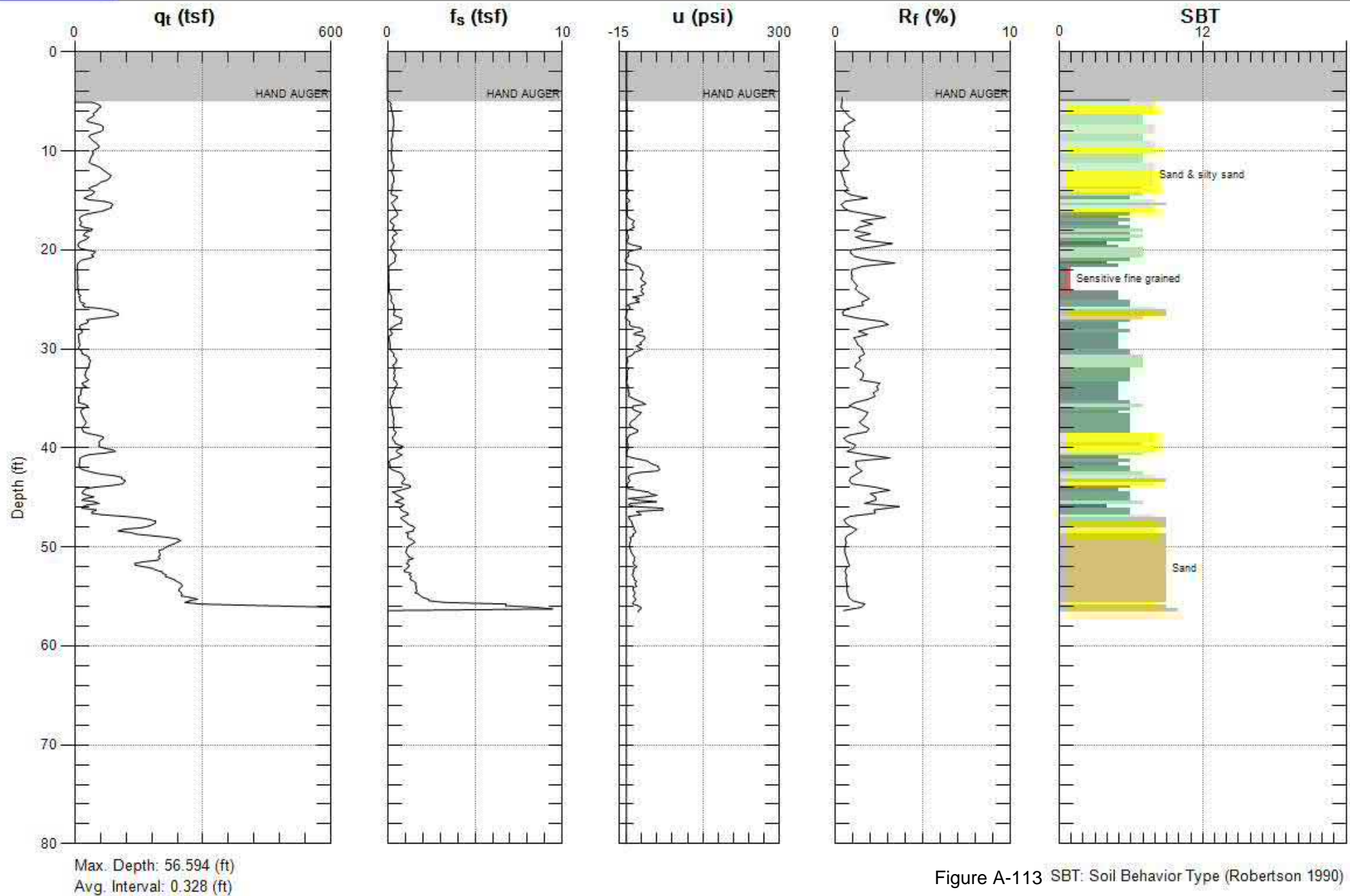


Figure A-113 SBT: Soil Behavior Type (Robertson 1990)

APPENDIX A – Boring Logs

The field investigation for the project included various types of explorations for the different project elements. Three (3) borings (PS-1 through PS-3) were advanced within Sherman Street and Custer Street for the pump station and sixty-seven (67) borings (WM-1 through WM-25 and FM-1 through FM-53) were advanced for the pipeline alignments. Thirty-seven (37) Cone Penetrometer Test (CPT) soundings were also advanced around the pump station site. Four (4) borings were advanced for the diversion structure connections in Friars Road (DS-1 through DS-6). In addition, borings were advanced for the four originally planned tunnel crossings: ONE borings (ex-1) for the I-805 crossing at Executive Drive, four borings (RC-1 through RC-4) at the railroad crossing near Rose Canyon, five (SC-1 through SC-5) near San Clemente Creek, and two (TC-1, 2) on West Morena Blvd. near Tecolote Creek. The Tecolote Creek crossing has been modified to be open trench and these borings are now considered part of the pipeline alignment borings. Some of the planned boring locations were eliminated based on nearby available information or due to utility conflicts, and therefore the numbering of the borings is not sequential.

The logs for the pump station borings, the pipeline alignment borings, and the diversion structure borings are presented in this appendix. The logs of borings for the tunnel crossings are presented in the Tunnel Report. Data for the CPT soundings are presented in the Faulting Report.

Field activities for the pump station borings were performed between April and August 2017 under the supervision of engineering geologists and engineers from our firm. Times and durations of field explorations varied based on traffic control permit constraints (typically 8:30 to 3:30 weekdays). Locations of the field explorations near the pump station, including CPT soundings and Borings PS-1 through PS-3, are presented in Figure 2. The remaining boring locations are presented on Figures 3a through 3e.

Procedures performed prior to any field activities included:

- Notifying Underground Service Alert (USA) 48 hours prior to drilling to mark out for subsurface utilities.
- Obtaining required County of San Diego Department of Environmental Health boring permits.
- Obtaining any necessary City of San Diego Right-of-Way permits.
- Generating plans and acquiring approved permits for traffic control.

Drilling for the borings was performed by Pacific Drilling or Cascade Drilling using hollow stem auger drilling methods. Either a truck-mounted Unimog Marl M5 drill rig with either 6-inch or 8-inch diameter hollow stem augers or a CME 85 with 6-inch diameter hollow stem augers were used to advance the borings. Each boring was advanced from the surface down to depths ranging from 2 feet to 81 feet. Borings shallower than 15 feet that met with refusal were moved and advanced to attempt to bypass the obstruction creating the refusal.

Relatively undisturbed samples of the subsurface materials were obtained within the exploratory borings using either a modified California sampler (2.5-inch outside diameter and 2-inch inside diameter) with thin stainless steel liners or a Standard Penetration Test (SPT) sampler (2.0-inch outside diameter and 1.5-inch inside diameter, without space for liners). The sampler was generally driven 18-inches into the material at the bottom of the boring by a 140-pound hammer falling 30-inches, and the blows required to advance the sampler was recorded in 6-inch increments for density correlations. The blow counts shown on the boring logs are for the final 12 inches of drive and have not been corrected for sample size or other corrections. The soil samples obtained were removed from the sampler, classified in the field, sealed to preserve the natural moisture content of the sample, and returned to the laboratory for further examination and testing.

Borings were backfilled according to San Diego County Site Assessment and Mitigation (SAM) Manual guidelines. The borings were finished with concrete or asphalt to approximately match the original surface. Soil cuttings collected from the borings were collected in 55 gallon steel drums affixed with non-hazardous labels. The drums were transported to a local City designated temporary storage location.

A monitoring well was completed in Boring PS-2. Perforated/slotted polyvinyl chloride (PVC) piping was installed from 6 feet below ground surface to the bottom of the boring at 61.5 feet. Solid pipe was installed at the top 6 feet. Filter sand was backfilled around the slotted pipe to one foot above the top of the slotted pipe. Bentonite was backfilled around the pipe over the filter sand to a depth of 2.5 feet. A traffic-rated vault was installed in concrete at the top of the boring, flush with the existing pavement surface. The monitoring well was developed and sampled. The groundwater samples recovered were sent to Eurofins Calscience analytical laboratory for further testing. Analytical laboratory test results are presented in Appendix C. A slug test was performed within the monitoring well for Boring PS-2.

A key to boring logs and the boring logs are presented in this Appendix.

APPENDIX B

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APPENDIX B – Geotechnical Laboratory Data

Geotechnical laboratory tests were performed on selected samples to aid in estimating soil properties and verify visual classifications of the materials. The tests include grain size, plasticity characteristics, corrosion potential, and R-value for pavement design. Test results are shown on the corresponding sample location on the logs of the borings in Appendix A. Detailed laboratory results are presented in this appendix.

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WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: DS-01
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	X 1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	X 1). NONE	X 1). NONE	X 1). NOT TESTED	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	X 3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S21	
Mass of Container & Wet Specimen, M1 (g)	528.89	
Mass of Container & Dry Specimen, M2 (g)	479.60	
Mass of Container, M3 (g)	104.85	
WATER CONTENT, wn (%)	13.15	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	763.56
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	763.56
Specimen Diameter, D (in) or ()	2.400
Specimen Length, L (in) or ()	5.150
Specimen Area, A (in^2) or ()	4.523
Specimen Volume, V (cm^3) or ()	381.68
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	124.9
DRY UNIT WEIGHT, DUW (pcf) or ()	110.4
VOID RATIO	0.527
DEGREE OF SATURATION, S (%)	67.36

MEASURED DIMENSIONS

	Length	Diameter
1	5.159	2.394
2	5.182	2.404
3	5.117	2.401
4	5.150	2.394
5	5.142	2.398
6	XXXXX	2.407
Average	5.150	2.400

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark grayish brown Silty SAND (SM)

TESTED BY: GD DATE: 8/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: DS-02
 Sample No.: 1
 Depth (ft): 5.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	X 2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY	X		4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE	X	1). NONE	X
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	X 2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	X 3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S219	
Mass of Container & Wet Specimen, M1 (g)	536.84	
Mass of Container & Dry Specimen, M2 (g)	458.34	
Mass of Container, M3 (g)	140.68	
WATER CONTENT, wn (%)	24.71	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1074.94
Mass of Container, M5 (g)	438.36
Mass of Wet Specimen, M6 (g)	636.58
Specimen Diameter, D (in) or ()	2.399
Specimen Length, L (in) or ()	4.659
Specimen Area, A (in^2) or ()	4.521
Specimen Volume, V (cm^3) or ()	345.13
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	115.1
DRY UNIT WEIGHT, DUW (pcf) or ()	92.3
VOID RATIO	0.826
DEGREE OF SATURATION, S (%)	80.82

MEASURED DIMENSIONS

	Length	Diameter
1	4.658	2.406
2	4.669	2.399
3	4.640	2.406
4	4.642	2.396
5	4.685	2.390
6	XXXXX	2.398
Average	4.659	2.399

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark olive gray Clayey SAND (SC)

TESTED BY: GD DATE: 8/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: DS-03
 Sample No.: 2
 Depth (ft): 5.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	X 1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	X 1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NOT TESTED	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	X 4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S215	
Mass of Container & Wet Specimen, M1 (g)	511.13	
Mass of Container & Dry Specimen, M2 (g)	430.58	
Mass of Container, M3 (g)	137.45	
WATER CONTENT, wn (%)	27.48	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	817.76
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	817.76
Specimen Diameter, D (in) or ()	2.405
Specimen Length, L (in) or ()	5.651
Specimen Area, A (in^2) or ()	4.542
Specimen Volume, V (cm^3) or ()	420.59
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	121.4
DRY UNIT WEIGHT, DUW (pcf) or ()	95.2
VOID RATIO	0.770
DEGREE OF SATURATION, S (%)	96.32

MEASURED DIMENSIONS

	Length	Diameter
1	5.661	2.416
2	5.647	2.405
3	5.645	2.394
4	5.664	2.401
5	5.640	2.407
6	XXXXX	2.405
Average	5.651	2.405

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive gray Silty SAND (SM)

TESTED BY: GD DATE: 8/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: DS-06
 Sample No.: 2
 Depth (ft): 5.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	X 1). HOMOGENEOUS	X 1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	X 3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S222	
Mass of Container & Wet Specimen, M1 (g)	503.86	
Mass of Container & Dry Specimen, M2 (g)	437.04	
Mass of Container, M3 (g)	140.49	
WATER CONTENT, wn (%)	22.53	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	626.72
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	626.72
Specimen Diameter, D (in) or ()	2.405
Specimen Length, L (in) or ()	4.217
Specimen Area, A (in^2) or ()	4.542
Specimen Volume, V (cm^3) or ()	313.81
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	124.7
DRY UNIT WEIGHT, DUW (pcf) or ()	101.7
VOID RATIO	0.657
DEGREE OF SATURATION, S (%)	92.66

MEASURED DIMENSIONS

	Length	Diameter
1	4.219	2.414
2	4.212	2.414
3	4.230	2.406
4	4.203	2.391
5	4.219	2.390
6	XXXXX	2.413
Average	4.217	2.405

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark grayish brown Silty SAND (SM)

TESTED BY: GD DATE: 8/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-01
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	1). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE X	2). GRAVEL	3). WET X	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT X	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH X
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S29	
Mass of Container & Wet Specimen, M1 (g)	531.86	
Mass of Container & Dry Specimen, M2 (g)	452.44	
Mass of Container, M3 (g)	125.99	
WATER CONTENT, wn (%)	24.33	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1065.97
Mass of Container, M5 (g)	210.56
Mass of Wet Specimen, M6 (g)	855.41
Specimen Diameter, D (in) or ()	2.410
Specimen Length, L (in) or ()	5.845
Specimen Area, A (in^2) or ()	4.562
Specimen Volume, V (cm^3) or ()	436.94
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	122.2
DRY UNIT WEIGHT, DUW (pcf) or ()	98.3
VOID RATIO	0.715
DEGREE OF SATURATION, S (%)	91.91

MEASURED DIMENSIONS	
Length	Diameter
1 5.849	2.392
2 5.862	2.413
3 5.839	2.421
4 5.840	2.395
5 5.836	2.419
6 XXXXX	2.420
Average 5.845	2.410

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive brown Poorly Graded SAND (SP)

TESTED BY: GD DATE: 5/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-02
 Sample No.: 2
 Depth (ft): 5.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND <input checked="" type="checkbox"/>		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT <input checked="" type="checkbox"/>			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM <input checked="" type="checkbox"/>	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) <input checked="" type="checkbox"/>	1). HOMOGENEOUS <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE
2). SOFT <input checked="" type="checkbox"/>	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM <input checked="" type="checkbox"/>
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S217	
Mass of Container & Wet Specimen, M1 (g)	442.43	
Mass of Container & Dry Specimen, M2 (g)	427.01	
Mass of Container, M3 (g)	137.94	
WATER CONTENT, wn (%)	5.33	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	716.37
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	716.37
Specimen Diameter, D (in) or ()	2.397
Specimen Length, L (in) or ()	5.954
Specimen Area, A (in^2) or ()	4.514
Specimen Volume, V (cm^3) or ()	440.44
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	101.5
DRY UNIT WEIGHT, DUW (pcf) or ()	96.4
VOID RATIO	0.749
DEGREE OF SATURATION, S (%)	19.24

MEASURED DIMENSIONS

	Length	Diameter
1	5.959	2.391
2	5.947	2.397
3	5.966	2.400
4	5.954	2.393
5	5.946	2.401
6	XXXXX	2.402
Average	5.954	2.397

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Very dark grayish brown Poorly Graded SAND with Silt (SP-SM)

TESTED BY: GD DATE: 5/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-04
 Sample No.: 2
 Depth (ft): 15.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD X		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S224	
Mass of Container & Wet Specimen, M1 (g)	531.64	
Mass of Container & Dry Specimen, M2 (g)	472.74	
Mass of Container, M3 (g)	140.75	
WATER CONTENT, wn (%)	17.74	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	758.01
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	758.01
Specimen Diameter, D (in) or ()	2.396
Specimen Length, L (in) or ()	4.980
Specimen Area, A (in^2) or ()	4.509
Specimen Volume, V (cm^3) or ()	367.92
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	128.6
DRY UNIT WEIGHT, DUW (pcf) or ()	109.2
VOID RATIO	0.543
DEGREE OF SATURATION, S (%)	88.21

MEASURED DIMENSIONS	
Length	Diameter
4.972	2.394
4.976	2.395
4.981	2.396
4.974	2.401
4.995	2.397
XXXXX	2.393
Average	2.396

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Brown Sandstone

TESTED BY: GD DATE: 5/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-06
 Sample No.: 4
 Depth (ft): 15.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE X	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH X
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S205	
Mass of Container & Wet Specimen, M1 (g)	566.94	
Mass of Container & Dry Specimen, M2 (g)	490.69	
Mass of Container, M3 (g)	139.34	
WATER CONTENT, wn (%)	21.70	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	933.59
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	933.59
Specimen Diameter, D (in) or ()	2.396
Specimen Length, L (in) or ()	5.997
Specimen Area, A (in^2) or ()	4.508
Specimen Volume, V (cm^3) or ()	442.94
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	131.6
DRY UNIT WEIGHT, DUW (pcf) or ()	108.1
VOID RATIO	0.559
DEGREE OF SATURATION, S (%)	104.81

MEASURED DIMENSIONS

	Length	Diameter
1	5.995	2.389
2	5.998	2.393
3	5.997	2.401
4	5.997	2.393
5	5.996	2.396
6	XXXXX	2.402
Average	5.997	2.396

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark yellowish brown Silty SAND (SM)

TESTED BY: GD DATE: 5/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-07
 Sample No.: 2
 Depth (ft): 5.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW X
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S221	
Mass of Container & Wet Specimen, M1 (g)	454.73	
Mass of Container & Dry Specimen, M2 (g)	432.55	
Mass of Container, M3 (g)	140.33	
WATER CONTENT, wn (%)	7.59	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	776.77
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	776.77
Specimen Diameter, D (in) or ()	2.394
Specimen Length, L (in) or ()	5.906
Specimen Area, A (in^2) or ()	4.500
Specimen Volume, V (cm^3) or ()	435.52
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	111.3
DRY UNIT WEIGHT, DUW (pcf) or ()	103.5
VOID RATIO	0.629
DEGREE OF SATURATION, S (%)	32.59

MEASURED DIMENSIONS	
Length	Diameter
1 5.901	2.398
2 5.905	2.404
3 5.910	2.400
4 5.899	2.381
5 5.915	2.389
6 XXXXX	2.390
Average 5.906	2.394

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive brown Poorly Graded SAND (SP)

TESTED BY: GD DATE: 5/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-07
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	X 1). COBBLES (>3")	2). MOIST	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	X 3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	X 1). HOMOGENEOUS	X 1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	X 3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S208	
Mass of Container & Wet Specimen, M1 (g)	574.44	
Mass of Container & Dry Specimen, M2 (g)	502.67	
Mass of Container, M3 (g)	146.59	
WATER CONTENT, wn (%)	20.16	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	834.00
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	834.00
Specimen Diameter, D (in) or ()	2.404
Specimen Length, L (in) or ()	5.564
Specimen Area, A (in^2) or ()	4.537
Specimen Volume, V (cm^3) or ()	413.67
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	125.9
DRY UNIT WEIGHT, DUW (pcf) or ()	104.7
VOID RATIO	0.609
DEGREE OF SATURATION, S (%)	89.34

MEASURED DIMENSIONS	
Length	Diameter
1 5.558	2.398
2 5.551	2.405
3 5.585	2.412
4 5.564	2.393
5 5.561	2.402
6 XXXXX	2.411
Average 5.564	2.404

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Grayish brown Poorly Graded SAND (SP)

TESTED BY: GD DATE: 5/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-08
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY <input checked="" type="checkbox"/>	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	2). SPOTTED	1). COARSE	2). SUBANGULAR <input checked="" type="checkbox"/>
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY <input checked="" type="checkbox"/>			4). MEDIUM <input checked="" type="checkbox"/>	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20) <input checked="" type="checkbox"/>	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF <input checked="" type="checkbox"/>	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	B63	
Mass of Container & Wet Specimen, M1 (g)	761.01	
Mass of Container & Dry Specimen, M2 (g)	716.37	
Mass of Container, M3 (g)	200.70	
WATER CONTENT, wn (%)	8.66	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	923.47
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	923.47
Specimen Diameter, D (in) or ()	2.378
Specimen Length, L (in) or ()	5.916
Specimen Area, A (in^2) or ()	4.441
Specimen Volume, V (cm^3) or ()	430.60
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	133.9
DRY UNIT WEIGHT, DUW (pcf) or ()	123.2
VOID RATIO	0.368
DEGREE OF SATURATION, S (%)	63.52

MEASURED DIMENSIONS	
Length	Diameter
1 5.922	2.374
2 5.857	2.364
3 5.810	2.378
4 5.926	2.394
5 6.067	2.379
6 XXXXX	2.379
Average 5.916	2.378

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark yellowish brown Sandy, Lean CLAY (CL)

TESTED BY: ADC DATE: 6/12/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-11
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	<input checked="" type="checkbox"/>	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY	<input checked="" type="checkbox"/>		4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW <input checked="" type="checkbox"/>
3). MED. STIFF	3). MEDIUM (PI >10 to 20) <input checked="" type="checkbox"/>	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF <input checked="" type="checkbox"/>	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	FJ10	
Mass of Container & Wet Specimen, M1 (g)	621.60	
Mass of Container & Dry Specimen, M2 (g)	538.84	
Mass of Container, M3 (g)	111.28	
WATER CONTENT, wn (%)	19.36	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	901.99
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	901.99
Specimen Diameter, D (in) or ()	2.388
Specimen Length, L (in) or ()	5.697
Specimen Area, A (in^2) or ()	4.479
Specimen Volume, V (cm^3) or ()	418.15
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	134.7
DRY UNIT WEIGHT, DUW (pcf) or ()	112.8
VOID RATIO	0.494
DEGREE OF SATURATION, S (%)	105.80

MEASURED DIMENSIONS	
Length	Diameter
1 5.861	2.386
2 5.807	2.386
3 5.905	2.386
4 5.848	2.384
5 5.062	2.393
6 XXXXX	2.394
Average 5.697	2.388

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive brown Sandy, Lean CLAY (CL)

TESTED BY: ADC DATE: 6/12/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-16
 Sample No.: 2
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	1). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY X			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S88	
Mass of Container & Wet Specimen, M1 (g)	607.07	
Mass of Container & Dry Specimen, M2 (g)	555.22	
Mass of Container, M3 (g)	113.09	
WATER CONTENT, wn (%)	11.73	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	929.87
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	929.87
Specimen Diameter, D (in) or ()	2.389
Specimen Length, L (in) or ()	5.950
Specimen Area, A (in^2) or ()	4.482
Specimen Volume, V (cm^3) or ()	437.01
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	132.8
DRY UNIT WEIGHT, DUW (pcf) or ()	118.9
VOID RATIO	0.418
DEGREE OF SATURATION, S (%)	75.80

MEASURED DIMENSIONS

	Length	Diameter
1	5.973	2.381
2	5.967	2.382
3	5.966	2.382
4	5.879	2.394
5	5.966	2.395
6	XXXXX	2.399
Average	5.950	2.389

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark yellowish brown Clayey SAND (SC)

TESTED BY: ADC DATE: 6/12/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-16
 Sample No.: 4
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY X			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF X	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	B22	
Mass of Container & Wet Specimen, M1 (g)	950.01	
Mass of Container & Dry Specimen, M2 (g)	825.70	
Mass of Container, M3 (g)	200.64	
WATER CONTENT, wn (%)	19.89	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	869.02
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	869.02
Specimen Diameter, D (in) or ()	2.398
Specimen Length, L (in) or ()	5.678
Specimen Area, A (in^2) or ()	4.517
Specimen Volume, V (cm^3) or ()	420.26
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	129.1
DRY UNIT WEIGHT, DUW (pcf) or ()	107.7
VOID RATIO	0.565
DEGREE OF SATURATION, S (%)	94.97

MEASURED DIMENSIONS

	Length	Diameter
1	5.665	2.398
2	5.692	2.398
3	5.659	2.413
4	5.687	2.388
5	5.685	2.396
6	XXXXX	2.396
Average	5.678	2.398

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown Sandy, Lean CLAY (CL)

TESTED BY: ADC DATE: 6/12/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-17
 Sample No.: 1
 Depth (ft): 5.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY X			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF X	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S3	
Mass of Container & Wet Specimen, M1 (g)	809.07	
Mass of Container & Dry Specimen, M2 (g)	723.53	
Mass of Container, M3 (g)	83.13	
WATER CONTENT, wn (%)	13.36	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	914.07
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	914.07
Specimen Diameter, D (in) or ()	2.382
Specimen Length, L (in) or ()	5.974
Specimen Area, A (in^2) or ()	4.456
Specimen Volume, V (cm^3) or ()	436.18
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	130.8
DRY UNIT WEIGHT, DUW (pcf) or ()	115.4
VOID RATIO	0.461
DEGREE OF SATURATION, S (%)	78.32

MEASURED DIMENSIONS	
Length	Diameter
1 5.970	2.383
2 5.975	2.382
3 5.977	2.382
4 5.971	2.379
5 5.976	2.381
6 XXXXX	2.384
Average 5.974	2.382

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Clayey SAND (SC)

TESTED BY: ADC DATE: 6/12/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-17
 Sample No.: 3
 Depth (ft): 15.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF X	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	B36	
Mass of Container & Wet Specimen, M1 (g)	1013.05	
Mass of Container & Dry Specimen, M2 (g)	903.60	
Mass of Container, M3 (g)	197.98	
WATER CONTENT, wn (%)	15.51	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1187.25
Mass of Container, M5 (g)	254.16
Mass of Wet Specimen, M6 (g)	933.09
Specimen Diameter, D (in) or ()	2.412
Specimen Length, L (in) or ()	5.966
Specimen Area, A (in^2) or ()	4.569
Specimen Volume, V (cm^3) or ()	446.70
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	130.4
DRY UNIT WEIGHT, DUW (pcf) or ()	112.9
VOID RATIO	0.493
DEGREE OF SATURATION, S (%)	84.94

MEASURED DIMENSIONS	
Length	Diameter
1 5.968	2.412
2 5.960	2.413
3 5.967	2.412
4 5.964	2.411
5 5.970	2.414
6 XXXXX	2.410
Average 5.966	2.412

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive yellow Silty SAND (SM)

TESTED BY: ADC DATE: 6/12/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-18
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY X	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY X			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20) X	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF X	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	B27	
Mass of Container & Wet Specimen, M1 (g)	779.48	
Mass of Container & Dry Specimen, M2 (g)	686.58	
Mass of Container, M3 (g)	200.49	
WATER CONTENT, wn (%)	19.11	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	916.37
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	916.37
Specimen Diameter, D (in) or ()	2.411
Specimen Length, L (in) or ()	5.982
Specimen Area, A (in^2) or ()	4.564
Specimen Volume, V (cm^3) or ()	447.35
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	127.9
DRY UNIT WEIGHT, DUW (pcf) or ()	107.4
VOID RATIO	0.570
DEGREE OF SATURATION, S (%)	90.53

MEASURED DIMENSIONS

	Length	Diameter
1	5.981	2.407
2	5.981	2.401
3	5.989	2.399
4	5.984	2.417
5	5.975	2.420
6	XXXXX	2.419
Average	5.982	2.411

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Lean CLAY (CL)

TESTED BY: ADC DATE: 6/13/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-18
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	0). BOULDERS (>12")	1). DRY <input checked="" type="checkbox"/>	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY <input checked="" type="checkbox"/>			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20) <input checked="" type="checkbox"/>	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF <input checked="" type="checkbox"/>	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer		
Container No.	S31			
Mass of Container & Wet Specimen, M1 (g)	627.15			
Mass of Container & Dry Specimen, M2 (g)	535.54			
Mass of Container, M3 (g)	98.21			
WATER CONTENT, wn (%)	20.95			
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10				

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	912.46
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	912.46
Specimen Diameter, D (in) or ()	2.397
Specimen Length, L (in) or ()	5.962
Specimen Area, A (in^2) or ()	4.511
Specimen Volume, V (cm^3) or ()	440.72
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	129.2
DRY UNIT WEIGHT, DUW (pcf) or ()	106.9
VOID RATIO	0.577
DEGREE OF SATURATION, S (%)	97.97

MEASURED DIMENSIONS	
Length	Diameter
1 5.961	2.391
2 5.961	2.390
3 5.966	2.394
4 5.959	2.404
5 5.965	2.401
6 XXXXX	2.399
Average 5.962	2.397

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Gray Lean CLAY with Sand (CL)

TESTED BY: ADC DATE: 6/13/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-19
 Sample No.: 2
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF X	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M62	
Mass of Container & Wet Specimen, M1 (g)	488.16	
Mass of Container & Dry Specimen, M2 (g)	441.00	
Mass of Container, M3 (g)	91.69	
WATER CONTENT, wn (%)	13.50	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	834.22
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	834.22
Specimen Diameter, D (in) or ()	2.390
Specimen Length, L (in) or ()	5.446
Specimen Area, A (in^2) or ()	4.487
Specimen Volume, V (cm^3) or ()	400.46
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	130.0
DRY UNIT WEIGHT, DUW (pcf) or ()	114.6
VOID RATIO	0.471
DEGREE OF SATURATION, S (%)	77.38

MEASURED DIMENSIONS	
Length	Diameter
1 5.421	2.385
2 5.458	2.389
3 5.453	2.393
4 5.442	2.394
5 5.458	2.388
6 XXXXX	2.392
Average 5.446	2.390

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark yellowish brown Sandy SILT with Gravel (ML)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-20
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND <input checked="" type="checkbox"/>		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED <input checked="" type="checkbox"/>	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD <input checked="" type="checkbox"/>		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M5	
Mass of Container & Wet Specimen, M1 (g)	465.89	
Mass of Container & Dry Specimen, M2 (g)	407.05	
Mass of Container, M3 (g)	91.23	
WATER CONTENT, wn (%)	18.63	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	830.75
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	830.75
Specimen Diameter, D (in) or ()	2.395
Specimen Length, L (in) or ()	5.850
Specimen Area, A (in^2) or ()	4.506
Specimen Volume, V (cm^3) or ()	432.00
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	120.1
DRY UNIT WEIGHT, DUW (pcf) or ()	101.2
VOID RATIO	0.666
DEGREE OF SATURATION, S (%)	75.57

MEASURED DIMENSIONS	
Length	Diameter
1 5.841	2.405
2 5.850	2.394
3 5.847	2.383
4 5.875	2.401
5 5.837	2.394
6 XXXXX	2.395
Average 5.850	2.395

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown Lean CLAY (CL)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-20
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	Major Minor	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT <input checked="" type="checkbox"/>			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED <input checked="" type="checkbox"/>	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20) <input checked="" type="checkbox"/>	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD <input checked="" type="checkbox"/>		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M6	
Mass of Container & Wet Specimen, M1 (g)	476.83	
Mass of Container & Dry Specimen, M2 (g)	415.88	
Mass of Container, M3 (g)	90.32	
WATER CONTENT, wn (%)	18.72	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	750.24
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	750.24
Specimen Diameter, D (in) or ()	2.384
Specimen Length, L (in) or ()	5.546
Specimen Area, A (in^2) or ()	4.462
Specimen Volume, V (cm^3) or ()	405.48
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	115.5
DRY UNIT WEIGHT, DUW (pcf) or ()	97.3
VOID RATIO	0.732
DEGREE OF SATURATION, S (%)	69.01

MEASURED DIMENSIONS	
Length	Diameter
1 5.541	2.376
2 5.538	2.375
3 5.570	2.381
4 5.554	2.401
5 5.525	2.389
6 XXXXX	2.379
Average 5.546	2.384

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown Elastic SILT (MH)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-21
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M91	
Mass of Container & Wet Specimen, M1 (g)	474.98	
Mass of Container & Dry Specimen, M2 (g)	457.33	
Mass of Container, M3 (g)	89.78	
WATER CONTENT, wn (%)	4.80	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	986.28
Mass of Container, M5 (g)	250.18
Mass of Wet Specimen, M6 (g)	736.10
Specimen Diameter, D (in) or ()	2.395
Specimen Length, L (in) or ()	5.659
Specimen Area, A (in^2) or ()	4.506
Specimen Volume, V (cm^3) or ()	417.89
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	110.0
DRY UNIT WEIGHT, DUW (pcf) or ()	104.9
VOID RATIO	0.606
DEGREE OF SATURATION, S (%)	21.38

MEASURED DIMENSIONS	
Length	Diameter
1 5.645	2.380
2 5.651	2.389
3 5.682	2.407
4 5.673	2.391
5 5.644	2.400
6 XXXXX	2.405
Average 5.659	2.395

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown Silty SAND (SM)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-21
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	Major Minor	1). DRY	1). UNIFORM	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE	1). NONE	1). NOT TESTED	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M8	
Mass of Container & Wet Specimen, M1 (g)	441.38	
Mass of Container & Dry Specimen, M2 (g)	380.21	
Mass of Container, M3 (g)	89.37	
WATER CONTENT, wn (%)	21.03	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1001.01
Mass of Container, M5 (g)	263.02
Mass of Wet Specimen, M6 (g)	737.99
Specimen Diameter, D (in) or ()	2.394
Specimen Length, L (in) or ()	5.748
Specimen Area, A (in^2) or ()	4.501
Specimen Volume, V (cm^3) or ()	423.92
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	108.7
DRY UNIT WEIGHT, DUW (pcf) or ()	89.8
VOID RATIO	0.877
DEGREE OF SATURATION, S (%)	64.74

MEASURED DIMENSIONS	
Length	Diameter
1 5.753	2.400
2 5.768	2.395
3 5.725	2.397
4 5.762	2.388
5 5.731	2.394
6 XXXXX	2.389
Average 5.748	2.394

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Grayish brown SILT with Sand (ML)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-22
 Sample No.: 2
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW X
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M1	
Mass of Container & Wet Specimen, M1 (g)	416.79	
Mass of Container & Dry Specimen, M2 (g)	386.00	
Mass of Container, M3 (g)	91.67	
WATER CONTENT, wn (%)	10.46	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	967.29
Mass of Container, M5 (g)	205.15
Mass of Wet Specimen, M6 (g)	762.14
Specimen Diameter, D (in) or ()	2.395
Specimen Length, L (in) or ()	5.839
Specimen Area, A (in^2) or ()	4.503
Specimen Volume, V (cm^3) or ()	430.87
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	110.4
DRY UNIT WEIGHT, DUW (pcf) or ()	100.0
VOID RATIO	0.686
DEGREE OF SATURATION, S (%)	41.17

MEASURED DIMENSIONS

	Length	Diameter
1	5.828	2.382
2	5.808	2.383
3	5.873	2.412
4	5.840	2.387
5	5.845	2.394
6	XXXXX	2.409
Average	5.839	2.395

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Strong brown Silty SAND (SM)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-22
 Sample No.: 4
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	1). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE X	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM X
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M9	
Mass of Container & Wet Specimen, M1 (g)	510.29	
Mass of Container & Dry Specimen, M2 (g)	469.63	
Mass of Container, M3 (g)	89.70	
WATER CONTENT, wn (%)	10.70	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	808.81
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	808.81
Specimen Diameter, D (in) or ()	2.405
Specimen Length, L (in) or ()	5.813
Specimen Area, A (in^2) or ()	4.543
Specimen Volume, V (cm^3) or ()	432.76
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	116.7
DRY UNIT WEIGHT, DUW (pcf) or ()	105.4
VOID RATIO	0.599
DEGREE OF SATURATION, S (%)	48.22

MEASURED DIMENSIONS

	Length	Diameter
1	5.793	2.403
2	5.857	2.405
3	5.785	2.407
4	5.831	2.403
5	5.801	2.404
6	XXXXX	2.408
Average	5.813	2.405

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Strong brown Silty SAND (SM)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-23
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	X 1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY	X		4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	X 1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	X 3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	X 5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M7	
Mass of Container & Wet Specimen, M1 (g)	434.86	
Mass of Container & Dry Specimen, M2 (g)	397.70	
Mass of Container, M3 (g)	91.00	
WATER CONTENT, wn (%)	12.12	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1027.87
Mass of Container, M5 (g)	272.50
Mass of Wet Specimen, M6 (g)	755.37
Specimen Diameter, D (in) or ()	2.366
Specimen Length, L (in) or ()	5.691
Specimen Area, A (in^2) or ()	4.396
Specimen Volume, V (cm^3) or ()	409.95
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	115.0
DRY UNIT WEIGHT, DUW (pcf) or ()	102.6
VOID RATIO	0.643
DEGREE OF SATURATION, S (%)	50.88

MEASURED DIMENSIONS

	Length	Diameter
1	5.694	2.351
2	5.686	2.358
3	5.693	2.383
4	5.711	2.376
5	5.670	2.354
6	XXXXX	2.373
Average	5.691	2.366

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Strong brown Clayey SAND (SC)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-23
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	1). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE X	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF X	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M60	
Mass of Container & Wet Specimen, M1 (g)	500.34	
Mass of Container & Dry Specimen, M2 (g)	459.59	
Mass of Container, M3 (g)	91.37	
WATER CONTENT, wn (%)	11.07	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	872.25
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	872.25
Specimen Diameter, D (in) or ()	2.371
Specimen Length, L (in) or ()	5.952
Specimen Area, A (in^2) or ()	4.415
Specimen Volume, V (cm^3) or ()	430.63
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	126.4
DRY UNIT WEIGHT, DUW (pcf) or ()	113.8
VOID RATIO	0.481
DEGREE OF SATURATION, S (%)	62.18

MEASURED DIMENSIONS

	Length	Diameter
1	5.956	2.364
2	5.938	2.359
3	5.951	2.379
4	5.957	2.376
5	5.957	2.373
6	XXXXX	2.375
Average	5.952	2.371

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Strong brown Silty SAND (SM)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-24
 Sample No.: 2
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	X 1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M3	
Mass of Container & Wet Specimen, M1 (g)	459.80	
Mass of Container & Dry Specimen, M2 (g)	430.81	
Mass of Container, M3 (g)	89.90	
WATER CONTENT, wn (%)	8.50	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Moisture content only. Bag sample.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm^3

Specimen Color and Description: Strong brown Poorly Graded SAND with Silt (SP-SM)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-24
 Sample No.: 4
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	X 1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NOT TESTED	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M73	
Mass of Container & Wet Specimen, M1 (g)	399.80	
Mass of Container & Dry Specimen, M2 (g)	375.18	
Mass of Container, M3 (g)	108.66	
WATER CONTENT, wn (%)	9.24	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Moisture content only. Bag sample.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm^3

Specimen Color and Description: Strong brown Silty SAND (SM)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-25
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW X
3). MED. STIFF X	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S216	
Mass of Container & Wet Specimen, M1 (g)	546.03	
Mass of Container & Dry Specimen, M2 (g)	505.32	
Mass of Container, M3 (g)	148.62	
WATER CONTENT, wn (%)	11.41	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	793.00
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	793.00
Specimen Diameter, D (in) or ()	2.401
Specimen Length, L (in) or ()	5.604
Specimen Area, A (in^2) or ()	4.527
Specimen Volume, V (cm^3) or ()	415.72
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	119.1
DRY UNIT WEIGHT, DUW (pcf) or ()	106.9
VOID RATIO	0.577
DEGREE OF SATURATION, S (%)	53.41

MEASURED DIMENSIONS	
Length	Diameter
1 5.597	2.398
2 5.574	2.394
3 5.613	2.412
4 5.643	2.404
5 5.592	2.394
6 XXXXX	2.403
Average 5.604	2.401

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown Silty SAND (SM)

TESTED BY: GD DATE: 8/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-25
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	X 1). HOMOGENEOUS	X 1). NONE	X 1). NONE	X 1). NOT TESTED	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	B72	
Mass of Container & Wet Specimen, M1 (g)	549.19	
Mass of Container & Dry Specimen, M2 (g)	517.60	
Mass of Container, M3 (g)	200.28	
WATER CONTENT, wn (%)	9.96	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

No liner. Bag sample. Moisture content only.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm^3

Specimen Color and Description: Dark yellowish brown Silty SAND (SM)

TESTED BY: GD DATE: 8/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-29
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR X
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW X
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	C4	
Mass of Container & Wet Specimen, M1 (g)	429.26	
Mass of Container & Dry Specimen, M2 (g)	402.84	
Mass of Container, M3 (g)	71.88	
WATER CONTENT, wn (%)	7.98	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	746.67
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	746.67
Specimen Diameter, D (in) or ()	2.413
Specimen Length, L (in) or ()	5.949
Specimen Area, A (in^2) or ()	4.574
Specimen Volume, V (cm^3) or ()	445.90
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	104.5
DRY UNIT WEIGHT, DUW (pcf) or ()	96.8
VOID RATIO	0.741
DEGREE OF SATURATION, S (%)	29.08

MEASURED DIMENSIONS	
Length	Diameter
1 5.933	2.399
2 5.949	2.416
3 5.959	2.426
4 5.962	2.412
5 5.944	2.411
6 XXXXX	2.415
Average 5.949	2.413

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Strong brown Silty SAND (SM)

TESTED BY: GD DATE: 8/18/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-29
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	X 1). NONE	X 1). NONE	X 1). NOT TESTED	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M7	
Mass of Container & Wet Specimen, M1 (g)	425.14	
Mass of Container & Dry Specimen, M2 (g)	376.02	
Mass of Container, M3 (g)	91.00	
WATER CONTENT, wn (%)	17.23	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

No unit weight conducted as only bag sample. Moisture content only.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown SILT with Sand (ML)

TESTED BY: GD DATE: 8/18/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-30
 Sample No.: 2
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED X	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE X	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M60	
Mass of Container & Wet Specimen, M1 (g)	442.68	
Mass of Container & Dry Specimen, M2 (g)	412.43	
Mass of Container, M3 (g)	91.37	
WATER CONTENT, wn (%)	9.42	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	548.23
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	548.23
Specimen Diameter, D (in) or ()	2.385
Specimen Length, L (in) or ()	4.468
Specimen Area, A (in^2) or ()	4.467
Specimen Volume, V (cm^3) or ()	327.08
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	104.6
DRY UNIT WEIGHT, DUW (pcf) or ()	95.6
VOID RATIO	0.763
DEGREE OF SATURATION, S (%)	33.36

MEASURED DIMENSIONS

	Length	Diameter
1	4.479	2.391
2	4.435	2.378
3	4.499	2.372
4	4.440	2.395
5	4.486	2.389
6	XXXXX	2.385
Average	4.468	2.385

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown Silty SAND with Gravel (SM)

TESTED BY: GD DATE: 8/18/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-30
 Sample No.: 4
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	X 1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	X 1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD	X	6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M3	
Mass of Container & Wet Specimen, M1 (g)	530.89	
Mass of Container & Dry Specimen, M2 (g)	474.80	
Mass of Container, M3 (g)	89.90	
WATER CONTENT, wn (%)	14.57	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

horizontally stratified broken sandstone

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	663.75
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	663.75
Specimen Diameter, D (in) or ()	2.386
Specimen Length, L (in) or ()	5.339
Specimen Area, A (in^2) or ()	4.471
Specimen Volume, V (cm^3) or ()	391.18
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	105.9
DRY UNIT WEIGHT, DUW (pcf) or ()	92.5
VOID RATIO	0.823
DEGREE OF SATURATION, S (%)	47.80

MEASURED DIMENSIONS

	Length	Diameter
1	5.340	2.378
2	5.351	2.395
3	5.303	2.372
4	5.371	2.390
5	5.329	2.390
6	XXXXX	2.391
Average	5.339	2.386

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown Silty SAND (SM)

TESTED BY: GD DATE: 8/18/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-34
 Sample No.: 2
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	B9	
Mass of Container & Wet Specimen, M1 (g)	1004.77	
Mass of Container & Dry Specimen, M2 (g)	929.10	
Mass of Container, M3 (g)	199.74	
WATER CONTENT, wn (%)	10.37	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	802.01
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	802.01
Specimen Diameter, D (in) or ()	2.375
Specimen Length, L (in) or ()	5.842
Specimen Area, A (in^2) or ()	4.430
Specimen Volume, V (cm^3) or ()	424.04
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	118.1
DRY UNIT WEIGHT, DUW (pcf) or ()	107.0
VOID RATIO	0.576
DEGREE OF SATURATION, S (%)	48.66

MEASURED DIMENSIONS	
Length	Diameter
1 5.834	2.380
2 5.847	2.378
3 5.861	2.377
4 5.821	2.368
5 5.846	2.372
6 XXXXX	2.374
Average 5.842	2.375

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive brown Silty SAND (SM)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-35
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT <small>Major Minor</small>	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED X	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW X
3). MED. STIFF	3). MEDIUM (PI >10 to 20) X	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF X	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer		
Container No.	M66			
Mass of Container & Wet Specimen, M1 (g)	555.74			
Mass of Container & Dry Specimen, M2 (g)	469.07			
Mass of Container, M3 (g)	101.66			
WATER CONTENT, wn (%)	23.59			

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	842.00
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	842.00
Specimen Diameter, D (in) or ()	2.395
Specimen Length, L (in) or ()	5.891
Specimen Area, A (in^2) or ()	4.503
Specimen Volume, V (cm^3) or ()	434.75
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	120.9
DRY UNIT WEIGHT, DUW (pcf) or ()	97.8
VOID RATIO	0.723
DEGREE OF SATURATION, S (%)	88.10

MEASURED DIMENSIONS	
Length	Diameter
1 5.897	2.403
2 5.895	2.394
3 5.888	2.392
4 5.906	2.396
5 5.871	2.388
6 XXXXX	2.394
Average 5.891	2.395

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Grayish brown Elastic SILT (MH)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-35
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY	X		4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	X 4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M67	
Mass of Container & Wet Specimen, M1 (g)	448.90	
Mass of Container & Dry Specimen, M2 (g)	386.09	
Mass of Container, M3 (g)	110.42	
WATER CONTENT, wn (%)	22.78	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Moisture content only. Bag sample.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm^3

Specimen Color and Description: Yellowish brown Lean CLAY with Sand (CL)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-38
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE X	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S221	
Mass of Container & Wet Specimen, M1 (g)	595.10	
Mass of Container & Dry Specimen, M2 (g)	537.33	
Mass of Container, M3 (g)	140.33	
WATER CONTENT, wn (%)	14.55	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	813.35
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	813.35
Specimen Diameter, D (in) or ()	2.389
Specimen Length, L (in) or ()	5.764
Specimen Area, A (in^2) or ()	4.481
Specimen Volume, V (cm^3) or ()	423.25
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	120.0
DRY UNIT WEIGHT, DUW (pcf) or ()	104.7
VOID RATIO	0.609
DEGREE OF SATURATION, S (%)	64.46

MEASURED DIMENSIONS

	Length	Diameter
1	5.774	2.407
2	5.767	2.405
3	5.736	2.383
4	5.771	2.370
5	5.770	2.383
6	XXXXX	2.384
Average	5.764	2.389

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown Silty SAND (SM)

TESTED BY: GD DATE: 7/21/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-38
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	X 3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:	4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY		OTHER:	OTHER:
6). HARD		6). POROUS			
		7). CEMENTED			
		8). FRIABLE			

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S218	
Mass of Container & Wet Specimen, M1 (g)	487.76	
Mass of Container & Dry Specimen, M2 (g)	424.72	
Mass of Container, M3 (g)	140.43	
WATER CONTENT, wn (%)	22.17	

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS	
Length	Diameter
1	
2	
3	
4	
5	
6	XXXXXX
Average	

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Sandy SILT (ML)

TESTED BY: GD DATE: 7/21/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-40
 Sample No.: 2
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW X
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF X	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S212	
Mass of Container & Wet Specimen, M1 (g)	549.81	
Mass of Container & Dry Specimen, M2 (g)	499.64	
Mass of Container, M3 (g)	137.50	
WATER CONTENT, wn (%)	13.85	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	811.39
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	811.39
Specimen Diameter, D (in) or ()	2.384
Specimen Length, L (in) or ()	5.988
Specimen Area, A (in^2) or ()	4.464
Specimen Volume, V (cm^3) or ()	438.04
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	115.6
DRY UNIT WEIGHT, DUW (pcf) or ()	101.6
VOID RATIO	0.660
DEGREE OF SATURATION, S (%)	56.71

MEASURED DIMENSIONS	
Length	Diameter
1 5.986	2.383
2 5.991	2.388
3 5.984	2.381
4 5.989	2.390
5 5.992	2.375
6 XXXXX	2.387
Average 5.988	2.384

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Sandy SILT (ML)

TESTED BY: GD DATE: 7/21/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-40
 Sample No.: 4
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	X 3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:	4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY		OTHER:	OTHER:
6). HARD		6). POROUS			
		7). CEMENTED			
		8). FRIABLE			

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S224	
Mass of Container & Wet Specimen, M1 (g)	279.08	
Mass of Container & Dry Specimen, M2 (g)	262.94	
Mass of Container, M3 (g)	140.74	
WATER CONTENT, wn (%)	13.21	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Moisture content only. Bag sample.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	#VALUE!
VOID RATIO	#VALUE!
DEGREE OF SATURATION, S (%)	#VALUE!

