

### NC04B Pure Water Pipeline Alignment Alternatives

#### **Memo Information**

Prepared By: Sean McCarty, Jeff Soriano

Date: July 27, 2017

#### **Purpose**

This memorandum is intended to analyze and compare additional alternative alignments not previously evaluated, including reassessing possible routes through Scripps Lake Drive for the North City Pure Water Pipeline (NCPWPL), formerly known as the Miramar Pipeline.

This memorandum is to provide an overview of the issues and is not intended to be a complete analysis of all facts and issues regarding alignment alternatives.

#### Background

The proposed alignment of the NCPWPL has been developed and refined through the course of the design. Three potential alignment routes were originally studied between Black Mountain Road and Miramar Reservoir, identified in *Figure 1*, below.



Figure 1: Alignment Alternatives Map from Miramar Pipeline Alternatives Report



Alternative B "Central" was ranked the most advantageous, as it resulted in the least direct impact to residential areas, had the lowest 50-year life cycle cost and provided the most construction schedule float. Its principal disadvantage is that it requires the most private commercial land easement acquisitions. For more information, please see **Attachment A**: the Miramar Pipeline Alternatives Analysis Report, dated September 17, 2015.

The 10% Engineering Design Report and 10% level engineering plans, dated March 16, 2016, utilized the recommendation of the aforementioned Alternative Analysis Report. Other notable considerations discussed in the 10% Engineering Design Report were the existing utilities found within Scripps Lake Drive and where "pinch points" were identified as issues that would deter possibilities for an additional pipeline within the roadway due to space limitations. One such pinch point is noted at the intersection of Scripps Ranch Blvd. and Scripps Lake Dr. *Figure 2* represents the pinch points presented in the 10% Engineering Design Report. To note, Scripps Lake Drive was later suggested to be further evaluated as an alternate alignment by others and will be discussed later within this memo in more detail.



#### Figure 2: Pinch Point Exhibit from 10% Engineering Design Report

An initial meeting on May 5, 2016 was held with Scripps Ranch Technology Park, LLC to present and discuss the pipeline alignment through three of their parcels (APN 319-170-25, 26 & 27) as proposed in the 10% design documents. This meeting concluded with Scripps Ranch Technology Park LLC, represented by Murphy Development, indicating that they would not grant the City easement through the three properties because it conflicted with their plans for development on these three lots.

Subsequent to the May 5, 2016 meeting with Murphy Development, the City, in conjunction with the detailed design consultant (HDR), began exploring additional alignment alternatives. A key consideration of the alignment analysis is the proposed location of the dechlorination facility. The facility will be located on City property at the reclaimed water tank site located at the southeast end of Meanley Drive. This is a

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very important facility that will remove chlorine from the pipeline prior to it discharging into the reservoir, as required by regulatory agencies. The location of this facility was selected because it provided adequate contact time to properly remove chlorine, it provides a safety gap prior to the water being expelled into the reservoir and it is located on property already owned by the City (see *Figure 3* for location).



#### Figure 3: Pure Water Pipeline Re-alignment to eliminate easements

As indicated in *Figure 3*, above, HDR evaluated the potential alignment alternatives that would intercept the proposed dechlorination facility and continue to either of three routes into the reservoir. In *Figure 3*, the three different alignments are shown through either: 1. APN 319-170-23; 2. APN 319-17-22 and; 3. along the pathway behind the Scripps Ranch Branch Library. Results are as described in Technical Memo #3 dated August 1, 2016 (Attachment B).



HDR recommended the path through APN 319-170-22 due to the decreased lineal footage of pipeline, trenching will not be as deep, most economical to construct and reduced energy costs due to minimizing the high point of the pipeline. The proposed alignment change eliminated the crossing through the original three Scripps Ranch Technology Park parcels but resulted in crossing of another one of their parcels at the end of Meanley Drive, it increased the length of pipeline within City roadway by 800 linear feet and resulted in an increase in construction cost of approximately \$840,000. The blue dashed line in *Figure 3* represents this proposed change. In addition, when evaluating the vacant parcel that this alignment now crosses and the concept sketch of their proposed development (**Attachment C**) obtained from Murphy Development, the NCPWPL design team felt strongly about the ability to locate the alignment alongside their future parking structure within their designated 20' undevelopable setback.