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Sandy SILT (ML)

TESTED BY: GD DATE: 7/21/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-41
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT <input checked="" type="checkbox"/>			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY <input checked="" type="checkbox"/>			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED <input checked="" type="checkbox"/>	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20) <input checked="" type="checkbox"/>	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF <input checked="" type="checkbox"/>	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S303	
Mass of Container & Wet Specimen, M1 (g)	441.53	
Mass of Container & Dry Specimen, M2 (g)	401.58	
Mass of Container, M3 (g)	143.28	
WATER CONTENT, wn (%)	15.47	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Horizontal layers of yellowish brown silt and grayish brown clay.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1103.58
Mass of Container, M5 (g)	386.60
Mass of Wet Specimen, M6 (g)	716.98
Specimen Diameter, D (in) or ()	2.388
Specimen Length, L (in) or ()	5.530
Specimen Area, A (in^2) or ()	4.478
Specimen Volume, V (cm^3) or ()	405.78
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	110.3
DRY UNIT WEIGHT, DUW (pcf) or ()	95.5
VOID RATIO	0.764
DEGREE OF SATURATION, S (%)	54.63

MEASURED DIMENSIONS

	Length	Diameter
1	5.521	2.370
2	5.530	2.387
3	5.542	2.375
4	5.531	2.394
5	5.528	2.402
6	XXXXX	2.398
Average	5.530	2.388

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown to grayish brown SILT and CLAY layers (CL/ML)

TESTED BY: GD DATE: 7/21/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-41
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	X 3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S213	
Mass of Container & Wet Specimen, M1 (g)	362.37	
Mass of Container & Dry Specimen, M2 (g)	349.21	
Mass of Container, M3 (g)	137.51	
WATER CONTENT, wn (%)	6.22	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Moisture content only. Bag sample.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown Silty SAND (SM)

TESTED BY: GD DATE: 7/21/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-43
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE X	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY X			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED X	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40) X	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF X	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S301	
Mass of Container & Wet Specimen, M1 (g)	510.25	
Mass of Container & Dry Specimen, M2 (g)	471.72	
Mass of Container, M3 (g)	143.60	
WATER CONTENT, wn (%)	11.74	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1069.23
Mass of Container, M5 (g)	272.55
Mass of Wet Specimen, M6 (g)	796.68
Specimen Diameter, D (in) or ()	2.369
Specimen Length, L (in) or ()	5.664
Specimen Area, A (in^2) or ()	4.408
Specimen Volume, V (cm^3) or ()	409.19
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	121.5
DRY UNIT WEIGHT, DUW (pcf) or ()	108.8
VOID RATIO	0.550
DEGREE OF SATURATION, S (%)	57.69

MEASURED DIMENSIONS

	Length	Diameter
1	5.672	2.367
2	5.666	2.371
3	5.659	2.359
4	5.653	2.377
5	5.671	2.374
6	XXXXX	2.367
Average	5.664	2.369

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Grayish brown to yellowish brown Clayey, Silty SAND (SC-SM)

TESTED BY: GD DATE: 7/21/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-43
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	X 1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	X 1). NONE	X 1). NONE	X 1). NOT TESTED	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	X 5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S202	
Mass of Container & Wet Specimen, M1 (g)	522.96	
Mass of Container & Dry Specimen, M2 (g)	477.23	
Mass of Container, M3 (g)	138.66	
WATER CONTENT, wn (%)	13.51	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Pockets of clay material.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1139.58
Mass of Container, M5 (g)	385.08
Mass of Wet Specimen, M6 (g)	754.50
Specimen Diameter, D (in) or ()	2.384
Specimen Length, L (in) or ()	5.557
Specimen Area, A (in^2) or ()	4.465
Specimen Volume, V (cm^3) or ()	406.61
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	115.8
DRY UNIT WEIGHT, DUW (pcf) or ()	102.1
VOID RATIO	0.652
DEGREE OF SATURATION, S (%)	55.97

MEASURED DIMENSIONS

	Length	Diameter
1	5.553	2.387
2	5.564	2.379
3	5.544	2.386
4	5.559	2.390
5	5.566	2.382
6	XXXXX	2.382
Average	5.557	2.384

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light yellowish brown Sandy SILT (ML)

TESTED BY: GD DATE: 7/21/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-44
 Sample No.: 4
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND <input checked="" type="checkbox"/>		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT <input checked="" type="checkbox"/>			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED <input checked="" type="checkbox"/>	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW <input checked="" type="checkbox"/>
3). MED. STIFF	3). MEDIUM (PI >10 to 20) <input checked="" type="checkbox"/>	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF <input checked="" type="checkbox"/>	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S216	
Mass of Container & Wet Specimen, M1 (g)	371.96	
Mass of Container & Dry Specimen, M2 (g)	333.47	
Mass of Container, M3 (g)	148.62	
WATER CONTENT, wn (%)	20.82	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	743.74
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	743.74
Specimen Diameter, D (in) or ()	2.386
Specimen Length, L (in) or ()	5.766
Specimen Area, A (in^2) or ()	4.472
Specimen Volume, V (cm^3) or ()	422.52
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	109.9
DRY UNIT WEIGHT, DUW (pcf) or ()	90.9
VOID RATIO	0.853
DEGREE OF SATURATION, S (%)	65.89

MEASURED DIMENSIONS	
Length	Diameter
1 5.780	2.385
2 5.749	2.395
3 5.766	2.389
4 5.761	2.385
5 5.773	2.375
6 XXXXX	2.388
Average 5.766	2.386

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Clayey SILT (ML)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-46
 Sample No.: 2
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT X	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE X	2). GRAVEL	3). WET	3). STREAKED X	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED X	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF X	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S205	
Mass of Container & Wet Specimen, M1 (g)	464.98	
Mass of Container & Dry Specimen, M2 (g)	416.56	
Mass of Container, M3 (g)	139.35	
WATER CONTENT, wn (%)	17.47	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	833.71
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	833.71
Specimen Diameter, D (in) or ()	2.376
Specimen Length, L (in) or ()	5.898
Specimen Area, A (in^2) or ()	4.432
Specimen Volume, V (cm^3) or ()	428.33
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	121.5
DRY UNIT WEIGHT, DUW (pcf) or ()	103.4
VOID RATIO	0.629
DEGREE OF SATURATION, S (%)	74.92

MEASURED DIMENSIONS

	Length	Diameter
1	5.906	2.385
2	5.908	2.386
3	5.887	2.385
4	5.905	2.375
5	5.882	2.368
6	XXXXX	2.354
Average	5.898	2.376

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark olive brown to light olive brown Sandy SILT (ML)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-46
 Sample No.: 4
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT <small>Major Minor</small>	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY X			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT X	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW X
3). MED. STIFF	3). MEDIUM (PI >10 to 20) X	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer		
Container No.	B40			
Mass of Container & Wet Specimen, M1 (g)	672.92			
Mass of Container & Dry Specimen, M2 (g)	584.30			
Mass of Container, M3 (g)	198.40			
WATER CONTENT, wn (%)	22.96			
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10				

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1075.15
Mass of Container, M5 (g)	273.16
Mass of Wet Specimen, M6 (g)	801.99
Specimen Diameter, D (in) or ()	2.390
Specimen Length, L (in) or ()	5.555
Specimen Area, A (in^2) or ()	4.487
Specimen Volume, V (cm^3) or ()	408.43
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	122.6
DRY UNIT WEIGHT, DUW (pcf) or ()	99.7
VOID RATIO	0.691
DEGREE OF SATURATION, S (%)	89.76

MEASURED DIMENSIONS	
Length	Diameter
1 5.543	2.378
2 5.547	2.395
3 5.558	2.381
4 5.560	2.399
5 5.566	2.383
6 XXXXX	2.405
Average 5.555	2.390

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark yellowish brown Sandy, Lean CLAY (CL)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-47
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	<input checked="" type="checkbox"/>	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	<input checked="" type="checkbox"/>		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED <input checked="" type="checkbox"/>	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD <input checked="" type="checkbox"/>		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S203	
Mass of Container & Wet Specimen, M1 (g)	466.89	
Mass of Container & Dry Specimen, M2 (g)	416.57	
Mass of Container, M3 (g)	147.94	
WATER CONTENT, wn (%)	18.73	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1024.18
Mass of Container, M5 (g)	272.90
Mass of Wet Specimen, M6 (g)	751.28
Specimen Diameter, D (in) or ()	2.385
Specimen Length, L (in) or ()	5.606
Specimen Area, A (in^2) or ()	4.468
Specimen Volume, V (cm^3) or ()	410.43
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	114.3
DRY UNIT WEIGHT, DUW (pcf) or ()	96.2
VOID RATIO	0.751
DEGREE OF SATURATION, S (%)	67.32

MEASURED DIMENSIONS	
Length	Diameter
1 5.599	2.391
2 5.630	2.377
3 5.593	2.386
4 5.607	2.386
5 5.602	2.379
6 XXXXX	2.391
Average 5.606	2.385

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown SILT (ML)

TESTED BY: GD DATE: 7/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-48
 Sample No.: 2
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND <input checked="" type="checkbox"/>		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM <input checked="" type="checkbox"/>	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) <input checked="" type="checkbox"/>	1). HOMOGENEOUS <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE
2). SOFT <input checked="" type="checkbox"/>	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW <input checked="" type="checkbox"/>
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S306	
Mass of Container & Wet Specimen, M1 (g)	516.02	
Mass of Container & Dry Specimen, M2 (g)	486.33	
Mass of Container, M3 (g)	145.10	
WATER CONTENT, wn (%)	8.70	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	736.76
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	736.76
Specimen Diameter, D (in) or ()	2.376
Specimen Length, L (in) or ()	5.988
Specimen Area, A (in^2) or ()	4.433
Specimen Volume, V (cm^3) or ()	435.02
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	105.7
DRY UNIT WEIGHT, DUW (pcf) or ()	97.3
VOID RATIO	0.733
DEGREE OF SATURATION, S (%)	32.05

MEASURED DIMENSIONS	
Length	Diameter
1 5.989	2.374
2 5.989	2.378
3 5.991	2.365
4 5.988	2.375
5 5.983	2.388
6 XXXXX	2.375
Average 5.988	2.376

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Poorly Graded SAND (SP)

TESTED BY: GD DATE: 7/25/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-48
 Sample No.: 4
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S215	
Mass of Container & Wet Specimen, M1 (g)	512.65	
Mass of Container & Dry Specimen, M2 (g)	457.10	
Mass of Container, M3 (g)	137.49	
WATER CONTENT, wn (%)	17.38	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Moisture content only. Bag sample.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Sandy SILT (ML)

TESTED BY: GD DATE: 7/25/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-49
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT <small>Major Minor</small>	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED X	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X		strong brown mottled with grayish brown	3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10) X	2). STRATIFIED X	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF X	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer		
Container No.	S219			
Mass of Container & Wet Specimen, M1 (g)	479.55			
Mass of Container & Dry Specimen, M2 (g)	437.94			
Mass of Container, M3 (g)	140.70			
WATER CONTENT, wn (%)	14.00			
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10				

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	739.94
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	739.94
Specimen Diameter, D (in) or ()	2.378
Specimen Length, L (in) or ()	5.709
Specimen Area, A (in^2) or ()	4.440
Specimen Volume, V (cm^3) or ()	415.42
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	111.2
DRY UNIT WEIGHT, DUW (pcf) or ()	97.5
VOID RATIO	0.728
DEGREE OF SATURATION, S (%)	51.92

MEASURED DIMENSIONS

	Length	Diameter
1	5.713	2.386
2	5.715	2.377
3	5.707	2.370
4	5.711	2.365
5	5.701	2.383
6	XXXXX	2.385
Average	5.709	2.378

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6, g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Strong brown Silty SAND (SM)

TESTED BY: GD DATE: 7/25/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-50
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	0). BOULDERS (>12")	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR <input checked="" type="checkbox"/>
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	<input checked="" type="checkbox"/>	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY	<input checked="" type="checkbox"/>		4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW <input checked="" type="checkbox"/>
3). MED. STIFF <input checked="" type="checkbox"/>	3). MEDIUM (PI >10 to 20) <input checked="" type="checkbox"/>	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer		
Container No.	M66			
Mass of Container & Wet Specimen, M1 (g)	654.27			
Mass of Container & Dry Specimen, M2 (g)	548.87			
Mass of Container, M3 (g)	101.64			
WATER CONTENT, wn (%)	23.57			
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10				

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	905.33
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	905.33
Specimen Diameter, D (in) or ()	2.391
Specimen Length, L (in) or ()	5.976
Specimen Area, A (in^2) or ()	4.488
Specimen Volume, V (cm^3) or ()	439.54
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	128.6
DRY UNIT WEIGHT, DUW (pcf) or ()	104.1
VOID RATIO	0.620
DEGREE OF SATURATION, S (%)	102.67

MEASURED DIMENSIONS	
Length	Diameter
1 5.978	2.389
2 5.964	2.383
3 5.984	2.397
4 5.975	2.390
5 5.980	2.390
6 XXXXX	2.394
Average 5.976	2.391

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive brown Lean CLAY with Sand and Gravel (SL)

TESTED BY: GD DATE: 8/18/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-50
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	X 2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	X 2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	X 5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S101	
Mass of Container & Wet Specimen, M1 (g)	455.75	
Mass of Container & Dry Specimen, M2 (g)	396.54	
Mass of Container, M3 (g)	95.31	
WATER CONTENT, wn (%)	19.66	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

horizontally fractured layered siltstone

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	653.96
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	653.96
Specimen Diameter, D (in) or ()	2.402
Specimen Length, L (in) or ()	5.103
Specimen Area, A (in^2) or ()	4.531
Specimen Volume, V (cm^3) or ()	378.88
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	107.8
DRY UNIT WEIGHT, DUW (pcf) or ()	90.1
VOID RATIO	0.872
DEGREE OF SATURATION, S (%)	60.88

MEASURED DIMENSIONS

	Length	Diameter
1	5.112	2.395
2	5.085	2.410
3	5.118	2.418
4	5.094	2.389
5	5.106	2.397
6	XXXXX	2.402
Average	5.103	2.402

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light brownish gray SILT (ML)

TESTED BY: GD DATE: 8/18/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-52
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S217	
Mass of Container & Wet Specimen, M1 (g)	549.78	
Mass of Container & Dry Specimen, M2 (g)	485.04	
Mass of Container, M3 (g)	137.96	
WATER CONTENT, wn (%)	18.65	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Moisture content only. Bag sample.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm^3

Specimen Color and Description: Yellowish brown Sandy SILT (ML)

TESTED BY: GD DATE: 7/25/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: FM-52
 Sample No.: 4
 Depth (ft): 15.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	X 7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE	X 1). NONE	X 1). NOT TESTED	X 1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	B64	
Mass of Container & Wet Specimen, M1 (g)	685.21	
Mass of Container & Dry Specimen, M2 (g)	606.69	
Mass of Container, M3 (g)	200.61	
WATER CONTENT, wn (%)	19.34	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Moisture content only. Bag sample.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Sandy SILT (ML)

TESTED BY: GD DATE: 7/25/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 0
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: PS-3
 Sample No.: 1
 Depth (ft): 5.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM X
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S38	
Mass of Container & Wet Specimen, M1 (g)	483.98	
Mass of Container & Dry Specimen, M2 (g)	438.92	
Mass of Container, M3 (g)	123.86	
WATER CONTENT, wn (%)	14.30	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	771.32
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	771.32
Specimen Diameter, D (in) or ()	2.409
Specimen Length, L (in) or ()	5.984
Specimen Area, A (in^2) or ()	4.557
Specimen Volume, V (cm^3) or ()	446.86
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	107.8
DRY UNIT WEIGHT, DUW (pcf) or ()	94.3
VOID RATIO	0.788
DEGREE OF SATURATION, S (%)	49.01

MEASURED DIMENSIONS

	Length	Diameter
1	5.991	2.411
2	5.984	2.412
3	5.996	2.414
4	5.990	2.401
5	5.957	2.405
6	XXXXX	2.410
Average	5.984	2.409

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive brown Silty SAND (SM)

TESTED BY: ADC DATE: 6/9/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 0
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: PS-3
 Sample No.: 3
 Depth (ft): 15.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM X
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S8	
Mass of Container & Wet Specimen, M1 (g)	561.41	
Mass of Container & Dry Specimen, M2 (g)	473.42	
Mass of Container, M3 (g)	115.21	
WATER CONTENT, wn (%)	24.56	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	840.42
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	840.42
Specimen Diameter, D (in) or ()	2.421
Specimen Length, L (in) or ()	5.876
Specimen Area, A (in^2) or ()	4.602
Specimen Volume, V (cm^3) or ()	443.08
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	118.4
DRY UNIT WEIGHT, DUW (pcf) or ()	95.1
VOID RATIO	0.773
DEGREE OF SATURATION, S (%)	85.78

MEASURED DIMENSIONS

	Length	Diameter
1	5.863	2.405
2	5.901	2.432
3	5.851	2.427
4	5.890	2.400
5	5.875	2.433
6	XXXXX	2.426
Average	5.876	2.421

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Poorly Graded SAND (SP)

TESTED BY: ADC DATE: 6/9/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 0
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: PS-3
 Sample No.: 7
 Depth (ft): 40.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM X
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S9	
Mass of Container & Wet Specimen, M1 (g)	530.45	
Mass of Container & Dry Specimen, M2 (g)	419.69	
Mass of Container, M3 (g)	87.90	
WATER CONTENT, wn (%)	33.38	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	857.65
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	857.65
Specimen Diameter, D (in) or ()	2.422
Specimen Length, L (in) or ()	5.948
Specimen Area, A (in^2) or ()	4.608
Specimen Volume, V (cm^3) or ()	449.16
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	119.2
DRY UNIT WEIGHT, DUW (pcf) or ()	89.4
VOID RATIO	0.886
DEGREE OF SATURATION, S (%)	101.72

MEASURED DIMENSIONS

	Length	Diameter
1	5.858	2.421
2	5.956	2.421
3	5.946	2.427
4	5.994	2.422
5	5.984	2.423
6	XXXXX	2.420
Average	5.948	2.422

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Greenish black Sandy SILT (ML)

TESTED BY: ADC DATE: 6/9/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-01
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY X	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF X	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM X
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S203	
Mass of Container & Wet Specimen, M1 (g)	555.94	
Mass of Container & Dry Specimen, M2 (g)	514.83	
Mass of Container, M3 (g)	147.95	
WATER CONTENT, wn (%)	11.21	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	713.59
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	713.59
Specimen Diameter, D (in) or ()	2.379
Specimen Length, L (in) or ()	6.033
Specimen Area, A (in^2) or ()	4.445
Specimen Volume, V (cm^3) or ()	439.45
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	101.4
DRY UNIT WEIGHT, DUW (pcf) or ()	91.2
VOID RATIO	0.849
DEGREE OF SATURATION, S (%)	35.63

MEASURED DIMENSIONS

	Length	Diameter
1	6.032	2.378
2	6.031	2.370
3	6.032	2.383
4	6.034	2.380
5	6.036	2.384
6	XXXXX	2.379
Average	6.033	2.379

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Very dark brown Silty SAND (SM)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
Project Name: Pure Water
Project Engineer: SF

Boring No.: WM-02
Sample No.: 1
Depth (ft): 7.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF X	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM X
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S220	
Mass of Container & Wet Specimen, M1 (g)	768.78	
Mass of Container & Dry Specimen, M2 (g)	671.95	
Mass of Container, M3 (g)	140.53	
WATER CONTENT, wn (%)	18.22	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	846.11
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	846.11
Specimen Diameter, D (in) or ()	2.382
Specimen Length, L (in) or ()	5.917
Specimen Area, A (in^2) or ()	4.457
Specimen Volume, V (cm^3) or ()	432.14
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	122.2
DRY UNIT WEIGHT, DUW (pcf) or ()	103.4
VOID RATIO	0.630
DEGREE OF SATURATION, S (%)	78.06

MEASURED DIMENSIONS

	Length	Diameter
1	5.986	2.381
2	5.858	2.381
3	5.913	2.387
4	5.948	2.377
5	5.879	2.384
6	XXXXX	2.383
Average	5.917	2.382

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Very dark brown Sandy SILT (ML)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-02
 Sample No.: 3
 Depth (ft): 15.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	X 3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT	X		3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	X 1). HOMOGENEOUS	X 1). NONE	X 1). NONE	X 1). NOT TESTED	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S210	
Mass of Container & Wet Specimen, M1 (g)	682.76	
Mass of Container & Dry Specimen, M2 (g)	584.65	
Mass of Container, M3 (g)	147.23	
WATER CONTENT, wn (%)	22.43	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

No unit weight due to excessive disturbance.

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	
Mass of Container, M5 (g)	
Mass of Wet Specimen, M6 (g)	
Specimen Diameter, D (in) or ()	
Specimen Length, L (in) or ()	
Specimen Area, A (in^2) or ()	
Specimen Volume, V (cm^3) or ()	
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	
DRY UNIT WEIGHT, DUW (pcf) or ()	
VOID RATIO	
DEGREE OF SATURATION, S (%)	

MEASURED DIMENSIONS

	Length	Diameter
1		
2		
3		
4		
5		
6	XXXXXX	
Average		

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive Poorly Graded SAND with Silt (SP-SM)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-03
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR <input checked="" type="checkbox"/>
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND <input checked="" type="checkbox"/>		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT <input checked="" type="checkbox"/>			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED <input checked="" type="checkbox"/>	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF <input checked="" type="checkbox"/>	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM <input checked="" type="checkbox"/>
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer		
Container No.	M3			
Mass of Container & Wet Specimen, M1 (g)	886.87			
Mass of Container & Dry Specimen, M2 (g)	724.07			
Mass of Container, M3 (g)	89.90			
WATER CONTENT, wn (%)	25.67			

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	849.61
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	849.61
Specimen Diameter, D (in) or ()	2.404
Specimen Length, L (in) or ()	5.981
Specimen Area, A (in^2) or ()	4.537
Specimen Volume, V (cm^3) or ()	444.66
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	119.3
DRY UNIT WEIGHT, DUW (pcf) or ()	94.9
VOID RATIO	0.776
DEGREE OF SATURATION, S (%)	89.34

MEASURED DIMENSIONS

	Length	Diameter
1	5.971	2.402
2	5.978	2.393
3	5.978	2.393
4	5.984	2.407
5	5.992	2.413
6	XXXXX	2.413
Average	5.981	2.404

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark olive brown Silty SAND (SM)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-06
 Sample No.: 3b
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT Major Minor	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE	0). BOULDERS (>12")	1). DRY	1). UNIFORM	X GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	X 1). COBBLES (>3")	2). MOIST	X 2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	X 3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY	X		4). MEDIUM	X 6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	X 1). NONE	X 1). NONE	X 1). NOT TESTED	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	X 2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	X 4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S21	
Mass of Container & Wet Specimen, M1 (g)	878.87	
Mass of Container & Dry Specimen, M2 (g)	846.00	
Mass of Container, M3 (g)	104.82	
WATER CONTENT, wn (%)	4.43	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

Rock approximately > 2.5" diameter x 2" high

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1209.18
Mass of Container, M5 (g)	278.77
Mass of Wet Specimen, M6 (g)	930.41
Specimen Diameter, D (in) or ()	2.398
Specimen Length, L (in) or ()	5.906
Specimen Area, A (in^2) or ()	4.516
Specimen Volume, V (cm^3) or ()	437.13
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	132.9
DRY UNIT WEIGHT, DUW (pcf) or ()	127.2
VOID RATIO	0.325
DEGREE OF SATURATION, S (%)	36.87

MEASURED DIMENSIONS

	Length	Diameter
1	5.741	2.392
2	5.992	2.400
3	6.015	2.395
4	5.944	2.404
5	5.840	2.410
6	XXXXX	2.387
Average	5.906	2.398

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive gray Clayey SAND (SC)

TESTED BY: ADC DATE: 6/13/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-06
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE X	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	X	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY X			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF	3). MEDIUM (PI >10 to 20) X	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD X		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	S41	
Mass of Container & Wet Specimen, M1 (g)	680.29	
Mass of Container & Dry Specimen, M2 (g)	600.39	
Mass of Container, M3 (g)	82.88	
WATER CONTENT, wn (%)	15.44	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	972.46
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	972.46
Specimen Diameter, D (in) or ()	2.394
Specimen Length, L (in) or ()	5.963
Specimen Area, A (in^2) or ()	4.500
Specimen Volume, V (cm^3) or ()	439.73
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	138.1
DRY UNIT WEIGHT, DUW (pcf) or ()	119.6
VOID RATIO	0.409
DEGREE OF SATURATION, S (%)	101.82

MEASURED DIMENSIONS	
Length	Diameter
1 5.969	2.395
2 5.945	2.392
3 5.964	2.392
4 5.971	2.397
5 5.966	2.394
6 XXXXX	2.392
Average 5.963	2.394

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive brown to dark yellowish brown Sandy, Lean CLAY (CL)

TESTED BY: ADC DATE: 6/13/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 0
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-07
 Sample No.: 3
 Depth (ft): 15.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	<input checked="" type="checkbox"/>	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY	<input checked="" type="checkbox"/>		4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED <input checked="" type="checkbox"/>	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW <input checked="" type="checkbox"/>
3). MED. STIFF	3). MEDIUM (PI >10 to 20) <input checked="" type="checkbox"/>	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF <input checked="" type="checkbox"/>	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	C8	
Mass of Container & Wet Specimen, M1 (g)	680.03	
Mass of Container & Dry Specimen, M2 (g)	581.89	
Mass of Container, M3 (g)	71.56	
WATER CONTENT, wn (%)	19.23	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

some oxidized seams

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	907.93
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	907.93
Specimen Diameter, D (in) or ()	2.387
Specimen Length, L (in) or ()	5.959
Specimen Area, A (in^2) or ()	4.473
Specimen Volume, V (cm^3) or ()	436.83
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	129.8
DRY UNIT WEIGHT, DUW (pcf) or ()	108.8
VOID RATIO	0.549
DEGREE OF SATURATION, S (%)	94.60

MEASURED DIMENSIONS

	Length	Diameter
1	5.966	2.377
2	5.948	2.389
3	5.962	2.381
4	5.962	2.386
5	5.959	2.396
6	XXXXX	2.390
Average	5.959	2.387

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Gray to dark grayish brown Lean CLAY with Sand (CL)

TESTED BY: ADC DATE: 6/13/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-10
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	Major Minor	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR <input checked="" type="checkbox"/>
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND <input checked="" type="checkbox"/>		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY <input checked="" type="checkbox"/>			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCI REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW <input checked="" type="checkbox"/>
3). MED. STIFF <input checked="" type="checkbox"/>	3). MEDIUM (PI >10 to 20) <input checked="" type="checkbox"/>	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	M62	
Mass of Container & Wet Specimen, M1 (g)	780.17	
Mass of Container & Dry Specimen, M2 (g)	700.90	
Mass of Container, M3 (g)	91.69	
WATER CONTENT, wn (%)	13.01	

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	873.70
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	873.70
Specimen Diameter, D (in) or ()	2.384
Specimen Length, L (in) or ()	5.959
Specimen Area, A (in^2) or ()	4.464
Specimen Volume, V (cm^3) or ()	435.90
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	125.1
DRY UNIT WEIGHT, DUW (pcf) or ()	110.7
VOID RATIO	0.522
DEGREE OF SATURATION, S (%)	67.25

MEASURED DIMENSIONS

	Length	Diameter
1	5.950	2.381
2	5.951	2.382
3	5.982	2.386
4	5.924	2.381
5	5.989	2.390
6	XXXXX	2.384
Average	5.959	2.384

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark brown to reddish brown Clayey SAND (SC)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-11
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT <small>Major Minor</small>	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY X			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW X
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF X	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer		
Container No.	M9			
Mass of Container & Wet Specimen, M1 (g)	755.40			
Mass of Container & Dry Specimen, M2 (g)	675.48			
Mass of Container, M3 (g)	89.74			
WATER CONTENT, wn (%)	13.64			

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	923.49
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	923.49
Specimen Diameter, D (in) or ()	2.386
Specimen Length, L (in) or ()	5.974
Specimen Area, A (in^2) or ()	4.471
Specimen Volume, V (cm^3) or ()	437.64
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	131.7
DRY UNIT WEIGHT, DUW (pcf) or ()	115.9
VOID RATIO	0.454
DEGREE OF SATURATION, S (%)	81.12

MEASURED DIMENSIONS	
Length	Diameter
1 5.982	2.388
2 5.940	2.382
3 5.980	2.390
4 5.982	2.383
5 5.985	2.383
6 XXXXX	2.389
Average 5.974	2.386

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Dark yellowish brown Clayey SAND (SC)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-11
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR <input checked="" type="checkbox"/>
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	<input checked="" type="checkbox"/>	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY <input checked="" type="checkbox"/>			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW <input checked="" type="checkbox"/>
3). MED. STIFF	3). MEDIUM (PI >10 to 20) <input checked="" type="checkbox"/>	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF <input checked="" type="checkbox"/>	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	N5	
Mass of Container & Wet Specimen, M1 (g)	606.00	
Mass of Container & Dry Specimen, M2 (g)	510.49	
Mass of Container, M3 (g)	104.63	
WATER CONTENT, wn (%)	23.53	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	909.02
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	909.02
Specimen Diameter, D (in) or ()	2.387
Specimen Length, L (in) or ()	5.936
Specimen Area, A (in^2) or ()	4.473
Specimen Volume, V (cm^3) or ()	435.09
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	130.4
DRY UNIT WEIGHT, DUW (pcf) or ()	105.6
VOID RATIO	0.596
DEGREE OF SATURATION, S (%)	106.53

MEASURED DIMENSIONS	
Length	Diameter
1 5.986	2.380
2 5.894	2.383
3 5.917	2.382
4 5.973	2.394
5 5.908	2.388
6 XXXXX	2.392
Average 5.936	2.387

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive Lean CLAY with Sand (CL)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-16
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE <input checked="" type="checkbox"/>	1). BOULDERS (>12")	1). DRY	1). UNIFORM <input checked="" type="checkbox"/>	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST <input checked="" type="checkbox"/>	2). SPOTTED	1). COARSE	2). SUBANGULAR <input checked="" type="checkbox"/>
3). CALIFORNIA SLEEVE	3). MODERATE <input checked="" type="checkbox"/>	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND	<input checked="" type="checkbox"/>	OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY <input checked="" type="checkbox"/>			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE <input checked="" type="checkbox"/>	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NONE <input checked="" type="checkbox"/>	1). NOT TESTED <input checked="" type="checkbox"/>	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED <input checked="" type="checkbox"/>	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW <input checked="" type="checkbox"/>
3). MED. STIFF	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF <input checked="" type="checkbox"/>	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	X2	
Mass of Container & Wet Specimen, M1 (g)	894.84	
Mass of Container & Dry Specimen, M2 (g)	807.50	
Mass of Container, M3 (g)	147.90	
WATER CONTENT, wn (%)	13.24	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	964.09
Mass of Container, M5 (g)	0.00
Mass of Wet Specimen, M6 (g)	964.09
Specimen Diameter, D (in) or ()	2.392
Specimen Length, L (in) or ()	5.998
Specimen Area, A (in^2) or ()	4.495
Specimen Volume, V (cm^3) or ()	441.84
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	136.2
DRY UNIT WEIGHT, DUW (pcf) or ()	120.3
VOID RATIO	0.401
DEGREE OF SATURATION, S (%)	89.09

MEASURED DIMENSIONS	
Length	Diameter
1 5.989	2.380
2 6.004	2.379
3 5.999	2.382
4 6.002	2.404
5 5.998	2.407
6 XXXXX	2.402
Average 5.998	2.392

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Yellowish brown Sandy, Lean CLAY (CL)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-16
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	1). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM X	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0) X	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW X
3). MED. STIFF X	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	X9	
Mass of Container & Wet Specimen, M1 (g)	1057.04	
Mass of Container & Dry Specimen, M2 (g)	926.20	
Mass of Container, M3 (g)	134.45	
WATER CONTENT, wn (%)	16.53	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1210.38
Mass of Container, M5 (g)	280.49
Mass of Wet Specimen, M6 (g)	929.89
Specimen Diameter, D (in) or ()	2.388
Specimen Length, L (in) or ()	5.854
Specimen Area, A (in^2) or ()	4.479
Specimen Volume, V (cm^3) or ()	429.68
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	135.1
DRY UNIT WEIGHT, DUW (pcf) or ()	115.9
VOID RATIO	0.454
DEGREE OF SATURATION, S (%)	98.33

MEASURED DIMENSIONS	
Length	Diameter
1 5.942	2.381
2 5.746	2.382
3 5.955	2.387
4 5.831	2.388
5 5.798	2.392
6 XXXXX	2.398
Average 5.854	2.388

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Olive Poorly Graded SAND with Silt (SP-SM)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-22
 Sample No.: 3
 Depth (ft): 10.0

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST X	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY X			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF X	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	X3	
Mass of Container & Wet Specimen, M1 (g)	997.00	
Mass of Container & Dry Specimen, M2 (g)	858.00	
Mass of Container, M3 (g)	138.05	
WATER CONTENT, wn (%)	19.31	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1075.57
Mass of Container, M5 (g)	209.88
Mass of Wet Specimen, M6 (g)	865.69
Specimen Diameter, D (in) or ()	2.411
Specimen Length, L (in) or ()	5.921
Specimen Area, A (in^2) or ()	4.565
Specimen Volume, V (cm^3) or ()	442.95
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	122.0
DRY UNIT WEIGHT, DUW (pcf) or ()	102.3
VOID RATIO	0.648
DEGREE OF SATURATION, S (%)	80.42

MEASURED DIMENSIONS

	Length	Diameter
1	5.967	2.412
2	5.910	2.410
3	5.954	2.407
4	5.828	2.414
5	5.948	2.412
6	XXXXX	2.410
Average	5.921	2.411

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Brownish yellow Sandy, Clayey SILT (CL-ML)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

WATER CONTENT, UNIT WEIGHT, AND VISUAL DESCRIPTION

ASTM D2216, D7263, D2488

Project Number: 60530732 Task Number: 2.01.05.20.00
 Project Name: Pure Water
 Project Engineer: SF

Boring No.: WM-24
 Sample No.: 5
 Depth (ft): 18.5

TYPE OF SAMPLE	VISUAL SIGNS OF DISTURBANCE (1)	SOIL COMPONENT	WATER CONTENT ADJECTIVES	COLOR	GRADATION	SHAPE OF GRAINS
1). BULK	1). NONE X	0). BOULDERS (>12")	1). DRY X	1). UNIFORM X	GRAVEL	1). ANGULAR
2). SPT	2). SLIGHT	1). COBBLES (>3")	2). MOIST	2). SPOTTED	1). COARSE	2). SUBANGULAR X
3). CALIFORNIA SLEEVE X	3). MODERATE	2). GRAVEL	3). WET	3). STREAKED	2). FINE	3). ROUNDED
4). CORE	4). EXCESSIVE	3). SAND X		OTHER:	SAND	4). SUBROUNDED
5). RINGS		4). SILT X			3). COARSE	5). FLAT
6). THIN WALL TUBE		5). CLAY			4). MEDIUM	6). ELONGATED
7). RECONSTITUTED		6). ORGANIC			5). FINE X	7). FLAT & ELONG.
		7). PEAT				
		OTHER:				

CONSISTENCY (MANUAL)	PLASTICITY ADJECTIVES	STRUCTURE	ORGANIC APPEARANCE	ODOR	HCl REACTION	MICA CONTENT
1). VERY SOFT	1). NON-PLASTIC (PI = 0)	1). HOMOGENEOUS X	1). NONE X	1). NONE X	1). NOT TESTED X	1). NONE X
2). SOFT	2). LOW (PI = 1 to 10)	2). STRATIFIED	2). FIBROUS	2). ORGANIC	2). NONE	2). LOW
3). MED. STIFF X	3). MEDIUM (PI >10 to 20)	3). FISSURED	3). DECOMPOSED	3). FUEL	3). WEAK	3). MEDIUM
4). STIFF	4). HIGH (PI >20 to 40)	4). SLICKENSIDED	OTHER:		4). STRONG	4). HIGH
5). VERY STIFF	5). VERY PLASTIC (PI >40)	5). BLOCKY			OTHER:	OTHER:
6). HARD		6). POROUS				
		7). CEMENTED				
		8). FRIABLE				

AS-RECEIVED WATER CONTENT (OVEN DRIED)

	Average	Sub-Specimen / Sub-Layer
Container No.	SNA1	
Mass of Container & Wet Specimen, M1 (g)	923.94	
Mass of Container & Dry Specimen, M2 (g)	894.10	
Mass of Container, M3 (g)	145.67	
WATER CONTENT, wn (%)	3.99	
Circle Approximate Max. Grain Size in "Sample" 3" 1-1/2" 3/4" 3/8" #4 #10 <#10		

Remarks

UNIT WEIGHT

Container No.	
Mass of Container and Wet Specimen, M4 (g)	1065.06
Mass of Container, M5 (g)	131.44
Mass of Wet Specimen, M6 (g)	933.62
Specimen Diameter, D (in) or ()	2.412
Specimen Length, L (in) or ()	5.626
Specimen Area, A (in^2) or ()	4.568
Specimen Volume, V (cm^3) or ()	421.12
assumed Specific Gravity, Gs	2.70
WET UNIT WEIGHT, WUW (pcf) or ()	138.4
DRY UNIT WEIGHT, DUW (pcf) or ()	133.1
VOID RATIO	0.266
DEGREE OF SATURATION, S (%)	40.40

MEASURED DIMENSIONS	
Length	Diameter
1 5.515	2.410
2 5.620	2.408
3 5.532	2.409
4 5.795	2.414
5 5.667	2.412
6 XXXXX	2.417
Average 5.626	2.412

This Excel spreadsheet is used to verify calculations.

$$w \text{ or } wn = ((M1 - M2) / (M2 - M3)) * 100$$

$$WUW = (M6.g) / (V, cm^3) * (62.43, lb/ft^3) \quad DUW = WUW / (1 + (w / 100))$$

$$S = (DUW * w * Gs) / ((Gs * \text{Unit Weight or Density of Water}) - DUW)$$

Unit Weight or Density of Water (20° C) = 62.32 pcf or 0.9982 g/cm³

Specimen Color and Description: Light olive brown Silty SAND (SM)

TESTED BY: ADC DATE: 6/19/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: DS-01

Project Name: Pure Water

Sample No.: 4a

Project Engineer: SF

Depth (ft): 15.0

Visual Description: Dark yellowish brown Clayey SAND (SC)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒ Method A ☒ not dispersed
Air Dried ☐ Method B ☐ is dispersed
Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐ ☐
Fine Fraction Washed on No. 200 sieve? ☐ ☐

By: ☒ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	LF3
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	325.09
Container Number		LF3	LF3	Dry, M2 (g)	305.44
Mass of Container and Dry Soil, (g)		305.44	268.89	Cont.,M3 (g)	205.15
Mass of Container, (g)		205.15	205.15	Water Content (%)	19.59
Dry Soil, Ws (g)		100.29	63.74		

SIEVING RESULTS

% error: 0.06

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	26.07		74.0
#60 / 60			
#100 / 40	49		51.1
#140 / 30			
#200 / 20	62.85		37.3
Pan	63.78	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 37.3 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 08/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: DS-01

Project Name: Pure Water

Sample No.: 7

Project Engineer: SF

Depth (ft): 30.0

Visual Description: Dark olive gray Silty SAND (SM)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
 SPT Sample ☒ Thin-Walled Tube ☒
 Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
 Sieves (1) - partial sample used & selected by Method(s)
 Method
 (a): Splitter; (use for dry soils or that which will segregate)
 (b): Quartering; (use for dry soils or that which will segregate)
 (c) : Representative scoop after mixing, or slice of intact sample.
 (use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒ Method A ☒ not dispersed
 Air Dried ☐ Method B ☐ is dispersed
 Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve ? ☐ No ☒ Yes
 Retained Fraction: 1st Split Washed ? ☐ No ☐ Yes
 Fine Fraction Washed on No. 200 sieve ? ☐ No ☐ Yes

By: ☒ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	747.61
Container Number		L19	L19	Dry, M2 (g)	614.13
Mass of Container and Dry Soil, (g)		614.13	552.32	Cont.,M3 (g)	134.87
Mass of Container, (g)		134.87	134.9	Water Content (%)	27.85
Dry Soil, Ws (g)		479.26	417.42		

SIEVING RESULTS

% error: 0.04

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	58.34		87.8
#60 / 60			
#100 / 40	329.17		31.3
#140 / 30			
#200 / 20	412.6		13.9
Pan	417.27	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
 % GRAVEL _____ D30 XXXXX D15 XXXXX
 % SAND _____ D10 XXXXX D50 XXXXX
 % FINES 13.9 Cu = XXXXX Cc = XXXXX
 Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 08/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: DS-02

Project Name: Pure Water

Sample No.: 2

Project Engineer: SF

Depth (ft): 10.0

Visual Description: Very dark gray Poorly Graded SAND with Silt (SP-SM)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

No ☒
Yes ☐

By: ☐ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B59
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	721.76
Container Number		B59	B59	Dry, M2 (g)	549.9
Mass of Container and Dry Soil, (g)		549.9	513.44	Cont.,M3 (g)	200.13
Mass of Container, (g)		200.13	200.18	Water Content (%)	49.14
Dry Soil, Ws (g)		349.77	313.26		

SIEVING RESULTS

% error: 0.03

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	25.62		92.7
#60 / 60			
#100 / 40	240.01		31.4
#140 / 30			
#200 / 20	308.21		11.9
Pan	313.34	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 11.9 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☒ Yes Amount Adjective: medium

SET-UP BY: GD DATE: 08/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: DS-03

Project Name: Pure Water

Sample No.: 7

Project Engineer: SF

Depth (ft): 30.0

Visual Description: Dark gray Clayey SAND (SC)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

☐ Whole sample used

☐ See Bulk Sample Processing Form

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

No ☒ By: ☐ Mortar & Pestle ☐ Hand
Yes ☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B96
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	713.97
Container Number		B96	B96	Dry, M2 (g)	584.22
Mass of Container and Dry Soil, (g)		584.22	427.6	Cont.,M3 (g)	204.67
Mass of Container, (g)		204.67	204.72	Water Content (%)	34.19
Dry Soil, Ws (g)		379.55	222.88		

SIEVING RESULTS

% error: 0.04

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	15.89		95.8
#60 / 60			
#100 / 40	163.67		56.9
#140 / 30			
#200 / 20	219.14		42.3
Pan	222.8	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 42.3 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30^2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☒ Yes Amount Adjective: medium

SET-UP BY: GD DATE: 08/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: DS-06

Project Name: Pure Water

Sample No.: 1

Project Engineer: SF

Depth (ft): 0~5

Visual Description: Dark grayish brown Silty SAND (SM)

SPECIMEN: Tested From:

Bulk Sample ☒ Other ☐
SPT Sample ☐ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

No ☒ Yes ☐
By: ☐ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B55
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	806.97
Container Number		B55	B55	Dry, M2 (g)	742
Mass of Container and Dry Soil, (g)		742	533.41	Cont.,M3 (g)	197.9
Mass of Container, (g)		197.9	197.96	Water Content (%)	11.94
Dry Soil, Ws (g)		544.1	335.45		

SIEVING RESULTS

% error: 0.03

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	113.13		79.2
#60 / 60			
#100 / 40	258.38		52.5
#140 / 30			
#200 / 20	331.91		39.0
Pan	335.34	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 39.0 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 08/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: DS-06

Project Name: Pure Water

Sample No.: 4

Project Engineer: SF

Depth (ft): 15.0

Visual Description: Dark grayish brown Poorly Graded SAND with Silt (SP-SM)

SPECIMEN: Tested From:

Selection Method:

Bulk Sample ☐ Other _____
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method

☐ Whole sample used
☐ See Bulk Sample Processing Form

(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c) : Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒ Method A ☒ not dispersed
Air Dried ☐ Method B ☐ is dispersed
Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve ? ☐ No ☒ Yes
Retained Fraction: 1st Split Washed ? ☐ No ☐ Yes
Fine Fraction Washed on No. 200 sieve ? ☐ No ☐ Yes

By: ☒ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B10
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	789.84
Container Number		B10	B10	Dry, M2 (g)	650.79
Mass of Container and Dry Soil, (g)		650.79	618.75	Cont.,M3 (g)	197.98
Mass of Container, (g)		197.98	198.01	Water Content (%)	30.71
Dry Soil, Ws (g)		452.81	420.74		

SIEVING RESULTS

% error: 0.02

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	19.34		95.7
#60 / 60			
#100 / 40	346.71		23.4
#140 / 30			
#200 / 20	416.9		7.9
Pan	420.64	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 7.9 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 08/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-01

Project Name: Pure Water

Sample No.: 1

Project Engineer: SF

Depth (ft): 0-5

Visual Description: Olive brown Silty SAND (SM)

SPECIMEN: Tested From:

Bulk Sample ☒ Other ☐
SPT Sample ☐ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒ Method A ☒ not dispersed
Air Dried ☐ Method B ☐ is dispersed
Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? No ☐ Yes ☐
Fine Fraction Washed on No. 200 sieve? No ☐ Yes ☐

By: ☒ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B38
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	794.35
Container Number		B38	B38	Dry, M2 (g)	735.41
Mass of Container and Dry Soil, (g)		735.41	575.58	Cont.,M3 (g)	200.34
Mass of Container, (g)		200.34	200.34	Water Content (%)	11.02
Dry Soil, Ws (g)		535.07	375.24		

SIEVING RESULTS

% error: 0.08

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	49.36		90.8
#60 / 60			
#100 / 40	252.7		52.8
#140 / 30			
#200 / 20	368.57		31.1
Pan	374.94	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 31.1 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30^2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 05/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-01

Project Name: Pure Water

Sample No.: 5

Project Engineer: SF

Depth (ft): 18.5

Visual Description: Dark gray Poorly Graded SAND with Silt (SP-SM)

SPECIMEN: Tested From:

Selection Method:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method

☐ Whole sample used

☐ See Bulk Sample Processing Form

(a): Splitter; (use for dry soils or that which will segregate)

(b): Quartering; (use for dry soils or that which will segregate)

(c): Representative scoop after mixing, or slice of intact sample.

(use for moist soils or that which will not segregate)

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒

Method A ☒ not dispersed

Air Dried ☐

Method B ☐ is dispersed

Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve ? No ☐ Yes ☒

Retained Fraction: 1st Split Washed ? No ☐ Yes ☐

Fine Fraction Washed on No. 200 sieve ? No ☐ Yes ☐

No ☒

Yes ☐

By: ☐ Mortar & Pestle ☐ Hand

☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B62
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	427.75
Container Number		B62	B62	Dry, M2 (g)	386.41
Mass of Container and Dry Soil, (g)		386.41	376.45	Cont.,M3 (g)	200.38
Mass of Container, (g)		200.38	200.38	Water Content (%)	22.22
Dry Soil, Ws (g)		186.03	176.07		

SIEVING RESULTS

% error: 0.02

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	66.53		64.2
#60 / 60			
#100 / 40	162.94		12.4
#140 / 30			
#200 / 20	175.59		5.6
Pan	176.03	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 5.6 Cu = XXXXX Cc = XXXXX

Cu = D60 / D10 Cc = D30^2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 05/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-02

Project Name: Pure Water

Sample No.: 4

Project Engineer: SF

Depth (ft): 15.0

Visual Description: Dark gray Silty SAND (SM)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

No ☒ Yes ☐
By: ☐ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐ ☐
Fine Fraction Washed on No. 200 sieve? ☐ ☐

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B68
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	597.16
Container Number		B68	B68	Dry, M2 (g)	506.7
Mass of Container and Dry Soil, (g)		506.7	472.41	Cont.,M3 (g)	200.19
Mass of Container, (g)		200.19	200.19	Water Content (%)	29.51
Dry Soil, Ws (g)		306.51	272.22		

SIEVING RESULTS

% error: 0.10

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	30.16		90.2
#60 / 60			
#100 / 40	219.54		28.4
#140 / 30			
#200 / 20	269.53		12.1
Pan	271.96	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 12.1 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☒ Yes Amount Adjective: high

SET-UP BY: GD DATE: 05/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-03

Project Name: Pure Water

Sample No.: 2

Project Engineer: SF

Depth (ft): 5.0

Visual Description: Olive brown Poorly Graded SAND with Silt (SP-SM)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

By: ☒ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B35
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	456.66
Container Number		B35	B35	Dry, M2 (g)	448.51
Mass of Container and Dry Soil, (g)		448.51	422.15	Cont.,M3 (g)	198.11
Mass of Container, (g)		198.11	198.11	Water Content (%)	3.25
Dry Soil, Ws (g)		250.4	224.04		

SIEVING RESULTS

% error: 0.08

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	26.42		89.4
#60 / 60			
#100 / 40	190.68		23.8
#140 / 30			
#200 / 20	223.01		10.9
Pan	223.86	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 10.9 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 05/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-04

Project Name: Pure Water

Sample No.: 1

Project Engineer: SF

Depth (ft): 10.0

Visual Description: Strong brown Silty SAND (SM)

SPECIMEN: Tested From:

Bulk Sample ☐
SPT Sample ☒
Calif. Sample ☐

Other ☐ ☒
Thin-Walled Tube ☐
Engr. Test Specimen's WC ☐

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c) : Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

No ☒ Yes ☐
By: ☐ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

Washing:

Whole Specimen Washed on No. 200 sieve ? No ☐ Yes ☒
Retained Fraction: 1st Split Washed ? ☐ ☐
Fine Fraction Washed on No. 200 sieve ? ☐ ☐

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B57
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	393.24
Container Number		B57	B57	Dry, M2 (g)	378.2
Mass of Container and Dry Soil, (g)		378.2	311.51	Cont.,M3 (g)	200.54
Mass of Container, (g)		200.54	200.54	Water Content (%)	8.47
Dry Soil, Ws (g)		177.66	110.97		

SIEVING RESULTS

% error: 0.01

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	58.78		66.9
#60 / 60			
#100 / 40	85.1		52.1
#140 / 30			
#200 / 20	109.45		38.4
Pan	110.96	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 38.4 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 05/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-06

Project Name: Pure Water

Sample No.: 5

Project Engineer: SF

Depth (ft): 18.5

Visual Description: Yellowish brown Well Graded SAND with Gravel (SW)

SPECIMEN: Tested From:

Selection Method:

Bulk Sample ☐

Other ☐

☒ Sieves (1) - whole sample used

SPT Sample ☒

Thin-Walled Tube ☐

Sieves (1) - partial sample used & selected by Method(s)

Calif. Sample ☐

Engr. Test Specimen's WC ☐

Method

☐ Whole sample used

☐ See Bulk Sample Processing Form

(a): Splitter; (use for dry soils or that which will segregate)

(b): Quartering; (use for dry soils or that which will segregate)

(c): Representative scoop after mixing, or slice of intact sample.

(use for moist soils or that which will not segregate)

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒

Method A ☒ not dispersed

Air Dried ☐

Method B ☐ is dispersed

Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve ? ☐ No ☒ Yes

Retained Fraction: 1st Split Washed ? ☐ No ☐ Yes

Fine Fraction Washed on No. 200 sieve ? ☐ No ☐ Yes

No ☒

Yes ☐

By: ☐ Mortar & Pestle ☐ Hand

☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B83
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	798.98
Container Number		B83	B83	Dry, M2 (g)	704.15
Mass of Container and Dry Soil, (g)		704.15	701.04	Cont.,M3 (g)	205.44
Mass of Container, (g)		205.44	205.44	Water Content (%)	19.02
Dry Soil, Ws (g)		498.71	495.6		

SIEVING RESULTS

% error: 0.03

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	453.85		9.0
#60 / 60			
#100 / 40	492.36		1.3
#140 / 30			
#200 / 20	495.38		0.7
Pan	495.46	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES ☐ D60 ☐ D85 ☐

% GRAVEL ☐ D30 ☐ D15 ☐

% SAND ☐ D10 ☐ D50 ☐

% FINES 0.7 Cu = ☐ Cc = ☐

Cu = D60 / D10 Cc = D30^2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☐ Yes Amount Adjective: ☐

SET-UP BY: GD DATE: 05/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-07

Project Name: Pure Water

Sample No.: 1

Project Engineer: SF

Depth (ft): 0-5

Visual Description: Dark yellowish brown Silty SAND (SM)

SPECIMEN: Tested From:

Bulk Sample ☒ Other ☐
SPT Sample ☐ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒ Method A ☒ not dispersed
Air Dried ☐ Method B ☐ is dispersed
Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐ ☐
Fine Fraction Washed on No. 200 sieve? ☐ ☐

No ☒ By: ☐ Mortar & Pestle ☐ Hand
Yes ☐ ☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B56
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	655.56
Container Number		B56	B56	Dry, M2 (g)	595.33
Mass of Container and Dry Soil, (g)		595.33	421.66	Cont.,M3 (g)	197.66
Mass of Container, (g)		197.66	197.66	Water Content (%)	15.15
Dry Soil, Ws (g)		397.67	224		

SIEVING RESULTS

% error: 0.00

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	47.08		88.2
#60 / 60			
#100 / 40	135.28		66.0
#140 / 30			
#200 / 20	219.16		44.9
Pan	223.99	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 44.9 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 05/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-07

Project Name: Pure Water

Sample No.: 3

Project Engineer: SF

Depth (ft): 10.0

Visual Description: Light olive brown Poorly Graded SAND (SP)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☒
 SPT Sample ☒ Thin-Walled Tube ☐
 Calif. Sample ☐ Engr. Test Specimen's WC ☐

Selection Method:

Sieves (1) - whole sample used
 Sieves (1) - partial sample used & selected by Method(s)
 Method
 (a): Splitter; (use for dry soils or that which will segregate)
 (b): Quartering; (use for dry soils or that which will segregate)
 (c): Representative scoop after mixing, or slice of intact sample.
 (use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
 Air Dried ☐
 Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
 Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
 Retained Fraction: 1st Split Washed? ☐
 Fine Fraction Washed on No. 200 sieve? ☐

By: ☒ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B73
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	819.65
Container Number		B73	B73	Dry, M2 (g)	712.83
Mass of Container and Dry Soil, (g)		712.83	689.48	Cont.,M3 (g)	200.86
Mass of Container, (g)		200.86	200.86	Water Content (%)	20.86
Dry Soil, Ws (g)		511.97	488.62		

SIEVING RESULTS

% error: 0.02

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	170.9		66.6
#60 / 60			
#100 / 40	458.78		10.4
#140 / 30			
#200 / 20	487.61		4.8
Pan	488.54	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
 % GRAVEL _____ D30 XXXXX D15 XXXXX
 % SAND _____ D10 XXXXX D50 XXXXX
 % FINES 4.8 Cu = XXXXX Cc = XXXXX
 Cu = D60 / D10 Cc = D30^2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 05/23/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-18

Project Name: Pure Water

Sample No.: 3

Project Engineer: SF

Depth (ft): 10.0

Visual Description: Light olive brown Lean CLAY (CL)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☐ Thin-Walled Tube ☒
Calif. Sample ☒ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

No ☒
Yes ☐

By: ☐ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B27
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	779.48
Container Number		B27	B27	Dry, M2 (g)	686.58
Mass of Container and Dry Soil, (g)		686.58	234.88	Cont.,M3 (g)	200.49
Mass of Container, (g)		200.49	200.49	Water Content (%)	19.11
Dry Soil, Ws (g)		486.09	34.39		

SIEVING RESULTS

% error: 0.23

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	0.96		99.8
#60 / 60			
#100 / 40	4.83		99.0
#140 / 30			
#200 / 20	33.04		93.2
Pan	34.31	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 93.2 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: ADC DATE: 06/13/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-24

Project Name: Pure Water

Sample No.: 1

Project Engineer: SF

Depth (ft): 5.0

Visual Description: Strong brown Silty SAND (SM)

SPECIMEN: Tested From:

Selection Method:

Bulk Sample ☐

☐ Other _____

☒ Sieves (1) - whole sample used

SPT Sample ☒

☐ Thin-Walled Tube

Sieves (1) - partial sample used & selected by Method(s)

Calif. Sample ☐

☐ Engr. Test Specimen's WC

Method

☐ Whole sample used

☐ See Bulk Sample Processing Form

(a): Splitter; (use for dry soils or that which will segregate)

(b): Quartering; (use for dry soils or that which will segregate)

(c): Representative scoop after mixing, or slice of intact sample.

(use for moist soils or that which will not segregate)

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒

Method A ☒ not dispersed

Air Dried ☐

Method B ☐ is dispersed

Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve ? ☐ No ☒ Yes

Retained Fraction: 1st Split Washed ? ☐ No ☐ Yes

Fine Fraction Washed on No. 200 sieve ? ☐ No ☐ Yes

No ☒

Yes ☐

By: ☐ Mortar & Pestle

☐ Pulverizer

☐ Hand

☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B50
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	412.11
Container Number		B50	B50	Dry, M2 (g)	399.88
Mass of Container and Dry Soil, (g)		399.88	359.62	Cont.,M3 (g)	200.37
Mass of Container, (g)		200.37	200.37	Water Content (%)	6.13
Dry Soil, Ws (g)		199.51	159.25		

SIEVING RESULTS

% error: 0.15

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	61.49		69.2
#60 / 60			
#100 / 40	146.37		26.6
#140 / 30			
#200 / 20	158.59		20.5
Pan	159.01	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX

% GRAVEL _____ D30 XXXXX D15 XXXXX

% SAND _____ D10 XXXXX D50 XXXXX

% FINES 20.5 Cu = XXXXX Cc = XXXXX

Cu = D60 / D10 Cc = D30^2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 07/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-25

Project Name: Pure Water

Sample No.: 1

Project Engineer: SF

Depth (ft): 0~5

Visual Description: Strong brown Clayey SAND (SC)

SPECIMEN: Tested From:

Bulk Sample ☒ Other _____
SPT Sample ☐ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

No ☒ Yes ☐
By: ☐ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐ ☐
Fine Fraction Washed on No. 200 sieve? ☐ ☐

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B7
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	654.63
Container Number		B7	B7	Dry, M2 (g)	612.13
Mass of Container and Dry Soil, (g)		612.13	488.21	Cont.,M3 (g)	198.1
Mass of Container, (g)		198.1	198.19	Water Content (%)	10.26
Dry Soil, Ws (g)		414.03	290.02		

SIEVING RESULTS

% error: 0.01

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	164.63		60.2
#60 / 60			
#100 / 40	265		36.0
#140 / 30			
#200 / 20	289.1		30.2
Pan	289.98	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 30.2 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30^2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 08/17/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-27

Project Name: Pure Water

Sample No.: 2

Project Engineer: SF

Depth (ft): 5.0

Visual Description: Light yellowish brown CLAY with Sand (CL)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve ? ☐ No ☒ Yes
Retained Fraction: 1st Split Washed ? ☐ No ☐ Yes
Fine Fraction Washed on No. 200 sieve ? ☐ No ☐ Yes

No ☒
Yes ☐

By: ☐ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B98
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	715.27
Container Number		B98	B98	Dry, M2 (g)	628.73
Mass of Container and Dry Soil, (g)		628.73	301.83	Cont.,M3 (g)	203.42
Mass of Container, (g)		203.42	203.5	Water Content (%)	20.35
Dry Soil, Ws (g)		425.31	98.33		

SIEVING RESULTS

% error: 0.17

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	40.47		90.5
#60 / 60			
#100 / 40	55.22		87.0
#140 / 30			
#200 / 20	94.03		77.9
Pan	98.5	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 77.9 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 08/18/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-32

Project Name: Pure Water

Sample No.: 2

Project Engineer: SF

Depth (ft): 10.0

Visual Description: Light olive brown Silty SAND (SM)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☐ Thin-Walled Tube ☒
Calif. Sample ☒ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒ Method A ☒ not dispersed
Air Dried ☐ Method B ☐ is dispersed
Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒

Retained Fraction: 1st Split Washed? No ☐ Yes ☐

Fine Fraction Washed on No. 200 sieve? No ☐ Yes ☐

No ☒

Yes ☐

By: ☐ Mortar & Pestle ☐ Hand

☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B36
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	665.95
Container Number		B36	B36	Dry, M2 (g)	600.81
Mass of Container and Dry Soil, (g)		600.81	492.75	Cont.,M3 (g)	197.98
Mass of Container, (g)		197.98	198.06	Water Content (%)	16.17
Dry Soil, Ws (g)		402.83	294.69		

SIEVING RESULTS

% error: 0.08

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	17.97		95.5
#60 / 60			
#100 / 40	219.71		45.5
#140 / 30			
#200 / 20	291.39		27.7
Pan	294.93	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 27.7 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 08/18/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-41

Project Name: Pure Water

Sample No.: 1

Project Engineer: SF

Depth (ft): 0~5

Visual Description: Light olive brown Sandy SILT (ML)

SPECIMEN: Tested From:

Bulk Sample ☒ Other _____
SPT Sample ☐ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

No ☒ Yes ☐
By: ☐ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐ ☐
Fine Fraction Washed on No. 200 sieve? ☐ ☐

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B1
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	640.99
Container Number		B1	B1	Dry, M2 (g)	574.06
Mass of Container and Dry Soil, (g)		574.06	370.06	Cont.,M3 (g)	200.05
Mass of Container, (g)		200.05	200.05	Water Content (%)	17.90
Dry Soil, Ws (g)		374.01	170.01		

SIEVING RESULTS

% error: 0.02

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	20.41		94.5
#60 / 60			
#100 / 40	58.66		84.3
#140 / 30			
#200 / 20	167.59		55.2
Pan	169.98	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 55.2 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: ADC DATE: 07/20/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-45

Project Name: Pure Water

Sample No.: 3

Project Engineer: SF

Depth (ft): 10.0

Visual Description: Light olive brown Silty CLAY (CL)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☐ Thin-Walled Tube ☒
Calif. Sample ☒ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used
☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒ Method A ☒ not dispersed
Air Dried ☐ Method B ☐ is dispersed
Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? No ☐ Yes ☐
Fine Fraction Washed on No. 200 sieve? No ☐ Yes ☐
By: ☐ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other
and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B68
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	648.35
Container Number		B68	B68	Dry, M2 (g)	572.66
Mass of Container and Dry Soil, (g)		572.66	213.21	Cont.,M3 (g)	200.17
Mass of Container, (g)		200.17	200.2	Water Content (%)	20.32
Dry Soil, Ws (g)		372.49	13.01		

SIEVING RESULTS

% error: 0.54

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	1.13		99.7
#60 / 60			
#100 / 40	2.04		99.5
#140 / 30			
#200 / 20	12.43		96.7
Pan	12.94	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 96.7 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 08/18/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-46

Project Name: Pure Water

Sample No.: 4

Project Engineer: SF

Depth (ft): 18.5

Visual Description: Dark yellowish brown Sandy, Lean CLAY (CL)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☐ Thin-Walled Tube ☒
Calif. Sample ☒ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

No ☒ By: ☐ Mortar & Pestle ☐ Hand
Yes ☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B40
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	672.92
Container Number		B40	B40	Dry, M2 (g)	584.3
Mass of Container and Dry Soil, (g)		584.3	337.85	Cont.,M3 (g)	198.4
Mass of Container, (g)		198.4	198.4	Water Content (%)	22.96
Dry Soil, Ws (g)		385.9	139.45		

SIEVING RESULTS

% error: 0.07

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	8.13		97.9
#60 / 60			
#100 / 40	68.83		82.2
#140 / 30			
#200 / 20	136.36		64.7
Pan	139.35	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 64.7 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 07/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-47

Project Name: Pure Water

Sample No.: 2

Project Engineer: SF

Depth (ft): 5.0

Visual Description: Light olive brown Fat CLAY (CH)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

☐ Whole sample used

☐ See Bulk Sample Processing Form

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

No ☒ By: ☐ Mortar & Pestle ☐ Hand
Yes ☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B56
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	434.78
Container Number		B56	B56	Dry, M2 (g)	401.58
Mass of Container and Dry Soil, (g)		401.58	213.19	Cont.,M3 (g)	197.61
Mass of Container, (g)		197.61	197.61	Water Content (%)	16.28
Dry Soil, Ws (g)		203.97	15.58		

SIEVING RESULTS

% error: 0.13

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	3.49		98.3
#60 / 60			
#100 / 40	4.19		97.9
#140 / 30			
#200 / 20	15.07		92.6
Pan	15.6	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 92.6 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 07/24/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-49

Project Name: Pure Water

Sample No.: 1

Project Engineer: SF

Depth (ft): 0~5

Visual Description: Brownish yellow Silty SAND (SM)

SPECIMEN: Tested From:

Bulk Sample ☒ Other _____
SPT Sample ☐ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒ Method A ☒ not dispersed
Air Dried ☐ Method B ☐ is dispersed
Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? No ☐ Yes ☐
Fine Fraction Washed on No. 200 sieve? No ☐ Yes ☐

By: ☒ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B14
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	634.65
Container Number		B14	B14	Dry, M2 (g)	585.5
Mass of Container and Dry Soil, (g)		585.5	456.61	Cont.,M3 (g)	200.71
Mass of Container, (g)		200.71	200.71	Water Content (%)	12.77
Dry Soil, Ws (g)		384.79	255.9		

SIEVING RESULTS

% error: 0.05

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	11.47		97.0
#60 / 60			
#100 / 40	201.48		47.6
#140 / 30			
#200 / 20	255.07		33.7
Pan	255.76	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 33.7 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: ADC DATE: 07/20/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: FM-52

Project Name: Pure Water

Sample No.: 4

Project Engineer: SF

Depth (ft): 15.0

Visual Description: Light olive brown Sandy SILT (ML)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

No ☒ By: ☐ Mortar & Pestle ☐ Hand
Yes ☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B64
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	685.21
Container Number		B64	B64	Dry, M2 (g)	606.69
Mass of Container and Dry Soil, (g)		606.69	330.42	Cont.,M3 (g)	200.61
Mass of Container, (g)		200.61	200.61	Water Content (%)	19.34
Dry Soil, Ws (g)		406.08	129.81		

SIEVING RESULTS

% error: 0.24

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	13.26		96.7
#60 / 60			
#100 / 40	23.66		94.2
#140 / 30			
#200 / 20	125.46		69.1
Pan	129.5	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 69.1 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: GD DATE: 07/25/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 1.01.05.20.00

Boring No.: PS-3

Project Name: Pure Water

Sample No.: 2

Project Engineer: SF

Depth (ft): 10.0

Visual Description: Light olive brown Poorly Graded SAND with Silt (SP-SM)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

By: ☒ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B40
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	517.29
Container Number		B40	B40	Dry, M2 (g)	434.8
Mass of Container and Dry Soil, (g)		434.8	420.14	Cont.,M3 (g)	198.41
Mass of Container, (g)		198.41	198.43	Water Content (%)	34.90
Dry Soil, Ws (g)		236.39	221.71		

SIEVING RESULTS

% error: 0.01

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	11.73		95.0
#60 / 60			
#100 / 40	190.74		19.3
#140 / 30			
#200 / 20	221.07		6.5
Pan	221.69	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 6.5 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30^2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☒ Yes Amount Adjective: high

SET-UP BY: ADC DATE: 06/09/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 1.01.05.20.00

Boring No.: PS-3

Project Name: Pure Water

Sample No.: 4

Project Engineer: SF

Depth (ft): 20.0

Visual Description: Olive gray Sandy SILT (ML)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
 SPT Sample ☒ Thin-Walled Tube ☒
 Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
 Sieves (1) - partial sample used & selected by Method(s)
 Method
 (a): Splitter; (use for dry soils or that which will segregate)
 (b): Quartering; (use for dry soils or that which will segregate)
 (c) : Representative scoop after mixing, or slice of intact sample.
 (use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒ Method A ☒ not dispersed
 Air Dried ☐ Method B ☐ is dispersed
 Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve ? ☐ No ☒ Yes
 Retained Fraction: 1st Split Washed ? ☐ No ☐ Yes
 Fine Fraction Washed on No. 200 sieve ? ☐ No ☐ Yes

By: ☒ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B26
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	470.42
Container Number		B26	B26	Dry, M2 (g)	396.16
Mass of Container and Dry Soil, (g)		396.16	273.7	Cont.,M3 (g)	199.73
Mass of Container, (g)		199.73	199.75	Water Content (%)	37.80
Dry Soil, Ws (g)		196.43	73.95		

SIEVING RESULTS

% error: 0.18

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	0.47		99.8
#60 / 60			
#100 / 40	16.6		91.5
#140 / 30			
#200 / 20	70.9		63.9
Pan	73.82	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
 % GRAVEL _____ D30 XXXXX D15 XXXXX
 % SAND _____ D10 XXXXX D50 XXXXX
 % FINES 63.9 Cu = XXXXX Cc = XXXXX
 Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☒ Yes Amount Adjective: medium

SET-UP BY: ADC DATE: 06/09/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 1.01.05.20.00

Boring No.: PS-3

Project Name: Pure Water

Sample No.: 9

Project Engineer: SF

Depth (ft): 50.0

Visual Description: Gray Poorly Graded SAND with Silt (SP-SM)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

No ☒ By: ☐ Mortar & Pestle ☐ Hand
Yes ☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B15
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	551.72
Container Number		B15	B15	Dry, M2 (g)	475.97
Mass of Container and Dry Soil, (g)		475.97	450.56	Cont.,M3 (g)	200.68
Mass of Container, (g)		200.68	200.69	Water Content (%)	27.52
Dry Soil, Ws (g)		275.29	249.87		

SIEVING RESULTS

% error: 0.03

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	44.6		83.8
#60 / 60			
#100 / 40	231.82		15.8
#140 / 30			
#200 / 20	249.55		9.4
Pan	249.8	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 9.4 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☒ Yes Amount Adjective: medium

SET-UP BY: ADC DATE: 06/09/17

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: WM-02

Project Name: Pure Water

Sample No.: 2

Project Engineer: SF

Depth (ft): 10.0

Visual Description: Olive Silty SAND (SM)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
 SPT Sample ☒ Thin-Walled Tube ☒
 Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
 Sieves (1) - partial sample used & selected by Method(s)
 Method
 (a): Splitter; (use for dry soils or that which will segregate)
 (b): Quartering; (use for dry soils or that which will segregate)
 (c) : Representative scoop after mixing, or slice of intact sample.
 (use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

Test Method (D1140)

As-Received ☒ Method A ☒ not dispersed
 Air Dried ☐ Method B ☐ is dispersed
 Oven-Dried ☐

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve ? ☐ No ☒ Yes
 Retained Fraction: 1st Split Washed ? ☐ No ☐ Yes
 Fine Fraction Washed on No. 200 sieve ? ☐ No ☐ Yes

By: ☒ Mortar & Pestle ☐ Hand
☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B3
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	436.27
Container Number		B3	B3	Dry, M2 (g)	391.59
Mass of Container and Dry Soil, (g)		391.59	316.81	Cont.,M3 (g)	198.05
Mass of Container, (g)		198.05	198.08	Water Content (%)	23.09
Dry Soil, Ws (g)		193.54	118.73		

SIEVING RESULTS

% error: 0.06

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	2.99		98.5
#60 / 60			
#100 / 40	98.74		49.0
#140 / 30			
#200 / 20	116.29		39.9
Pan	118.66	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
 % GRAVEL _____ D30 XXXXX D15 XXXXX
 % SAND _____ D10 XXXXX D50 XXXXX
 % FINES 39.9 Cu = XXXXX Cc = XXXXX
 Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☒ No ☐ Yes Amount Adjective: _____

SET-UP BY: ADC DATE: 06/19/15

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

GRADATION OF SOILS by Sieving using Soil Sieve Sizes & with Water Content

ASTM D1140 and D2216

Project Number: 60530732

Task Number: 2.01.05.20.00

Boring No.: WM-16

Project Name: Pure Water

Sample No.: 2

Project Engineer: SF

Depth (ft): 8.0

Visual Description: Yellowish brown Lean CLAY with Sand (CL)

SPECIMEN: Tested From:

Bulk Sample ☐ Other ☐
SPT Sample ☒ Thin-Walled Tube ☒
Calif. Sample ☐ Engr. Test Specimen's WC ☒

Selection Method:

Sieves (1) - whole sample used
Sieves (1) - partial sample used & selected by Method(s)
Method
(a): Splitter; (use for dry soils or that which will segregate)
(b): Quartering; (use for dry soils or that which will segregate)
(c): Representative scoop after mixing, or slice of intact sample.
(use for moist soils or that which will not segregate)

☐ Whole sample used

☐ See Bulk Sample Processing Form

Preparation: Sample/Specimen:

As-Received ☒
Air Dried ☐
Oven-Dried ☐

Test Method (D1140)

Method A ☒ not dispersed
Method B ☐ is dispersed

Oven-Dried Soil Broken Up Before Selecting Partial sample:

Washing:

Whole Specimen Washed on No. 200 sieve? No ☐ Yes ☒
Retained Fraction: 1st Split Washed? ☐
Fine Fraction Washed on No. 200 sieve? ☐

No ☒ By: ☐ Mortar & Pestle ☐ Hand
Yes ☐ Pulverizer ☐ Other

and Soil Soaked for: 5 hrs.

Water Content

MASS OF TEST SPECIMEN (g)	Total Test Specimen with Coarse Fraction	Partial or Whole Test Specimen	Soil Retained (after washing)	As Received	
				Container No.	B56
Min. sieve size in sieving sequence (3)	#N/A	# 200	+200	Wet, M1 (g)	665.37
Container Number		B56	B56	Dry, M2 (g)	604.45
Mass of Container and Dry Soil, (g)		604.45	302.49	Cont.,M3 (g)	197.62
Mass of Container, (g)		197.62	197.64	Water Content (%)	14.97
Dry Soil, Ws (g)		406.83	104.85		

SIEVING RESULTS

% error: 0.05

See (1)	Sieve No.	Cum. Mass Retained (g)	Total Specimen % Finer N'
	3 "		
	2 "		
	1 1/2"		
	1 "		
	3/4 "		
	1/2 "		
	3/8 "		
	4		
	Pan		XXXXXXXXXX

Req. Mass of Test Spec. for 1% (kg)
3"= 70
1 1/2"=10
3/4"= 1.1
3/8"= 0.25
#4 = 0.1
#10 = 0.1

Sieve No. / (2)	Cum. Mass Retained (g)	Partial Test Specimen	Total Specimen % Finer N'
3 "			
2 "			
1 1/2"			
1 "			
3/4 "			
1/2 "			
3/8 "			
#4 / 325			
#10 / 180			
#20 / 115			
#40 / 75	10.04		97.5
#60 / 60			
#100 / 40	53.98		86.7
#140 / 30			
#200 / 20	103.94		74.5
Pan	104.8	XXXXXXXXXX	XXXXXXXXXX

SUMMARY: Shape & Filter Parameters

% COBBLES _____ D60 XXXXX D85 XXXXX
% GRAVEL _____ D30 XXXXX D15 XXXXX
% SAND _____ D10 XXXXX D50 XXXXX
% FINES 74.5 Cu = XXXXX Cc = XXXXX
Cu = D60 / D10 Cc = D30*2 / (D60 * D10)

(1) X in box denotes sieve on which split was made

(2) Proposed allowable amount of soil retained on 8" dia. Sieve.

(3) Sieve size given, denotes min. sieve size used in the appropriate sieving sequence.

Mica Noted: ☐ No ☒ Yes Amount Adjective: low

SET-UP BY: ADC DATE: 06/19/15

CALCULATED BY: ADC

CHECKED BY: ADC

SUBMITTED BY: A. D. Camacho

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 2.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	DS-01	DS-01	DS-01	DS-01	DS-01		DS-02	
Sample No.	2	5	8b	10	11		3a	
Depth (ft.)	5	20	35	45	50		15	
Container No.	S10	S203	S217	S204	S205		S302	
Mass of Container and Wet Specimen, M1 (g)	335.04	467.70	275.27	458.24	430.36		502.09	
Mass Container and Dry Specimen, M2 (g)	313.74	402.86	237.61	367.89	355.53		392.14	
Mass Container, M3 (g)	84.01	147.88	137.94	149.08	139.32		144.91	
Water Content, w (%)	9.3	25.4	37.8	41.3	34.6		44.5	
Unified Soil Classification Group Symbol (Visual)	SM	SM	CL	ML	ML		ML	
Boring No.	DS-03	DS-03	DS-03					
Sample No.	6a	9b	11a					
Depth (ft.)	25	40	50					
Container No.	S304	S308	S206					
Mass of Container and Wet Specimen, M1 (g)	560.94	455.03	676.79					
Mass Container and Dry Specimen, M2 (g)	477.04	360.30	527.01					
Mass Container, M3 (g)	144.68	145.06	139.91					
Water Content, w (%)	25.2	44	38.7					
Unified Soil Classification Group Symbol (Visual)	SM	CL-ML	ML					

$$w = ((M1 - M2) / (M2 - M3)) * 100$$

Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ±0.1 %	Water Content Reported to ±1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

TESTED BY: GD

Date 8/17/2017

CALCULATED BY: ADC

CHECKED BY: ADC

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 2.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	FM-08	FM-08	FM-10-1	FM-11	FM-14	FM-14	FM-14	FM-14
Sample No.	2	4	2	4	2a	2b	4a	4b/c
Depth (ft.)	5	15	5	15	5	5	15	15
Container No.	S7	S11	M1	M8	M6	M5	M7	M70
Mass of Container and Wet Specimen, M1 (g)	538.08	481.80	425.52	529.78	402.47	419.08	467.34	603.94
Mass Container and Dry Specimen, M2 (g)	488.09	420.44	376.48	427.68	384.97	372.79	418.55	532.39
Mass Container, M3 (g)	80.32	95.71	91.72	89.38	90.32	91.24	91.02	105.47
Water Content, w (%)	12.3	18.9	17.2	30.2	5.9	16.4	14.9	16.8
Unified Soil Classification Group Symbol (Visual)	SC	CL	CL	ML	SP	SC	SM	CL
Boring No.	FM-16	FM-17						
Sample No.	3	2						
Depth (ft.)	15	10						
Container No.	M89	M72						
Mass of Container and Wet Specimen, M1 (g)	713.17	708.82						
Mass Container and Dry Specimen, M2 (g)	647.42	618.18						
Mass Container, M3 (g)	109.50	108.58						
Water Content, w (%)	12.2	17.8						
Unified Soil Classification Group Symbol (Visual)	SM-ML	SM-ML						

$$w = ((M1 - M2) / (M2 - M3)) * 100$$

Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ±0.1 %	Water Content Reported to ±1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

TESTED BY: ADC

Date 6/12/2017

CALCULATED BY: ADC

CHECKED BY: ADC

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 2.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	FM-09	FM-09	FM-18	FM-18				
Sample No.	1	3	2	4				
Depth (ft.)	5	15	5	15				
Container No.	S12	S39	S19	S33				
Mass of Container and Wet Specimen, M1 (g)	549.43	383.19	641.91	586.22				
Mass Container and Dry Specimen, M2 (g)	513.35	375.62	571.02	512.63				
Mass Container, M3 (g)	83.55	113.44	102.54	110.13				
Water Content, w (%)	8.4	2.9	15.1	18.3				
Unified Soil Classification Group Symbol (Visual)	SM	SW	ML	ML				
Boring No.								
Sample No.								
Depth (ft.)								
Container No.								
Mass of Container and Wet Specimen, M1 (g)								
Mass Container and Dry Specimen, M2 (g)								
Mass Container, M3 (g)								
Water Content, w (%)								
Unified Soil Classification Group Symbol (Visual)								

$$w = (M1 - M2) / (M2 - M3) \times 100$$

Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ± 0.1 %	Water Content Reported to ± 1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

TESTED BY: ADC

Date 6/13/2017

CALCULATED BY: ADC

CHECKED BY: ADC

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 2.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	FM-26	FM-27	FM-28	FM-28	FM-29	FM-45	FM-45	
Sample No.	3	4	1	3	1	2	4	
Depth (ft.)	15	15	5	15	0~5	5	15	
Container No.	S306	S309	S33	S34	S39	M9	M8	
Mass of Container and Wet Specimen, M1 (g)	522.52	484.00	348.02	427.14	521.13	445.50	504.52	
Mass Container and Dry Specimen, M2 (g)	463.55	430.22	330.46	382.49	493.40	386.97	437.09	
Mass Container, M3 (g)	145.08	143.28	110.10	117.99	113.42	89.73	89.38	
Water Content, w (%)	18.5	18.7	8	16.9	7.3	19.7	19.4	
Unified Soil Classification Group Symbol (Visual)	ML	ML	SM	ML	SM	ML	ML	
Boring No.								
Sample No.								
Depth (ft.)								
Container No.								
Mass of Container and Wet Specimen, M1 (g)								
Mass Container and Dry Specimen, M2 (g)								
Mass Container, M3 (g)								
Water Content, w (%)								
Unified Soil Classification Group Symbol (Visual)								

$$w = ((M1 - M2) / (M2 - M3)) * 100$$

Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ±0.1 %	Water Content Reported to ±1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

TESTED BY: GD

Date 8/18/2017

CALCULATED BY: ADC

CHECKED BY: ADC

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 2.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	FM-31	FM-31	FM-32	FM-32	FM-33	FM-33	FM-50	
Sample No.	2	4	1	3	3	7	1	
Depth (ft.)	5	15	5	15	7	15	0~5	
Container No.	M1	M91	M6	M67	M89	M72	M73	
Mass of Container and Wet Specimen, M1 (g)	582.45	473.05	438.15	480.39	417.72	475.72	639.34	
Mass Container and Dry Specimen, M2 (g)	508.57	412.52	384.75	424.27	371.64	426.68	588.57	
Mass Container, M3 (g)	91.70	89.78	90.32	110.41	109.49	108.61	108.67	
Water Content, w (%)	17.7	18.8	18.1	17.9	17.6	15.4	10.6	
Unified Soil Classification Group Symbol (Visual)	SC	CL	SM	CL	CL	CL	SM	
Boring No.								
Sample No.								
Depth (ft.)								
Container No.								
Mass of Container and Wet Specimen, M1 (g)								
Mass Container and Dry Specimen, M2 (g)								
Mass Container, M3 (g)								
Water Content, w (%)								
Unified Soil Classification Group Symbol (Visual)								

$$w = ((M1 - M2) / (M2 - M3)) * 100$$

Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ±0.1 %	Water Content Reported to ±1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

TESTED BY: GD

Date 8/18/2017

CALCULATED BY: ADC

CHECKED BY: ADC

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 2.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	FM-34	FM-34	FM-35					
Sample No.	3	4	2a,b					
Depth (ft.)	15	18.5	5					
Container No.	M70	M72	M89					
Mass of Container and Wet Specimen, M1 (g)	389.04	339.62	457.53					
Mass Container and Dry Specimen, M2 (g)	355.33	314.09	415.54					
Mass Container, M3 (g)	105.51	108.61	109.53					
Water Content, w (%)	13.5	12.4	13.7					
Unified Soil Classification Group Symbol (Visual)	SM	MH	SM					
Boring No.								
Sample No.								
Depth (ft.)								
Container No.								
Mass of Container and Wet Specimen, M1 (g)								
Mass Container and Dry Specimen, M2 (g)								
Mass Container, M3 (g)								
Water Content, w (%)								
Unified Soil Classification Group Symbol (Visual)								

$$w = ((M1 - M2) / (M2 - M3)) * 100$$

Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ±0.1 %	Water Content Reported to ±1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

TESTED BY: GD

Date 7/24/2017

CALCULATED BY: ADC

CHECKED BY: ADC

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 2.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	FM-36b	FM-36b	FM-44					
Sample No.	2	4	1a,b					
Depth (ft.)	5	15	5					
Container No.	S31	S209	S206					
Mass of Container and Wet Specimen, M1 (g)	230.25	406.37	339.80					
Mass Container and Dry Specimen, M2 (g)	220.73	386.37	318.10					
Mass Container, M3 (g)	98.24	146.53	139.92					
Water Content, w (%)	7.8	8.3	12.2					
Unified Soil Classification Group Symbol (Visual)	SC	SM	SC					
Boring No.								
Sample No.								
Depth (ft.)								
Container No.								
Mass of Container and Wet Specimen, M1 (g)								
Mass Container and Dry Specimen, M2 (g)								
Mass Container, M3 (g)								
Water Content, w (%)								
Unified Soil Classification Group Symbol (Visual)								

$$w = ((M1 - M2) / (M2 - M3)) * 100$$

Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ±0.1 %	Water Content Reported to ±1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

TESTED BY: GD

Date 7/21/2017

CALCULATED BY: ADC

CHECKED BY: ADC

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 2.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	FM-49	FM-49						
Sample No.	2	4						
Depth (ft.)	5	15						
Container No.	S304	S308						
Mass of Container and Wet Specimen, M1 (g)	406.85	381.02						
Mass Container and Dry Specimen, M2 (g)	367.03	365.12						
Mass Container, M3 (g)	144.65	145.10						
Water Content, w (%)	17.9	7.2						
Unified Soil Classification Group Symbol (Visual)	ML	SM						
Boring No.								
Sample No.								
Depth (ft.)								
Container No.								
Mass of Container and Wet Specimen, M1 (g)								
Mass Container and Dry Specimen, M2 (g)								
Mass Container, M3 (g)								
Water Content, w (%)								
Unified Soil Classification Group Symbol (Visual)								

$$w = (M1 - M2) / (M2 - M3) \times 100$$

Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ± 0.1 %	Water Content Reported to ± 1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

TESTED BY: GD

Date 7/25/2017

CALCULATED BY: ADC

CHECKED BY: ADC

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 1.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	PS-3	PS-3	PS-3					
Sample No.	11a	12a	13					
Depth (ft.)	60	65	70					
Container No.	S30	C6	S6					
Mass of Container and Wet Specimen, M1 (g)	270.39	296.80	315.03					
Mass Container and Dry Specimen, M2 (g)	244.48	235.02	266.14					
Mass Container, M3 (g)	120.41	71.88	79.82					
Water Content, w (%)	20.9	37.9	26.2					
Unified Soil Classification Group Symbol (Visual)	SP	CH	ML					
Boring No.								
Sample No.								
Depth (ft.)								
Container No.								
Mass of Container and Wet Specimen, M1 (g)								
Mass Container and Dry Specimen, M2 (g)								
Mass Container, M3 (g)								
Water Content, w (%)								
Unified Soil Classification Group Symbol (Visual)								

$$w = ((M1 - M2) / (M2 - M3)) * 100$$

Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ±0.1 %	Water Content Reported to ±1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

TESTED BY: ADC

Date 6/9/2017

CALCULATED BY: ADC

CHECKED BY: ADC

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 2.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	WM-01	WM-01	WM-01	WM-03	WM-03	WM-11	WM-16	WM-21
Sample No.	2	4a	5	2a	5a	2	1	1
Depth (ft.)	5	15	18.5	5	18.5	6	6	5
Container No.	S2	S219	S300	S211	S222	M60	M66	M67
Mass of Container and Wet Specimen, M1 (g)	520.90	471.42	631.91	553.10	571.39	405.03	305.11	560.88
Mass Container and Dry Specimen, M2 (g)	489.39	424.74	527.61	483.36	499.04	376.26	299.96	494.58
Mass Container, M3 (g)	100.09	140.70	143.53	148.48	140.49	91.37	101.65	110.42
Water Content, w (%)	8.1	16.4	27.2	20.8	20.2	10.1	2.6	17.3
Unified Soil Classification Group Symbol (Visual)	SM	ML	SP	ML	SM	SC	SP-SM	SC
Boring No.	WM-21	WM-24						
Sample No.	3	2						
Depth (ft.)	15	5						
Container No.	M73	F110						
Mass of Container and Wet Specimen, M1 (g)	564.84	599.74						
Mass Container and Dry Specimen, M2 (g)	496.36	546.40						
Mass Container, M3 (g)	108.68	109.55						
Water Content, w (%)	17.7	12.2						
Unified Soil Classification Group Symbol (Visual)	SC	SM						

$$w = ((M1 - M2) / (M2 - M3)) * 100$$

Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ±0.1 %	Water Content Reported to ±1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

TESTED BY: ADC

Date 6/19/2017

CALCULATED BY: ADC

CHECKED BY: ADC

**LABORATORY WATER CONTENT: AS - RECEIVED CONDITION
ASTM D 2216**

Project Number: 60530732

Task Number: 2.01.05.20.00

Project Name: Pure Water

Project Engineer: SF

Test Method: B (0.1%)

Boring No.	WM-06	WM-08	WM-23	WM-23				
Sample No.	2	4a	1	3				
Depth (ft.)	5	15.5	5	15				
Container No.	S45	S34	S27	S32				
Mass of Container and Wet Specimen, M1 (g)	498.55	455.65	538.70	490.98				
Mass Container and Dry Specimen, M2 (g)	464.86	401.68	501.61	460.40				
Mass Container, M3 (g)	77.64	117.98	79.00	78.29				
Water Content, w (%)	8.7	19	8.8	8				
Unified Soil Classification Group Symbol (Visual)	SM	ML	CL	CL				
Boring No.								
Sample No.								
Depth (ft.)								
Container No.								
Mass of Container and Wet Specimen, M1 (g)								
Mass Container and Dry Specimen, M2 (g)								
Mass Container, M3 (g)								
Water Content, w (%)								
Unified Soil Classification Group Symbol (Visual)								

$$w = ((M1 - M2) / (M2 - M3)) * 100$$

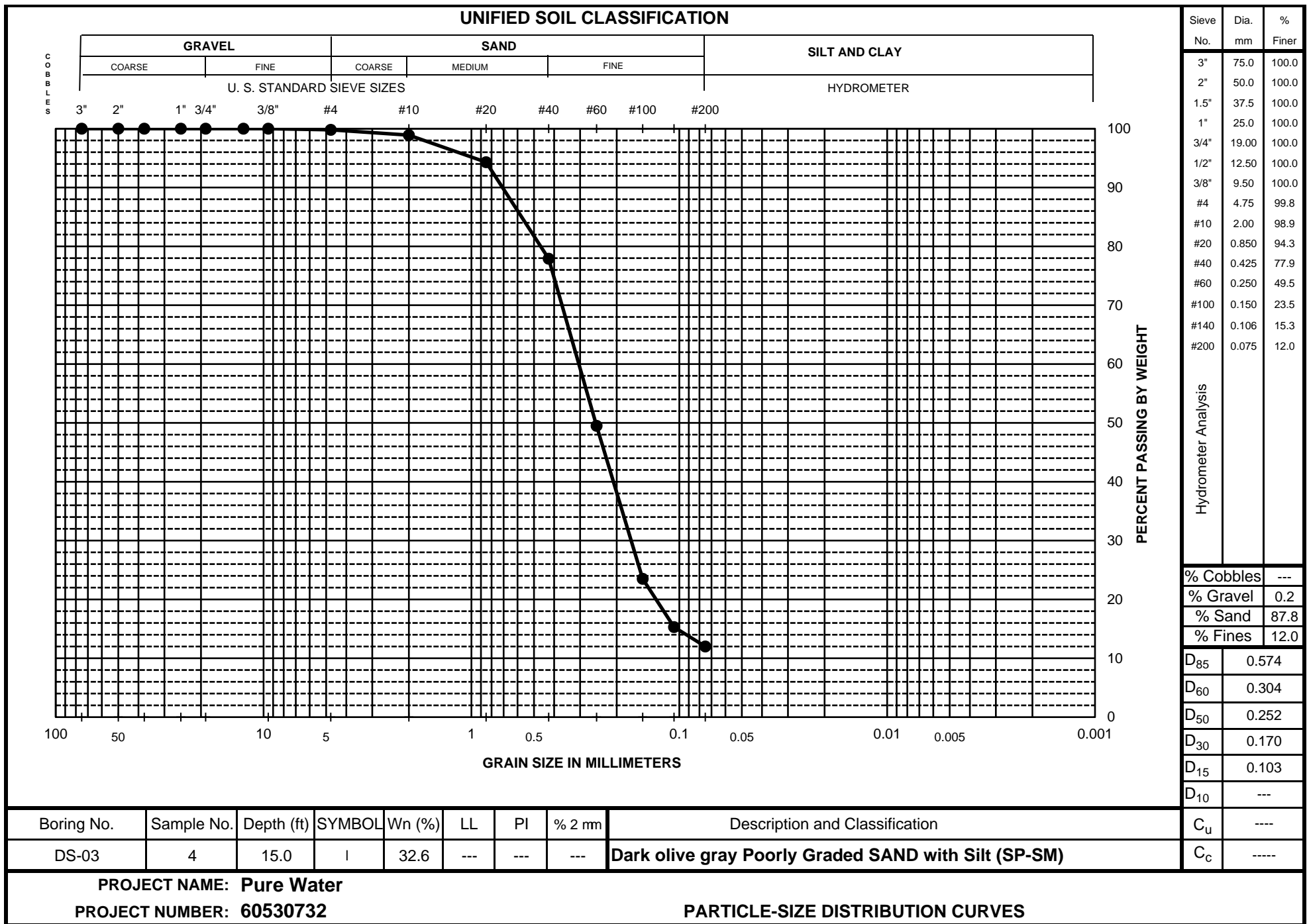
Recommended Minimum Mass of Moist Test Specimen in Accordance with ASTM D 2216			
Approximate Maximum Grain Size in Standard Sieve Size	Water Content Reported to ±0.1 %	Water Content Reported to ±1 %	
No. 10	20 g	To be representative not less than 20 g shall be used.	
No. 4	100 g	To be representative not less than 20 g shall be used.	
3/8-in.	500 g	50 g	
3/4-in.	2.5 kg	250 g	
1 1/2-in.	10 kg	1 kg	
3-in.	50 kg	5 kg	

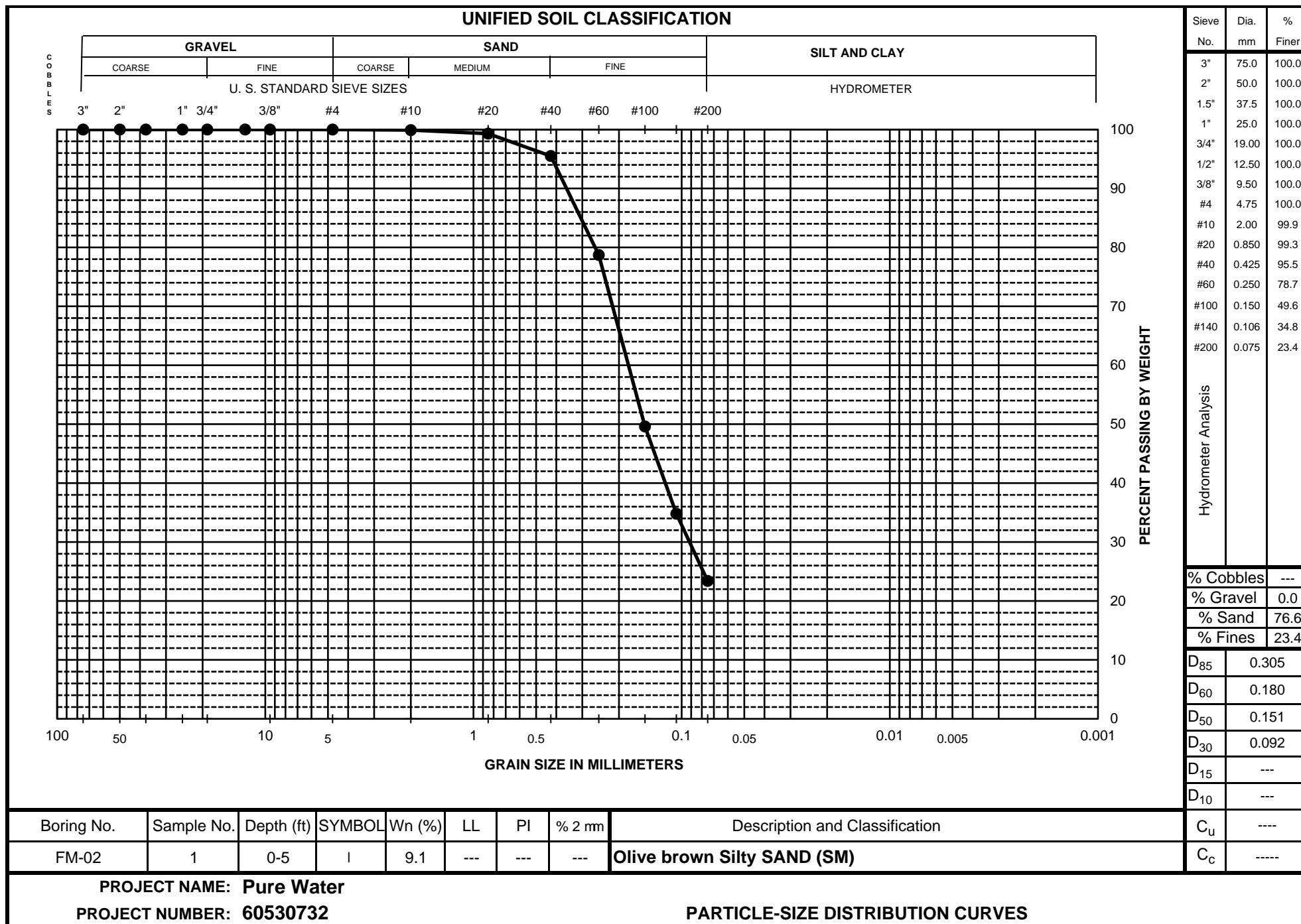
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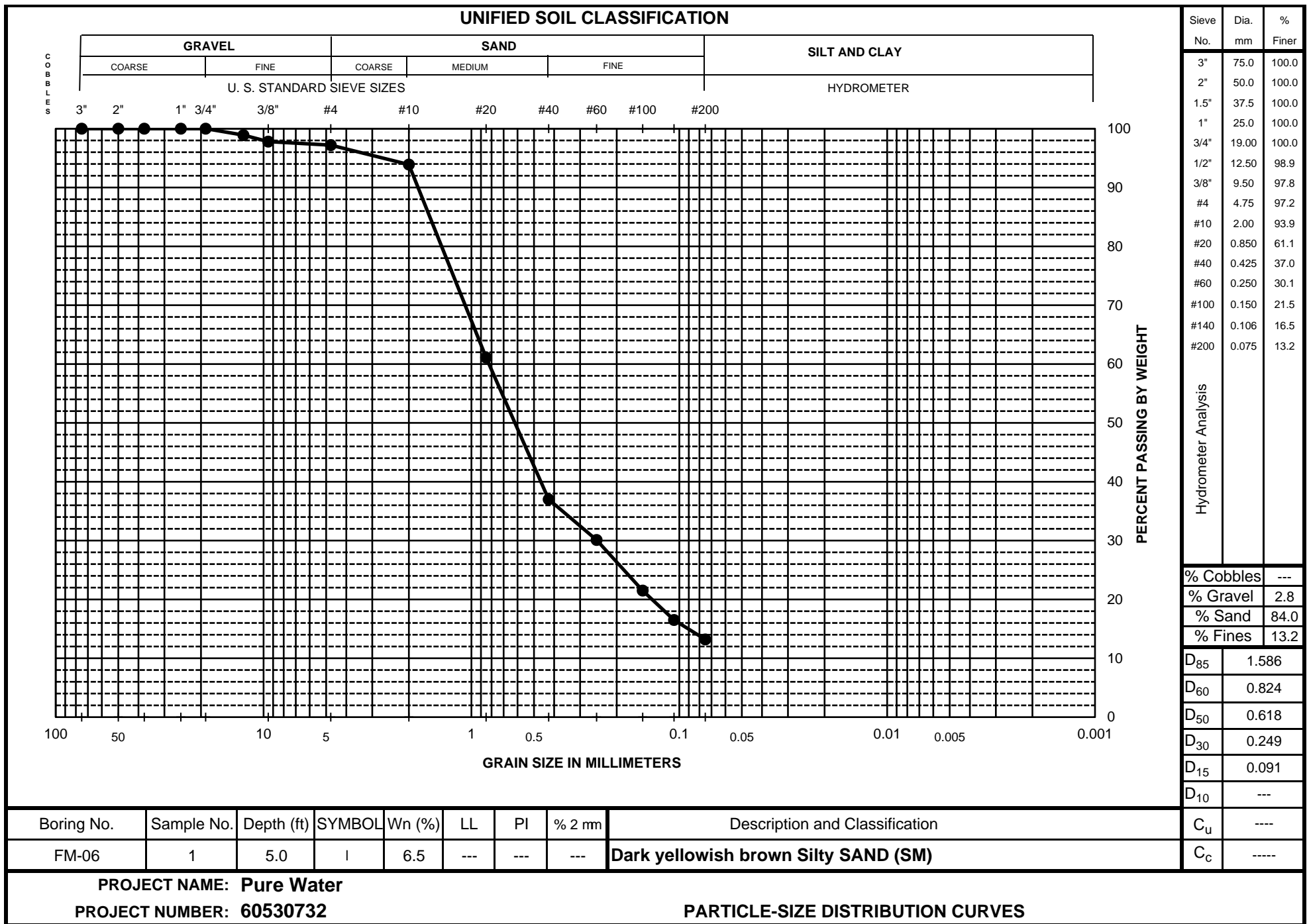
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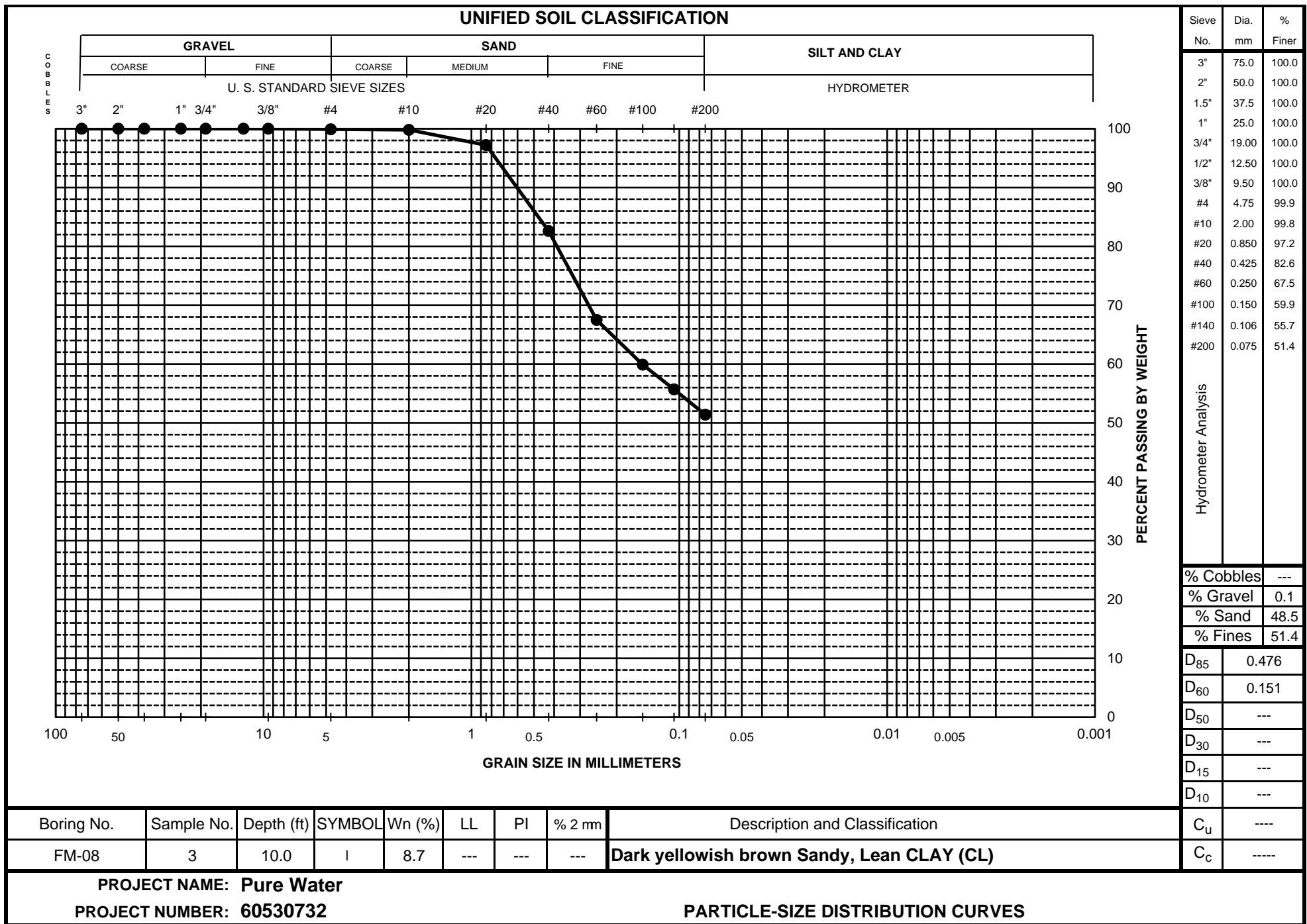
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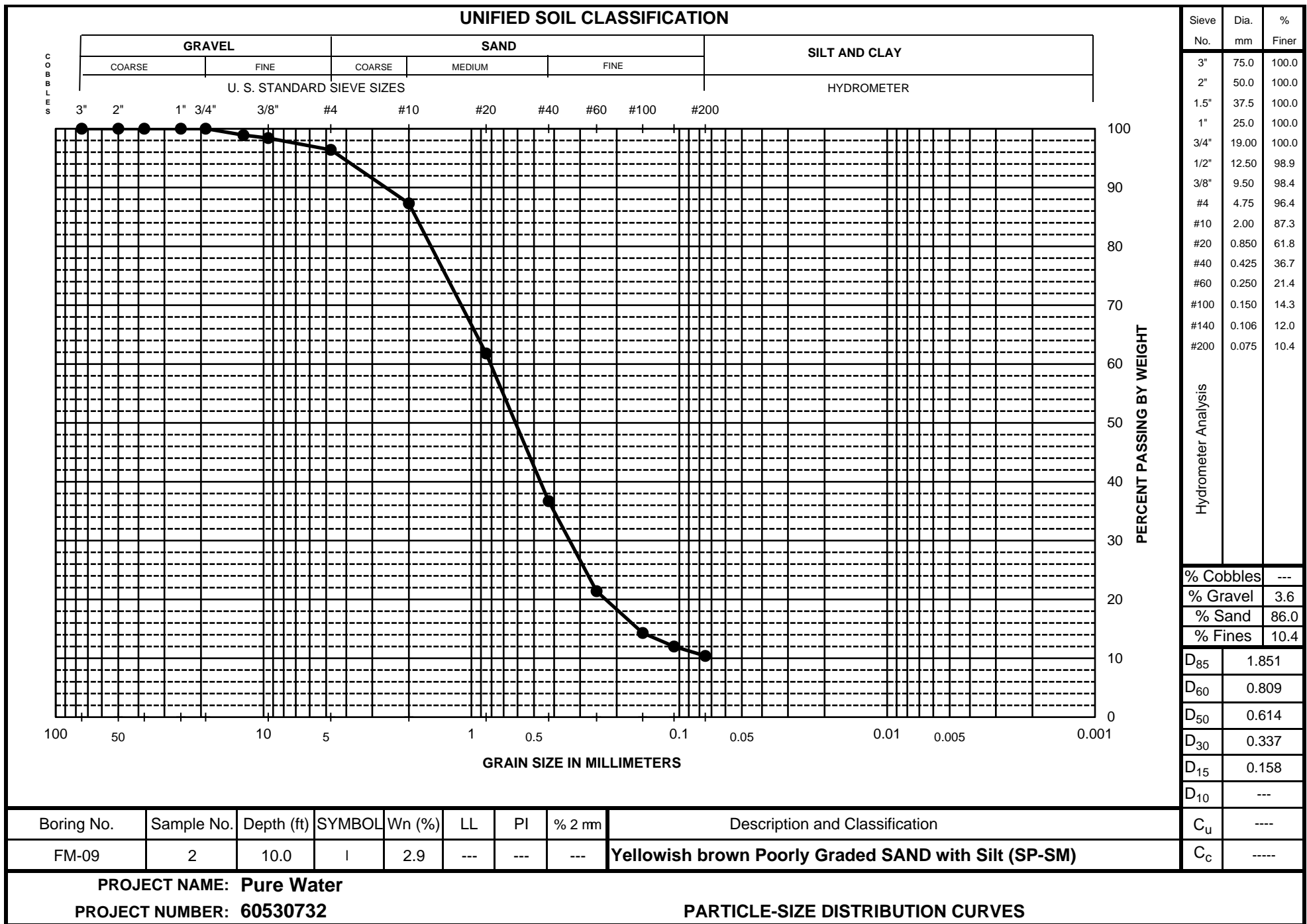
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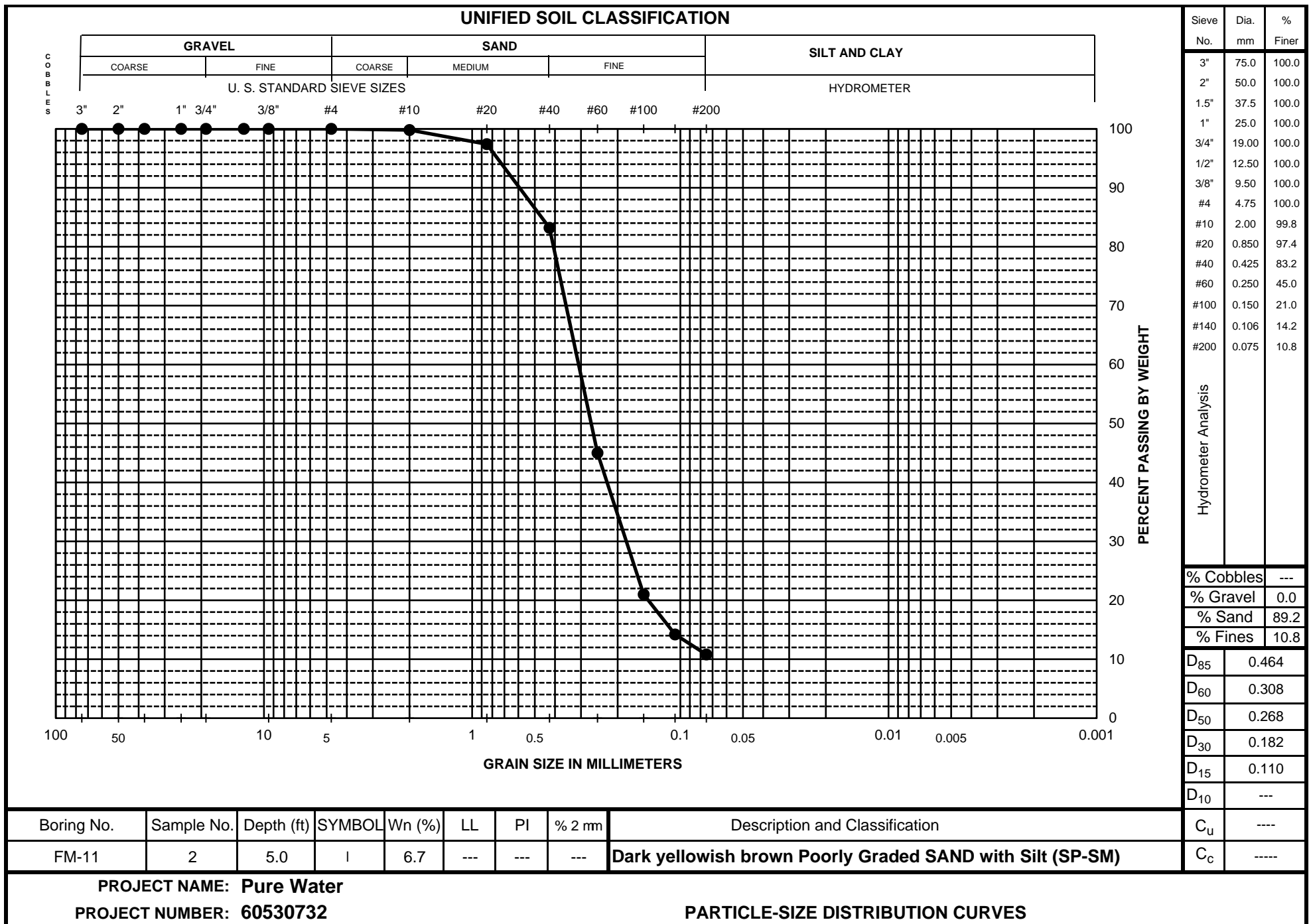