Following further evaluation of the Meanley Drive route, the design consultant looked into a cost saving route along Hoyt Park Drive that would eliminate the need to tunnel under the crossing of the San Diego County Water Authority's (SDCWA) 96" Aqueduct at Meanley Drive. The findings were that this relocation was feasible and would eliminate the tunneling of the pipeline under the SDCWA Aqueduct and it would reduce the overall pipeline length by 400' within the roadway. This alignment revision is as shown in *Figure 4*.



Figure 4: Pure Water Pipeline Current Proposed Alignment (Scripps Ranch Area) – Reroute alignment to Hoyt Park Drive.



On August 16, 2016, the City Project Manager updated Murphy Development of the latest evaluation for avoiding the three parcels, after which they requested that the City work directly with their Engineer, James Roberts.

A site meeting was held with Murphy Development at APN 319-170-22 on September 9, 2016 to review the new proposed easement location and discuss the geotechnical and property surveys necessary for the design. At that time, Murphy Development requested that the pipeline be pushed close to the property boundary, so that the easement would not impede future site development. In addition, prior to further consideration of this alignment by Murphy Development and before granting the approval to conduct geotechnical borings on their property, they requested that the City look into some other suggested alignments to the west of this parcel and report the findings back to them. A memo dated, September 14, 2016 (Attachment D) was sent to Murphy Development that summarized the issues discovered with these suggested alignments. After receipt of the September 14, 2016 memo, Murphy Development provided the City permission to conduct the necessary geotechnical investigations on their property.

On February 2, 2017, the City Project Manager provided James Roberts a copy of the "tentative" 60% alignment for Murphy Development's review and because the Murphy Development proposed improvements was only concept in nature, the City had requested their input. A follow-up meeting was held on March 27, 2017 to discuss the alignment with Murphy Development. At this meeting Murphy Development told the City that they were no longer interested in allowing an easement through their property because they were concerned that the alignment would cut off access to the only ingress/egress into the lot and for apprehensions that the location of the pipeline would result in the need for them to mitigate for a pipeline falling within the structural influence line of their future parking structure. This was despite the City indicating that alignment adjustments can still be made with their input, offering to work hours opposite their contractor and buttoning up the site after every work shift. The City of San Diego received a letter dated April 13, 2017 from James G. Sandler an attorney representing Murphy Development. This letter is included as **Attachment E**.

Finally, prior to the Pure Water Program's presentation scheduled on May 4, 2017 to the Scripps Miramar Planning Group, Mr. Wally Wulfeck requested a response to the possibility of different alignments in this same area as suggested by him. This email exchange and City response can be seen in **Attachment F**.

#### **Alternative Alignments**

Due to the latest challenges presented to the City by Murphy Development's sudden reconsideration, the following alternative alignments were evaluated.

#### 1. Scripps Lake Drive

Although Scripps Lake Drive was already looked into as previously discussed during the 10% design phase, the City was requested to look at this alignment again by members of the Scripps Miramar Planning Group at the meeting held on May 4, 2017. This alignment would deviate from the current proposed alignment at the Dechlorination Facility, head west on Meanley Drive, turn north on Scripps Ranch Blvd., head east on Scripps Lake Drive then enter the Miramar Water Treatment Plant Site and enter Miramar Reservoir at the current alignment. *Figure 5* represents the proposed Scripps Lake Drive Alignment Alternative.





Figure 5: Scripps Lake Drive Alignment Alternative

*Figure 6a* shows a closer look at the existing utilities found within Scripps Lake Drive. The information provided in these figures denotes the findings from further research into the existing utilities within these roads. As seen in these figures, additional utilities were discovered making this route more problematic for constructing a new 48" diameter pipeline due to space limitation, needing to meet separation requirements and the need to relocate large utilities found through this corridor. The additional utilities that were not described or discovered in the earlier research were a fiber optic line, SDG&E electrical, SDG&E electrical vault, City water pressure reducing station and a County Water Authority facility that is critical for the operation of the 48" aqueduct.

*Figure 6b* shows a closer look at the existing utilities found where Scripps Lake Drive narrows adjacent to the Scripps Ranch Branch Library. This is to also demonstrate the issues with available space for the new 48" diameter pipeline.





Figure 6a: Existing Utilities along Scripps Lake Drive



Figure 6b: Existing Utilities along narrower portion of Scripps Lake Drive



*Figure 7* represents the existing utilities found within Meanley Drive. The City is currently in the process of working with SDG&E for their relocation of an electrical duct bank to allow space and proper clearance with the existing sanitary sewer for the single proposed 48" diameter Pure Water pipeline. The ability to obtain additional space for another 48" diameter pipeline would not be possible without the acquisition of easement within parcels APN 319-170-35, APN 319-170-37 and APN 319-170-38 (refer to *Figure 5* for location).