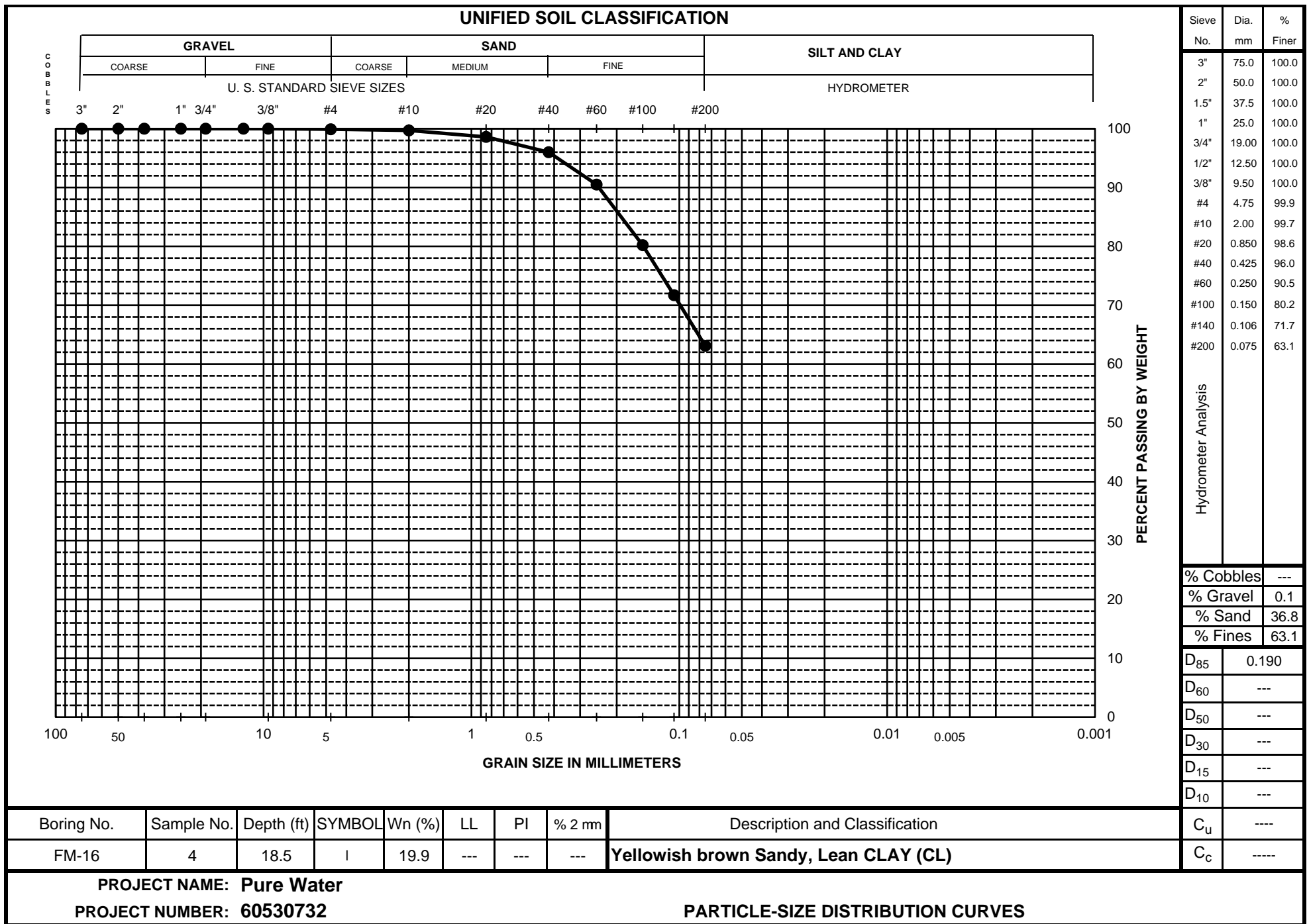








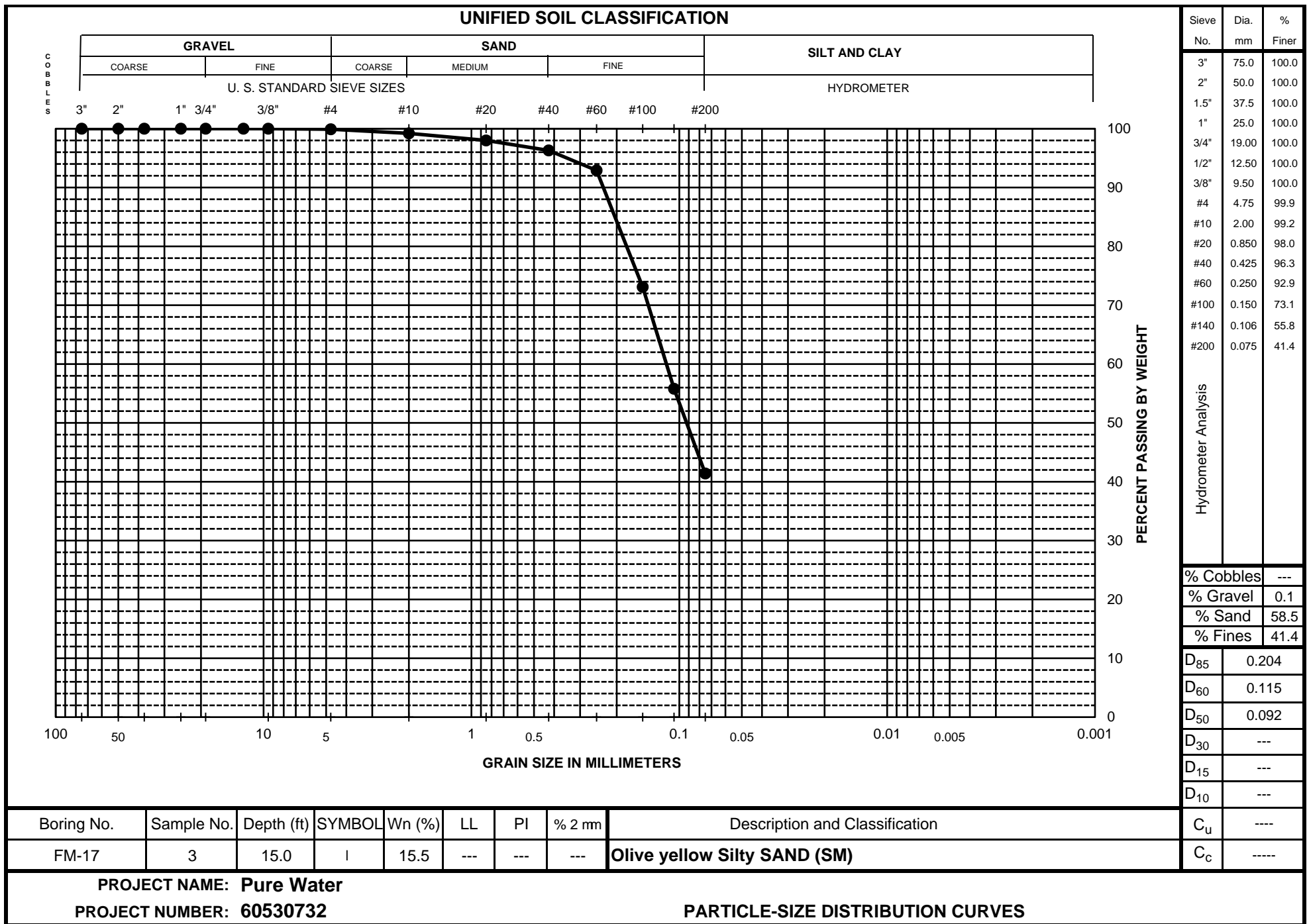


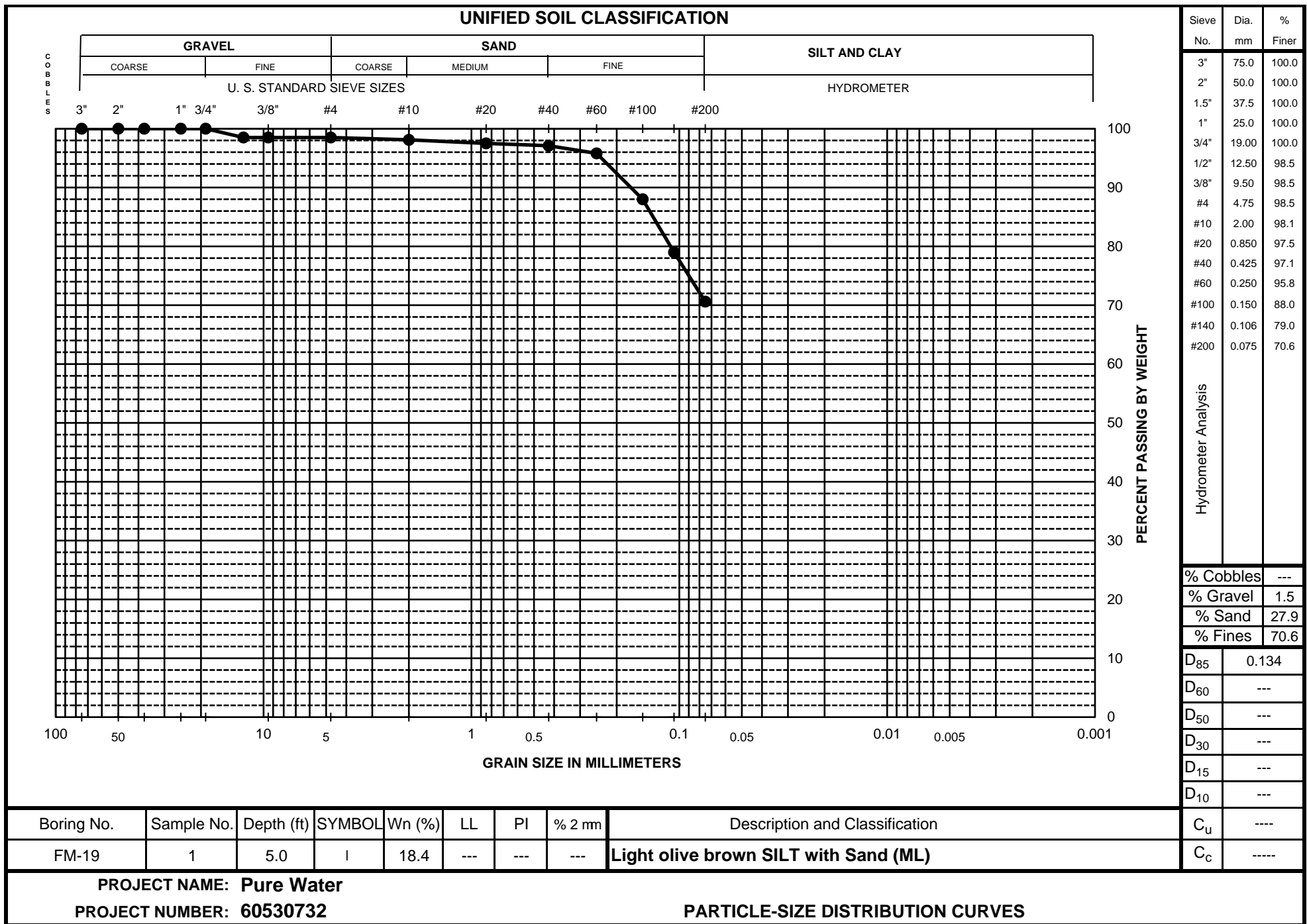


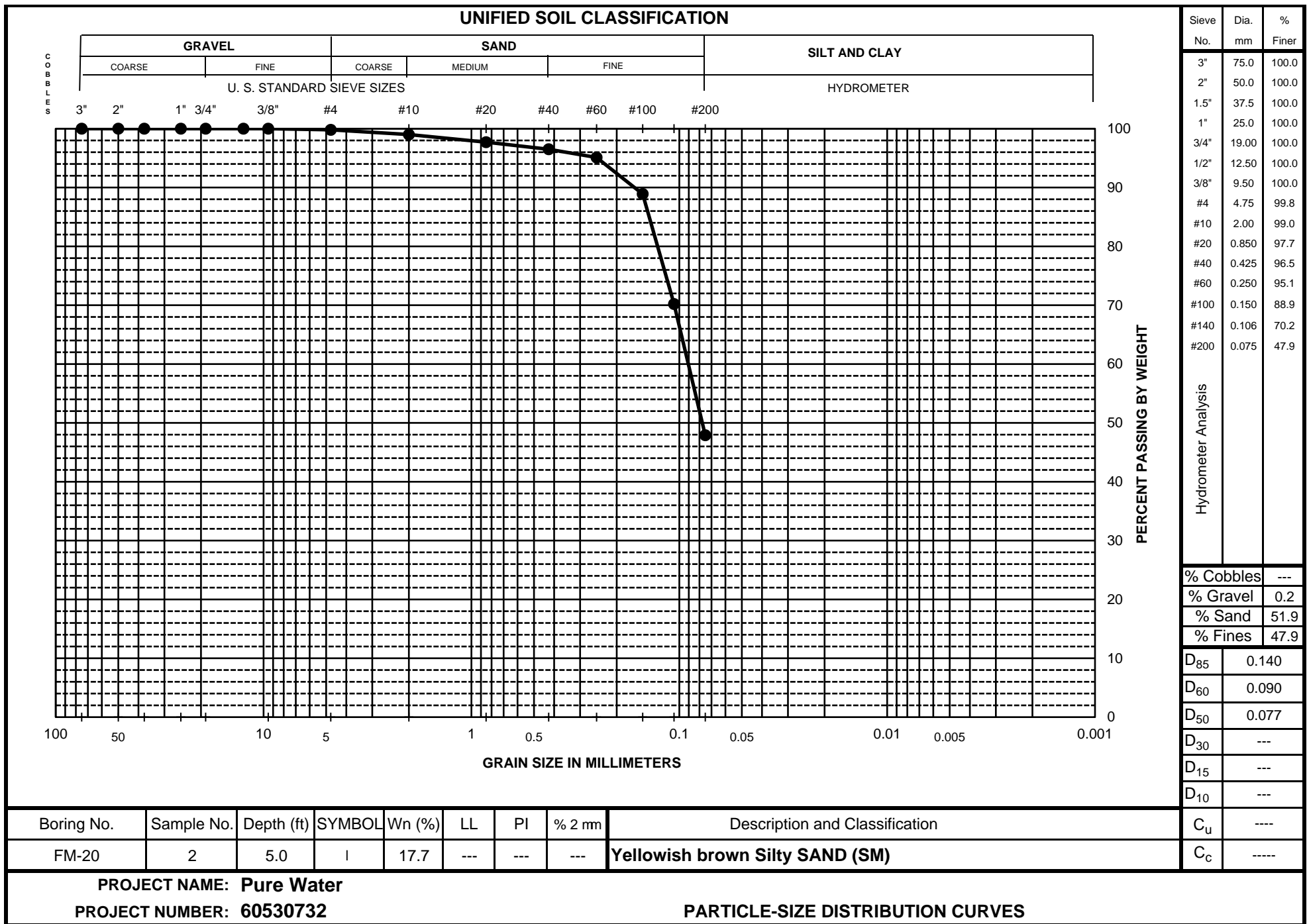
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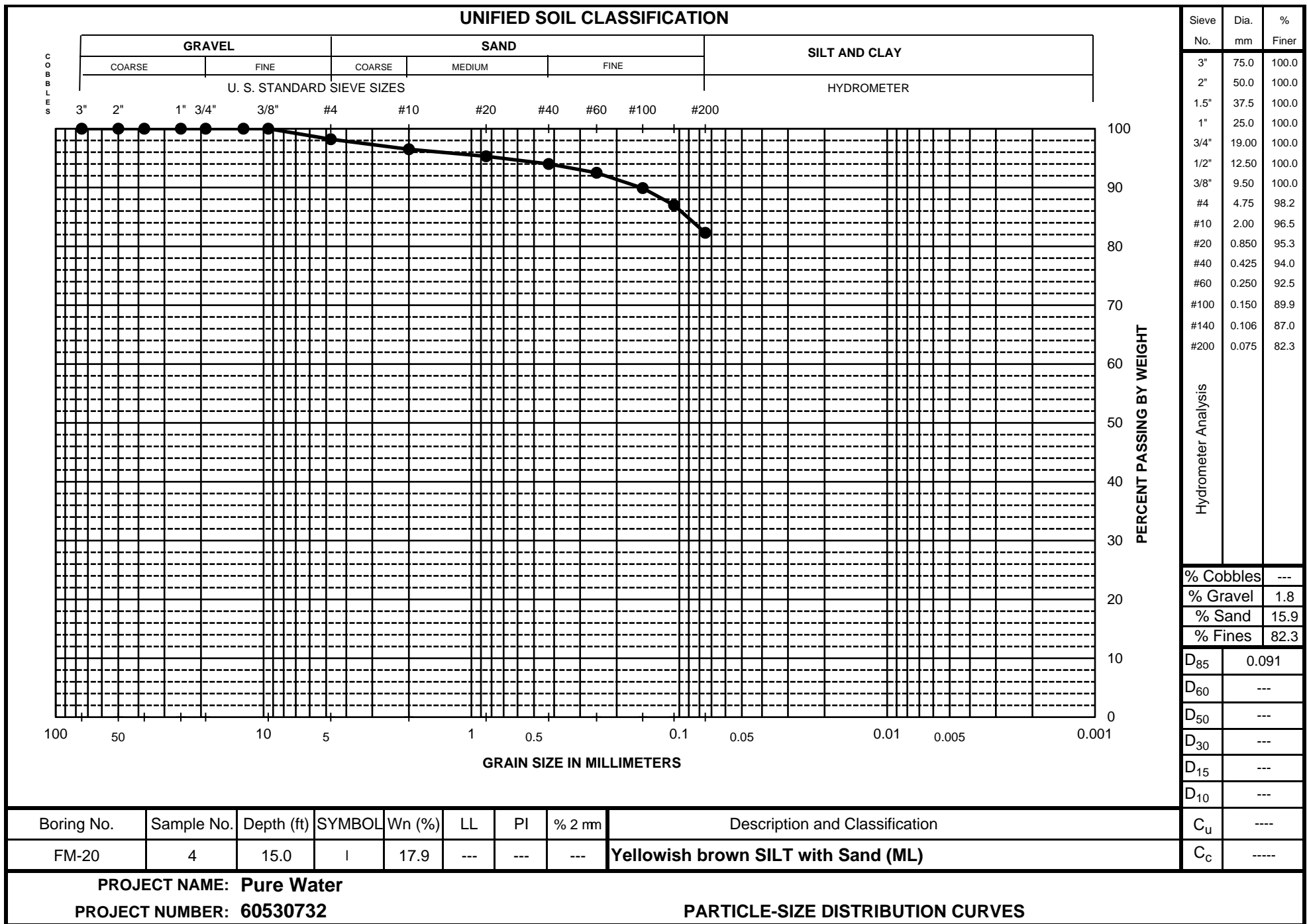
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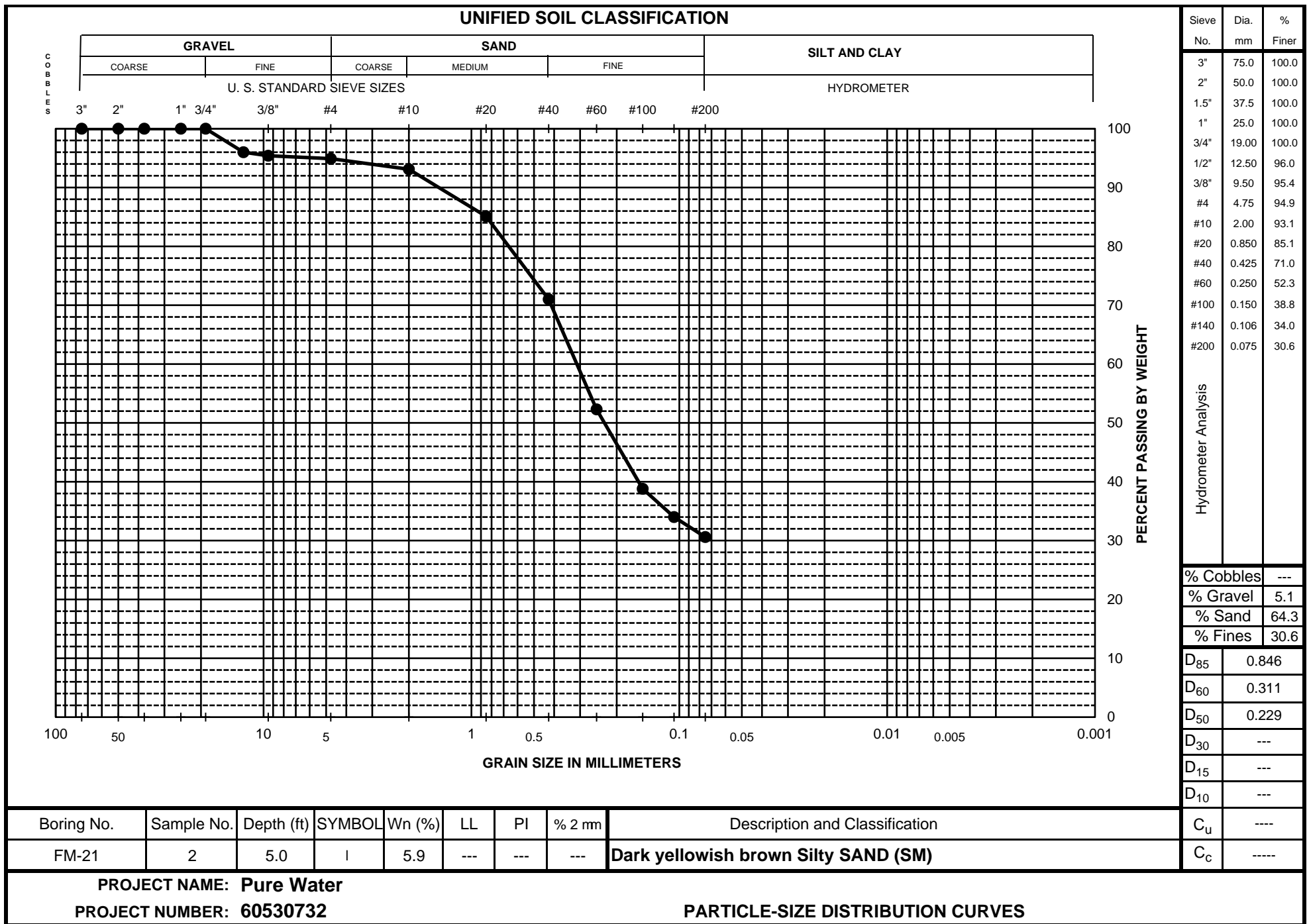
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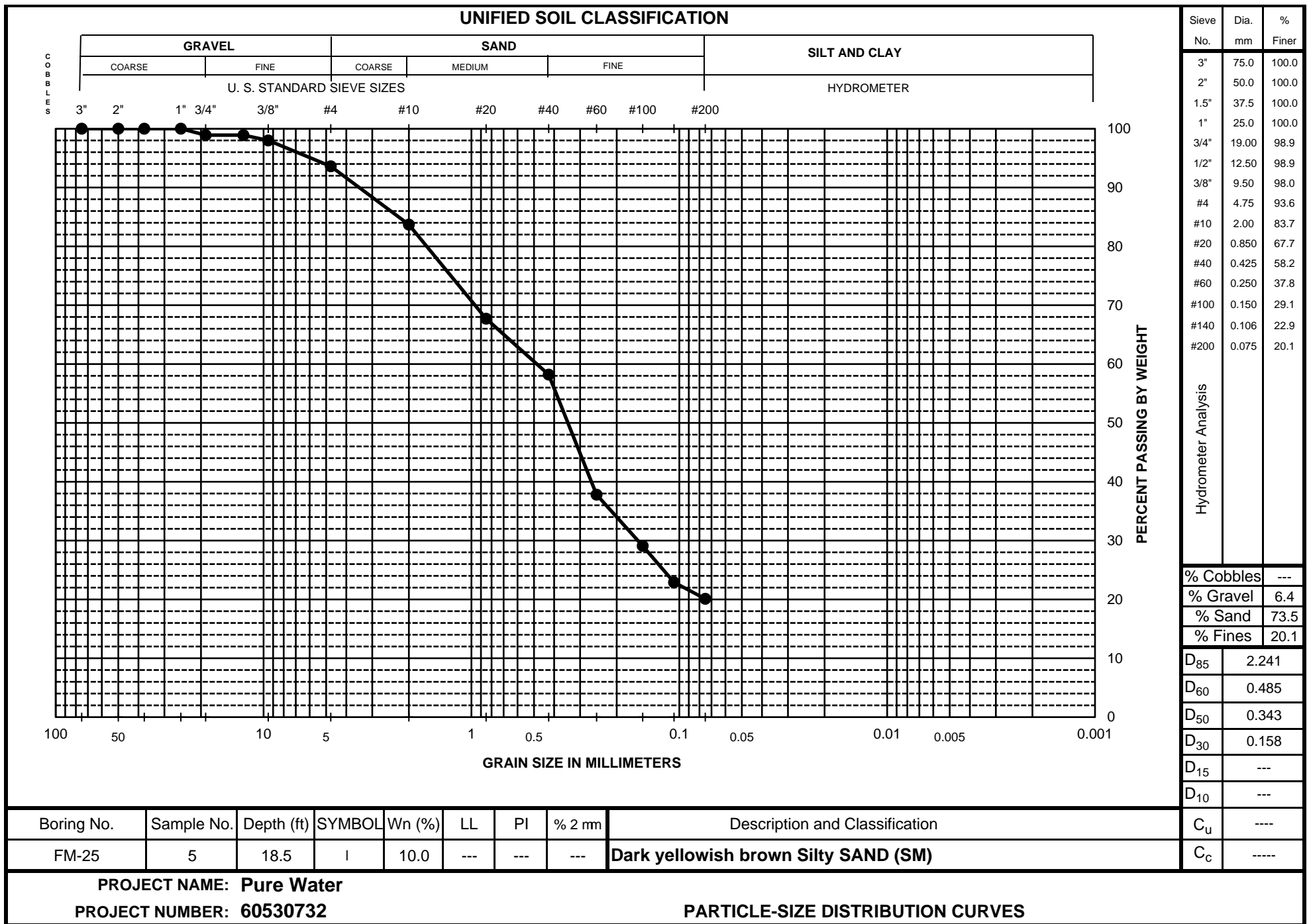


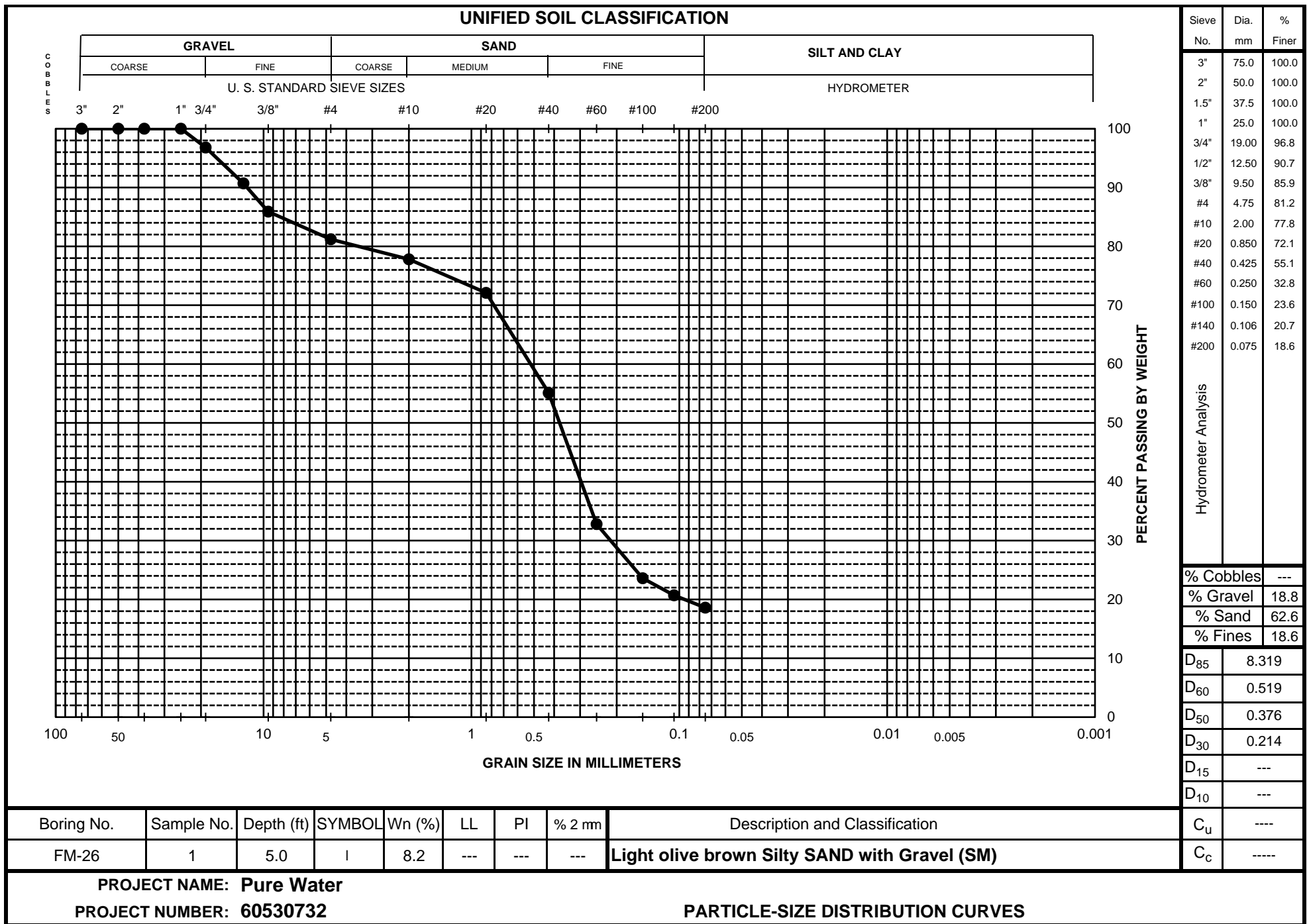


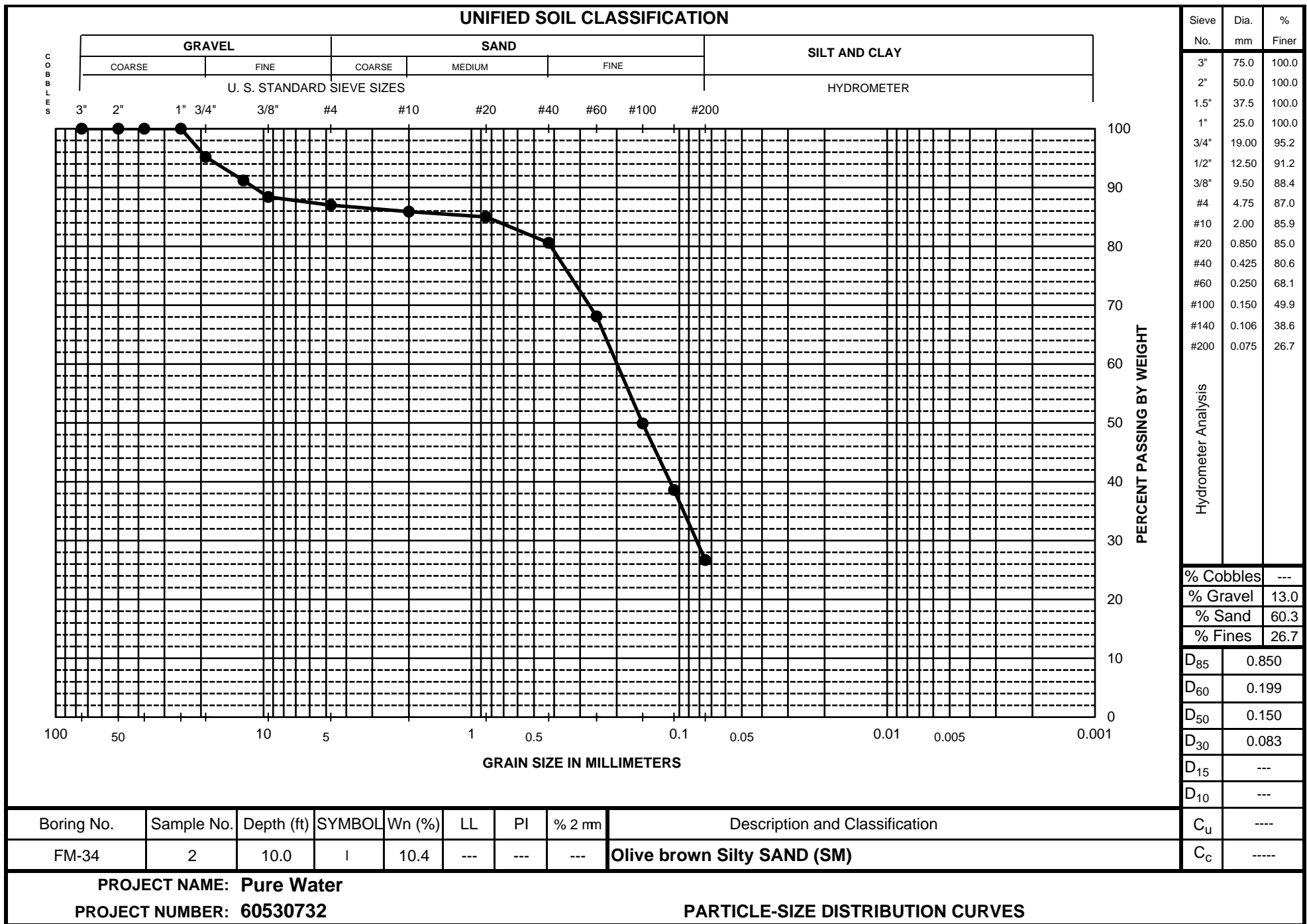


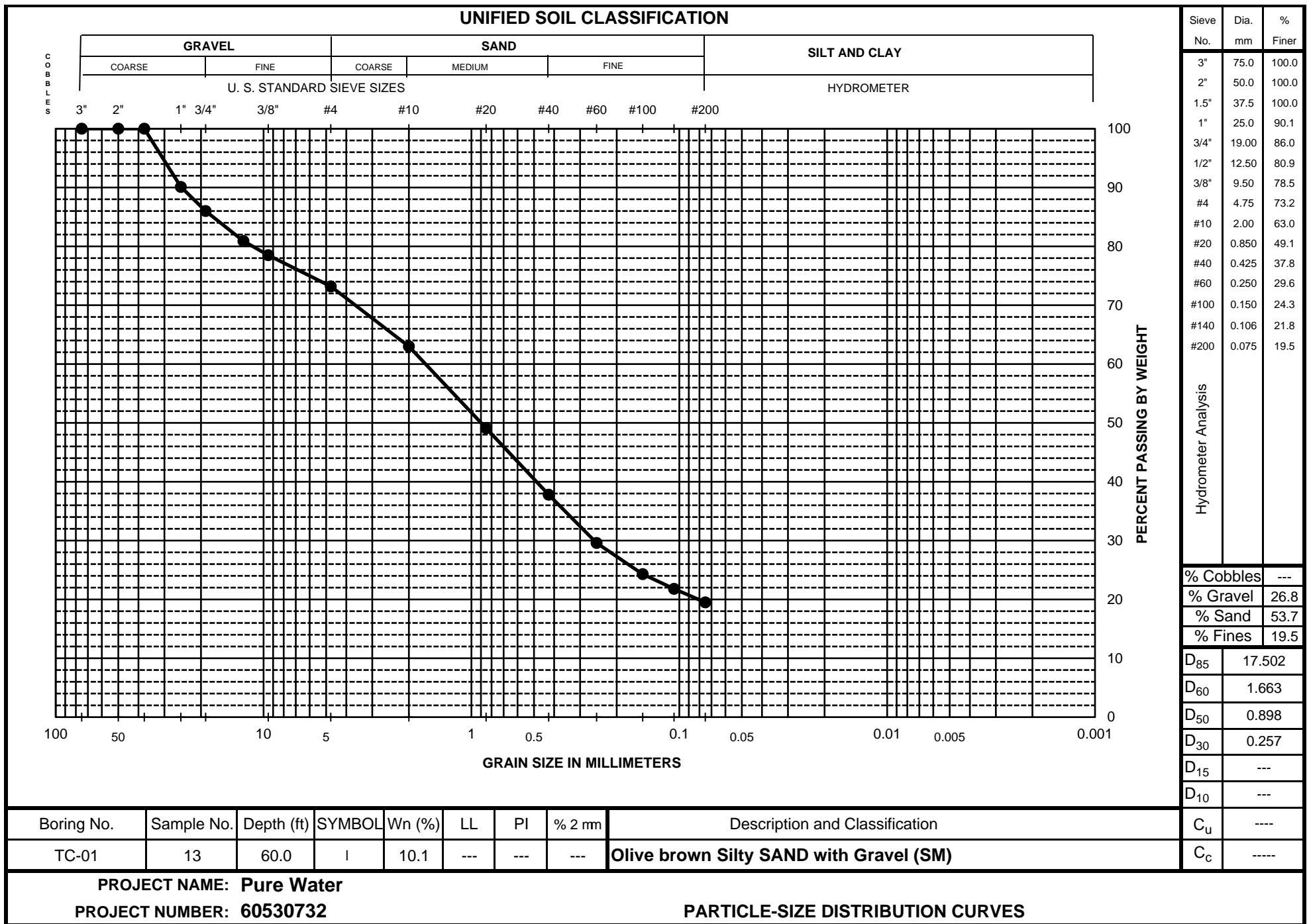


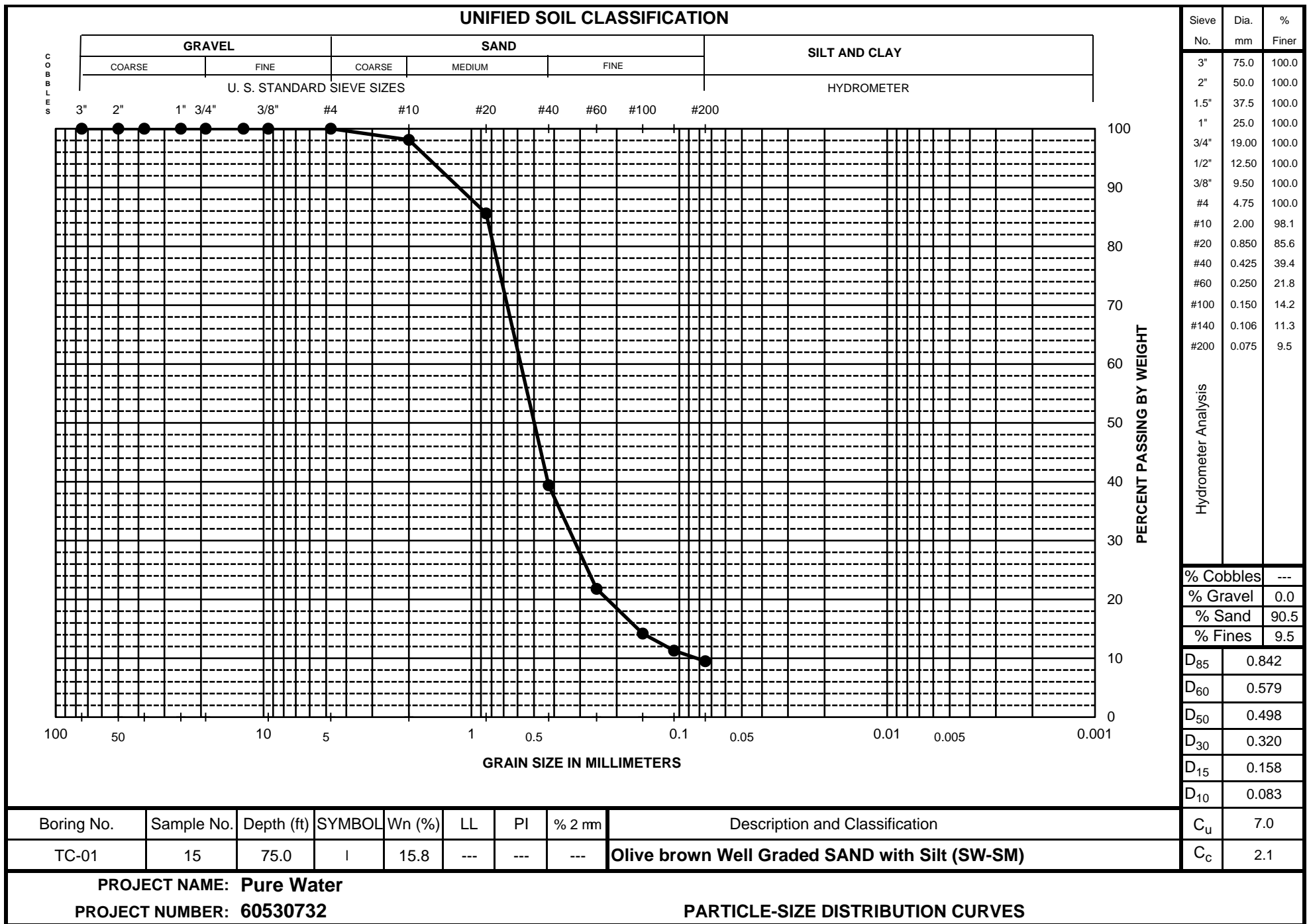


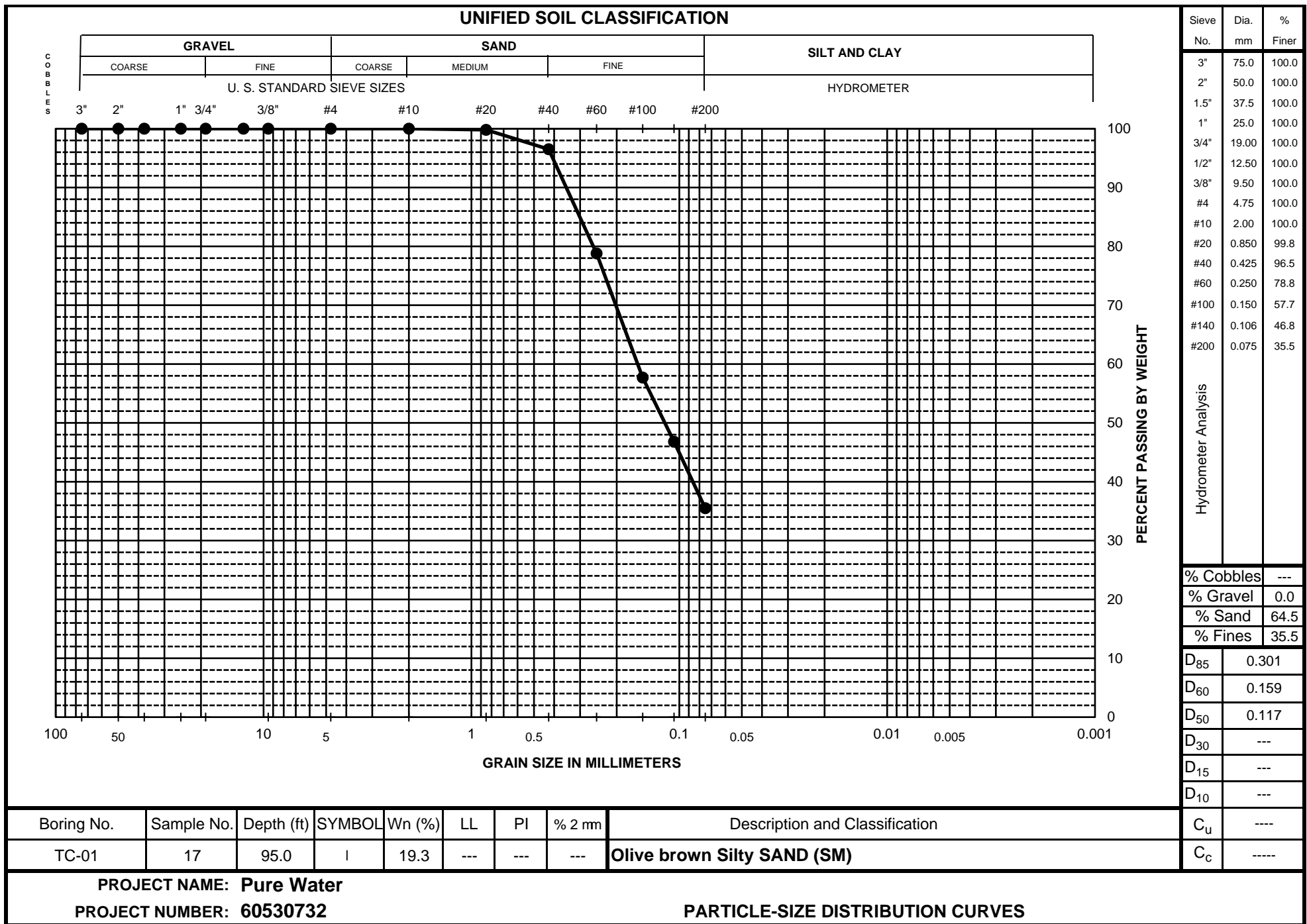


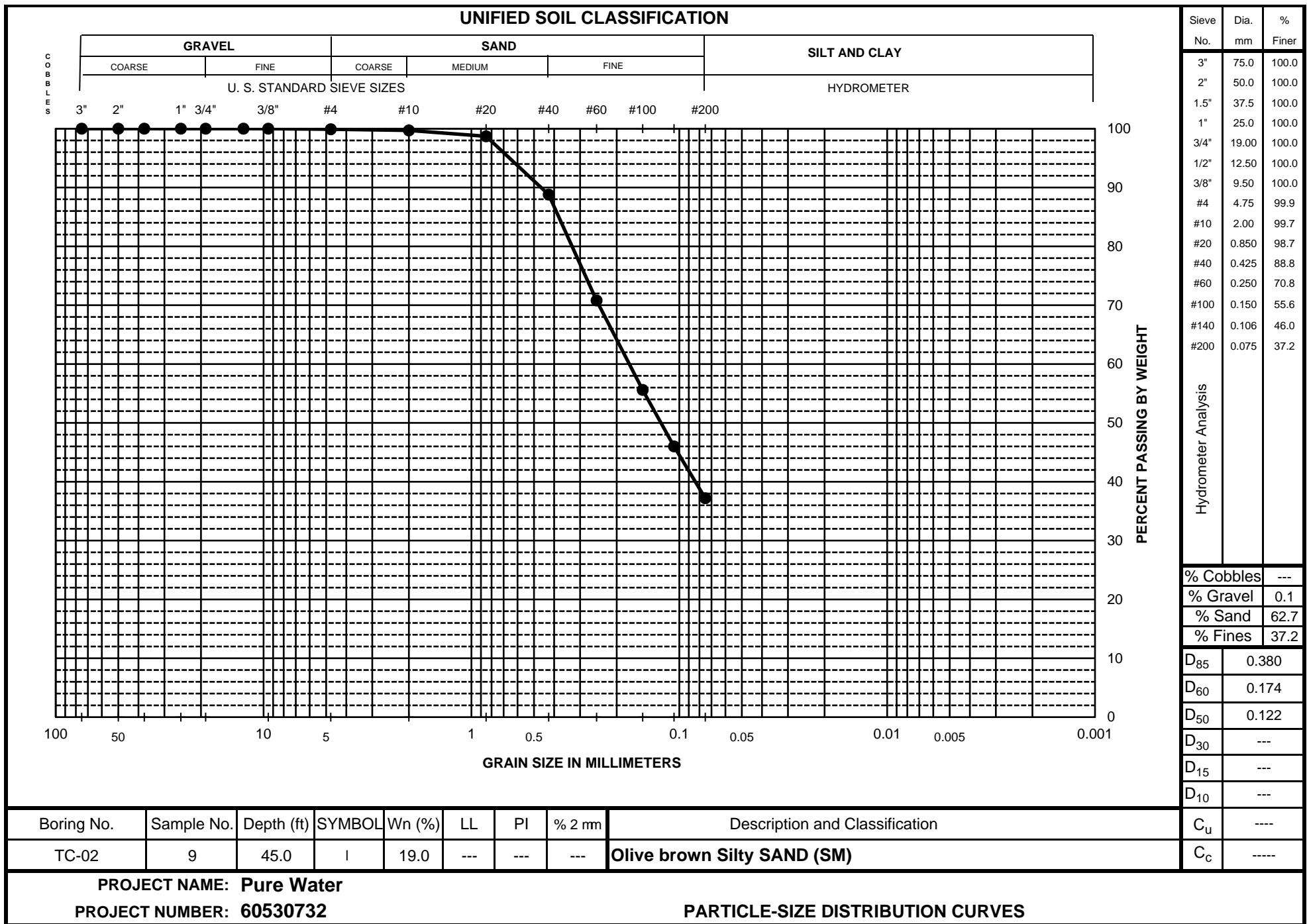


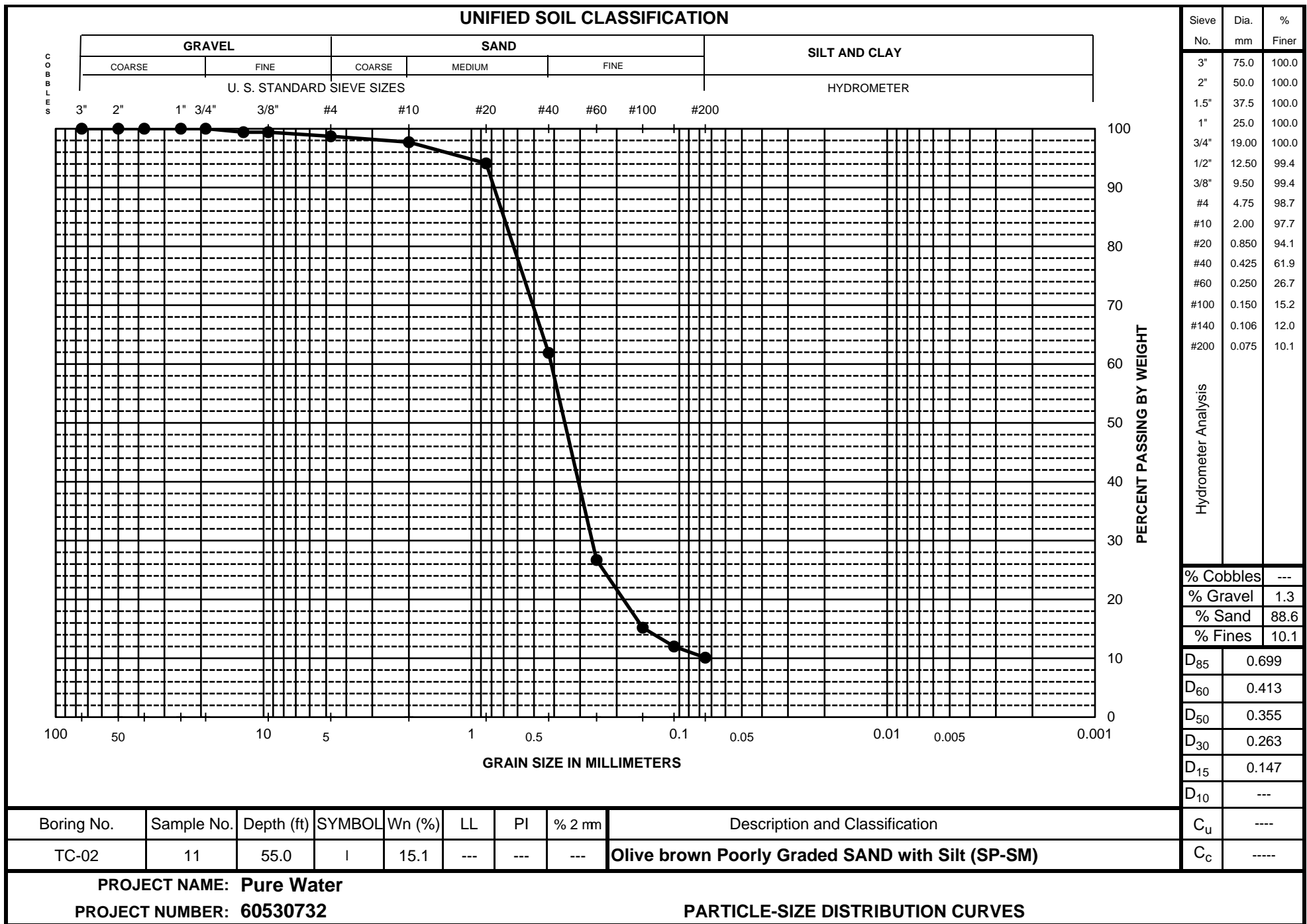


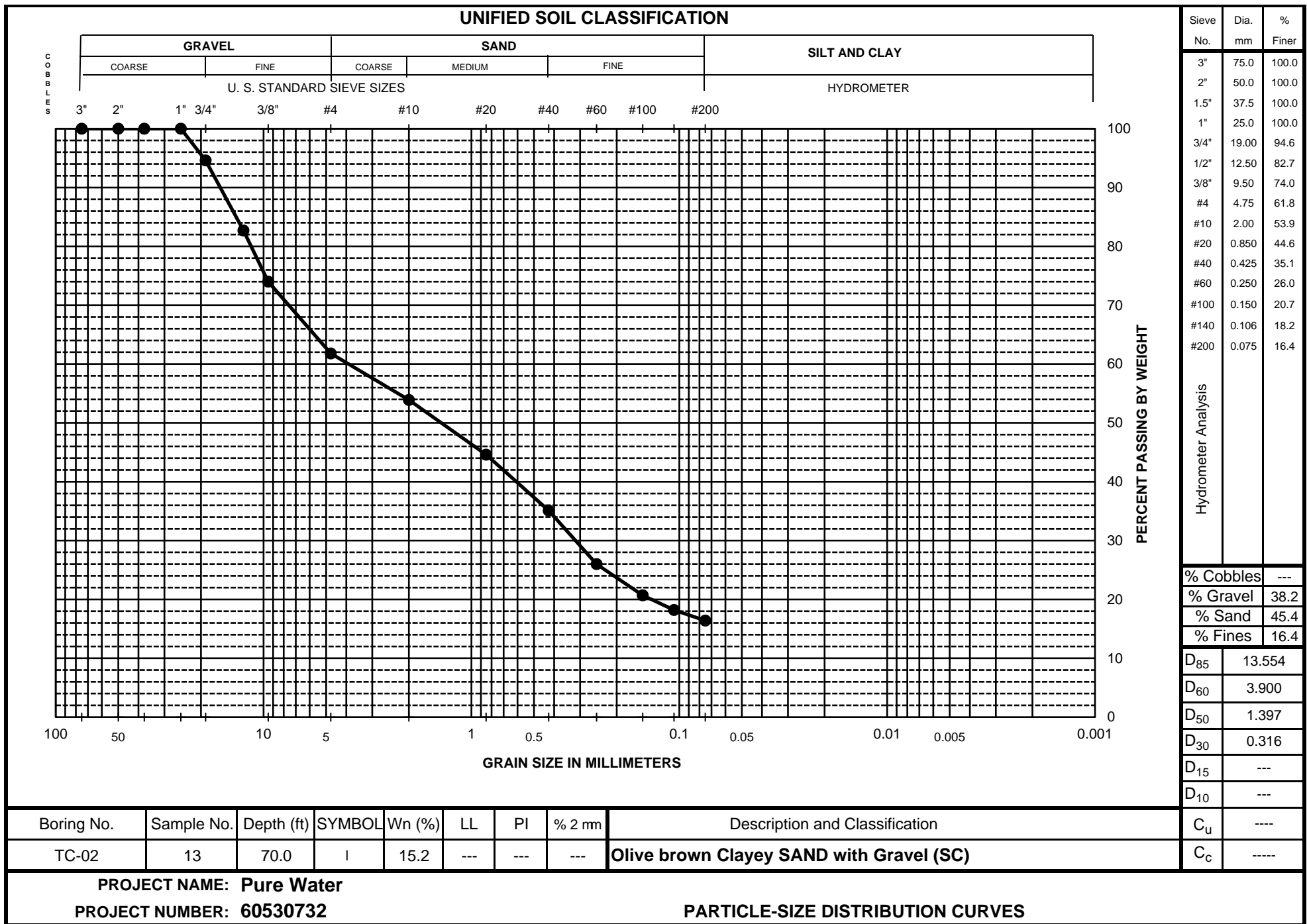


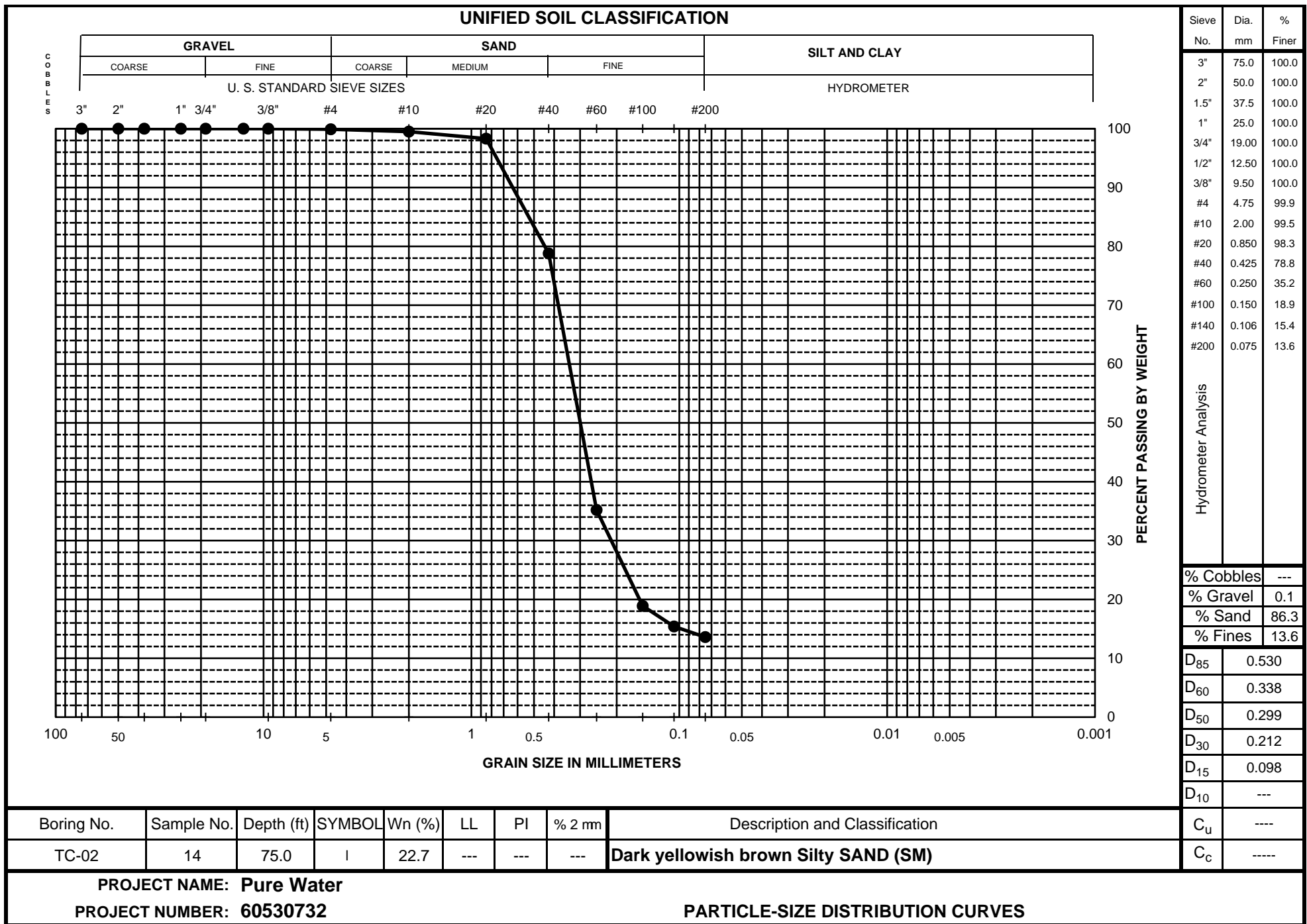


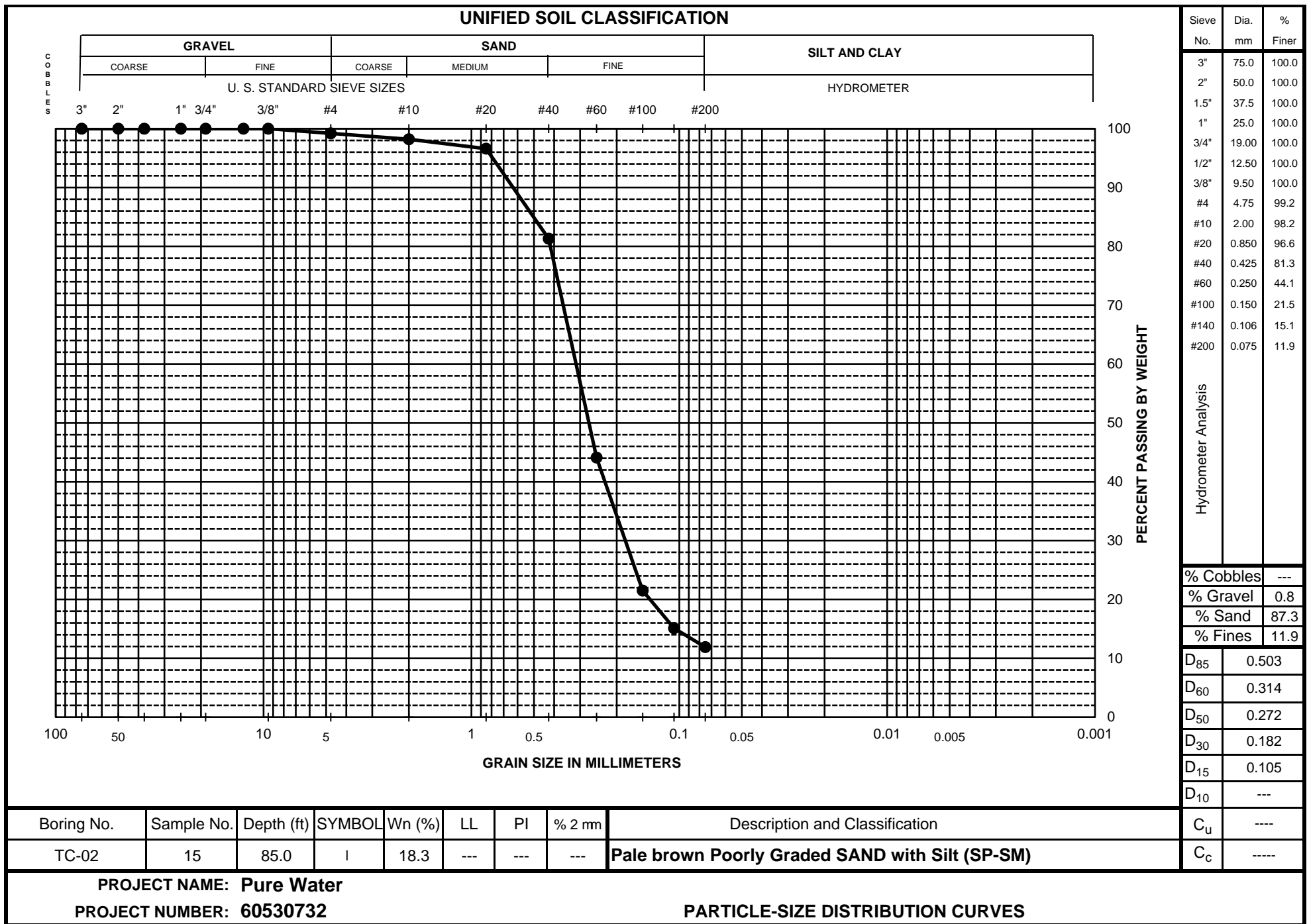


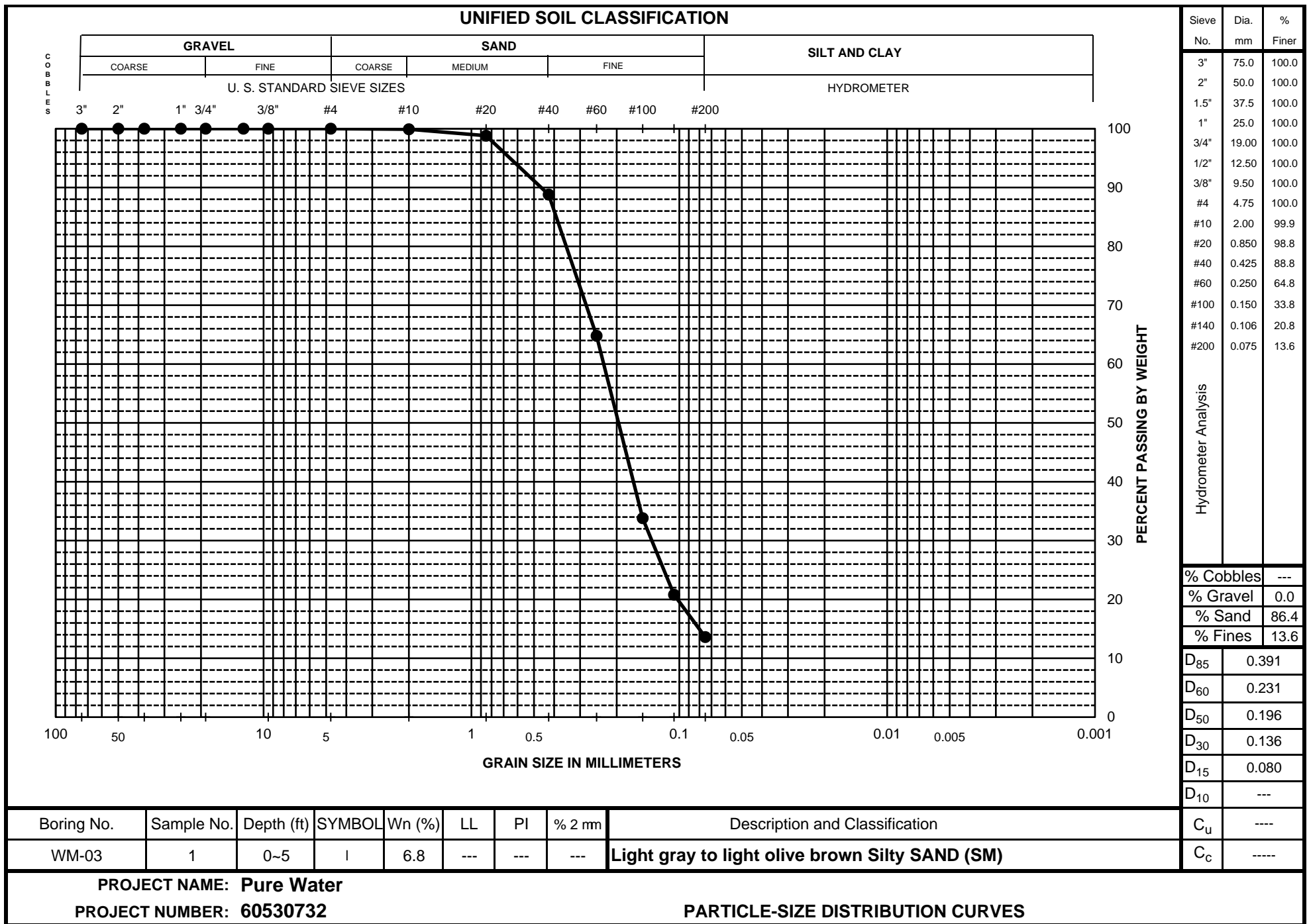


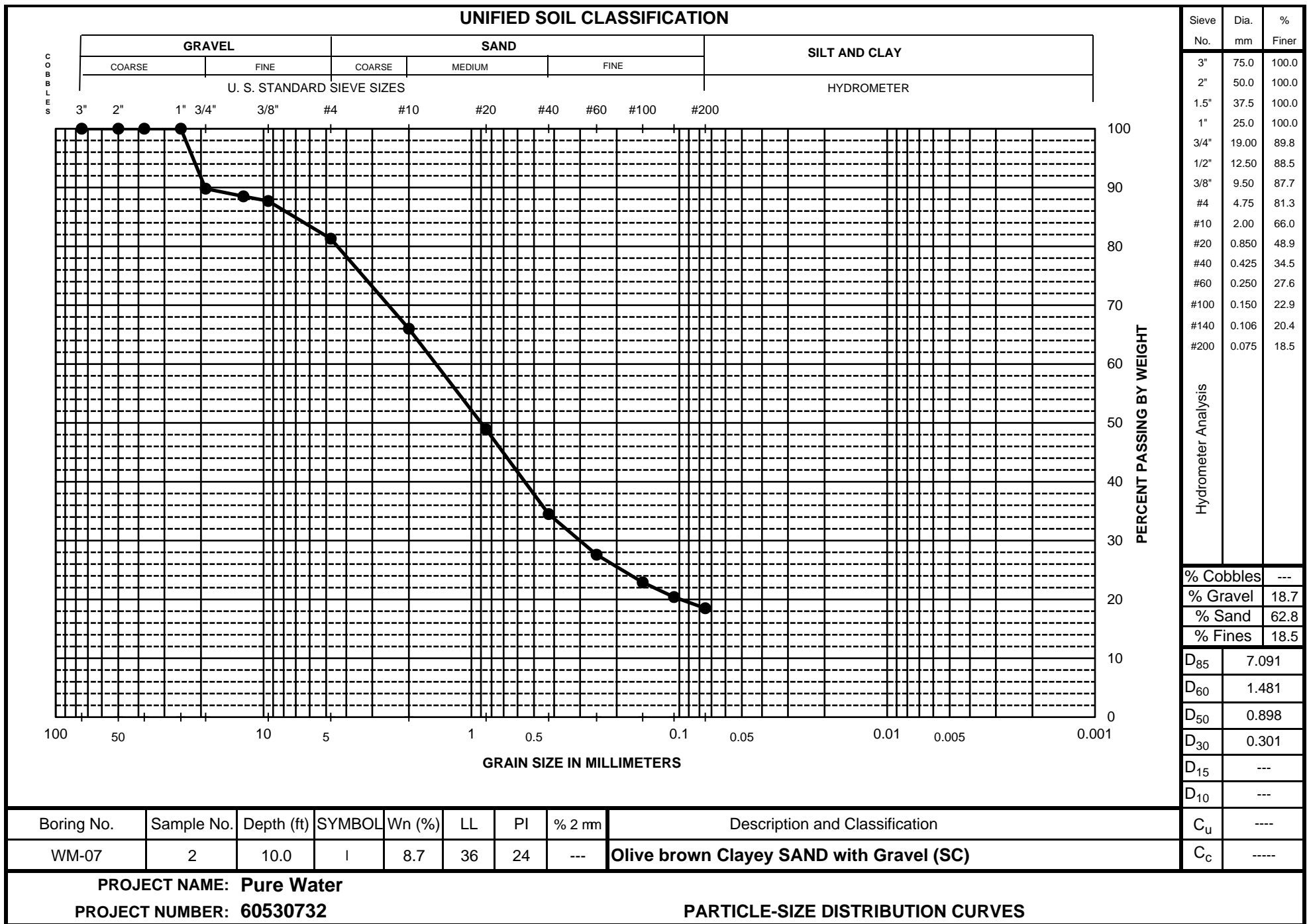


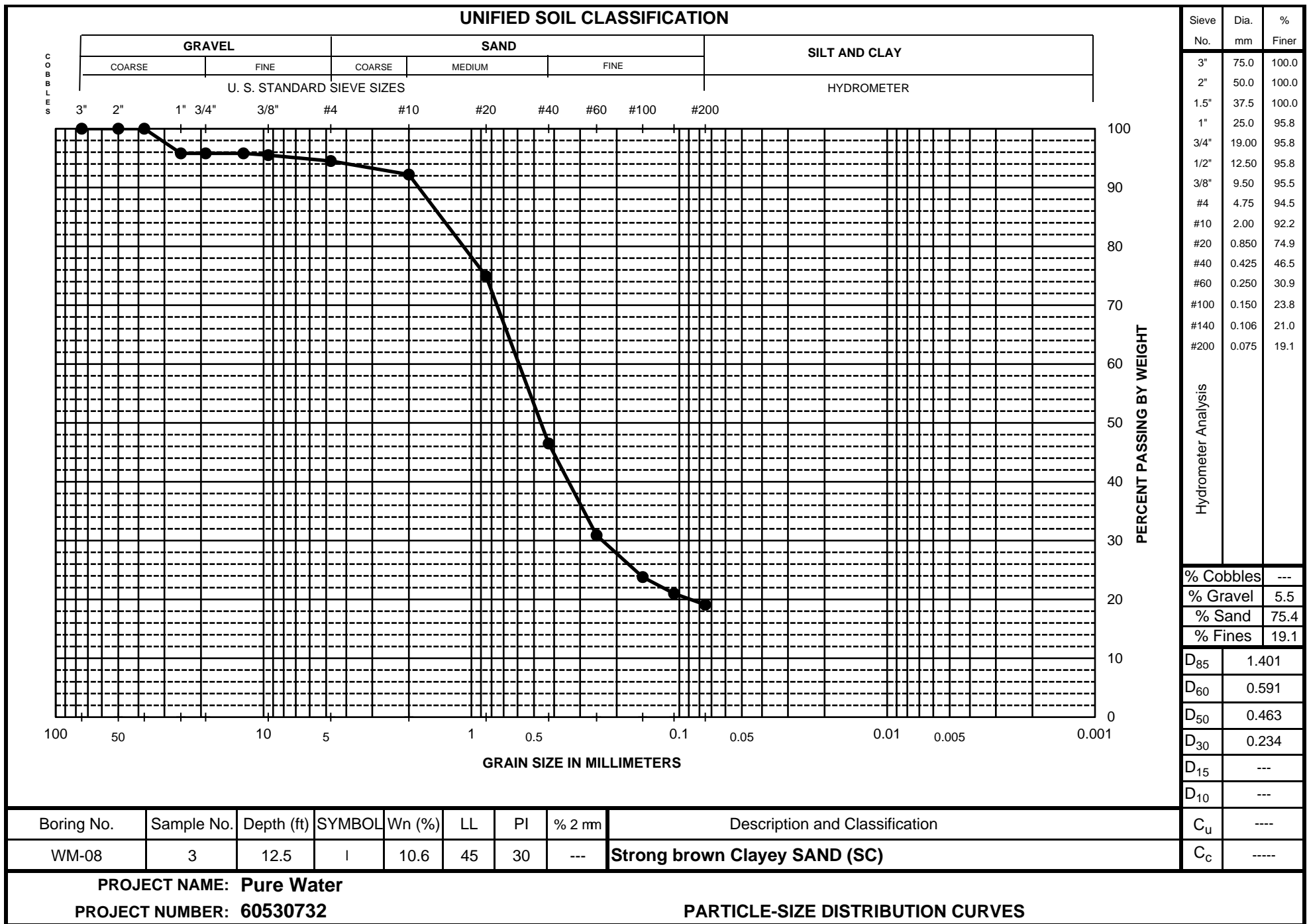


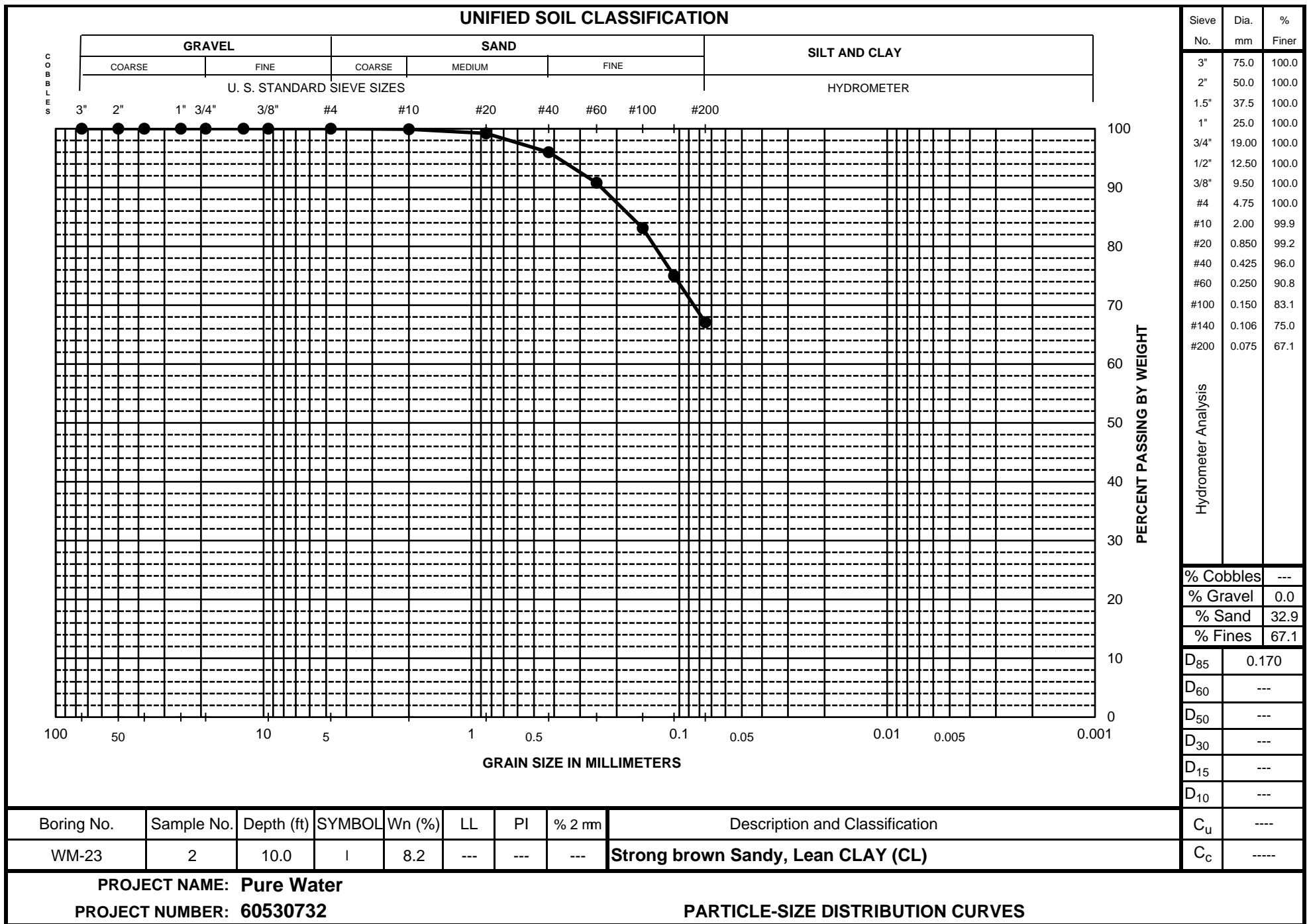


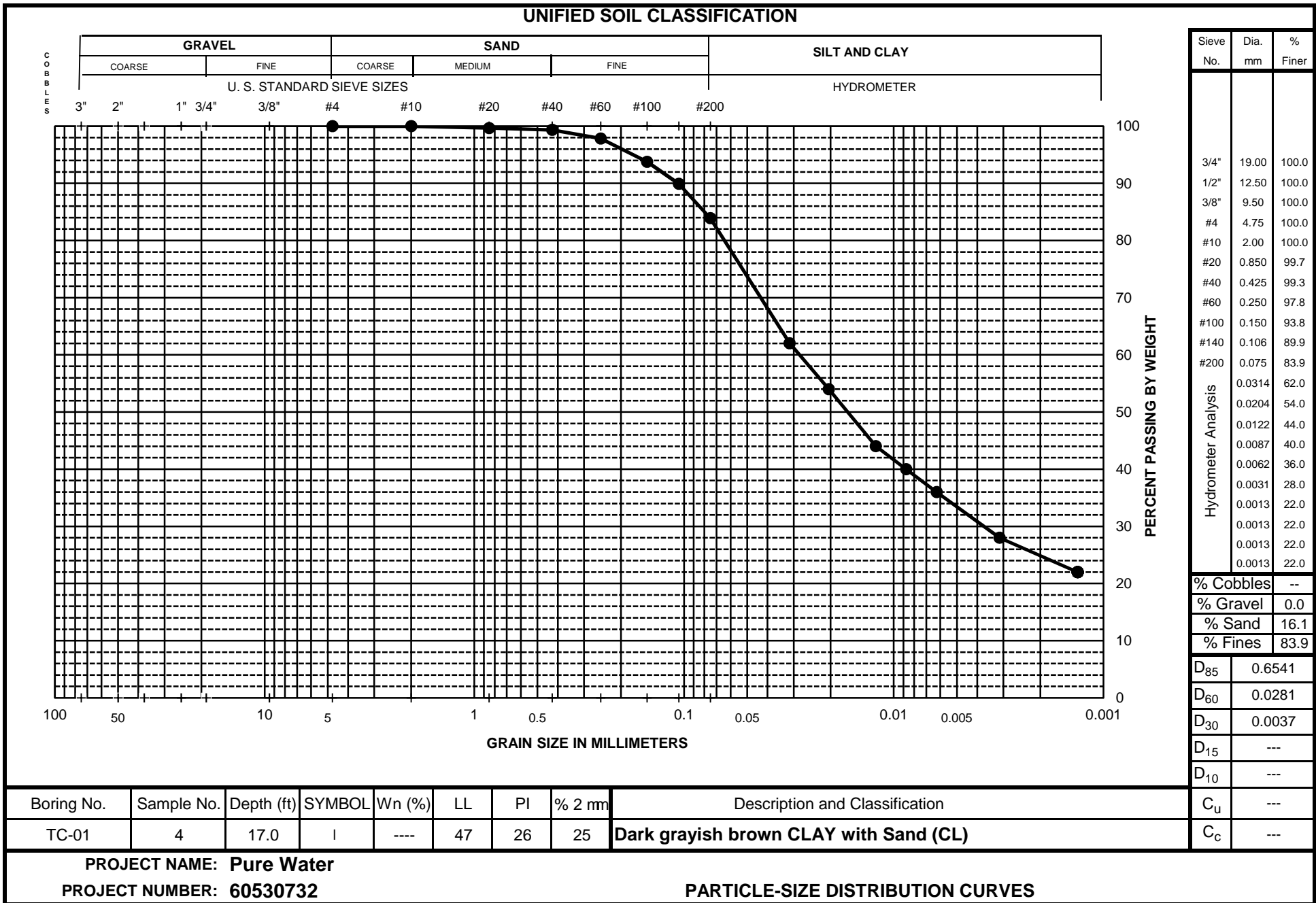


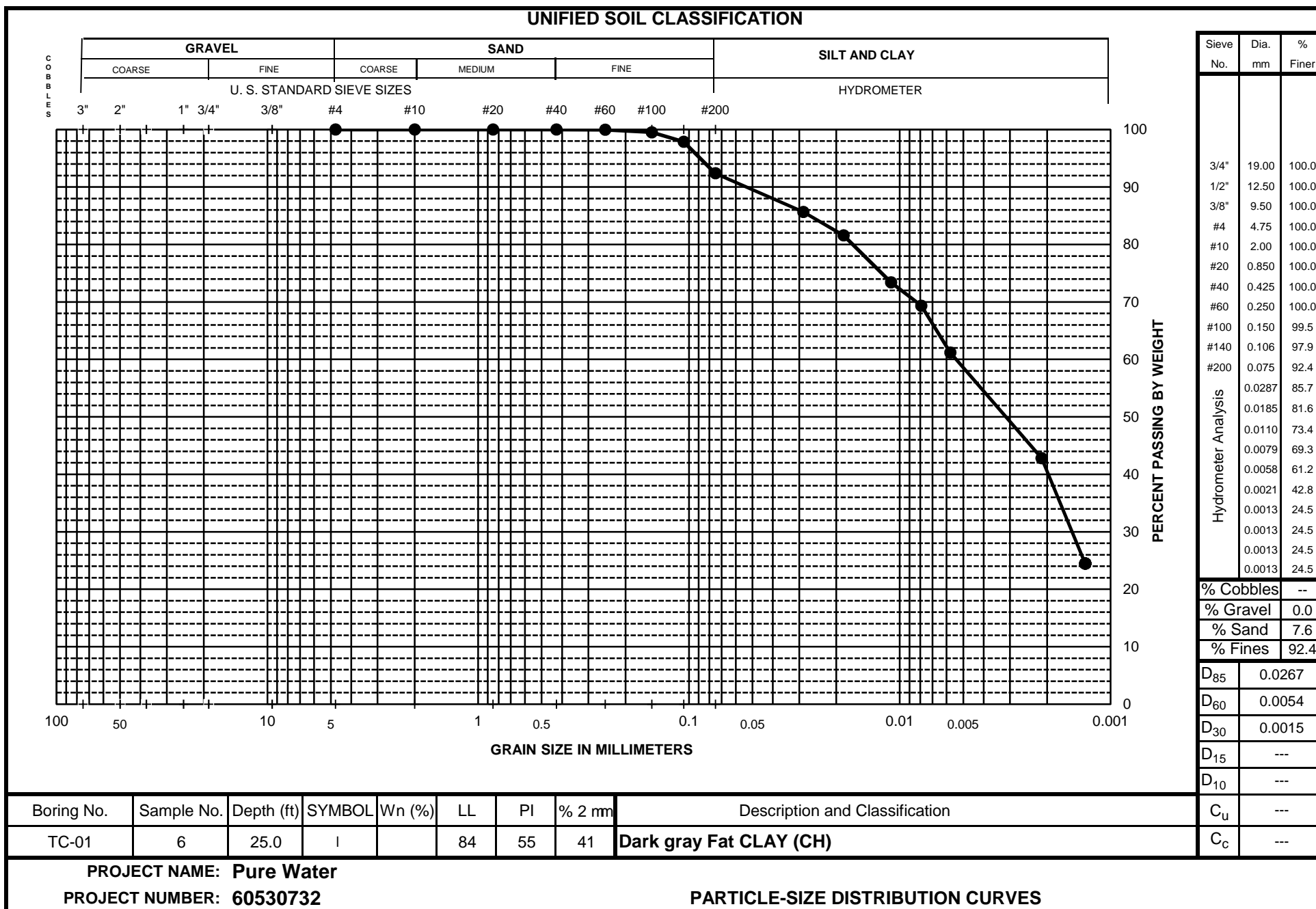


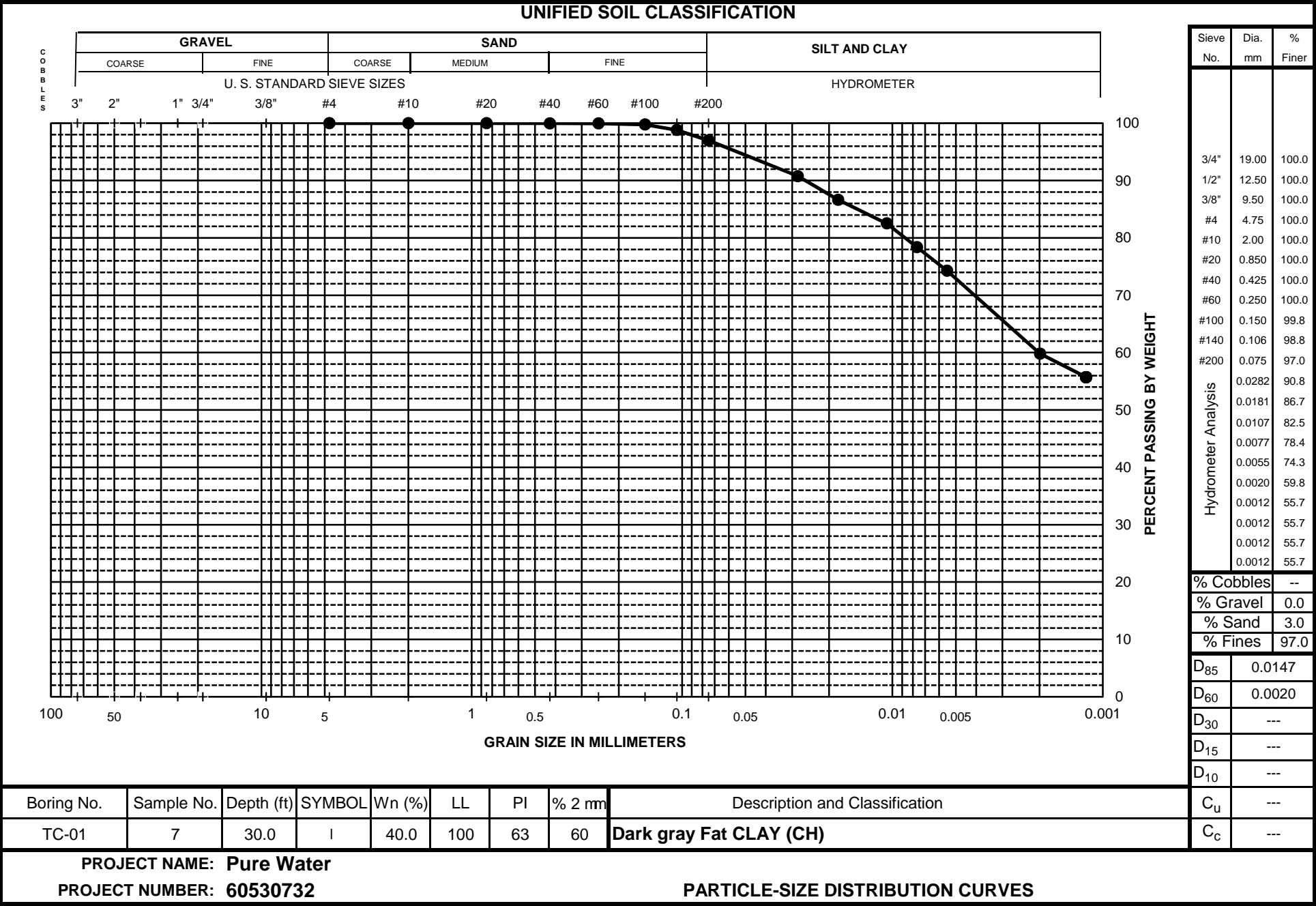


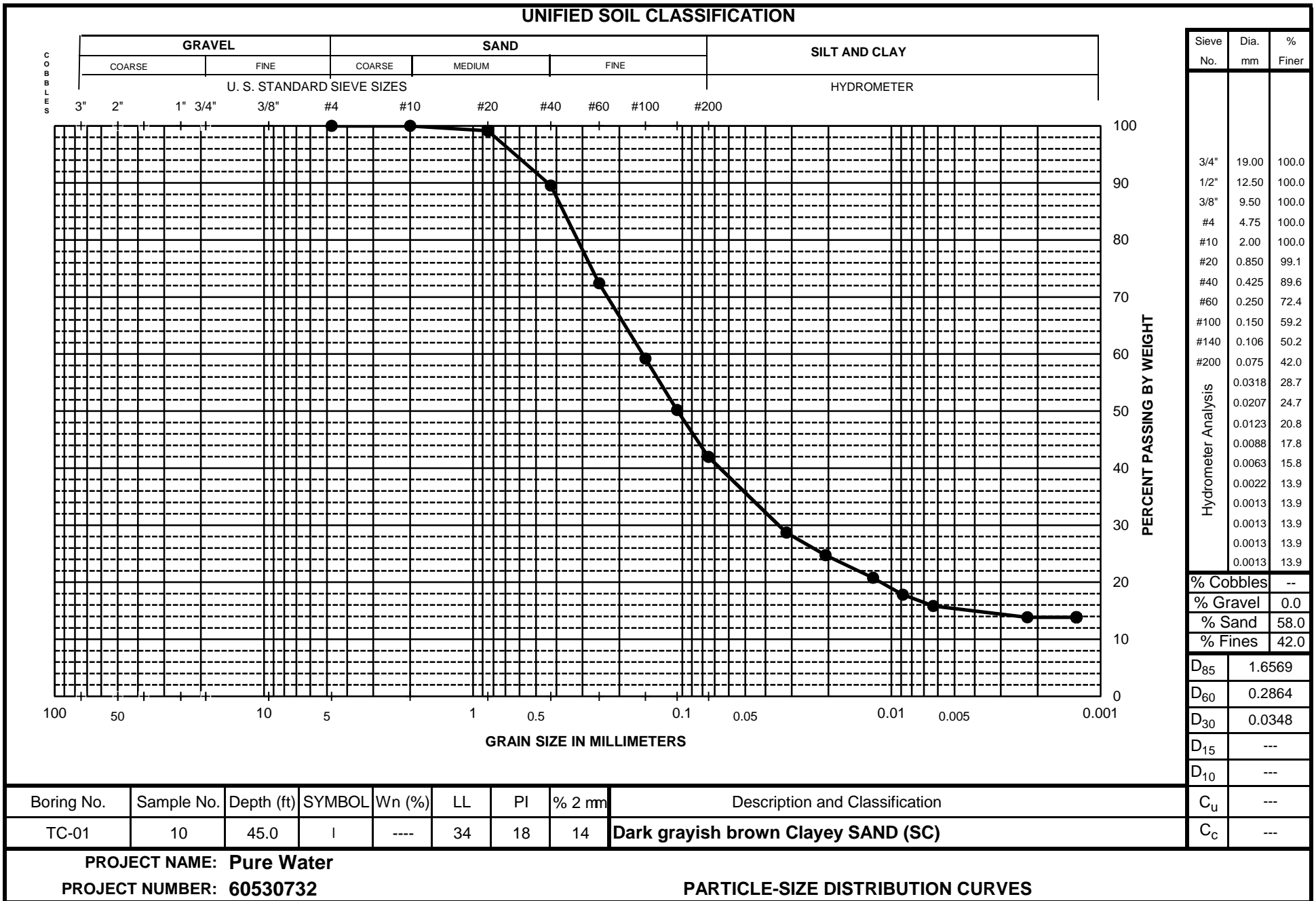


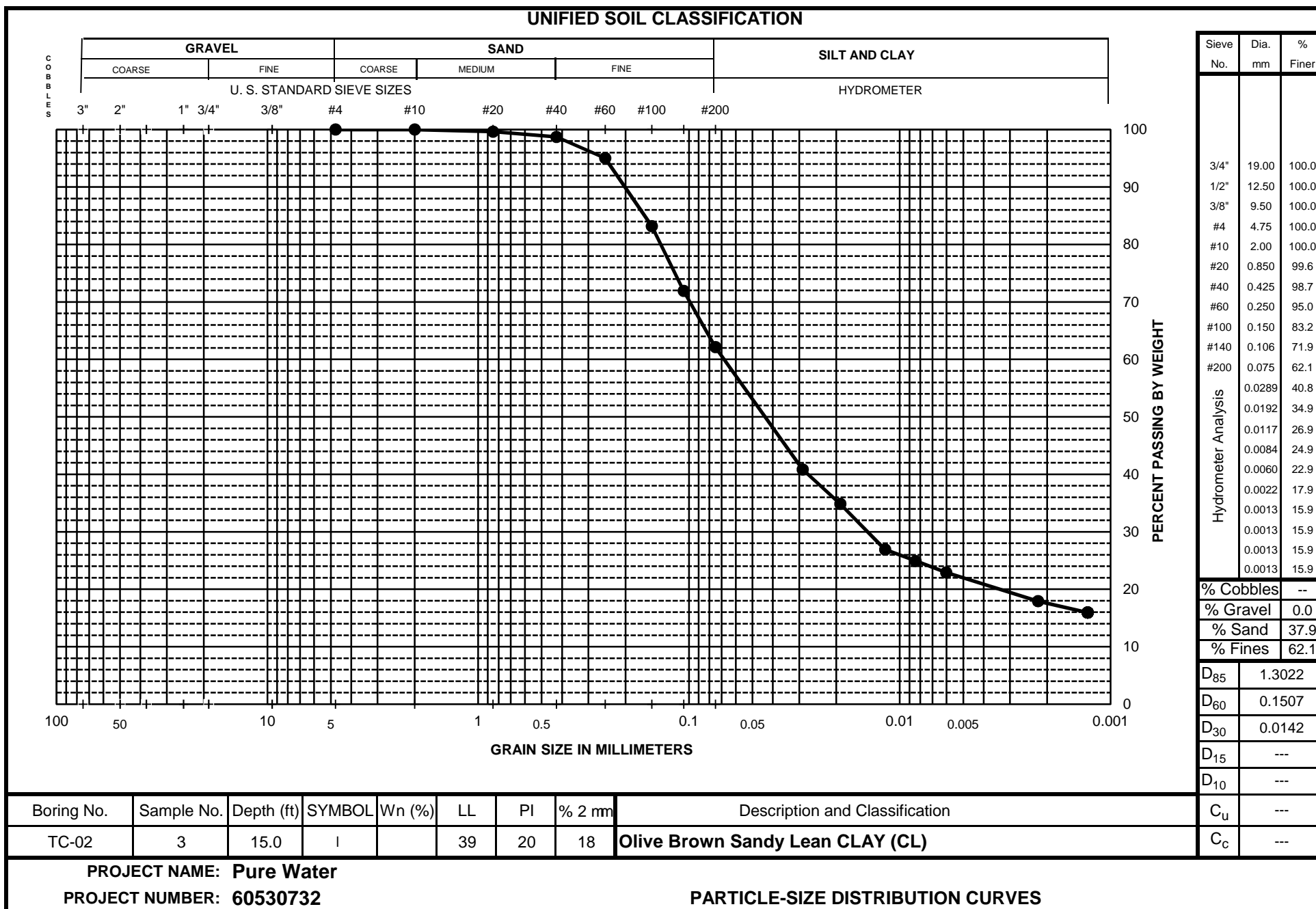


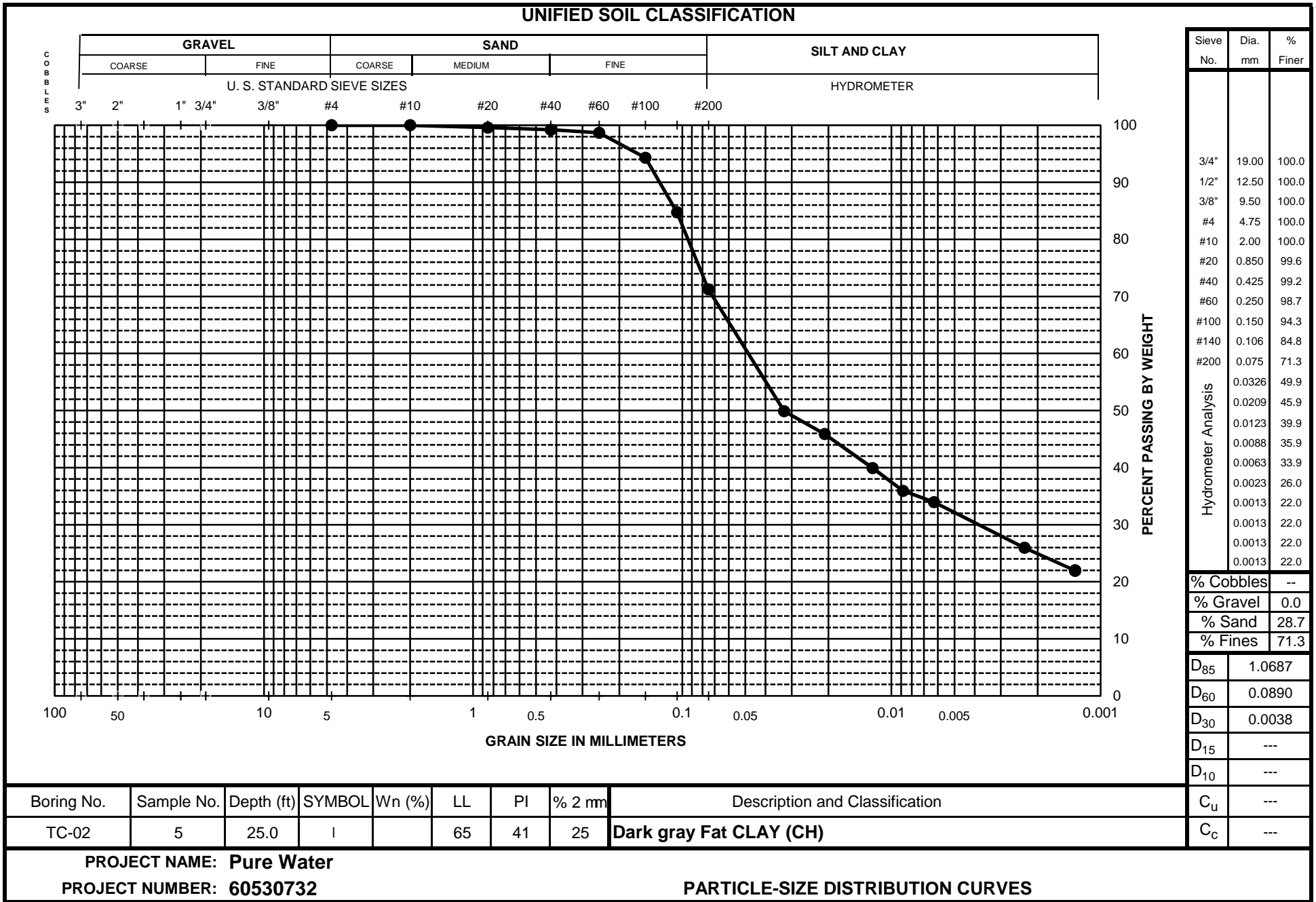


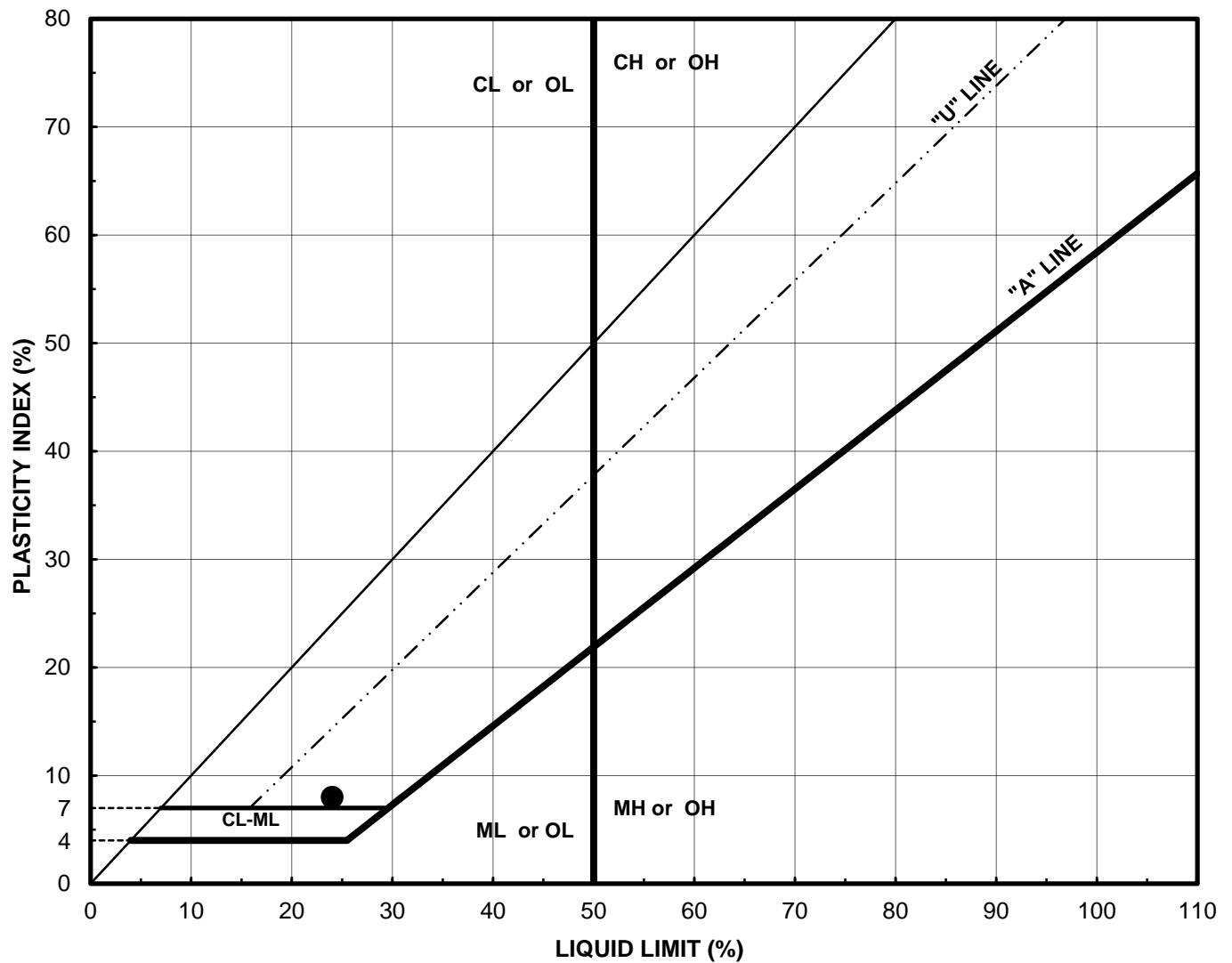




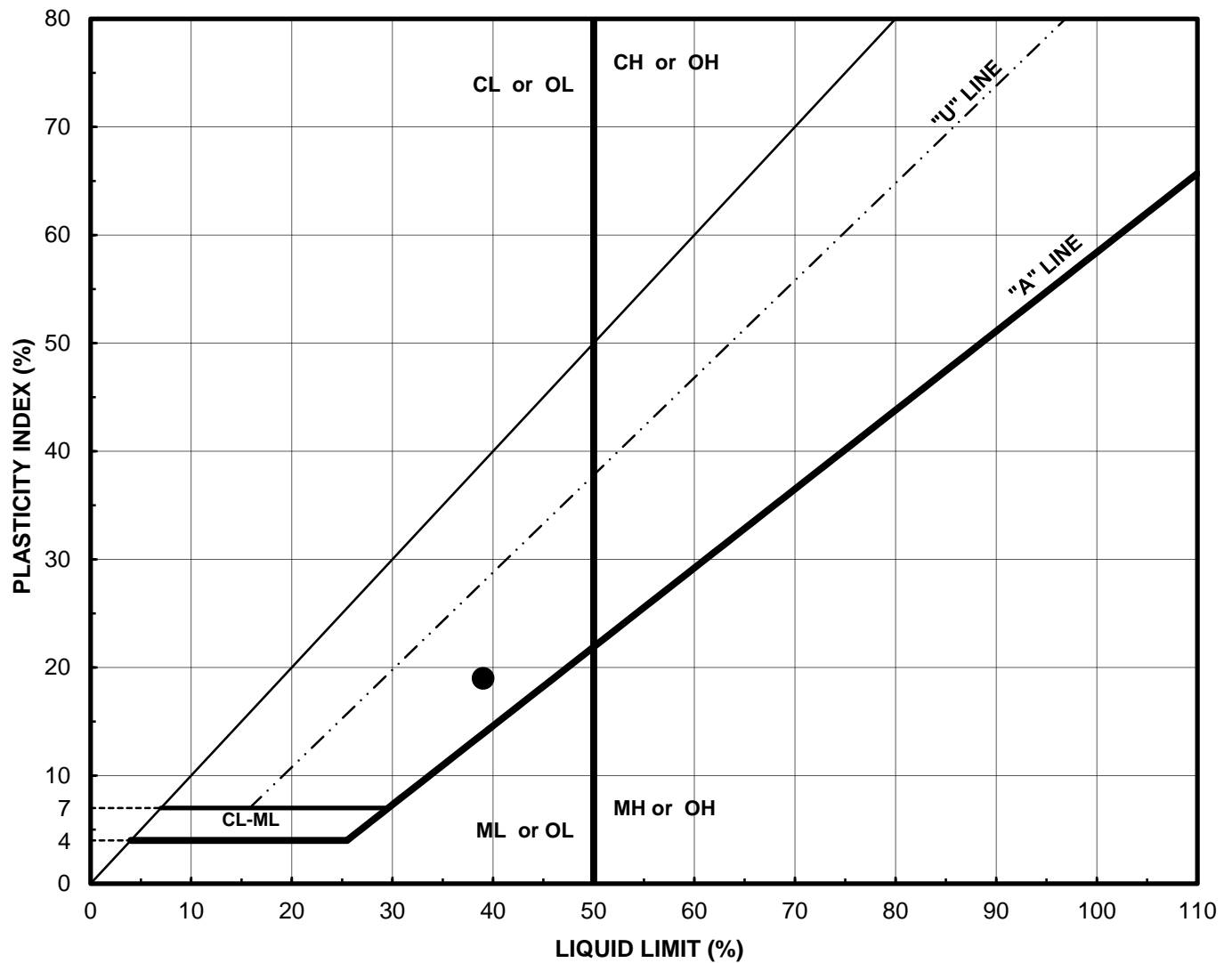




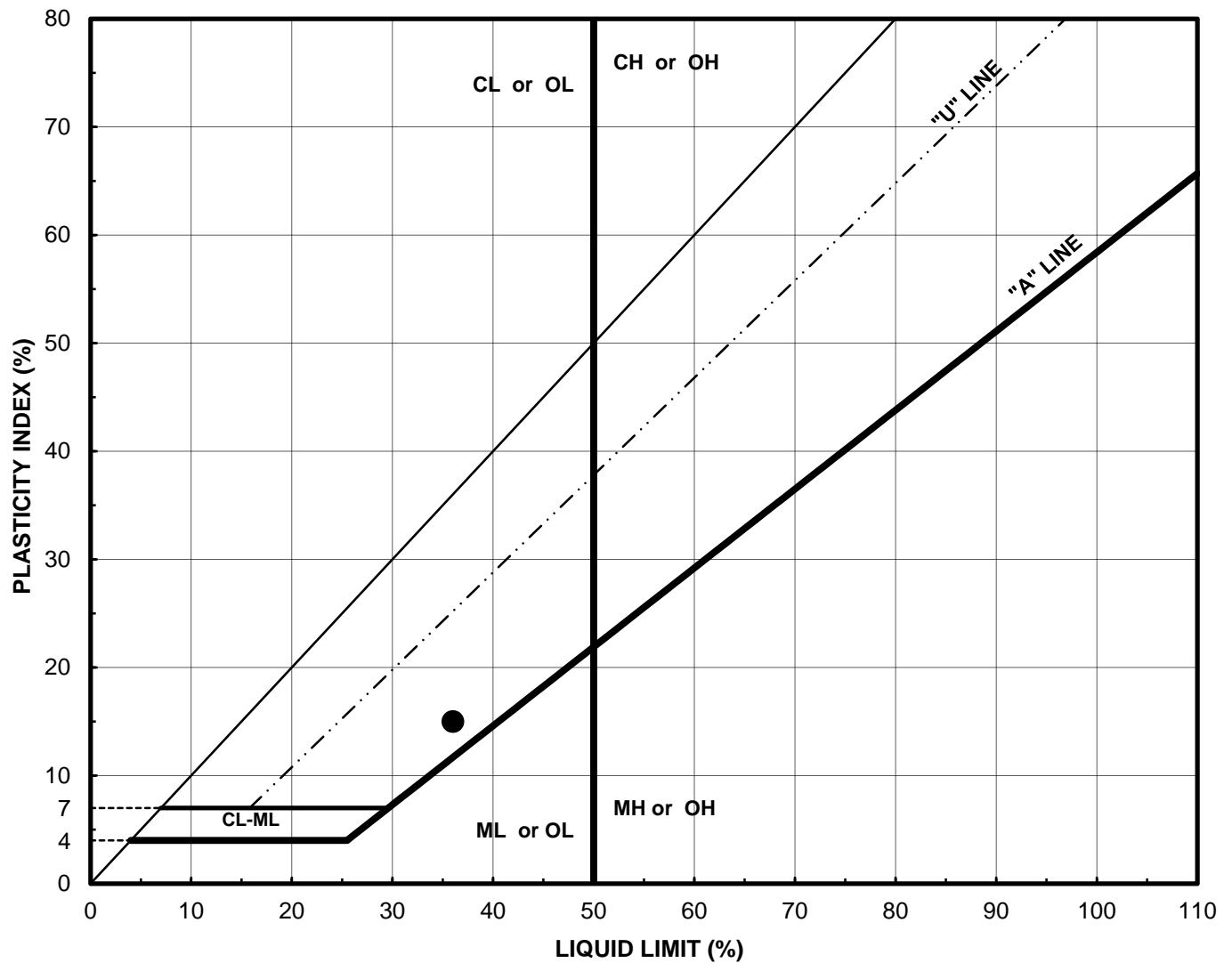




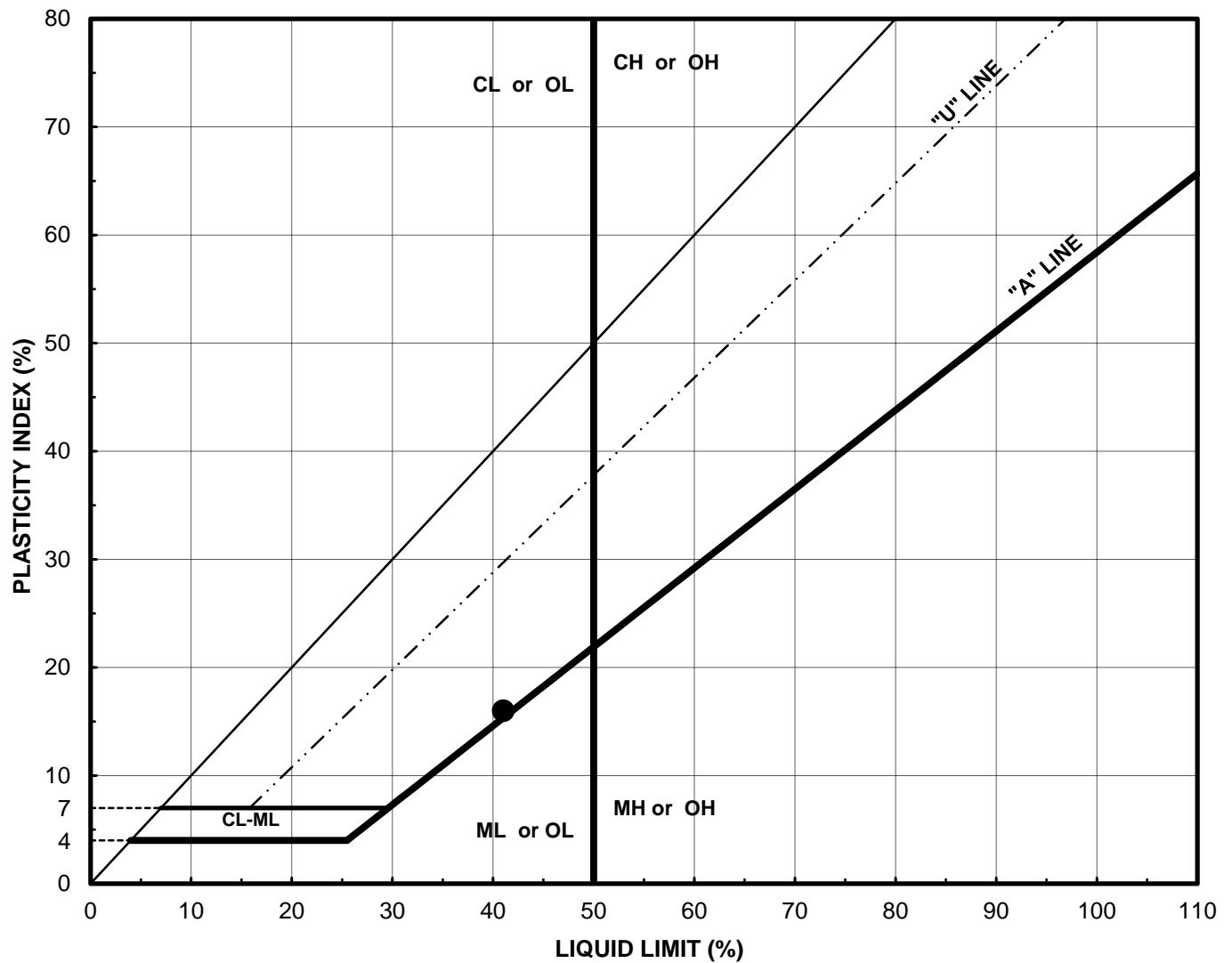
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
DS-01	4a	15.0	19.6	24	8	Dark yellowish brown Clayey SAND (SC)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