Figure 7: Existing Utilities along Meanley Drive

A Preliminary Opinion of Probable Project Cost has been developed for the Scripps Lake Drive Alignment Alternative and is presented in *Table 5*, "Alternate Alignment Comparisons".

In addition to the Capital Costs, long-term energy consumption at the pump station will increase due to a motor size increase from 1,000 HP to 1,250 HP motors. The additional energy costs, based on \$0.07 kW/Hr for power provided by the proposed power generation facility, yields an increase of \$92,000 annually. When calculated over the estimated 50 year life cycle this equates to an additional \$3.2 million.

The advantages and disadvantages of the Scripps Ranch Drive Alignment are summarized in Table 1.



#### Table 1: Scripps Lake Drive Alignment Alternative Advantages/Disadvantages

Advantages	Disadvantages
	Additional 4,500 LF of Pipeline
	Additional 4,500 LF of pipeline would require increasing motor requirements to 1,250 HP at Pump Station resulting in an annual increased energy cost of approximately \$92,000. When calculated over the estimated 50 year life cycle this equates to an additional \$3.2 million.
	Alignment is in heavily congested utility corridor
Eliminates Scripps Lake Drive Tunnel	<ul> <li>No space for 48" pipeline trench along Scripps Lake Drive without having to relocate critical utilities and/or meeting required separation requirements (Figure 6a and 6b)</li> </ul>
Access to pipeline air vacuum valves would be	<ul> <li>No space for two 48" pipeline trenches within Meanley Drive; (Figure 7)</li> </ul>
within City right-of-way.	• Due to space limitation, pipe would need to be constructed at greater depths than existing utilities. This would require the use of tunneling methods.
	<ul> <li>To make space, easement acquisition would be required across three separate parcels at an approximate cost of \$810,000</li> </ul>
	Volcanic geotechnical formation
	<ul> <li>This material is reported to have a compressive strength of up to 50,000 psi which would require blasting.</li> <li>Possible dam or aqueduct damage/failure due to blasting methods.</li> </ul>
	Increased construction cost of \$7,153,162 when compared to current alignment

#### 2. Modified Alignment through KBS Horizon, LLC Parking Lot (APN 319-170-23)

The Modified Alignment through APN 319-170-23 deviates from the alignment as originally analyzed by HDR through this property. This alignment hugs the western boundary of the property, within the existing paved parking lot utilized by the tenants of this parcel, as shown as the yellow line in *Figure 8*. The red line in this figure is to provide reference to the location of the previously investigated alignment through this property.





Figure 8: Modified Alignment APN 319-170-23 Alternative

A Preliminary Opinion of Probable Project Cost has been developed for the Modified Alignment APN 319-170-23 Alternative and is presented in *Table 5*, "Alternate Alignment Comparisons".

Similar to Alternative 1, long-term energy consumption at the pump station will increase due to a motor size increase from 1,000 HP to 1,250 HP motors. The additional energy costs, based on \$0.07 kW/Hr for power provided by the proposed Power Generation Facility, yields an increase of \$92,000 annually. When calculated over the estimated 50 year life cycle this equates to an additional \$3.2 million.



The advantages and disadvantages of the Modified Alignment APN 319-170-23 are summarized in *Table* 2.

Table 2:	Modified	Alianment	through	KBS Horizon.	LLC Parking	a Lot Advanta	ge/Disadvantages

Advantages	Disadvantages
Alignment west of Dechlorination facility remains unchanged.	Raise highpoint of pipeline results in, increasing motor requirements to 1,250 HP at Pump Station resulting in increased annual energy cost of approximately \$92,000. When calculated over the estimated 50 year life cycle this equates to an additional \$3.2 million. Alignment will require access through parking lot for construction activities. Tunneling pit would be in close proximity to existing building. Will impact available parking to existing business
	tenants. Will require parking lot restoration after construction. Requires easement acquisition with estimated cost of \$737,000. Increased construction cost of \$780,592 when compared to current alignment.

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#### 3. Modified Alignment through KBS Horizon, LLC Ingress/Egress and Landscaped Area

This modified alignment is similar to that mentioned as Alternative No. 2 but instead would impact the same property at a location that would head northerly within the landscaped areas, just west of the asphalt parking lot. *Figure 9* represents this proposed alignment.