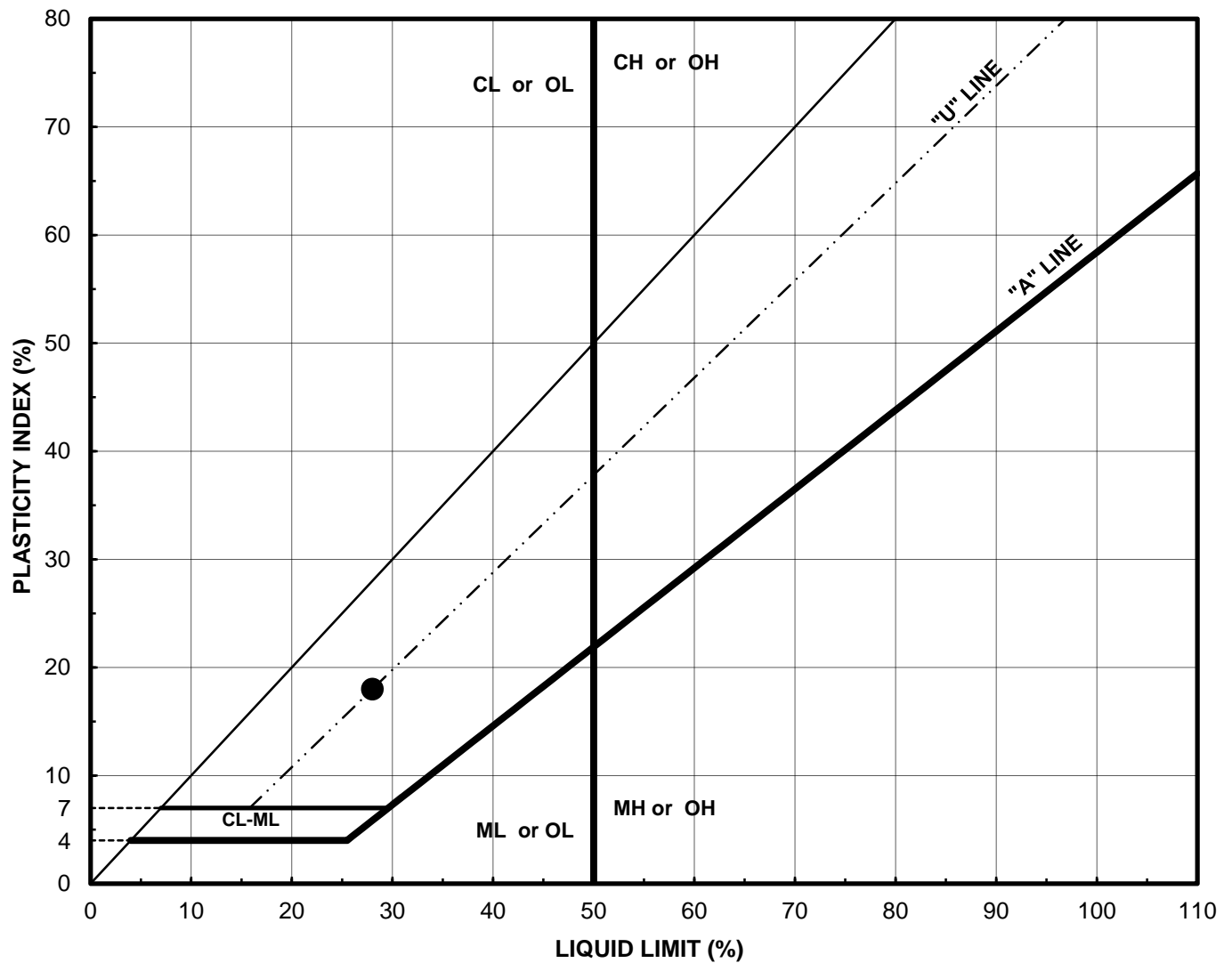
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
DS-01	8b	35.0	37.8	39	19	Gray Lean CLAY with Sand (CL)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



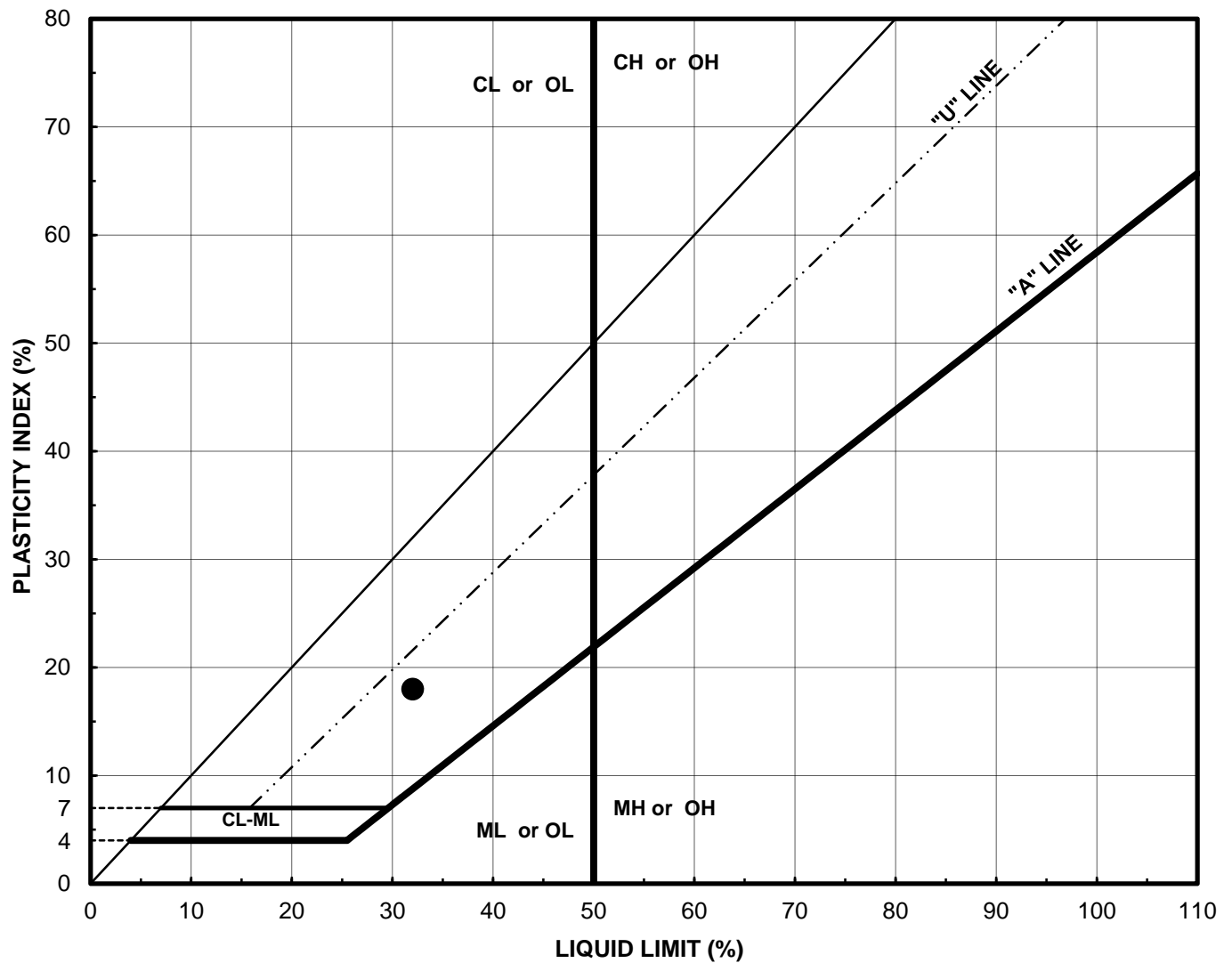
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
DS-03	7	30.0	34.2	36	15	Dark gray Clayey SAND (SC)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



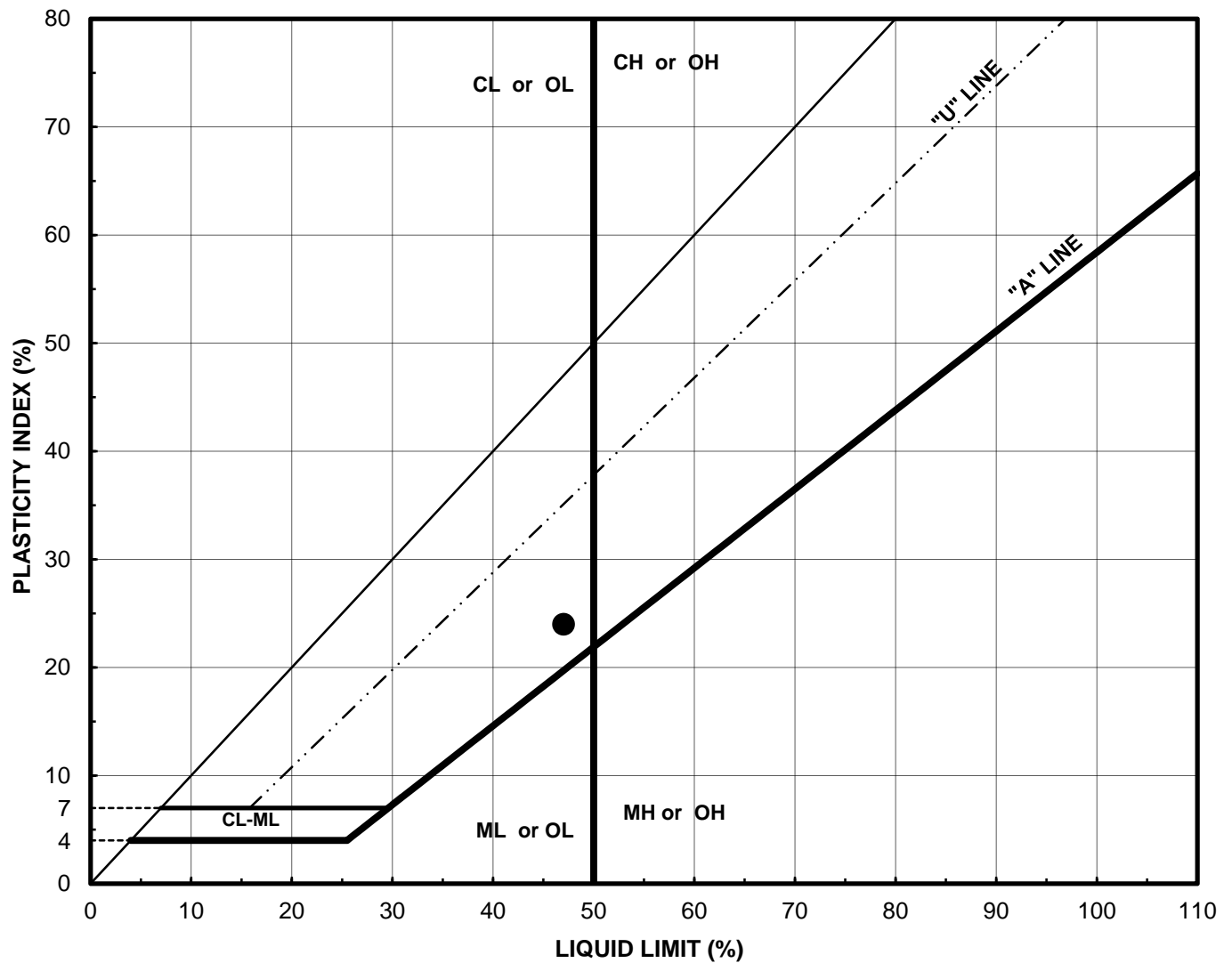
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
DS-06	7	30.0	35.7	41	16	Dark gray Sandy, Lean CLAY (CL)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



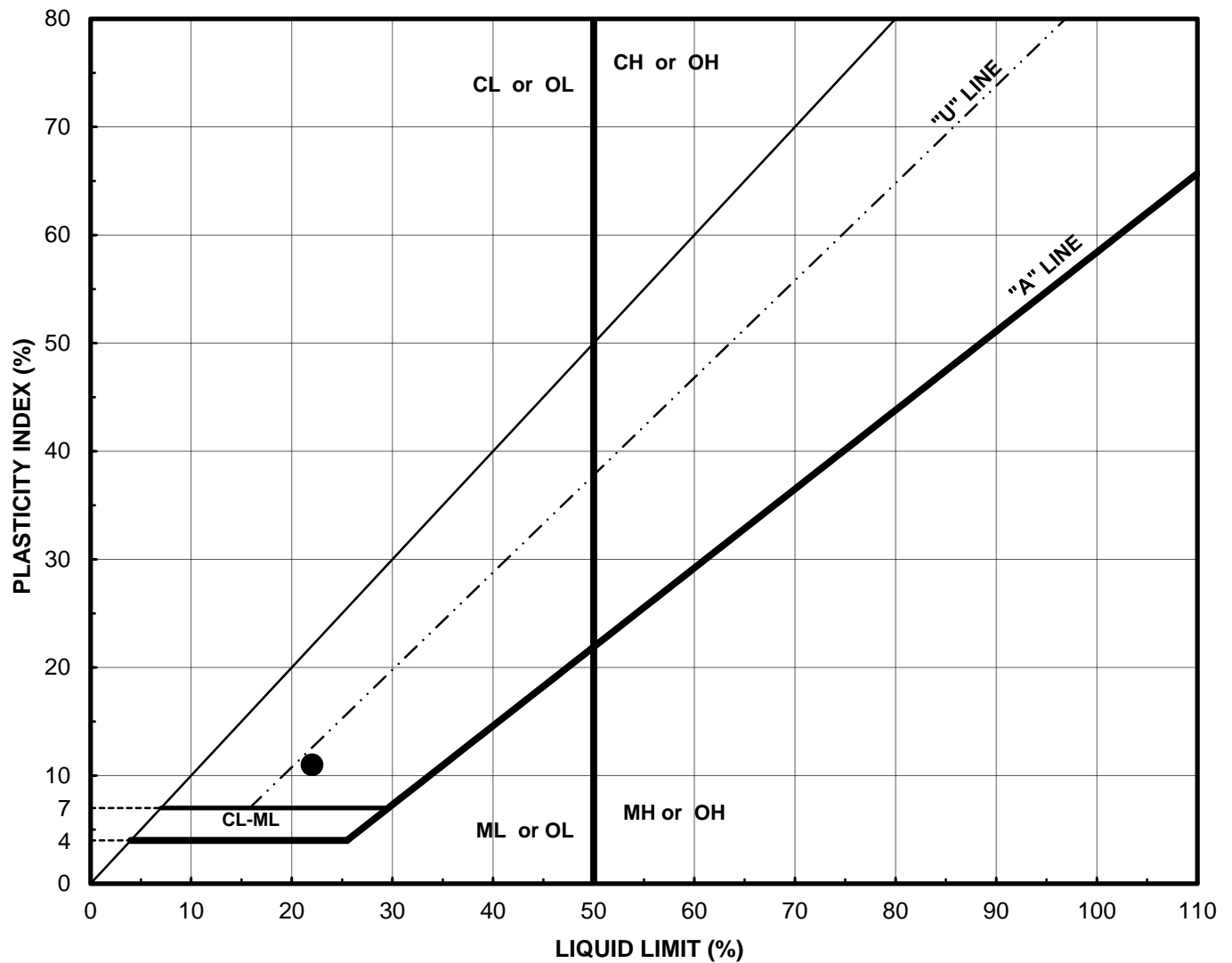
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-08	4	15.0	18.9	28	18	Light brown Lean CLAY (CL)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



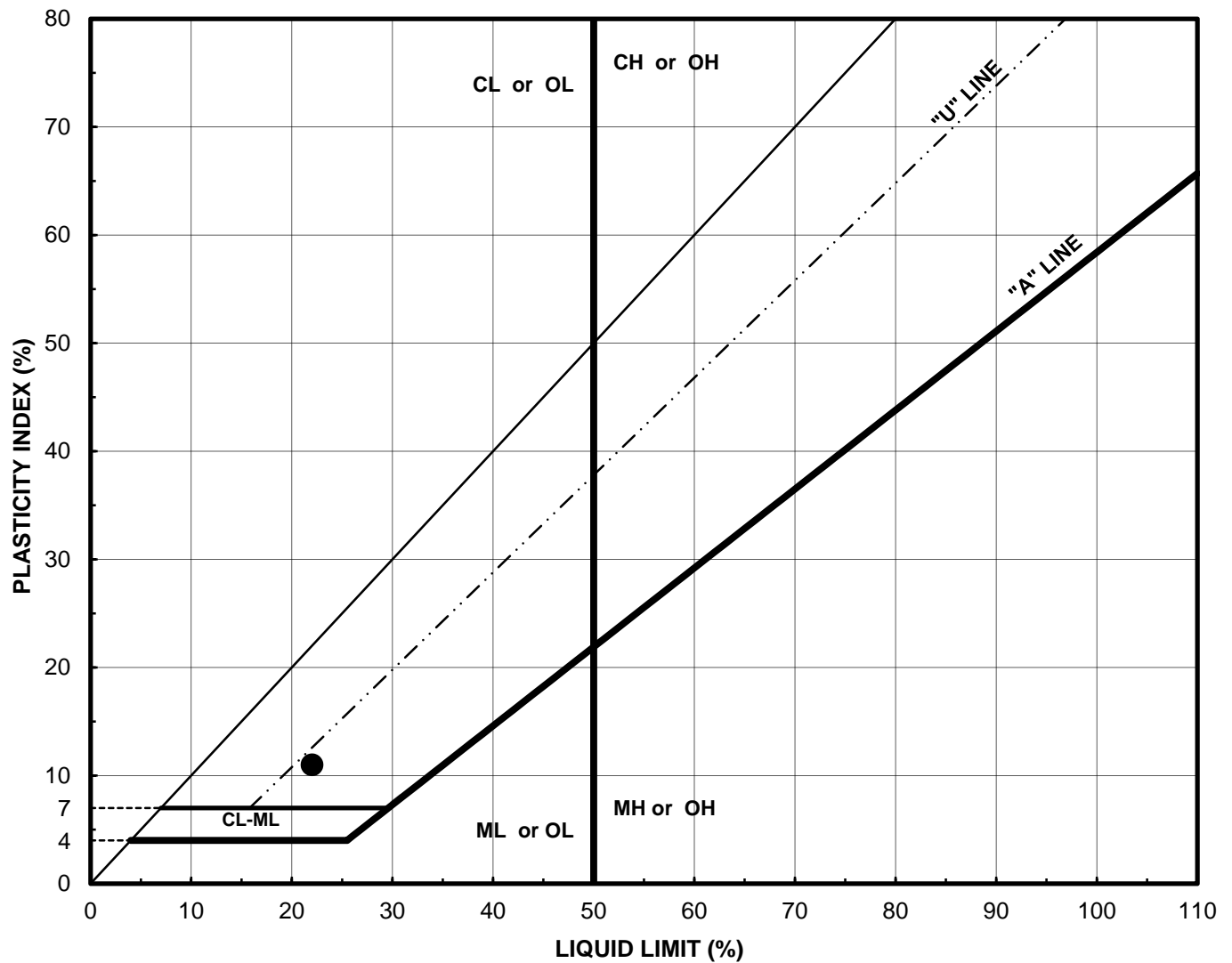
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-11	3	10.0	19.4	32	18	Olive brown Sandy, Lean CLAY (CL)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



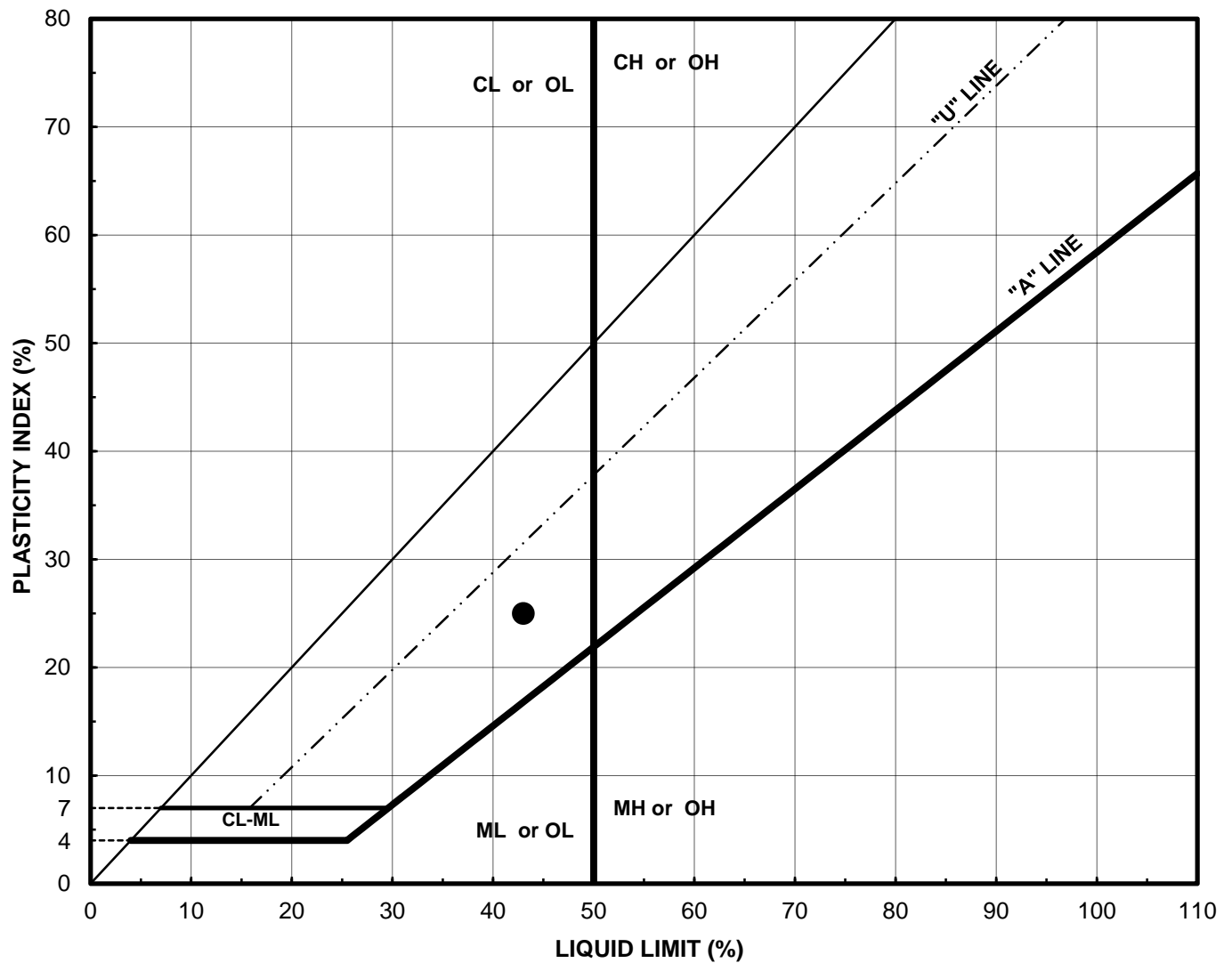
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-20	3	10.0	18.6	47	24	Yellowish brown Lean CLAY (CL)
Project Name: Pure Water						
Project Number: 60530732						



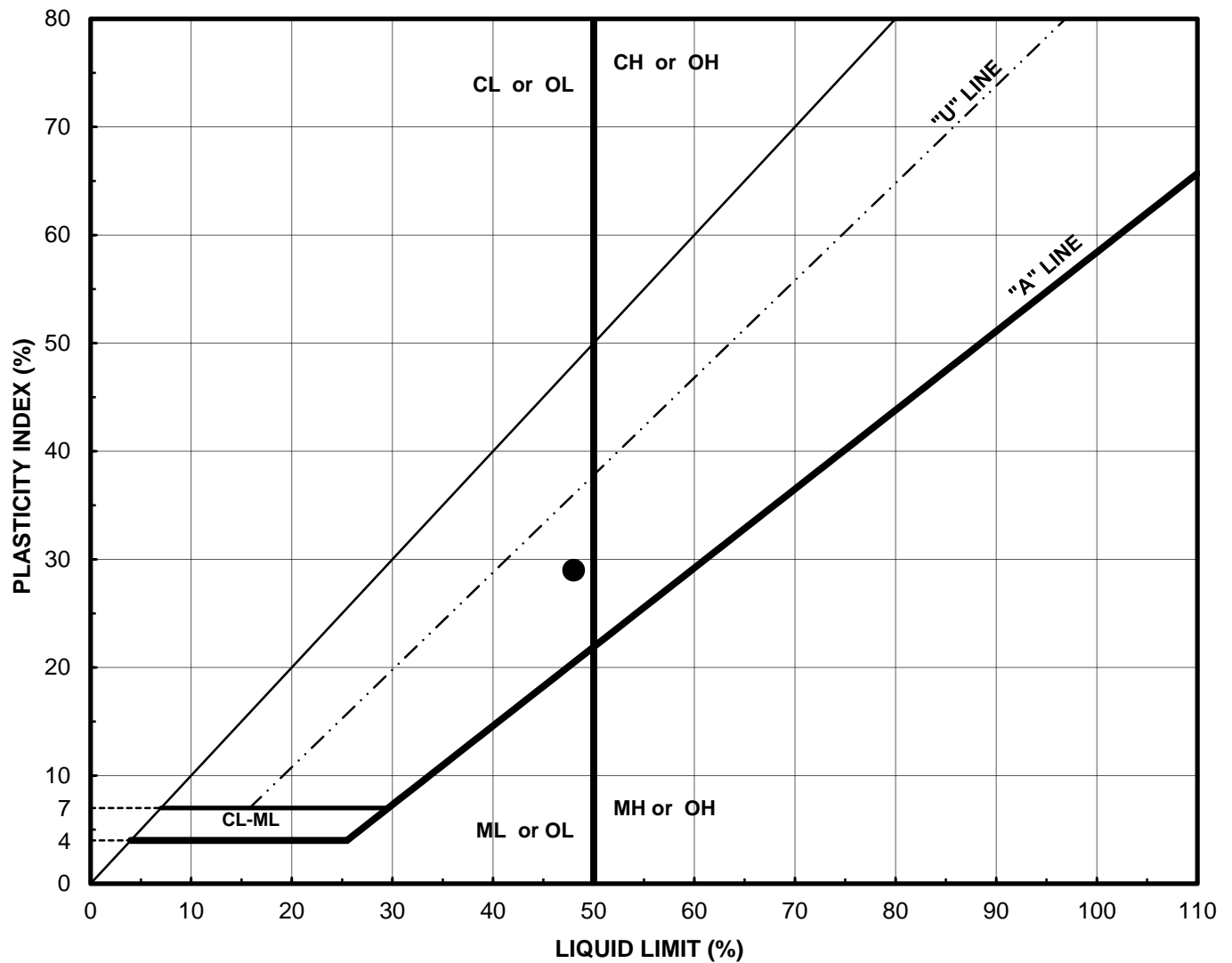
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-23	1	0~5	NA	22	11	Strong brown Lean CLAY (CL)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



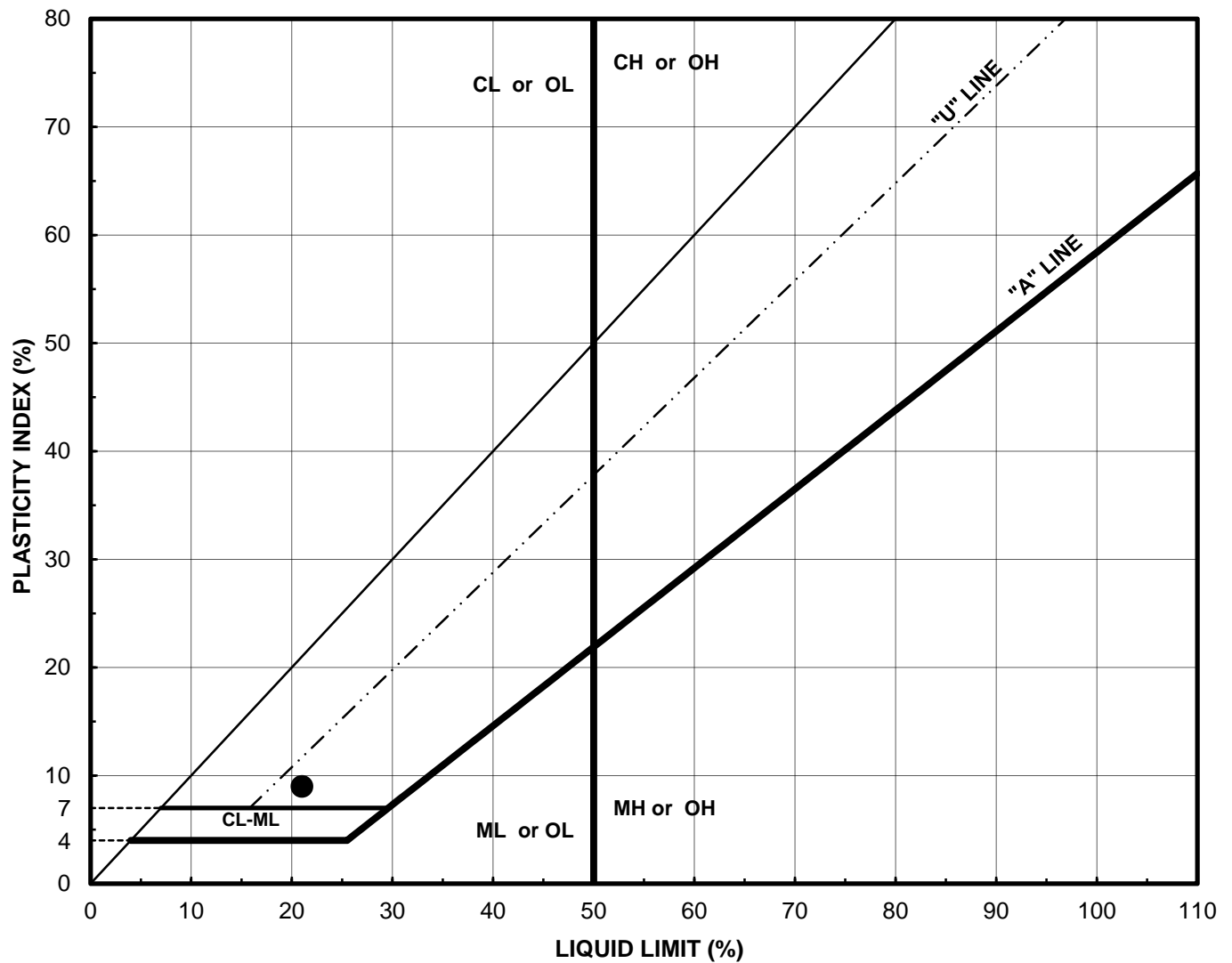
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-25	1	0-5	10.3	22	11	Strong brown Clayey SAND (SC)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



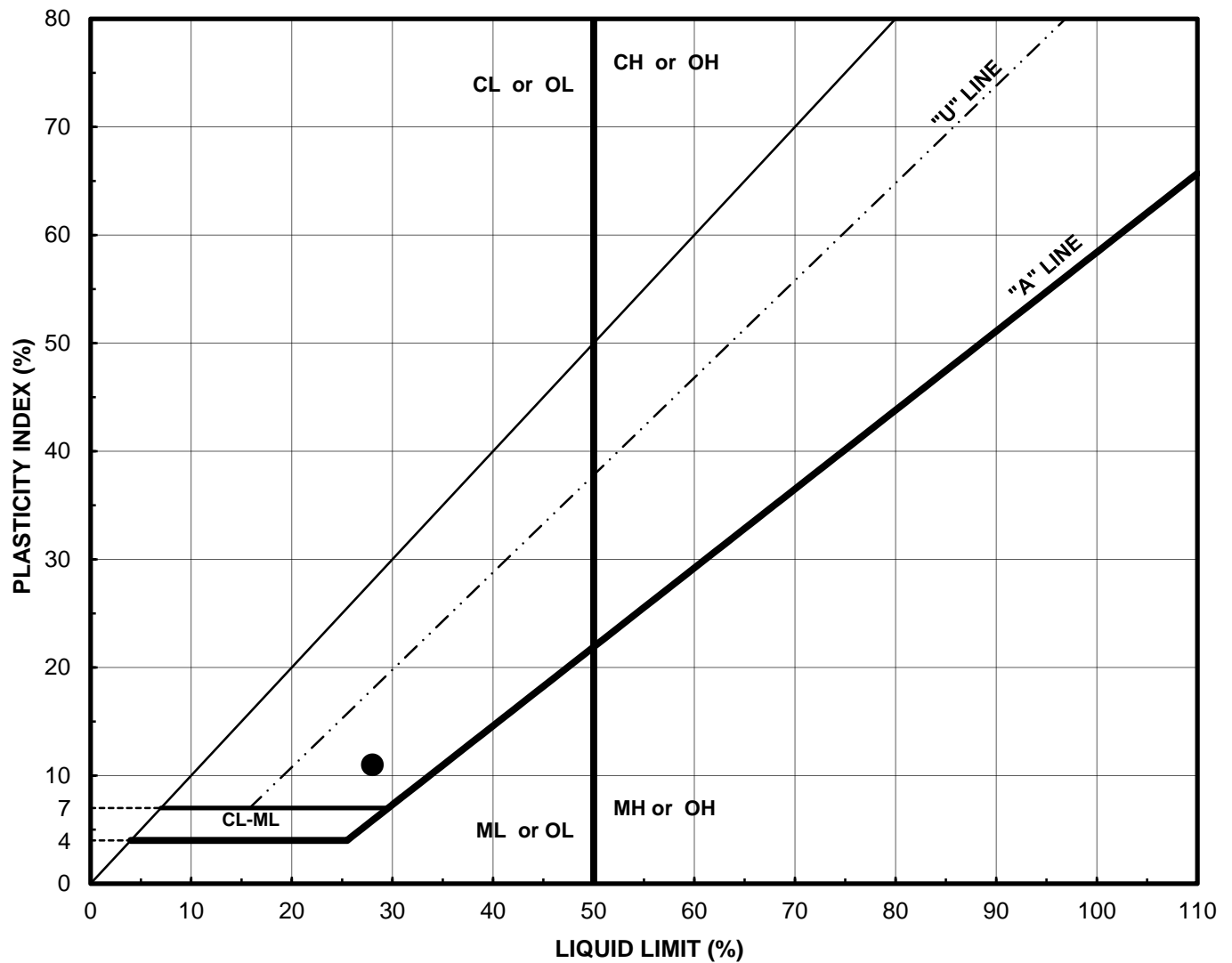
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-27	2	5.0	20.3	43	25	Light yellowish brown CLAY with Sand (CL)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



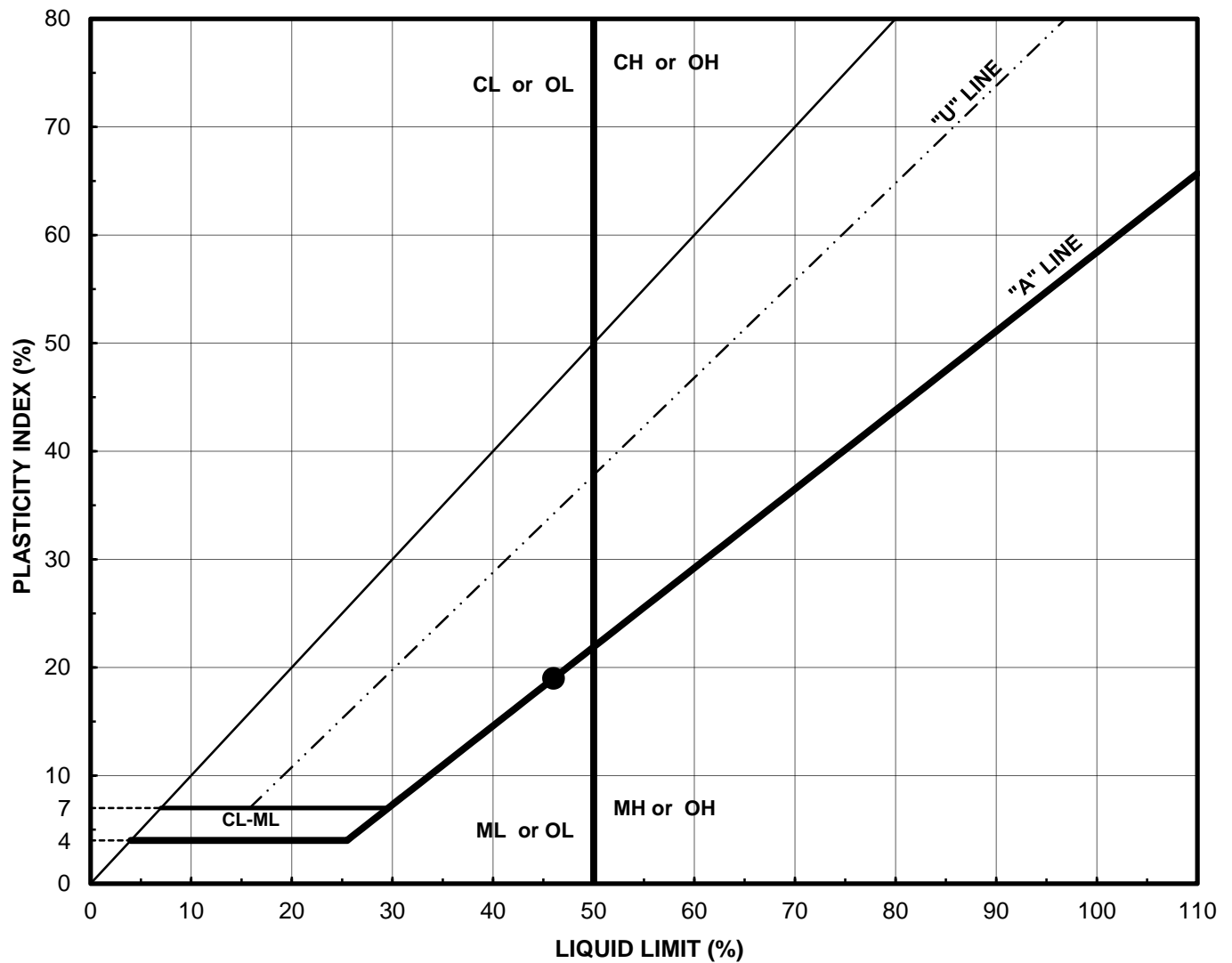
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-31	3	10.0	NA	48	29	Yellowish brown Silty CLAY (CL)
Project Name: Pure Water						
Project Number: 60530732						



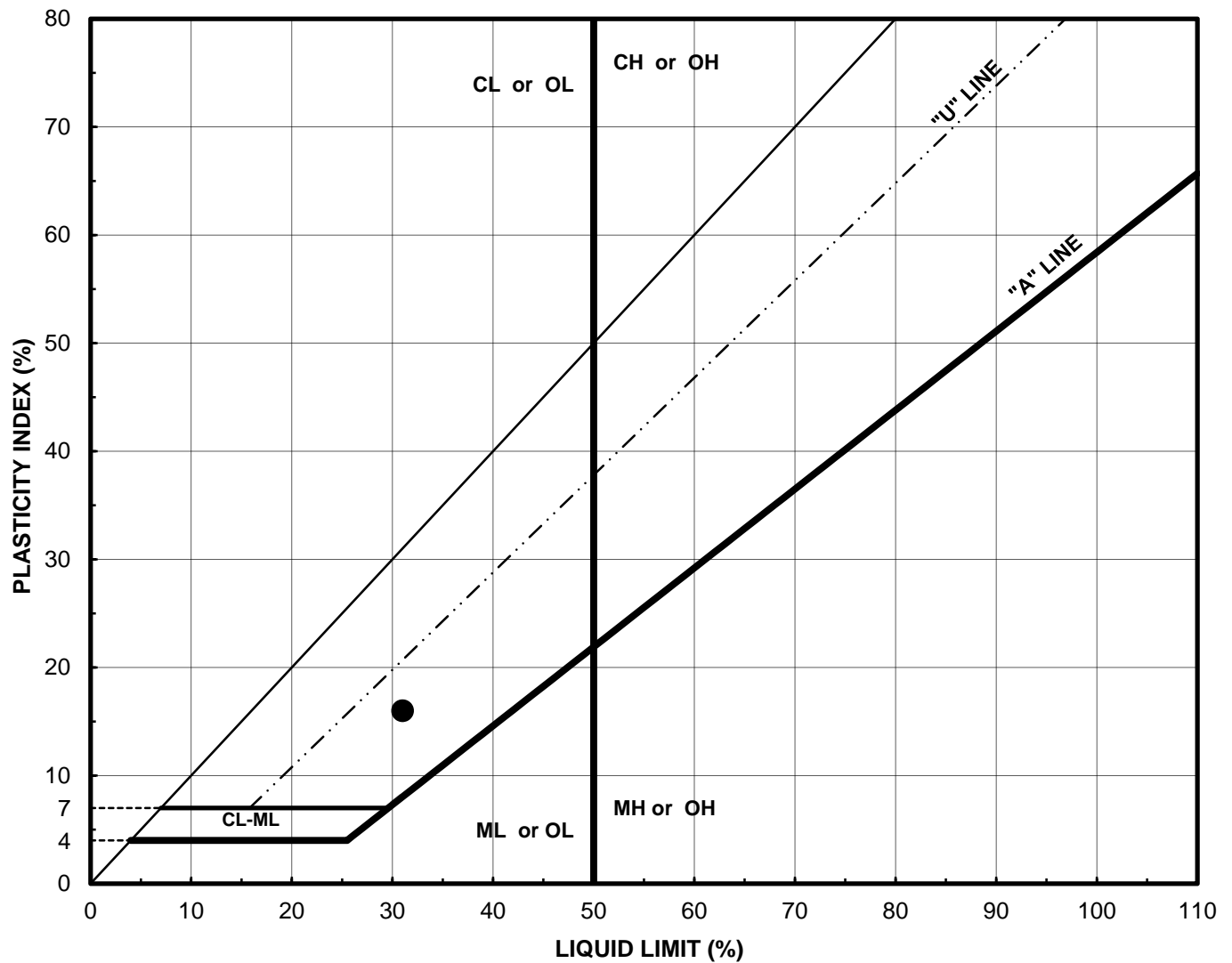
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-36b	2	5.0	7.8	21	9	Strong brown Clayey SAND (SC)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



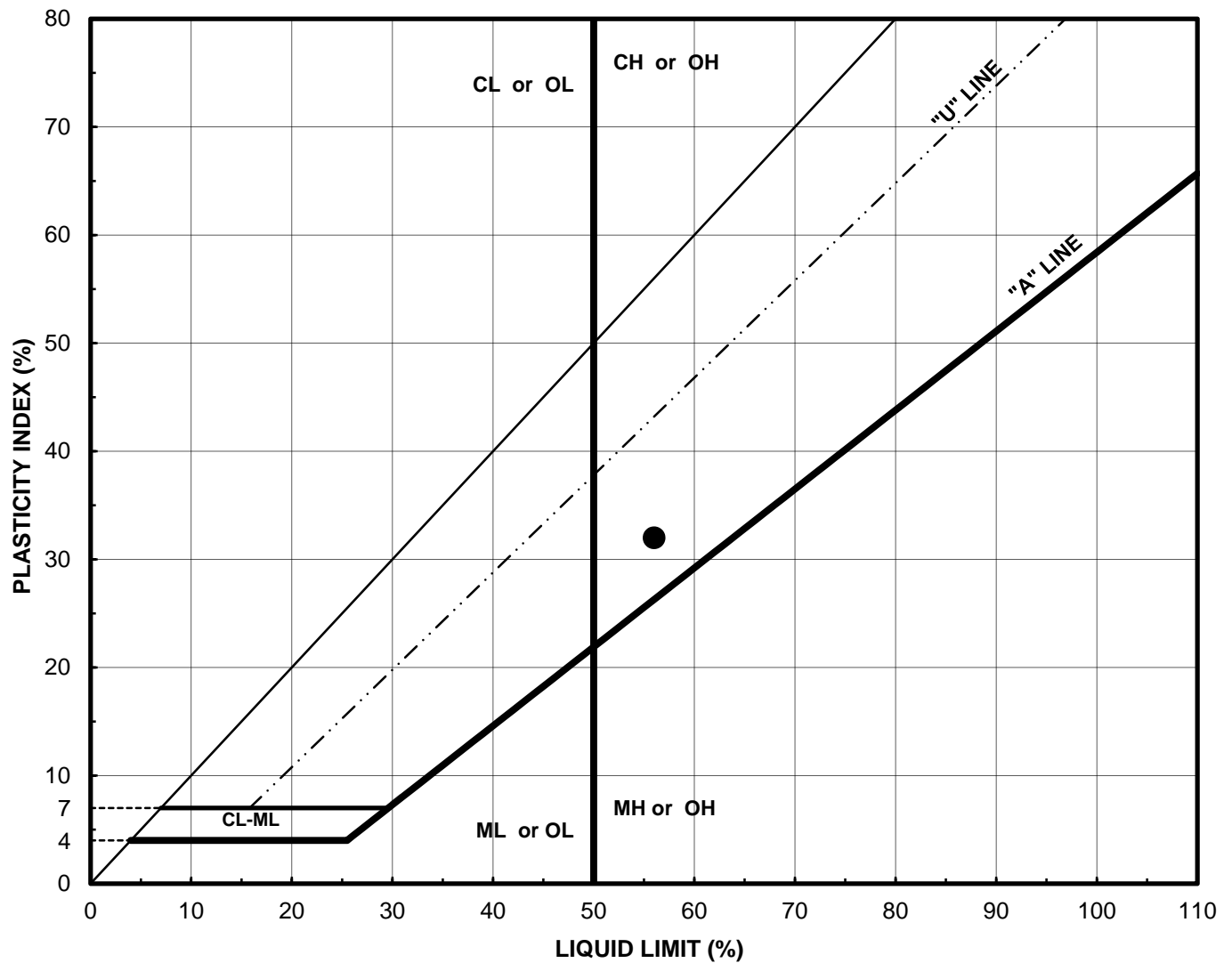
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-44	1	5.0	12.2	28	11	Yellowish brown Clayey SAND (SC)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



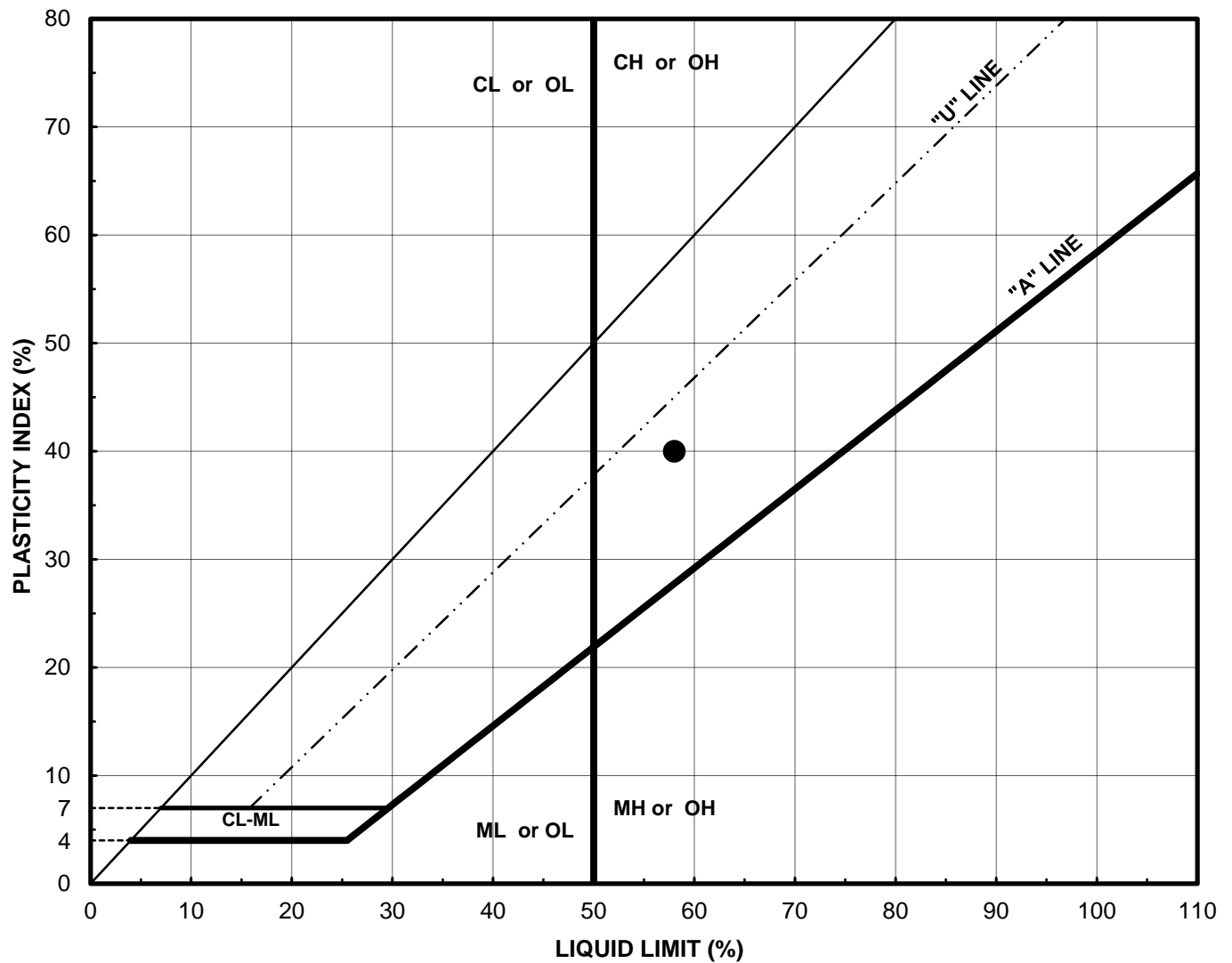
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-45	3	10.0	20.3	46	19	Light olive brown Silty CLAY (CL)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



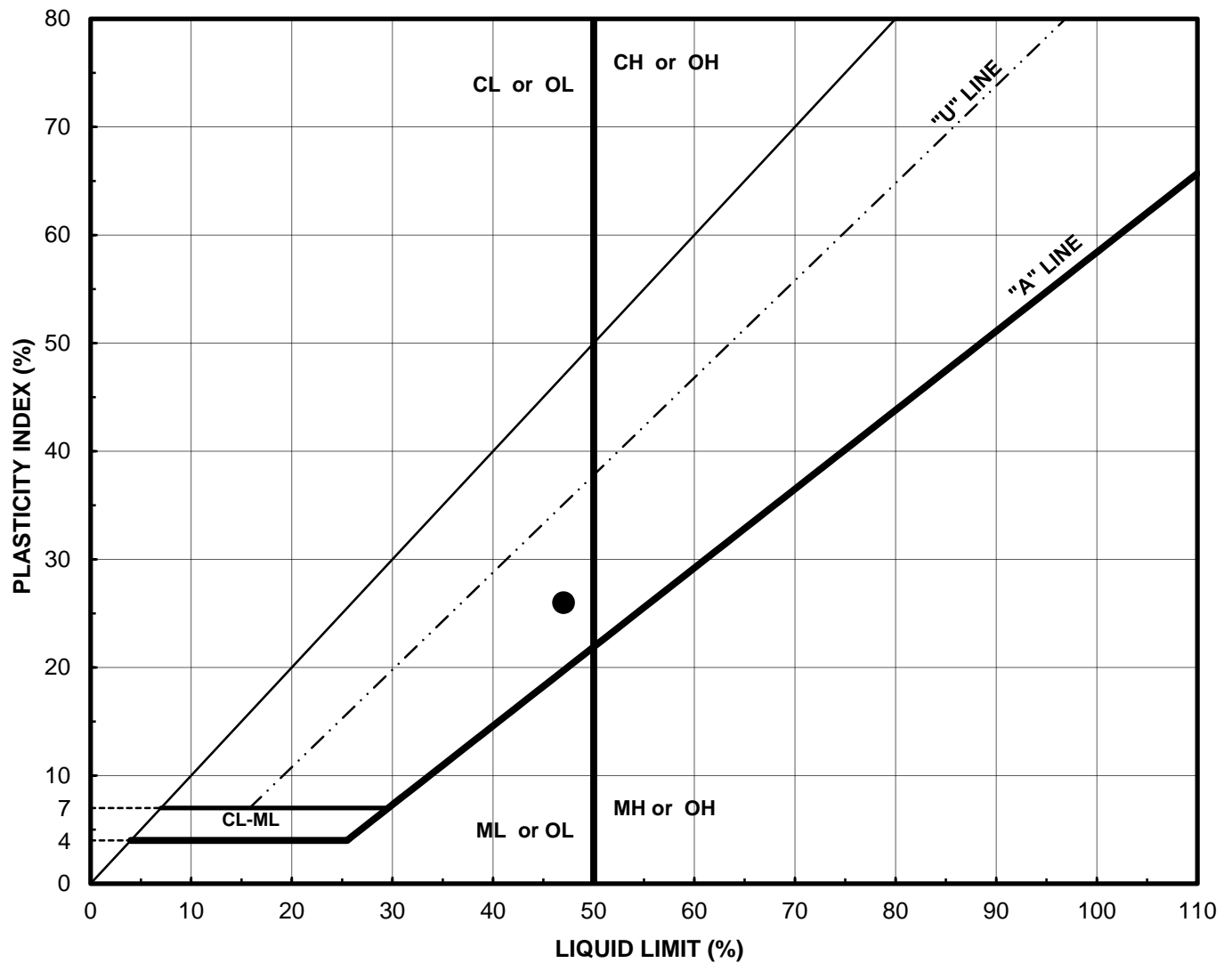
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-46	4	18.5	23.0	31	16	Dark yellowish brown Sandy, Lean CLAY (CL)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



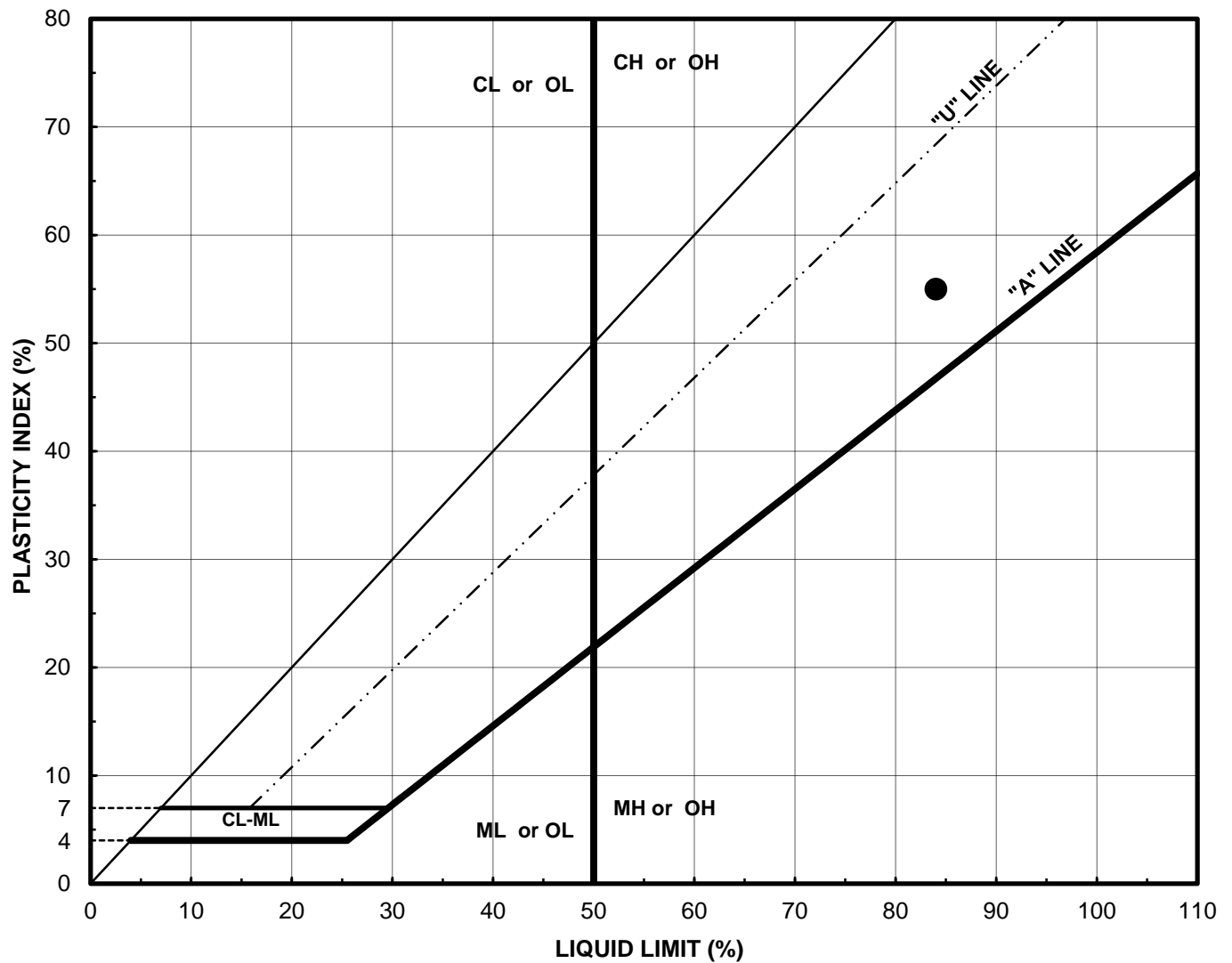
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
FM-47	2	5.0	16.3	56	32	Light olive brown Fat CLAY (CH)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



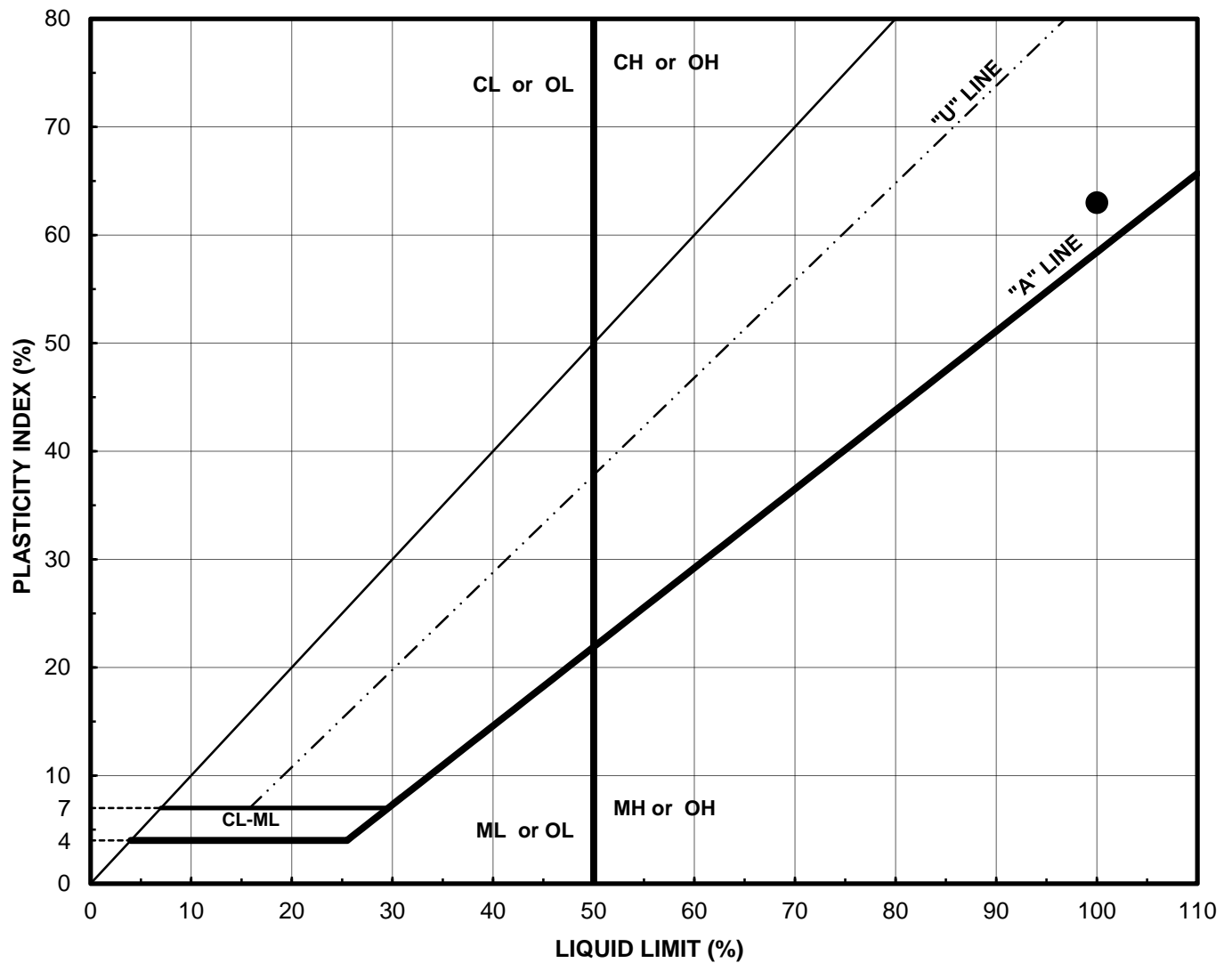
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
PS-3	12a	65.0	37.9	58	40	Dark gray Fat CLAY (CH)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



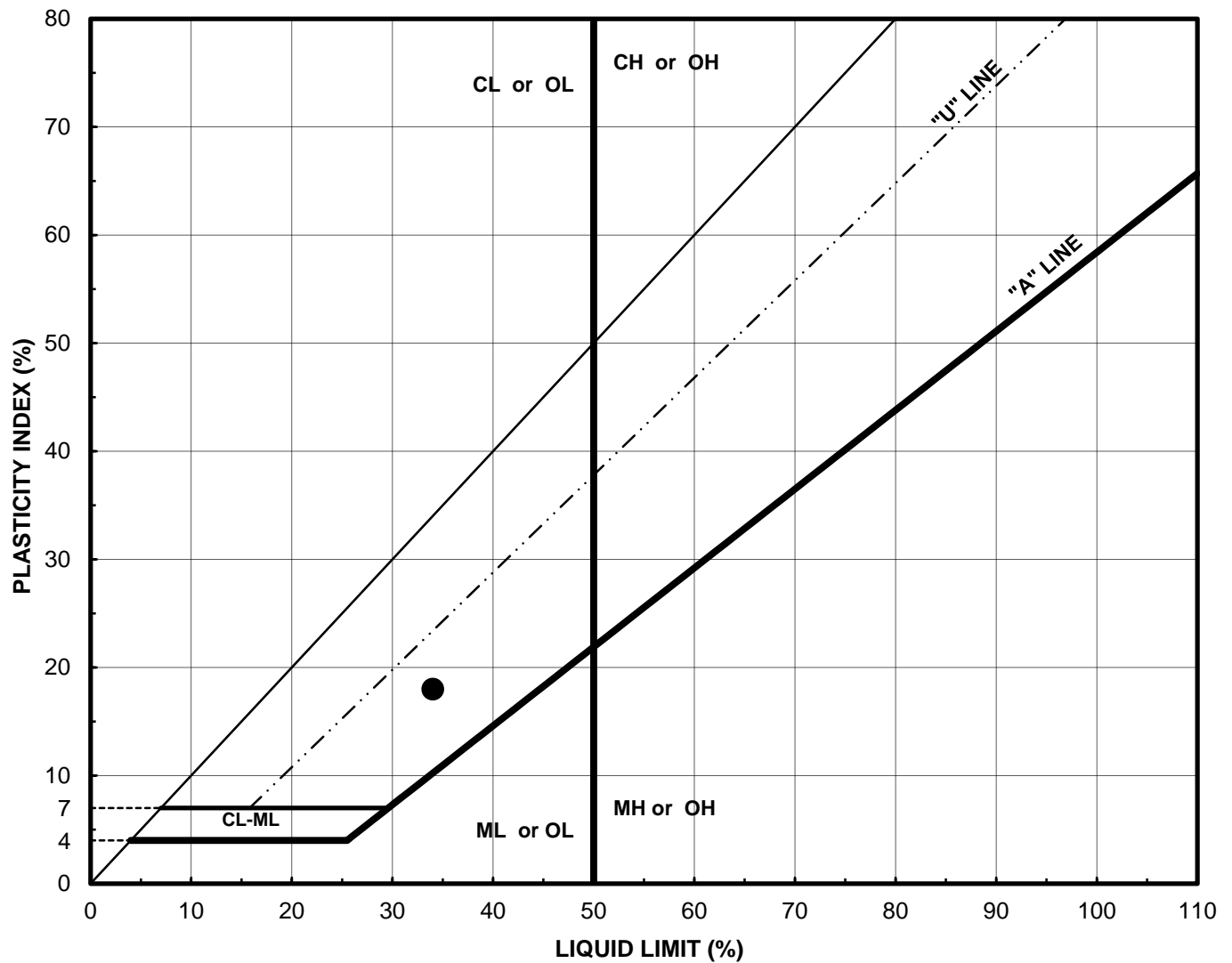
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
TC-01	4	17.0	NA	47	26	Dark grayish brown CLAY with Sand (CL)
Project Name: Pure Water						
Project Number: 60530732						



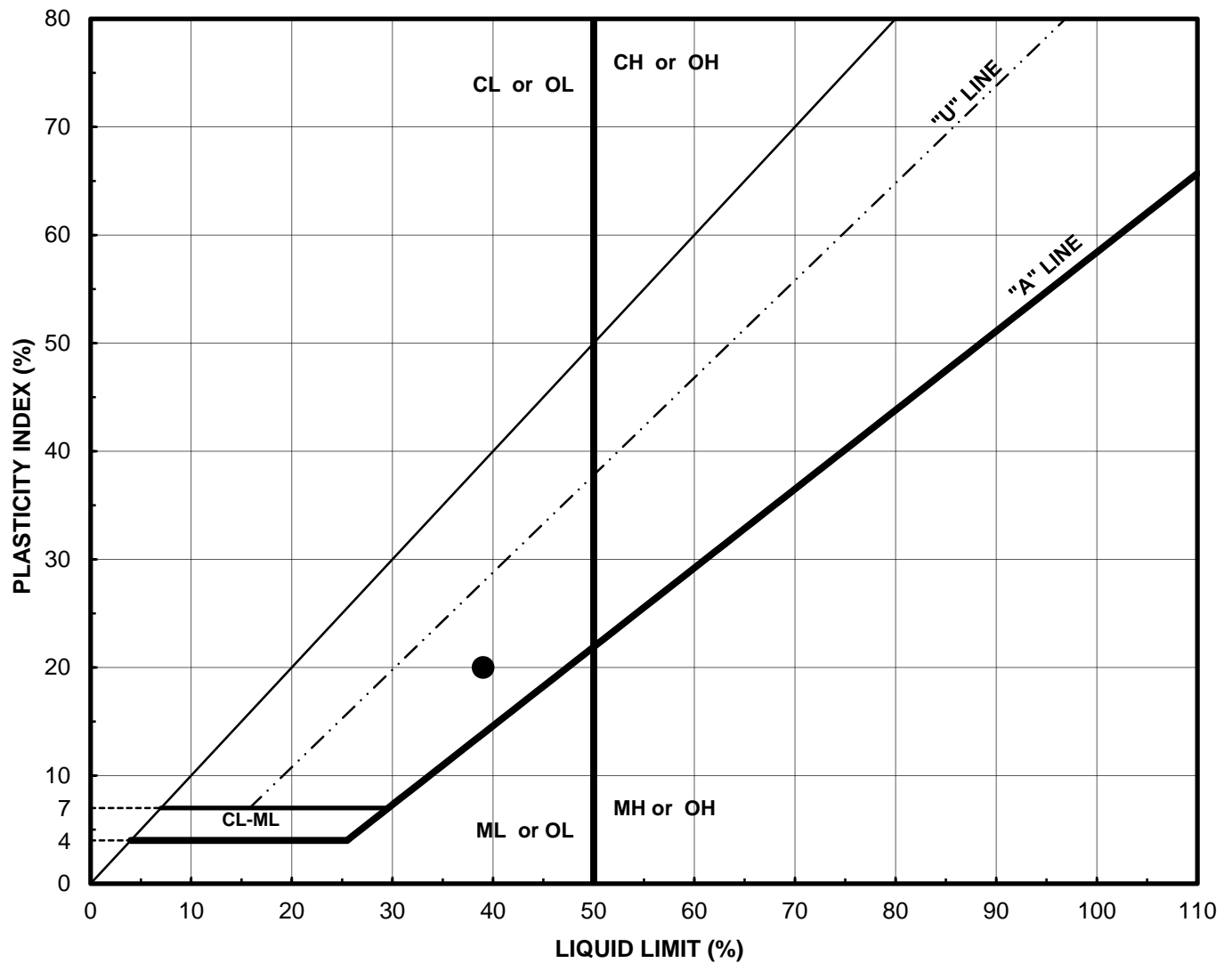
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
TC-01	6	25.0	----	84	55	Dark gray Fat CLAY (CH)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



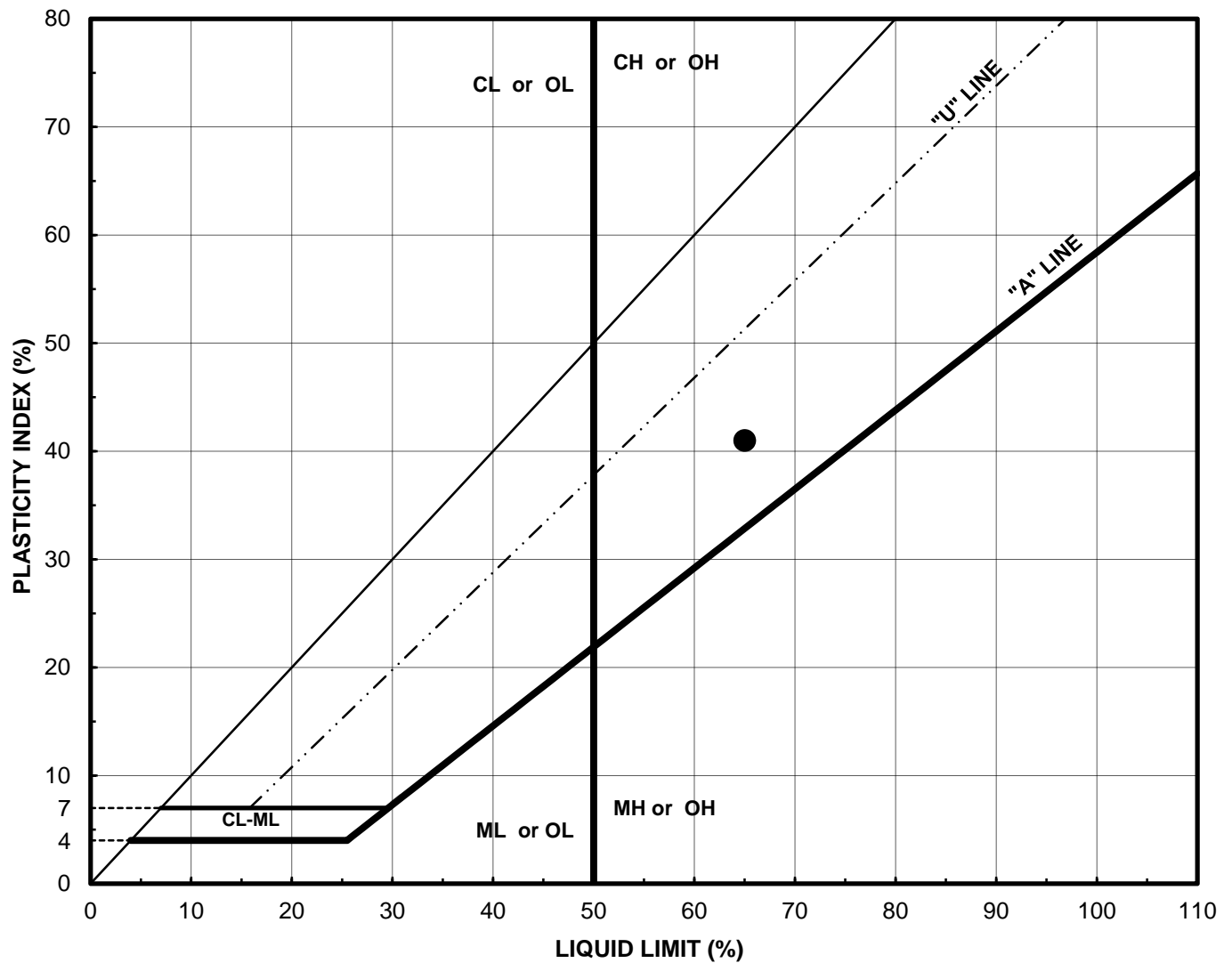
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
TC-01	7	30.0	40.0	100	63	Dark gray Fat CLAY (CH)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



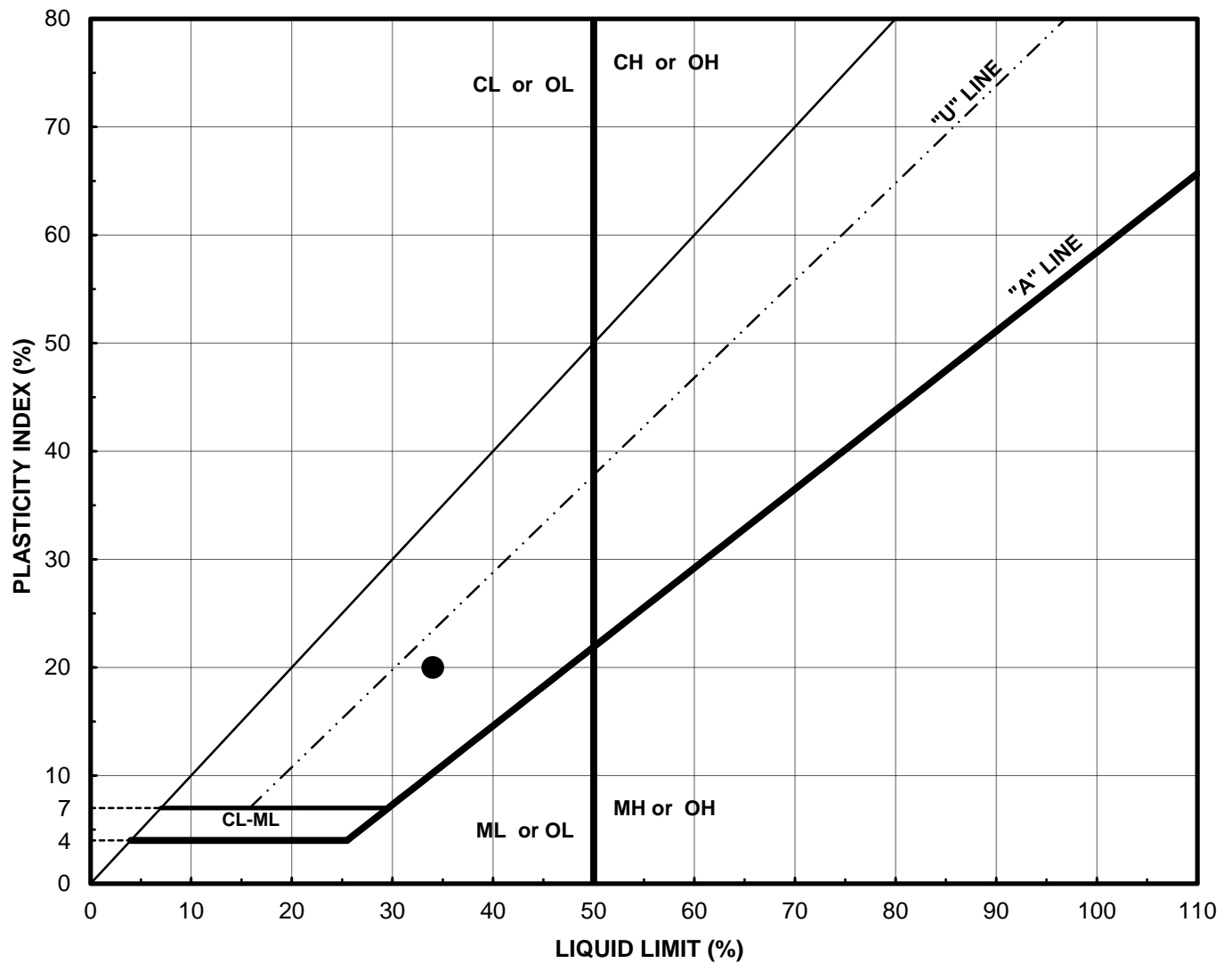
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
TC-01	10	45.0	NA	34	18	Dark grayish brown Clayey SAND (SC)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



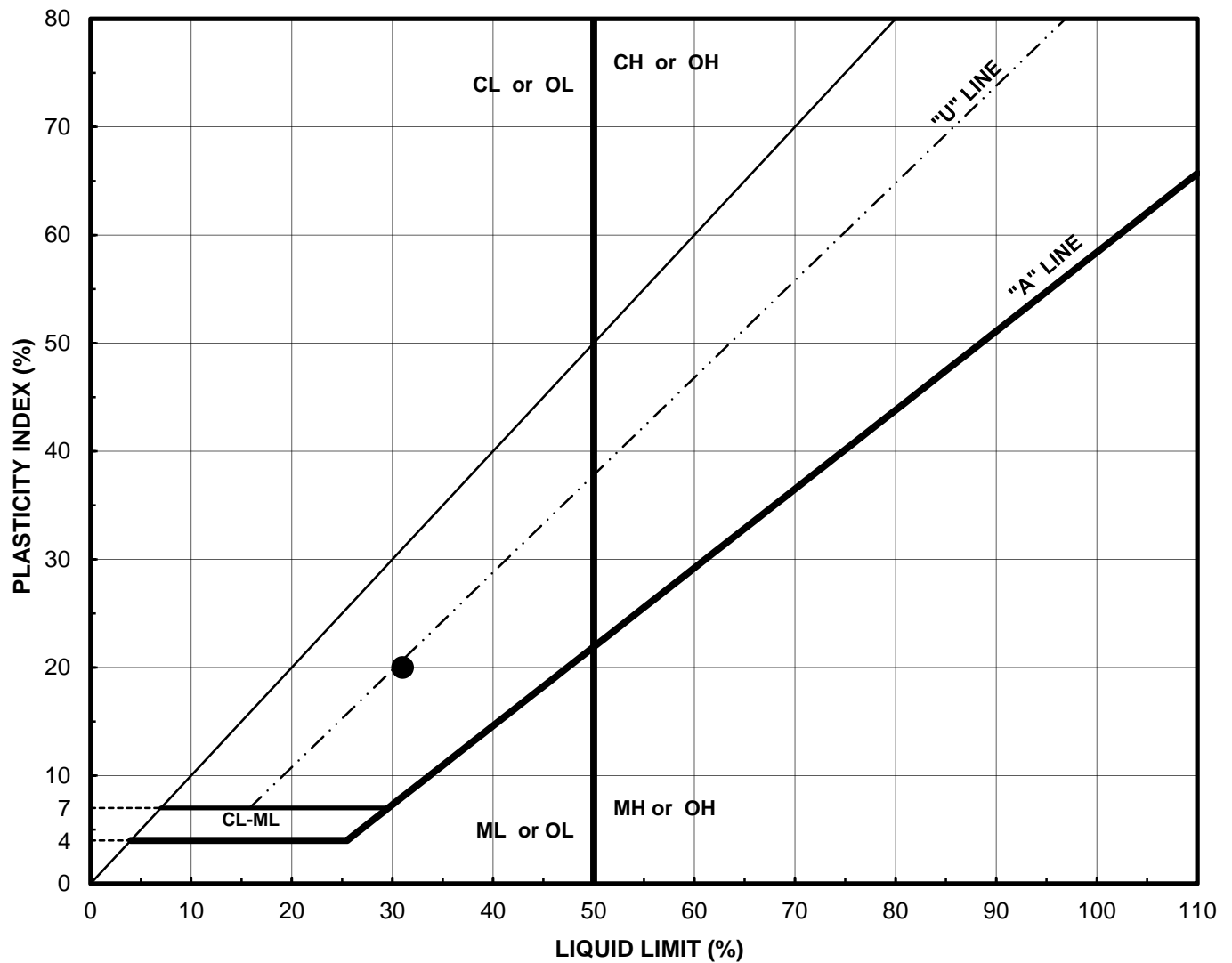
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
TC-02	3	15.0	----	39	20	Olive brown Sandy Lean CLAY (CL)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



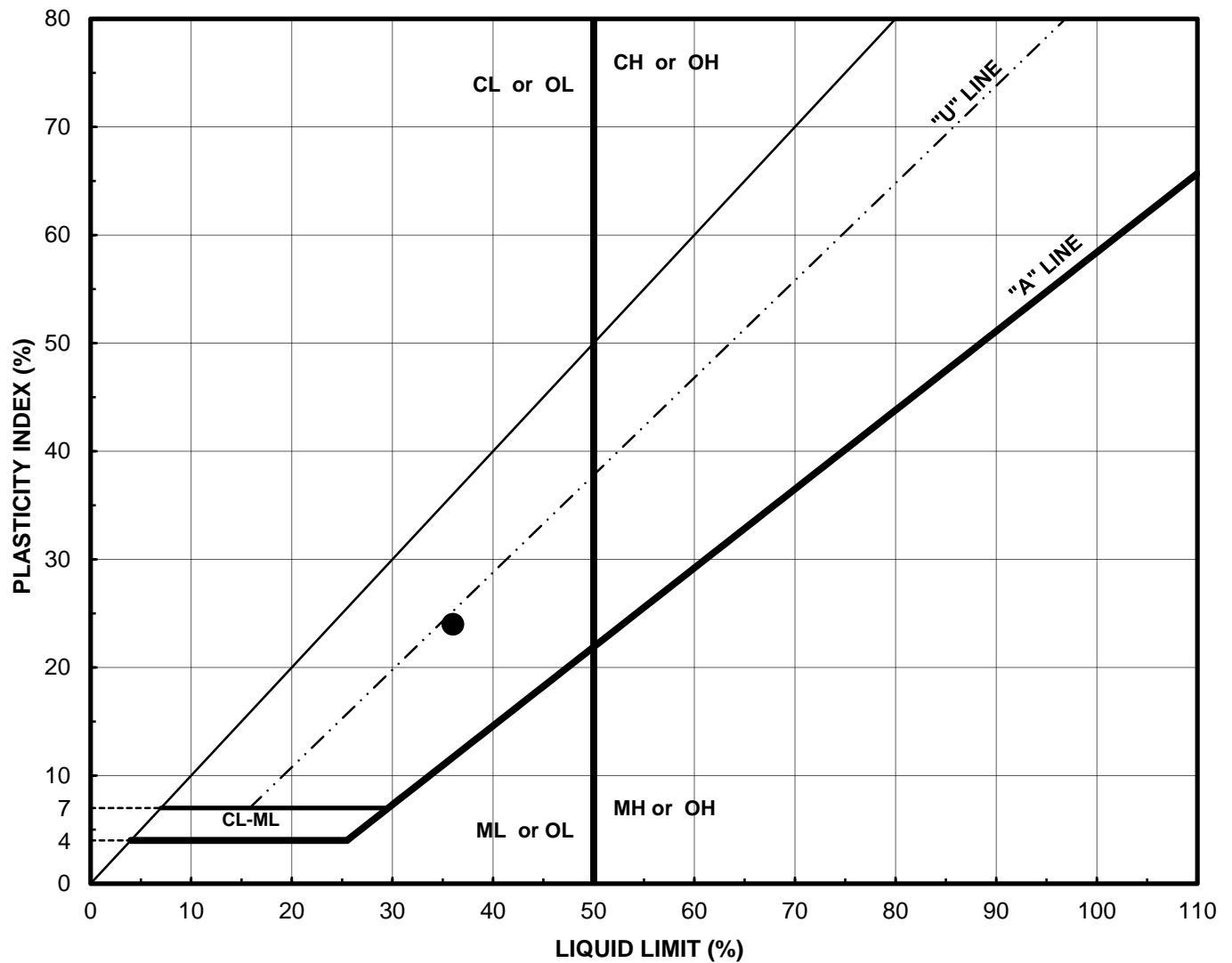
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
TC-02	5	25.0	----	65	41	Dark gray Fat CLAY (CH)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



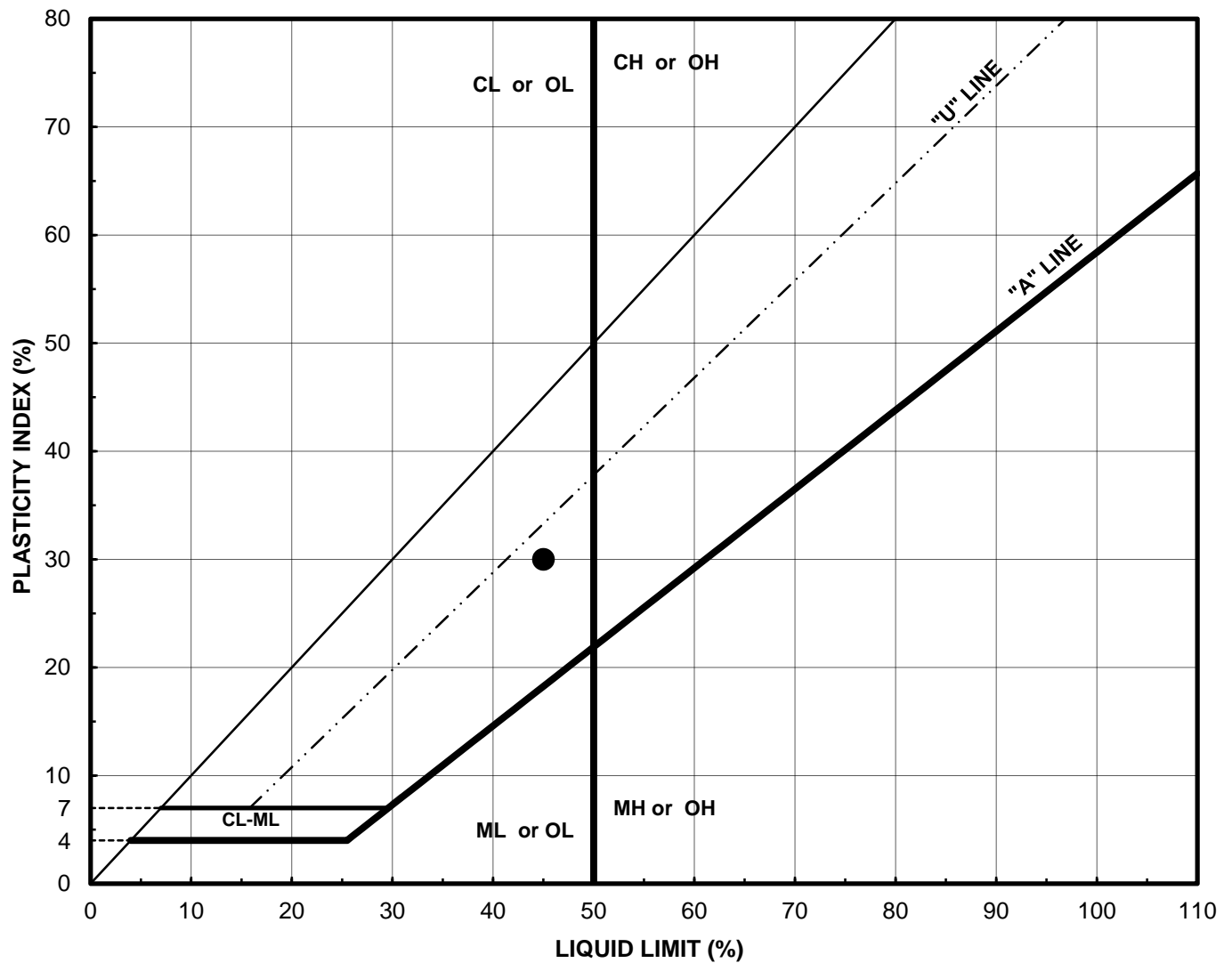
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
WM-01	1	0~5	----	34	20	Yellowish brown Clayey SAND (SC)
Project Name: SR-210 Project Number: 60512688						
PLASTICITY CHART						



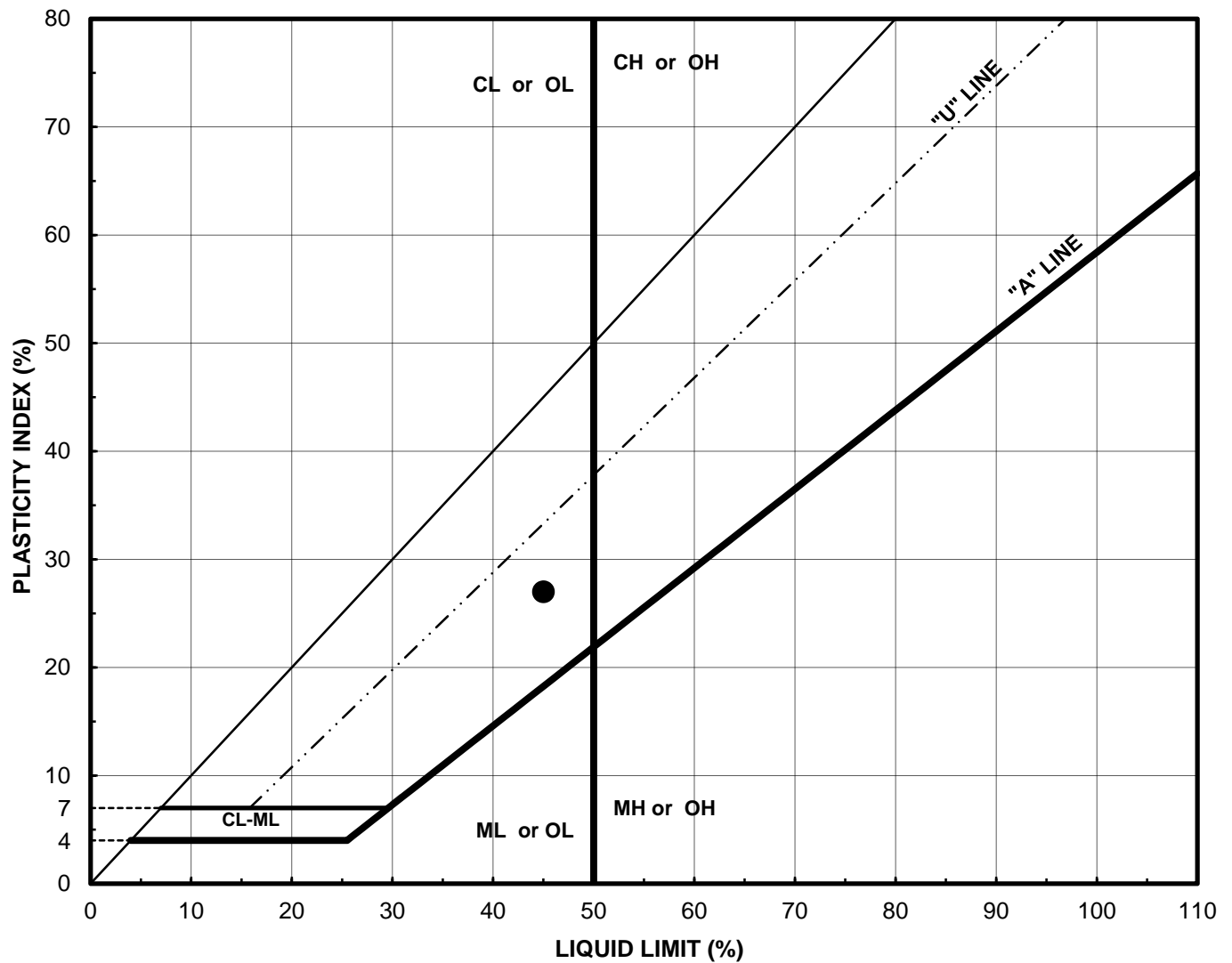
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
WM-06	1	0-5	NA	31	20	Dark yellowish brown Lean CLAY with Sand (CL)
<div> <div>Project Name: Pure Water</div> <div>Project Number: 60530732</div> </div> <div>PLASTICITY CHART</div>						



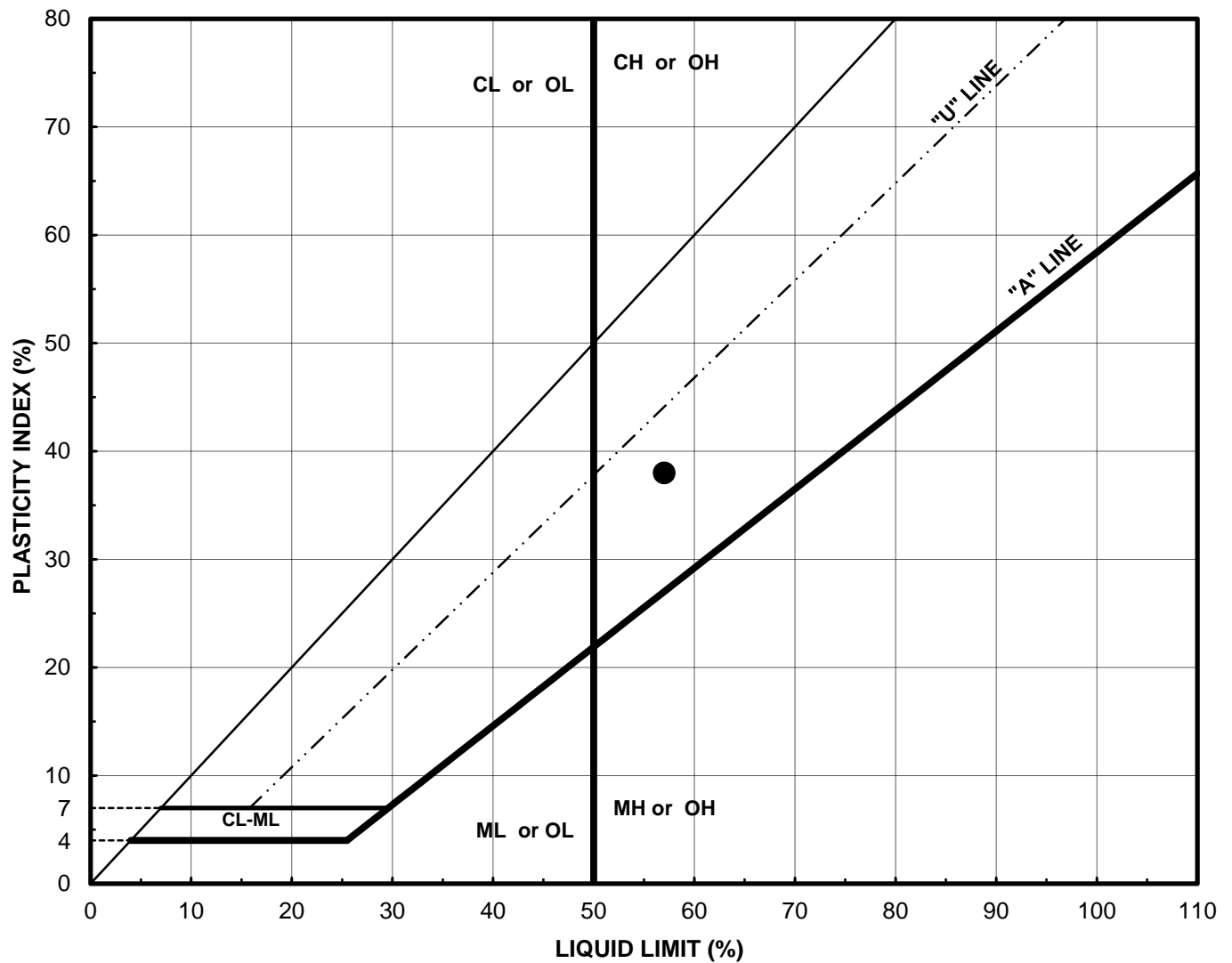
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
WM-07	2	10.0	8.7	36	24	Olive brown Clayey SAND with Gravel (SC)
Project Name: Pure Water						
Project Number: 60530732						



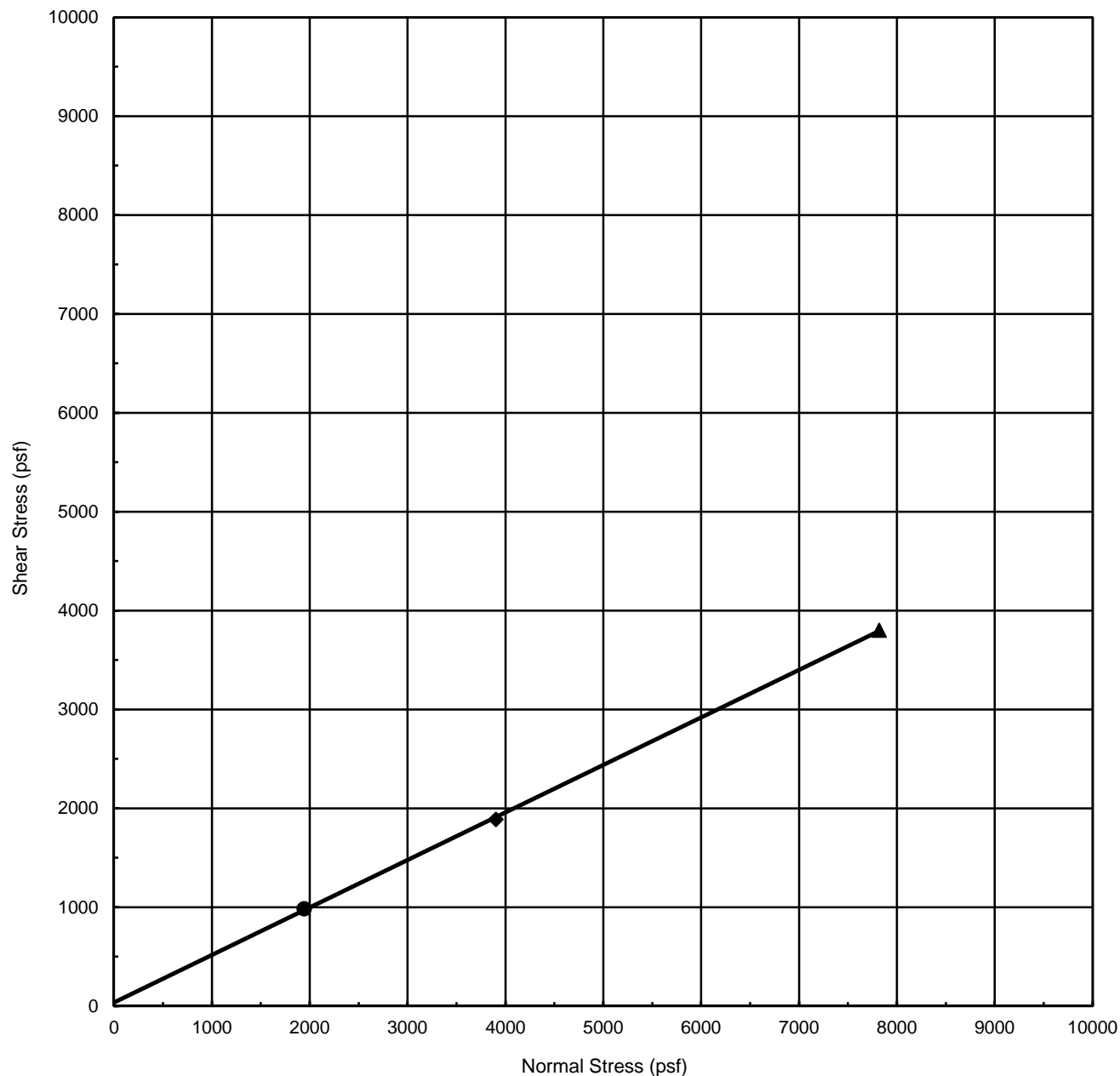
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
WM-08	3	12.5	10.6	45	30	Strong brown Clayey SAND (SC)
Project Name: Pure Water						
Project Number: 60530732						
PLASTICITY CHART						



Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
WM-11	5	18.5	23.5	45	27	Olive Lean CLAY with Sand (CL)
<div> <div>Project Name: SR-210</div> <div>Project Number: 60512688</div> </div> <div>PLASTICITY CHART</div>						



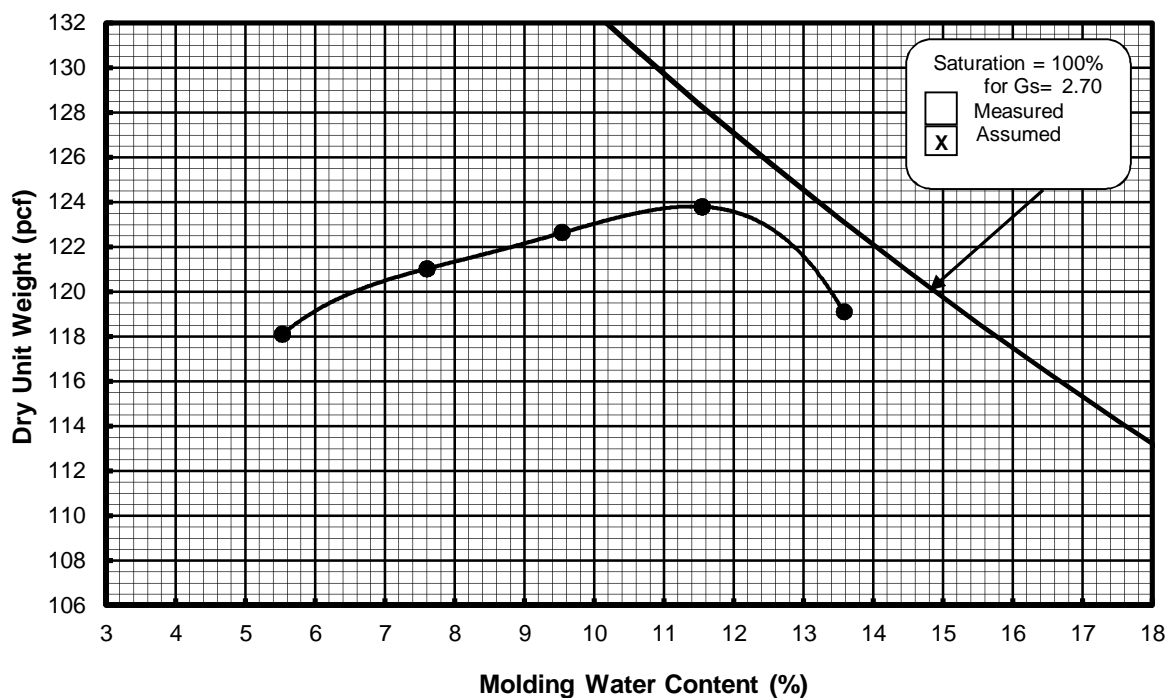
Boring Number	Sample Number	Depth (ft)	Water Content (%)	LL	PI	DESCRIPTION / CLASSIFICATION
WM-24	4b	15.0	17.4	57	38	Light olive brown Fat CLAY (CH)
<div> <div>Project Name: SR-210</div> <div>Project Number: 60512688</div> </div> <div>PLASTICITY CHART</div>						



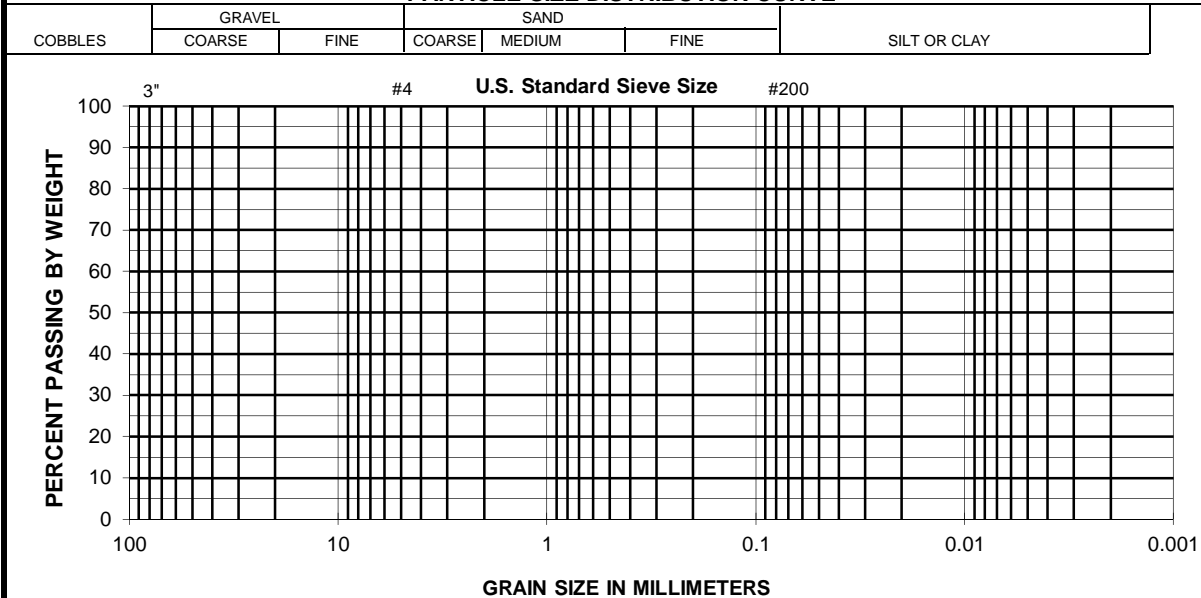
Peak Values are : ,solid trend line				Ultimate Values are: m ,dashed trend line														
Exploration No.:		TC-01		Strength Intercept (C) :			35.3		psf		Peak	XXXXXX		psf		Ultimate		
Sample No.:		7					1.7		kPa			XXXXXX		kPa				
Depth (ft m)		30.0		9.1		Friction Angle (f) :			26			degree		XXXXXX			degree	
Description:		Dark gray Fat CLAY (CH)						Shear rate :		0.0010 (in/min) ,		0.0025 (cm/min)						
SYMBOL		% Water	Total Unit Weight		Dry Unit Weight		Normal Stress		Peak Stress		Ultimate Stress							
		Content	(pcf)	(kN/m³)	(pcf)	(kN/m³)	(psf)	(kPa)	(psf)	(kPa)	(psf)	(kPa)						
Initial / Set up		40.0	111.2	17.5	79.4	12.5	XXXXXX	XXXXXX	XXXXXX	XXXXXX	XXXXXX	XXXXXX						
pre-shear	I spec. 1	67.0	109.2	17.2	65.4	10.3	1943	93	984	47	XXXXXX	XXXXXX						
	U spec. 2	36.7	126.3	19.8	92.4	14.5	3902	187	1888	90	XXXXXX	XXXXXX						
	p spec. 3	24.5	132.7	20.9	106.6	16.8	7819	374	3800	182	XXXXXX	XXXXXX						
AECOM		Pure Water								DIRECT SHEAR TEST ASTM D 3080								
		Project Number: 60530732																
		Test Date: 5/3/2017																

COMPACTION CURVE

Test Method: ☒ ASTM D 1557 ☐ ASTM D 698 ☐ CA-DWR: S-10 ☐ Other Effort
 Compaction Procedure: Specimen Preparation Method: **Moist**



PARTICLE-SIZE DISTRIBUTION CURVE



NOTATION: ☒ Representative of entire sample ☐ Representative of compacted specimen ☐ Representative of compacted specimen and entire sample

Boring Number	Sample Number	Depth (ft.)	Optimum WC (%)	Maximum DUW (pcf)	Description and/or Classification
FM-01	1	0~5	11.4	123.9	Olive brown Silty SAND (SM)

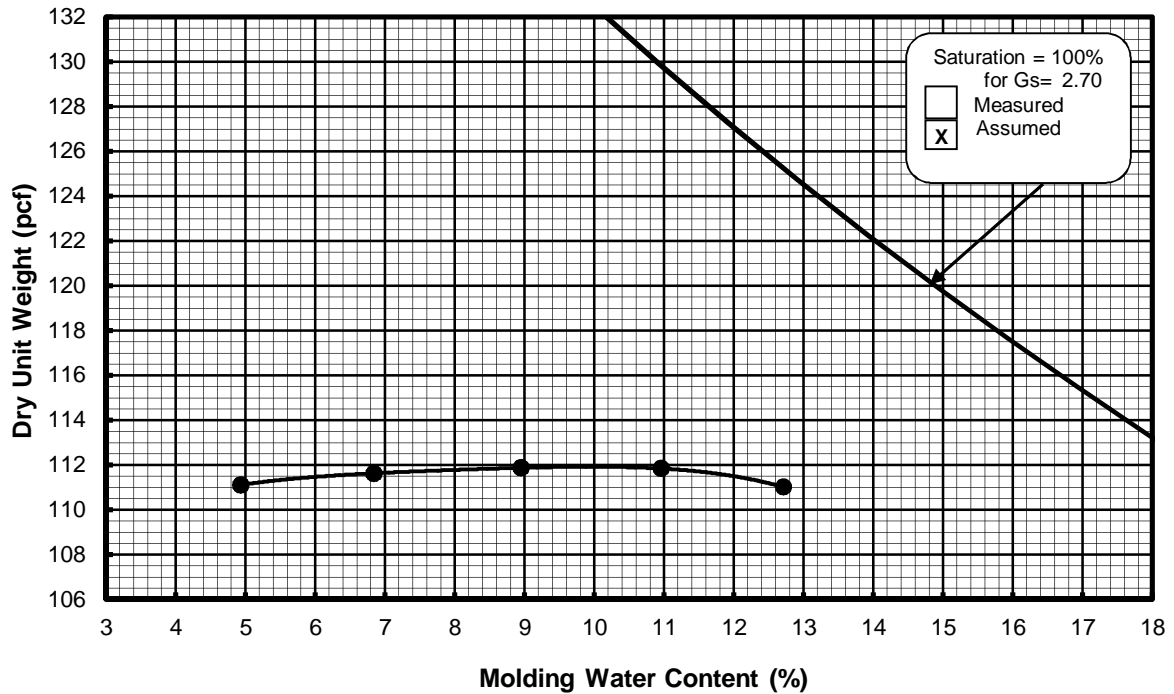
PROJECT NAME: Pure Water
PROJECT NUMBER: 60530732

**COMPACTION AND INDEX
PROPERTY DATA**

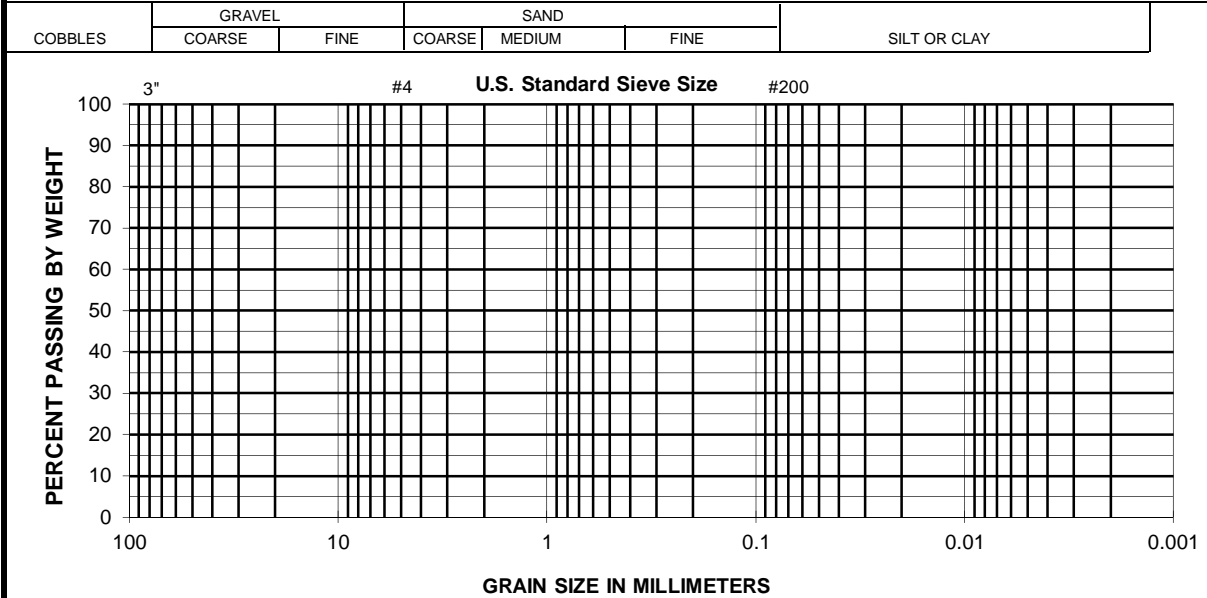
SUBMITTED BY: A. D. Camacho

COMPACTION CURVE

Test Method: ☒ ASTM D 1557 ☐ ASTM D 698 ☐ CA-DWR: S-10 ☐ Other Effort
 Compaction Procedure: Specimen Preparation Method: **Moist**



PARTICLE-SIZE DISTRIBUTION CURVE



NOTATION: ☒ Representative of entire sample ☐ Representative of compacted specimen ☐ Representative of compacted specimen and entire sample

Boring Number	Sample Number	Depth (ft.)	Optimum WC (%)	Maximum DUW (pcf)	Description and/or Classification
FM-02	1	0~5	10.2	112.0	Olive brown Silty SAND (SM)

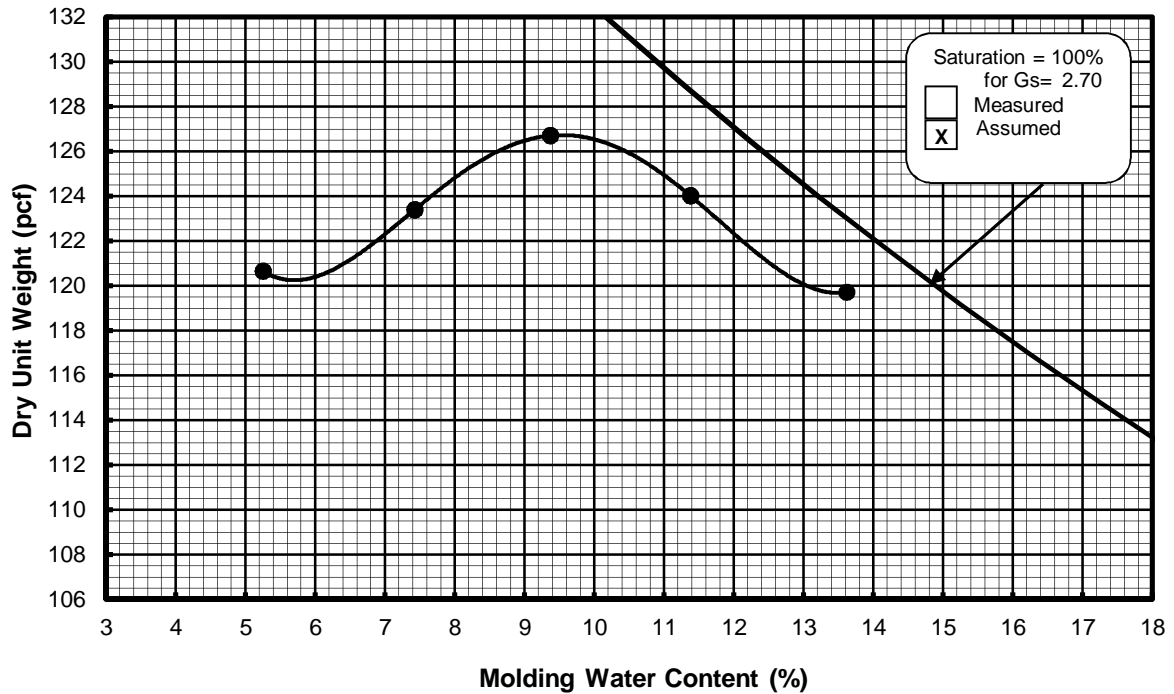
PROJECT NAME: Pure Water
PROJECT NUMBER: 60530732

**COMPACTION AND INDEX
PROPERTY DATA**

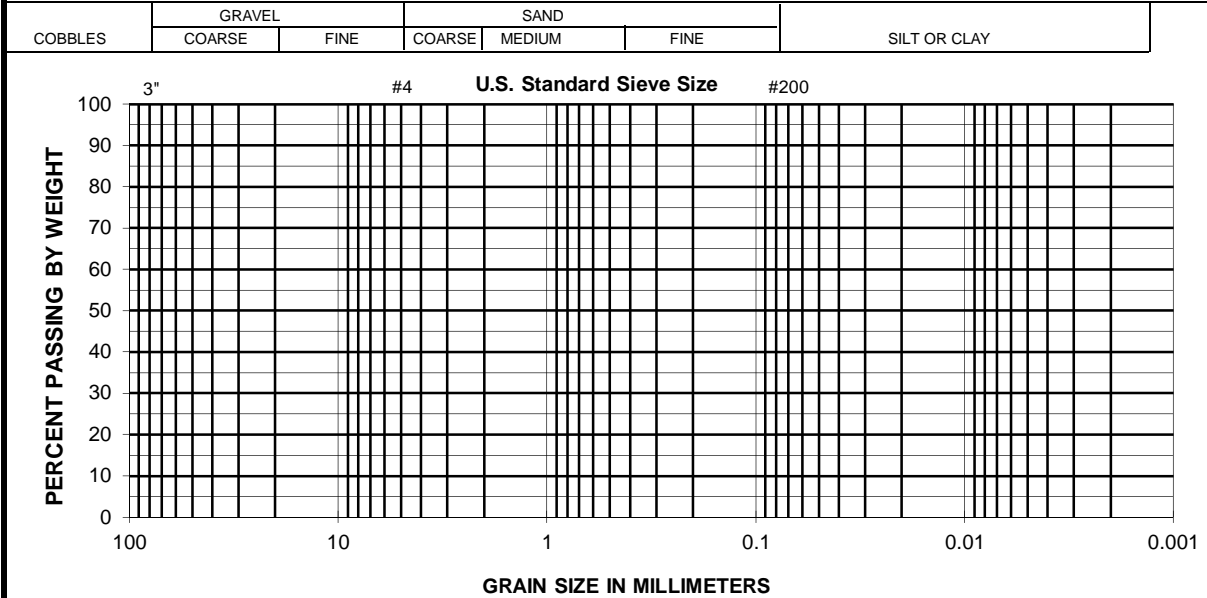
SUBMITTED BY: A. D. Camacho

COMPACTION CURVE

Test Method: ☒ ASTM D 1557 ☒ ASTM D 698 ☐ CA-DWR: S-10 ☐ Other Effort
 Compaction Procedure: Specimen Preparation Method: **Moist**



PARTICLE-SIZE DISTRIBUTION CURVE



NOTATION: ☒ Representative of entire sample ☐ Representative of compacted specimen ☐ Representative of compacted specimen and entire sample

Boring Number	Sample Number	Depth (ft.)	Optimum WC (%)	Maximum DUW (pcf)	Description and/or Classification
FM-07	1	0~5	9.4	126.7	Dark yellowish brown Silty SAND (SM)

PROJECT NAME: Pure Water
PROJECT NUMBER: 60530732

**COMPACTION AND INDEX
PROPERTY DATA**

SUBMITTED BY: A. D. Camacho



Table 1 - Laboratory Tests on Soil Samples

AECOM
Pure Water - City of San Diego
Your #60413030.29865969.00000, HDR Lab #17-0400LAB
22-Jun-17

Sample ID

			FM-01	FM-02	FM-03	FM-06	FM-08
Resistivity							
as-received	ohm-cm		16,000	1,680	1,120	2,680	1,280
minimum	ohm-cm		5,600	1,400	1,080	2,680	600
pH			7.7	8.5	8.7	7.8	8.0
Electrical							
Conductivity	mS/cm		0.08	0.08	0.26	0.05	0.36
Chemical Analyses							
Cations							
calcium	Ca ²⁺	mg/kg	16	6.2	31	5.0	13
magnesium	Mg ²⁺	mg/kg	2.9	1.9	18	1.6	5.9
sodium	Na ¹⁺	mg/kg	58	82	272	57	309
potassium	K ¹⁺	mg/kg	21	9.3	102	5.3	38
Anions							
carbonate	CO ₃ ²⁻	mg/kg	14	ND	128	ND	ND
bicarbonate	HCO ₃ ¹⁻	mg/kg	88	70	162	64	73
fluoride	F ¹⁻	mg/kg	8.6	4.5	16	10	7.3
chloride	Cl ¹⁻	mg/kg	16	37	27	17	19
sulfate	SO ₄ ²⁻	mg/kg	24	36	70	30	599
phosphate	PO ₄ ³⁻	mg/kg	ND	2.3	3.9	ND	ND
Other Tests							
ammonium	NH ₄ ¹⁺	mg/kg	ND	ND	ND	ND	ND
nitrate	NO ₃ ¹⁻	mg/kg	32	22	5.2	5.4	6.5
sulfide	S ²⁻	qual	na	na	na	na	na
Redox	mV		na	na	na	na	na

Minimum resistivity per CTM 643, Chlorides per CTM 422, Sulfates per CTM 417

Electrical conductivity in millisiemens/cm and chemical analyses were made on a 1:5 soil-to-water extract.

mg/kg = milligrams per kilogram (parts per million) of dry soil.

Redox = oxidation-reduction potential in millivolts

ND = not detected

na = not analyzed



Table 1 - Laboratory Tests on Soil Samples

AECOM
Pure Water - City of San Diego
Your #60413030.29865969.00000, HDR Lab #17-0400LAB
22-Jun-17

Sample ID

			FM-10-1	FM-14	FM-18	WM-06
Resistivity						
as-received	ohm-cm		2,120	6,400	2,760	6,000
minimum	ohm-cm		1,280	1,600	1,280	1,120
pH			8.3	8.4	7.9	7.6
Electrical						
Conductivity	mS/cm		0.06	0.18	0.09	0.21
Chemical Analyses						
Cations						
calcium	Ca ²⁺	mg/kg	9.3	18	12	16
magnesium	Mg ²⁺	mg/kg	5.1	8.0	7.9	6.5
sodium	Na ¹⁺	mg/kg	67	183	105	171
potassium	K ¹⁺	mg/kg	7.1	17	12	92
Anions						
carbonate	CO ₃ ²⁻	mg/kg	ND	44	ND	ND
bicarbonate	HCO ₃ ¹⁻	mg/kg	140	189	119	159
fluoride	F ¹⁻	mg/kg	4.7	10	8.9	34
chloride	Cl ¹⁻	mg/kg	13	29	29	111
sulfate	SO ₄ ²⁻	mg/kg	27	62	48	91
phosphate	PO ₄ ³⁻	mg/kg	2.5	ND	2.7	ND
Other Tests						
ammonium	NH ₄ ¹⁺	mg/kg	ND	ND	ND	ND
nitrate	NO ₃ ¹⁻	mg/kg	6.7	45	5.4	84
sulfide	S ²⁻	qual	na	na	na	na
Redox	mV		na	na	na	na

Minimum resistivity per CTM 643, Chlorides per CTM 422, Sulfates per CTM 417

Electrical conductivity in millisiemens/cm and chemical analyses were made on a 1:5 soil-to-water extract.

mg/kg = milligrams per kilogram (parts per million) of dry soil.

Redox = oxidation-reduction potential in millivolts

ND = not detected

na = not analyzed



Table 1 - Laboratory Tests on Soil Samples

AECOM
Pure Water City of San Diego
HDR Lab #17-0414LAB
29-Jun-17

Sample ID

WM-24 S-1 WM-11 S-1 WM-03 @-1
 @ 0-5' SM @ 0-5' SM @ 0-5' SM

Resistivity		Units			
as-received		ohm-cm	3,440	6,400	44,000
saturated		ohm-cm	1,240	1,360	5,600
pH			7.7	8.7	8.5
Electrical					
Conductivity		mS/cm	0.11	0.22	0.06
Chemical Analyses					
Cations					
calcium	Ca ²⁺	mg/kg	39	16	38
magnesium	Mg ²⁺	mg/kg	12	34	12
sodium	Na ¹⁺	mg/kg	94	246	39
potassium	K ¹⁺	mg/kg	46	26	28
Anions					
carbonate	CO ₃ ²⁻	mg/kg	ND	35	ND
bicarbonate	HCO ₃ ¹⁻	mg/kg	122	198	119
fluoride	F ¹⁻	mg/kg	3.4	4.9	2.3
chloride	Cl ¹⁻	mg/kg	27	84	3.7
sulfate	SO ₄ ²⁻	mg/kg	85	134	27
phosphate	PO ₄ ³⁻	mg/kg	6.6	2.4	6.7
Other Tests					
ammonium	NH ₄ ¹⁺	mg/kg	ND	ND	ND
nitrate	NO ₃ ¹⁻	mg/kg	26	17	37
sulfide	S ²⁻	qual	na	na	na
Redox		mV	na	na	na

Resistivity per ASTM G187, Cations per ASTM D6919, Anions per ASTM D4327, and Alkalinity per APHA 2320-B.
 Electrical conductivity in millisiemens/cm and chemical analyses were made on a 1:5 soil-to-water extract.
 mg/kg = milligrams per kilogram (parts per million) of dry soil.
 Redox = oxidation-reduction potential in millivolts
 ND = not detected
 na = not analyzed



Table 1 - Laboratory Tests on Soil Samples

AECOM
Pure Water - City of San Diego
HDR Lab #17-0507LAB
28-Jul-17

Sample ID			FM-20 @ 0-5 ft	FM-21 @ 0-5 ft	FM-35 @ 0-5 ft	FM-36b @ 0-5 ft	FM-38 @ 0-5 ft
Resistivity							
as-received	ohm-cm		6,800	21,200	4,400	26,400	13,200
minimum	ohm-cm		2,120	1,120	1,400	1,080	1,240
pH			8.1	7.9	7.6	7.9	6.7
Electrical							
Conductivity	mS/cm		0.07	0.26	0.10	0.23	0.07
Chemical Analyses							
Cations							
calcium	Ca ²⁺	mg/kg	43	29	47	39	35
magnesium	Mg ²⁺	mg/kg	27	15	25	ND	14
sodium	Na ¹⁺	mg/kg	92	246	123	231	75
potassium	K ¹⁺	mg/kg	8.0	44	19	24	22
Anions							
carbonate	CO ₃ ²⁻	mg/kg	ND	ND	ND	ND	ND
bicarbonate	HCO ₃ ¹⁻	mg/kg	95	302	82	229	15
fluoride	F ¹⁻	mg/kg	20	7.7	10	12	3.3
chloride	Cl ¹⁻	mg/kg	17	126	26	94	5.3
sulfate	SO ₄ ²⁻	mg/kg	33	146	109	148	92
phosphate	PO ₄ ³⁻	mg/kg	17	6.4	2.4	1.7	1.7
Other Tests							
ammonium	NH ₄ ¹⁺	mg/kg	ND	ND	ND	ND	ND
nitrate	NO ₃ ¹⁻	mg/kg	6.0	13	8.0	59	4.3
sulfide	S ²⁻	qual	na	na	na	na	na
Redox	mV		na	na	na	na	na

Minimum resistivity per CTM 643, Chlorides per CTM 422, Sulfates per CTM 417

Electrical conductivity in millisiemens/cm and chemical analyses were made on a 1:5 soil-to-water extract.

mg/kg = milligrams per kilogram (parts per million) of dry soil.

Redox = oxidation-reduction potential in millivolts

ND = not detected

na = not analyzed



Table 1 - Laboratory Tests on Soil Samples

AECOM
Pure Water - City of San Diego
HDR Lab #17-0507LAB
28-Jul-17

Sample ID			FM-41 @ 0-5 ft	FM-49 @ 0-5 ft	FM-52 @ 0-5 ft
Resistivity		Units			
	as-received	ohm-cm	3,040	5,200	10,800
	minimum	ohm-cm	480	880	1,280
pH			7.2	8.1	8.9
Electrical					
Conductivity		mS/cm	0.63	0.25	0.25
Chemical Analyses					
Cations					
calcium	Ca ²⁺	mg/kg	75	33	45
magnesium	Mg ²⁺	mg/kg	20	24	ND
sodium	Na ¹⁺	mg/kg	456	271	260
potassium	K ¹⁺	mg/kg	71	19	45
Anions					
carbonate	CO ₃ ²⁻	mg/kg	ND	ND	ND
bicarbonate	HCO ₃ ¹⁻	mg/kg	122	98	433
fluoride	F ¹⁻	mg/kg	16	36	37
chloride	Cl ¹⁻	mg/kg	657	192	28
sulfate	SO ₄ ²⁻	mg/kg	288	138	227
phosphate	PO ₄ ³⁻	mg/kg	2.7	14	3.4
Other Tests					
ammonium	NH ₄ ¹⁺	mg/kg	ND	ND	ND
nitrate	NO ₃ ¹⁻	mg/kg	10	4.6	6.2
sulfide	S ²⁻	qual	na	na	na
Redox		mV	na	na	na

Minimum resistivity per CTM 643, Chlorides per CTM 422, Sulfates per CTM 417

Electrical conductivity in millisiemens/cm and chemical analyses were made on a 1:5 soil-to-water extract.

mg/kg = milligrams per kilogram (parts per million) of dry soil.

Redox = oxidation-reduction potential in millivolts

ND = not detected

na = not analyzed



Table 1 - Laboratory Tests on Soil Samples

AECOM
Pure Water - City of San Diego
HDR Lab #17-0562LAB
28-Aug-17

Sample ID			DS-01 @ 0-5 ft	FM-25 @ 0-5 ft	FM-29 @ 0-5 ft	FM-50 @ 0-5 ft
Resistivity	Units					
as-received	ohm-cm		13,600	14,800	14,400	7,200
minimum	ohm-cm		2,200	2,680	840	2,600
pH			7.6	7.3	7.1	7.3
Electrical						
Conductivity	mS/cm		0.09	0.11	0.29	0.12
Chemical Analyses						
Cations						
calcium	Ca ²⁺	mg/kg	ND	25	24	31
magnesium	Mg ²⁺	mg/kg	7.3	6.9	7.6	7.7
sodium	Na ¹⁺	mg/kg	80	88	227	133
potassium	K ¹⁺	mg/kg	7.8	8.5	10	17
Anions						
carbonate	CO ₃ ²⁻	mg/kg	ND	ND	ND	35
bicarbonate	HCO ₃ ¹⁻	mg/kg	79	101	214	247
fluoride	F ¹⁻	mg/kg	2.0	3.1	6.4	4.6
chloride	Cl ¹⁻	mg/kg	25	5.5	121	4.0
sulfate	SO ₄ ²⁻	mg/kg	41	78	163	35
phosphate	PO ₄ ³⁻	mg/kg	ND	ND	ND	4.4
Other Tests						
ammonium	NH ₄ ¹⁺	mg/kg	ND	ND	ND	ND
nitrate	NO ₃ ¹⁻	mg/kg	5.2	4.6	4.5	4.8
sulfide	S ²⁻	qual	na	na	na	na
Redox	mV		na	na	na	na

Minimum resistivity per CTM 643, Chlorides per CTM 422, Sulfates per CTM 417

Electrical conductivity in millisiemens/cm and chemical analyses were made on a 1:5 soil-to-water extract.

mg/kg = milligrams per kilogram (parts per million) of dry soil.

Redox = oxidation-reduction potential in millivolts

ND = not detected

na = not analyzed

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APPENDIX C

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APPENDIX C – Groundwater Sampling

On March 31, 2017, AECOM personnel sampled the groundwater monitoring well PS-2. Depth to groundwater was measured using an electronic water level indicator and measured to the closest hundredth-inch from the top of the well casing. The well was purged by manually bailing using a disposal bailer using low flow purge techniques in accordance with the County of San Diego Department of Environmental Health Site Assessment and Mitigation (SAM) Manual guidelines. Field parameters including pH, temperature, and electric conductivity were regularly monitored during purging. Purging continued until measurements stabilized to generally within 10 percent of the previous reading. Field data was recorded on the sampling forms in our project files, and is available for review if requested.

One groundwater sample was collected using a disposal bailer and placed in unused, laboratory-supplied sample containers. The sample was stored in the field in a chilled cooler pending transport to a California Department of Health Services-Certified analytical laboratory, Eurofins Calscience, Inc., under proper chain-of-custody documentation.

The groundwater sample was analyzed for total petroleum hydrocarbons (TPH) extended range by EPA Modified Method 8015B, Title 22 metals including mercury by EPA Methods 6010B and 7470A, organochlorine pesticides (OCPs) by EPA Method 8081A, polychlorinated biphenyls (PCBs) by EPA Method 8082, semi-volatile organic compounds (SVOCs) by EPA Method 8270C, and volatile organic compounds (VOCs) by EPA Method 8260B.

Purged water generated during sampling was contained in one California Department of Transportation 55-gallon drum and transported to the project temporary storage facility on City property pending disposal evaluation.

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**WORK ORDER NUMBER: 17-03-2408***The difference is service*

AIR | SOIL | WATER | MARINE CHEMISTRY

Analytical Report For**Client:** AECOM**Client Project Name:** Pure Water**Attention:** Steve Fitzwilliam
401 West A Street
Suite 1200
San Diego, CA 92101-2421*Vikas Patel*

Approved for release on 04/11/2017 by:
Vikas Patel
Project Manager

ResultLink ▶

Email your PM ▶

Eurofins Calscience, Inc. (Calscience) certifies that the test results provided in this report meet all NELAC requirements for parameters for which accreditation is required or available. Any exceptions to NELAC requirements are noted in the case narrative. The original report of subcontracted analyses, if any, is attached to this report. The results in this report are limited to the sample(s) tested and any reproduction thereof must be made in its entirety. The client or recipient of this report is specifically prohibited from making material changes to said report and, to the extent that such changes are made, Calscience is not responsible, legally or otherwise. The client or recipient agrees to indemnify Calscience for any defense to any litigation which may arise.

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 Work Order Number: 17-03-2408

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Work Order Narrative

Work Order: 17-03-2408Page 1 of 1

Condition Upon Receipt:

Samples were received under Chain-of-Custody (COC) on 03/31/17. They were assigned to Work Order 17-03-2408.

Unless otherwise noted on the Sample Receiving forms all samples were received in good condition and within the recommended EPA temperature criteria for the methods noted on the COC. The COC and Sample Receiving Documents are integral elements of the analytical report and are presented at the back of the report.

Holding Times:

All samples were analyzed within prescribed holding times (HT) and/or in accordance with the Calscience Sample Acceptance Policy unless otherwise noted in the analytical report and/or comprehensive case narrative, if required.

Any parameter identified in 40CFR Part 136.3 Table II that is designated as "analyze immediately" with a holding time of ≤ 15 minutes (40CFR-136.3 Table II, footnote 4), is considered a "field" test and the reported results will be qualified as being received outside of the stated holding time unless received at the laboratory within 15 minutes of the collection time.

Quality Control:

All quality control parameters (QC) were within established control limits except where noted in the QC summary forms or described further within this report.

Subcontractor Information:

Unless otherwise noted below (or on the subcontract form), no samples were subcontracted.

Additional Comments:

Air - Sorbent-extracted air methods (EPA TO-4A, EPA TO-10, EPA TO-13A, EPA TO-17): Analytical results are converted from mass/sample basis to mass/volume basis using client-supplied air volumes.

Solid - Unless otherwise indicated, solid sample data is reported on a wet weight basis, not corrected for % moisture. All QC results are always reported on a wet weight basis.

QC Association Summary

Work Order: 17-03-2408

Page 1 of 1

<u>Client Sample ID</u>	<u>Method Name</u>	<u>Type</u>	<u>Ext Name</u>	<u>Instrument</u>	<u>MS/MSD/SDP</u>	<u>LCS/LCSD</u>
PS-2	EPA 6010B/7470A CAC Title 22 Metals		EPA 3010A Total	ICP 7300	170403SA5	170403LA5
PS-2	EPA 7470A Mercury		EPA 7470A Total	Mercury 07	170407SA2	170407LA2
PS-2	EPA 8015B (M) C6-C44		EPA 3510C	GC 47	*2	170404B02A
PS-2	EPA 8081A Organochlorine Pesticides		EPA 3510C	GC 44	*2	170404L03
PS-2	EPA 8082 PCB Aroclors		EPA 3510C	GC 66	*2	170404L04
PS-2	EPA 8260B Volatile Organics + Oxygenates		EPA 5030C	GC/MS V V	170404S011	170401L029
PS-2	EPA 8260B Volatile Organics + Oxygenates	R	EPA 5030C	GC/MS V V	170404S015	170404L033
PS-2	EPA 8270C Semi-Volatile Organics		EPA 3510C	GC/MS TT	*2	170403L04A

2 = Limited sample received, no MS/MSD performed

R = Rerun

Detections Summary

Client: AECOM
 401 West A Street, Suite 1200
 San Diego, CA 92101-2421

Work Order: 17-03-2408
 Project Name: Pure Water
 Received: 03/31/17

Attn: Steve Fitzwilliam

Page 1 of 1

Client SampleID

<u>Analyte</u>	<u>Result</u>	<u>Qualifiers</u>	<u>RL</u>	<u>Units</u>	<u>Method</u>	<u>Extraction</u>
PS-2 (17-03-2408-1)						
Barium	0.142		0.0100	mg/L	EPA 6010B	EPA 3010A Total
Molybdenum	0.0287		0.0100	mg/L	EPA 6010B	EPA 3010A Total
C6-C44 Total	73		50	ug/L	EPA 8015B (M)	EPA 3510C
c-1,2-Dichloroethene	480		10	ug/L	EPA 8260B	EPA 5030C
t-1,2-Dichloroethene	2.6		1.0	ug/L	EPA 8260B	EPA 5030C

Subcontracted analyses, if any, are not included in this summary.

* MDL is shown

Analytical Report

AECOM 401 West A Street, Suite 1200 San Diego, CA 92101-2421	Date Received: 03/31/17 Work Order: 17-03-2408 Preparation: EPA 3510C Method: EPA 8015B (M) Units: ug/L
--	---

Project: Pure Water Page 1 of 2

Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
PS-2	17-03-2408-1-G	03/31/17 09:25	Aqueous	GC 47	04/04/17	04/05/17 04:44	170404B02A

Comment(s): - The total concentration includes individual carbon range concentrations (estimated), if any, below the RL reported as ND.

<u>Parameter</u>	<u>Result</u>	<u>RL</u>	<u>DF</u>	<u>Qualifiers</u>
C6	ND	50	1.00	
C7	ND	50	1.00	
C8	ND	50	1.00	
C9-C10	ND	50	1.00	
C11-C12	ND	50	1.00	
C13-C14	ND	50	1.00	
C15-C16	ND	50	1.00	
C17-C18	ND	50	1.00	
C19-C20	ND	50	1.00	
C21-C22	ND	50	1.00	
C23-C24	ND	50	1.00	
C25-C28	ND	50	1.00	
C29-C32	ND	50	1.00	
C33-C36	ND	50	1.00	
C37-C40	ND	50	1.00	
C41-C44	ND	50	1.00	
C6-C44 Total	73	50	1.00	

<u>Surrogate</u>	<u>Rec. (%)</u>	<u>Control Limits</u>	<u>Qualifiers</u>
n-Octacosane	106	68-140	

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM
 401 West A Street, Suite 1200
 San Diego, CA 92101-2421

Date Received: 03/31/17
 Work Order: 17-03-2408
 Preparation: EPA 3510C
 Method: EPA 8015B (M)
 Units: ug/L

Project: Pure Water

Page 2 of 2

Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
Method Blank	099-15-472-642	N/A	Aqueous	GC 47	04/04/17	04/05/17 00:54	170404B02A

Parameter	Result	RL	DF	Qualifiers
C6	ND	50	1.00	
C7	ND	50	1.00	
C8	ND	50	1.00	
C9-C10	ND	50	1.00	
C11-C12	ND	50	1.00	
C13-C14	ND	50	1.00	
C15-C16	ND	50	1.00	
C17-C18	ND	50	1.00	
C19-C20	ND	50	1.00	
C21-C22	ND	50	1.00	
C23-C24	ND	50	1.00	
C25-C28	ND	50	1.00	
C29-C32	ND	50	1.00	
C33-C36	ND	50	1.00	
C37-C40	ND	50	1.00	
C41-C44	ND	50	1.00	
C6-C44 Total	ND	50	1.00	

Surrogate	Rec. (%)	Control Limits	Qualifiers
n-Octacosane	91	68-140	

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM 401 West A Street, Suite 1200 San Diego, CA 92101-2421	Date Received: 03/31/17 Work Order: 17-03-2408 Preparation: EPA 3010A Total Method: EPA 6010B Units: mg/L
Project: Pure Water	Page 1 of 2

Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
PS-2	17-03-2408-1-F	03/31/17 09:25	Aqueous	ICP 7300	04/03/17	04/04/17 15:06	170403LA5
<u>Parameter</u>	<u>Result</u>		<u>RL</u>	<u>DF</u>		<u>Qualifiers</u>	
Antimony	ND		0.0150	1.00			
Arsenic	ND		0.0100	1.00			
Barium	0.142		0.0100	1.00			
Beryllium	ND		0.0100	1.00			
Cadmium	ND		0.0100	1.00			
Chromium	ND		0.0100	1.00			
Cobalt	ND		0.0100	1.00			
Copper	ND		0.0100	1.00			
Lead	ND		0.0100	1.00			
Molybdenum	0.0287		0.0100	1.00			
Nickel	ND		0.0100	1.00			
Selenium	ND		0.0150	1.00			
Silver	ND		0.00500	1.00			
Thallium	ND		0.0150	1.00			
Vanadium	ND		0.0100	1.00			
Zinc	ND		0.0100	1.00			


 Return to Contents

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM 401 West A Street, Suite 1200 San Diego, CA 92101-2421 Project: Pure Water	Date Received: 03/31/17 Work Order: 17-03-2408 Preparation: EPA 3010A Total Method: EPA 6010B Units: mg/L Page 2 of 2
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Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
Method Blank	097-01-003-16394	N/A	Aqueous	ICP 7300	04/03/17	04/04/17 11:17	170403LA5

Parameter	Result	RL	DF	Qualifiers
Antimony	ND	0.0150	1.00	
Arsenic	ND	0.0100	1.00	
Barium	ND	0.0100	1.00	
Beryllium	ND	0.0100	1.00	
Cadmium	ND	0.0100	1.00	
Chromium	ND	0.0100	1.00	
Cobalt	ND	0.0100	1.00	
Copper	ND	0.0100	1.00	
Lead	ND	0.0100	1.00	
Molybdenum	ND	0.0100	1.00	
Nickel	ND	0.0100	1.00	
Selenium	ND	0.0150	1.00	
Silver	ND	0.00500	1.00	
Thallium	ND	0.0150	1.00	
Vanadium	ND	0.0100	1.00	
Zinc	ND	0.0100	1.00	

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RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM 401 West A Street, Suite 1200 San Diego, CA 92101-2421 Project: Pure Water	Date Received: 03/31/17 Work Order: 17-03-2408 Preparation: EPA 7470A Total Method: EPA 7470A Units: mg/L
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Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
PS-2	17-03-2408-1-F	03/31/17 09:25	Aqueous	Mercury 07	04/07/17	04/07/17 14:53	170407LA2

<u>Parameter</u>	<u>Result</u>	<u>RL</u>	<u>DF</u>	<u>Qualifiers</u>
Mercury	ND	0.000500	1.00	

Method Blank	099-04-008-8169	N/A	Aqueous	Mercury 07	04/07/17	04/07/17 14:48	170407LA2
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<u>Parameter</u>	<u>Result</u>	<u>RL</u>	<u>DF</u>	<u>Qualifiers</u>
Mercury	ND	0.000500	1.00	

Analytical Report

AECOM 401 West A Street, Suite 1200 San Diego, CA 92101-2421	Date Received: 03/31/17 Work Order: 17-03-2408 Preparation: EPA 3510C Method: EPA 8081A Units: ug/L
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Project: Pure Water Page 1 of 2

Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
PS-2	17-03-2408-1-H	03/31/17 09:25	Aqueous	GC 44	04/04/17	04/07/17 08:02	170404L03

<u>Parameter</u>	<u>Result</u>	<u>RL</u>	<u>DF</u>	<u>Qualifiers</u>
Alpha-BHC	ND	0.096	1.00	
Gamma-BHC	ND	0.096	1.00	
Beta-BHC	ND	0.096	1.00	
Heptachlor	ND	0.096	1.00	
Delta-BHC	ND	0.096	1.00	
Aldrin	ND	0.096	1.00	
Heptachlor Epoxide	ND	0.096	1.00	
Endosulfan I	ND	0.096	1.00	
Dieldrin	ND	0.096	1.00	
4,4'-DDE	ND	0.096	1.00	
Endrin	ND	0.096	1.00	
Endrin Aldehyde	ND	0.096	1.00	
4,4'-DDD	ND	0.096	1.00	
Endosulfan II	ND	0.096	1.00	
4,4'-DDT	ND	0.096	1.00	
Endosulfan Sulfate	ND	0.096	1.00	
Methoxychlor	ND	0.096	1.00	
Chlordane	ND	0.96	1.00	
Toxaphene	ND	1.9	1.00	
Endrin Ketone	ND	0.096	1.00	

<u>Surrogate</u>	<u>Rec. (%)</u>	<u>Control Limits</u>	<u>Qualifiers</u>
Decachlorobiphenyl	72	50-135	
2,4,5,6-Tetrachloro-m-Xylene	93	50-135	

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM 401 West A Street, Suite 1200 San Diego, CA 92101-2421	Date Received: 03/31/17 Work Order: 17-03-2408 Preparation: EPA 3510C Method: EPA 8081A Units: ug/L
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Project: Pure Water Page 2 of 2

Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
Method Blank	099-12-529-949	N/A	Aqueous	GC 44	04/04/17	04/06/17 16:44	170404L03

Parameter	Result	RL	DF	Qualifiers
Alpha-BHC	ND	0.10	1.00	
Gamma-BHC	ND	0.10	1.00	
Beta-BHC	ND	0.10	1.00	
Heptachlor	ND	0.10	1.00	
Delta-BHC	ND	0.10	1.00	
Aldrin	ND	0.10	1.00	
Heptachlor Epoxide	ND	0.10	1.00	
Endosulfan I	ND	0.10	1.00	
Dieldrin	ND	0.10	1.00	
4,4'-DDE	ND	0.10	1.00	
Endrin	ND	0.10	1.00	
Endrin Aldehyde	ND	0.10	1.00	
4,4'-DDD	ND	0.10	1.00	
Endosulfan II	ND	0.10	1.00	
4,4'-DDT	ND	0.10	1.00	
Endosulfan Sulfate	ND	0.10	1.00	
Methoxychlor	ND	0.10	1.00	
Chlordane	ND	1.0	1.00	
Toxaphene	ND	2.0	1.00	
Endrin Ketone	ND	0.10	1.00	

Surrogate	Rec. (%)	Control Limits	Qualifiers
Decachlorobiphenyl	79	50-135	
2,4,5,6-Tetrachloro-m-Xylene	85	50-135	

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM
401 West A Street, Suite 1200
San Diego, CA 92101-2421

Date Received: 03/31/17
Work Order: 17-03-2408
Preparation: EPA 3510C
Method: EPA 8082
Units: ug/L

Project: Pure Water

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Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
PS-2	17-03-2408-1-H	03/31/17 09:25	Aqueous	GC 66	04/04/17	04/06/17 14:33	170404L04

Parameter	Result	RL	DF	Qualifiers
Aroclor-1016	ND	0.96	1.00	
Aroclor-1221	ND	0.96	1.00	
Aroclor-1232	ND	0.96	1.00	
Aroclor-1242	ND	0.96	1.00	
Aroclor-1248	ND	0.96	1.00	
Aroclor-1254	ND	0.96	1.00	
Aroclor-1260	ND	0.96	1.00	
Aroclor-1262	ND	0.96	1.00	
Aroclor-1268	ND	0.96	1.00	

Surrogate	Rec. (%)	Control Limits	Qualifiers
Decachlorobiphenyl	77	50-135	
2,4,5,6-Tetrachloro-m-Xylene	81	50-135	

Method Blank	099-12-533-1272	N/A	Aqueous	GC 66	04/04/17	04/06/17 13:40	170404L04
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Parameter	Result	RL	DF	Qualifiers
Aroclor-1016	ND	1.0	1.00	
Aroclor-1221	ND	1.0	1.00	
Aroclor-1232	ND	1.0	1.00	
Aroclor-1242	ND	1.0	1.00	
Aroclor-1248	ND	1.0	1.00	
Aroclor-1254	ND	1.0	1.00	
Aroclor-1260	ND	1.0	1.00	
Aroclor-1262	ND	1.0	1.00	
Aroclor-1268	ND	1.0	1.00	

Surrogate	Rec. (%)	Control Limits	Qualifiers
Decachlorobiphenyl	73	50-135	
2,4,5,6-Tetrachloro-m-Xylene	72	50-135	

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM
401 West A Street, Suite 1200
San Diego, CA 92101-2421

Date Received: 03/31/17
Work Order: 17-03-2408
Preparation: EPA 3510C
Method: EPA 8270C
Units: ug/L

Project: Pure Water

Page 1 of 6

Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
PS-2	17-03-2408-1-I	03/31/17 09:25	Aqueous	GC/MS TT	04/03/17	04/04/17 20:53	170403L04A

Parameter	Result	RL	DF	Qualifiers
Carbazole	ND	10	1.00	
Acenaphthene	ND	10	1.00	
Acenaphthylene	ND	10	1.00	
Aniline	ND	10	1.00	
Anthracene	ND	10	1.00	
Azobenzene	ND	10	1.00	
Benzidine	ND	50	1.00	
Benzo (a) Anthracene	ND	10	1.00	
Benzo (a) Pyrene	ND	10	1.00	
Benzo (b) Fluoranthene	ND	10	1.00	
Benzo (g,h,i) Perylene	ND	10	1.00	
Benzo (k) Fluoranthene	ND	10	1.00	
Benzoic Acid	ND	50	1.00	
Benzyl Alcohol	ND	10	1.00	
Bis(2-Chloroethoxy) Methane	ND	10	1.00	
Bis(2-Chloroethyl) Ether	ND	25	1.00	
Bis(2-Chloroisopropyl) Ether	ND	10	1.00	
Bis(2-Ethylhexyl) Phthalate	ND	10	1.00	
4-Bromophenyl-Phenyl Ether	ND	10	1.00	
Butyl Benzyl Phthalate	ND	10	1.00	
4-Chloro-3-Methylphenol	ND	10	1.00	
4-Chloroaniline	ND	10	1.00	
2-Chloronaphthalene	ND	10	1.00	
2-Chlorophenol	ND	10	1.00	
4-Chlorophenyl-Phenyl Ether	ND	10	1.00	
Chrysene	ND	10	1.00	
Di-n-Butyl Phthalate	ND	10	1.00	
Di-n-Octyl Phthalate	ND	10	1.00	
Dibenz (a,h) Anthracene	ND	10	1.00	
Dibenzofuran	ND	10	1.00	
1,2-Dichlorobenzene	ND	10	1.00	
1,3-Dichlorobenzene	ND	10	1.00	
1,4-Dichlorobenzene	ND	10	1.00	
3,3'-Dichlorobenzidine	ND	25	1.00	
2,4-Dichlorophenol	ND	10	1.00	

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM
401 West A Street, Suite 1200
San Diego, CA 92101-2421

Date Received: 03/31/17
Work Order: 17-03-2408
Preparation: EPA 3510C
Method: EPA 8270C
Units: ug/L

Project: Pure Water

Page 2 of 6

Parameter	Result	RL	DF	Qualifiers
Diethyl Phthalate	ND	10	1.00	
Dimethyl Phthalate	ND	10	1.00	
2,4-Dimethylphenol	ND	10	1.00	
4,6-Dinitro-2-Methylphenol	ND	50	1.00	
2,4-Dinitrophenol	ND	50	1.00	
2,4-Dinitrotoluene	ND	10	1.00	
2,6-Dinitrotoluene	ND	10	1.00	
Fluoranthene	ND	10	1.00	
Fluorene	ND	10	1.00	
Hexachloro-1,3-Butadiene	ND	10	1.00	
Hexachlorobenzene	ND	10	1.00	
Hexachlorocyclopentadiene	ND	25	1.00	
Hexachloroethane	ND	10	1.00	
Indeno (1,2,3-c,d) Pyrene	ND	10	1.00	
Isophorone	ND	10	1.00	
2-Methylnaphthalene	ND	10	1.00	
1-Methylnaphthalene	ND	10	1.00	
2-Methylphenol	ND	10	1.00	
3/4-Methylphenol	ND	10	1.00	
N-Nitroso-di-n-propylamine	ND	10	1.00	
N-Nitrosodimethylamine	ND	10	1.00	
N-Nitrosodiphenylamine	ND	10	1.00	
Naphthalene	ND	10	1.00	
4-Nitroaniline	ND	10	1.00	
3-Nitroaniline	ND	10	1.00	
2-Nitroaniline	ND	10	1.00	
Nitrobenzene	ND	25	1.00	
4-Nitrophenol	ND	10	1.00	
2-Nitrophenol	ND	10	1.00	
Pentachlorophenol	ND	10	1.00	
Phenanthrene	ND	10	1.00	
Phenol	ND	10	1.00	
Pyrene	ND	10	1.00	
Pyridine	ND	10	1.00	
1,2,4-Trichlorobenzene	ND	10	1.00	
2,4,6-Trichlorophenol	ND	10	1.00	
2,4,5-Trichlorophenol	ND	10	1.00	
2,6-Dichlorophenol	ND	10	1.00	

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM	Date Received:	03/31/17
401 West A Street, Suite 1200	Work Order:	17-03-2408
San Diego, CA 92101-2421	Preparation:	EPA 3510C
	Method:	EPA 8270C
	Units:	ug/L

Project: Pure Water Page 3 of 6

<u>Surrogate</u>	<u>Rec. (%)</u>	<u>Control Limits</u>	<u>Qualifiers</u>
2-Fluorobiphenyl	75	33-120	
2-Fluorophenol	55	24-120	
Nitrobenzene-d5	88	38-120	
p-Terphenyl-d14	95	41-137	
Phenol-d6	37	16-120	
2,4,6-Tribromophenol	85	27-159	

Analytical Report

AECOM
401 West A Street, Suite 1200
San Diego, CA 92101-2421

Date Received: 03/31/17
Work Order: 17-03-2408
Preparation: EPA 3510C
Method: EPA 8270C
Units: ug/L

Project: Pure Water

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Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
Method Blank	095-01-003-4354	N/A	Aqueous	GC/MS TT	04/03/17	04/04/17 17:04	170403L04A

Parameter	Result	RL	DF	Qualifiers
Carbazole	ND	10	1.00	
Acenaphthene	ND	10	1.00	
Acenaphthylene	ND	10	1.00	
Aniline	ND	10	1.00	
Anthracene	ND	10	1.00	
Azobenzene	ND	10	1.00	
Benzidine	ND	50	1.00	
Benzo (a) Anthracene	ND	10	1.00	
Benzo (a) Pyrene	ND	10	1.00	
Benzo (b) Fluoranthene	ND	10	1.00	
Benzo (g,h,i) Perylene	ND	10	1.00	
Benzo (k) Fluoranthene	ND	10	1.00	
Benzoic Acid	ND	50	1.00	
Benzyl Alcohol	ND	10	1.00	
Bis(2-Chloroethoxy) Methane	ND	10	1.00	
Bis(2-Chloroethyl) Ether	ND	25	1.00	
Bis(2-Chloroisopropyl) Ether	ND	10	1.00	
Bis(2-Ethylhexyl) Phthalate	ND	10	1.00	
4-Bromophenyl-Phenyl Ether	ND	10	1.00	
Butyl Benzyl Phthalate	ND	10	1.00	
4-Chloro-3-Methylphenol	ND	10	1.00	
4-Chloroaniline	ND	10	1.00	
2-Chloronaphthalene	ND	10	1.00	
2-Chlorophenol	ND	10	1.00	
4-Chlorophenyl-Phenyl Ether	ND	10	1.00	
Chrysene	ND	10	1.00	
Di-n-Butyl Phthalate	ND	10	1.00	
Di-n-Octyl Phthalate	ND	10	1.00	
Dibenz (a,h) Anthracene	ND	10	1.00	
Dibenzofuran	ND	10	1.00	
1,2-Dichlorobenzene	ND	10	1.00	
1,3-Dichlorobenzene	ND	10	1.00	
1,4-Dichlorobenzene	ND	10	1.00	
3,3'-Dichlorobenzidine	ND	25	1.00	
2,4-Dichlorophenol	ND	10	1.00	

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM
401 West A Street, Suite 1200
San Diego, CA 92101-2421

Date Received: 03/31/17
Work Order: 17-03-2408
Preparation: EPA 3510C
Method: EPA 8270C
Units: ug/L

Project: Pure Water

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Parameter	Result	RL	DF	Qualifiers
Diethyl Phthalate	ND	10	1.00	
Dimethyl Phthalate	ND	10	1.00	
2,4-Dimethylphenol	ND	10	1.00	
4,6-Dinitro-2-Methylphenol	ND	50	1.00	
2,4-Dinitrophenol	ND	50	1.00	
2,4-Dinitrotoluene	ND	10	1.00	
2,6-Dinitrotoluene	ND	10	1.00	
Fluoranthene	ND	10	1.00	
Fluorene	ND	10	1.00	
Hexachloro-1,3-Butadiene	ND	10	1.00	
Hexachlorobenzene	ND	10	1.00	
Hexachlorocyclopentadiene	ND	25	1.00	
Hexachloroethane	ND	10	1.00	
Indeno (1,2,3-c,d) Pyrene	ND	10	1.00	
Isophorone	ND	10	1.00	
2-Methylnaphthalene	ND	10	1.00	
1-Methylnaphthalene	ND	10	1.00	
2-Methylphenol	ND	10	1.00	
3/4-Methylphenol	ND	10	1.00	
N-Nitroso-di-n-propylamine	ND	10	1.00	
N-Nitrosodimethylamine	ND	10	1.00	
N-Nitrosodiphenylamine	ND	10	1.00	
Naphthalene	ND	10	1.00	
4-Nitroaniline	ND	10	1.00	
3-Nitroaniline	ND	10	1.00	
2-Nitroaniline	ND	10	1.00	
Nitrobenzene	ND	25	1.00	
4-Nitrophenol	ND	10	1.00	
2-Nitrophenol	ND	10	1.00	
Pentachlorophenol	ND	10	1.00	
Phenanthrene	ND	10	1.00	
Phenol	ND	10	1.00	
Pyrene	ND	10	1.00	
Pyridine	ND	10	1.00	
1,2,4-Trichlorobenzene	ND	10	1.00	
2,4,6-Trichlorophenol	ND	10	1.00	
2,4,5-Trichlorophenol	ND	10	1.00	
2,6-Dichlorophenol	ND	10	1.00	

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM	Date Received:	03/31/17
401 West A Street, Suite 1200	Work Order:	17-03-2408
San Diego, CA 92101-2421	Preparation:	EPA 3510C
	Method:	EPA 8270C
	Units:	ug/L
Project: Pure Water		Page 6 of 6

<u>Surrogate</u>	<u>Rec. (%)</u>	<u>Control Limits</u>	<u>Qualifiers</u>
2-Fluorobiphenyl	80	33-120	
2-Fluorophenol	58	24-120	
Nitrobenzene-d5	88	38-120	
p-Terphenyl-d14	92	41-137	
Phenol-d6	37	16-120	
2,4,6-Tribromophenol	85	27-159	

Analytical Report

AECOM
401 West A Street, Suite 1200
San Diego, CA 92101-2421

Date Received: 03/31/17
Work Order: 17-03-2408
Preparation: EPA 5030C
Method: EPA 8260B
Units: ug/L

Project: Pure Water

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Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
PS-2	17-03-2408-1-A	03/31/17 09:25	Aqueous	GC/MS V V	04/01/17	04/02/17 02:30	170401L029

Parameter	Result	RL	DF	Qualifiers
Acetone	ND	20	1.00	
Benzene	ND	0.50	1.00	
Bromobenzene	ND	1.0	1.00	
Bromochloromethane	ND	1.0	1.00	
Bromodichloromethane	ND	1.0	1.00	
Bromoform	ND	1.0	1.00	
Bromomethane	ND	10	1.00	
2-Butanone	ND	10	1.00	
n-Butylbenzene	ND	1.0	1.00	
sec-Butylbenzene	ND	1.0	1.00	
tert-Butylbenzene	ND	1.0	1.00	
Carbon Disulfide	ND	10	1.00	
Carbon Tetrachloride	ND	0.50	1.00	
Chlorobenzene	ND	1.0	1.00	
Chloroethane	ND	5.0	1.00	
Chloroform	ND	1.0	1.00	
Chloromethane	ND	10	1.00	
2-Chlorotoluene	ND	1.0	1.00	
4-Chlorotoluene	ND	1.0	1.00	
Dibromochloromethane	ND	1.0	1.00	
1,2-Dibromo-3-Chloropropane	ND	5.0	1.00	
1,2-Dibromoethane	ND	1.0	1.00	
Dibromomethane	ND	1.0	1.00	
1,2-Dichlorobenzene	ND	1.0	1.00	
1,3-Dichlorobenzene	ND	1.0	1.00	
1,4-Dichlorobenzene	ND	1.0	1.00	
Dichlorodifluoromethane	ND	1.0	1.00	
1,1-Dichloroethane	ND	1.0	1.00	
1,2-Dichloroethane	ND	0.50	1.00	
1,1-Dichloroethene	ND	1.0	1.00	
t-1,2-Dichloroethene	2.6	1.0	1.00	
1,2-Dichloropropane	ND	1.0	1.00	
1,3-Dichloropropane	ND	1.0	1.00	
2,2-Dichloropropane	ND	1.0	1.00	
1,1-Dichloropropene	ND	1.0	1.00	

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM
401 West A Street, Suite 1200
San Diego, CA 92101-2421

Date Received: 03/31/17
Work Order: 17-03-2408
Preparation: EPA 5030C
Method: EPA 8260B
Units: ug/L

Project: Pure Water

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Parameter	Result	RL	DF	Qualifiers
c-1,3-Dichloropropene	ND	0.50	1.00	
t-1,3-Dichloropropene	ND	0.50	1.00	
Ethylbenzene	ND	1.0	1.00	
2-Hexanone	ND	10	1.00	
Isopropylbenzene	ND	1.0	1.00	
p-Isopropyltoluene	ND	1.0	1.00	
Methylene Chloride	ND	10	1.00	
4-Methyl-2-Pentanone	ND	10	1.00	
Naphthalene	ND	10	1.00	
n-Propylbenzene	ND	1.0	1.00	
Styrene	ND	1.0	1.00	
1,1,1,2-Tetrachloroethane	ND	1.0	1.00	
1,1,2,2-Tetrachloroethane	ND	1.0	1.00	
Tetrachloroethene	ND	1.0	1.00	
Toluene	ND	1.0	1.00	
1,2,3-Trichlorobenzene	ND	1.0	1.00	
1,2,4-Trichlorobenzene	ND	1.0	1.00	
1,1,1-Trichloroethane	ND	1.0	1.00	
1,1,2-Trichloro-1,2,2-Trifluoroethane	ND	10	1.00	
1,1,2-Trichloroethane	ND	1.0	1.00	
Trichloroethene	ND	1.0	1.00	
Trichlorofluoromethane	ND	10	1.00	
1,2,3-Trichloropropane	ND	5.0	1.00	
1,2,4-Trimethylbenzene	ND	1.0	1.00	
1,3,5-Trimethylbenzene	ND	1.0	1.00	
Vinyl Acetate	ND	10	1.00	
Vinyl Chloride	ND	0.50	1.00	
p/m-Xylene	ND	1.0	1.00	
o-Xylene	ND	1.0	1.00	
Methyl-t-Butyl Ether (MTBE)	ND	1.0	1.00	
Tert-Butyl Alcohol (TBA)	ND	10	1.00	
Diisopropyl Ether (DIPE)	ND	2.0	1.00	
Ethyl-t-Butyl Ether (ETBE)	ND	2.0	1.00	
Tert-Amyl-Methyl Ether (TAME)	ND	2.0	1.00	
Ethanol	ND	100	1.00	
Surrogate	Rec. (%)	Control Limits	Qualifiers	
1,4-Bromofluorobenzene	87	77-120		
Dibromofluoromethane	102	80-128		

RL: Reporting Limit. DF: Dilution Factor. MDL: Method Detection Limit.

Analytical Report

AECOM	Date Received:	03/31/17
401 West A Street, Suite 1200	Work Order:	17-03-2408
San Diego, CA 92101-2421	Preparation:	EPA 5030C
	Method:	EPA 8260B
	Units:	ug/L
Project: Pure Water		Page 3 of 6

<u>Surrogate</u>	<u>Rec. (%)</u>	<u>Control Limits</u>	<u>Qualifiers</u>
1,2-Dichloroethane-d4	95	80-129	
Toluene-d8	98	80-120	

Client Sample Number	Lab Sample Number	Date/Time Collected	Matrix	Instrument	Date Prepared	Date/Time Analyzed	QC Batch ID
PS-2	17-03-2408-1-B	03/31/17 09:25	Aqueous	GC/MS V V	04/04/17	04/04/17 18:55	170404L033

<u>Parameter</u>	<u>Result</u>	<u>RL</u>	<u>DF</u>	<u>Qualifiers</u>
c-1,2-Dichloroethene	480	10	10.0	

<u>Surrogate</u>	<u>Rec. (%)</u>	<u>Control Limits</u>	<u>Qualifiers</u>
1,4-Bromofluorobenzene	91	77-120	
Dibromofluoromethane	96	80-128	
1,2-Dichloroethane-d4	90	80-129	
Toluene-d8	97	80-120	