Figure 9: Modified Alignment through KBS Ingress/Egress and Landscaped Area

A Preliminary Opinion of Probable Project Cost has been developed for the Modified Alignment APN 319-170-23 Alternative and is presented in *Table 5*, "Alternate Alignment Comparisons".

The advantages and disadvantages of the Modified Alignment APN 319-170-23 are summarized in *Table 3*.



# Table 3: Modified Alignment through KBS Horizon, LLC Ingress/Egress and Landscaped Areas Advantages/Disadvantages

Advantages	Disadvantages
Alignment west of Dechlorination facility remains unchanged.	Requires easement acquisition with estimated cost of \$716,000.
Pumps remain at 1000 HP. Property Owner has indicated initial approval of proposed easement	Maintenance access would be required through easement.
proposed easement	Would require temporary construction easement within Scripps Ranch Technology Park property (20' set-back area) for grading at bottom of slope and allow enough area for equipment movement during construction.
	Increased cost of \$1,061,314 when compared to current alignment

#### <u>4. Modified Alignment completely within 20' wide setback of Scripps Ranch Technology Park Parcel</u> (APN 319-170-22)

This modified alignment deviates from the current proposed alignment through Scripps Ranch Technology Park's parcel (APN 319-170-22) by way of maintaining the pipeline alignment within the 20' wide setback and outside of the future parking lot's estimated structural line of influence. The City had to make some conservative assumptions for the location of the property owner's future parking structure to further evaluate this possibility. Assumptions made were that the parking structure would be built along the setback boundary located 20' from property line with an estimated ground elevation of 655' MSL. The result was that it is feasible to avoid encroaching within the structural influence line of the parking structure by offsetting the centerline of the 48" pipeline 8' from the property line in order to be outside of an area that would impact and/or set restrictions on the design of their parking structure. Figure 10 represents the general location of this alternative.





Figure 10: Modified Alignment completely within 20' wide setback of Scripps Ranch Technology Park Parcel (APN 319-170-22)

A Preliminary Opinion of Probable Project Cost has been developed for the Modified Alignment APN 319-170-23 Alternative and is presented in *Table 5*, "Alternate Alignment Comparisons".

As previously offered, to address Murphy Development's concern regarding cutting off their access to the lot, the City can enforce that our contractor work the shift opposite of their operations and backfill and/or place steel plates over open trench at the end of each day.

The advantages and disadvantages of the Modified Alignment APN 319-170-23 are summarized in *Table 4*.



## Table 4: Modified Alignment completely within 20' wide setback of Scripps Ranch Technology Park Parcel (APN 319-170-22) Advantages/Disadvantages

Advantages	Disadvantages
Alignment west of Dechlorination facility remains	Maintenance access would be required through a permanent easement.
unchanged.	Would require grading at bottom of slope and permanent retaining wall.
Pumps remain at 1000 HP	Increased construction cost of \$371,500 when compared to current alignment.

#### **Alternative Alignment Comparisons**

A Preliminary Opinion of Probable Project Construction Cost has been developed for the current proposed alignment alternative and is presented in *Table 5* to allow side-by-side comparisons to each of the four alternatives.

Note: Easements are assumed 25' wide for permanent, 15' wide for temporary construction; Increase capital cost are a result of pump size increases @ 4 each; Alternatives 1 and 2 will result in increased energy cost; Positive value in "Difference" column indicates a cost increase and Negative value indicates a cost decrease.



TABLE 5: Alternative	Alignment	Comparis	sons																					
		Current F	Proposed Align	ment		1. So	cripps Lake	Drive Alignme	int	2.	Modified A	lignment th	ough KBS Parl	ding Lot		Modified A	Vignment th	rough KBS La	indscape	4. Modifi	ed Alignme	nt through	Murphy Develo	pment Setback
Item	Quantity	Unit	Unit Cost	Total Cost	Quantity	Unit	Unit Cost	Total Cost	Difference	Quantity	Unit	Unit Cost	Total Cost	Difference	Quantity	Unit	Unit Cost	Total Cost	Difference	Quantity	Unit	Unit Cost	Total Cost	Difference
Pipeline	731	LF	\$1,054	\$770,474	5,350	LF	\$1,054	\$6,324,000	\$5,553,526	812	LF	\$1,054	\$855,848	\$85,374	835	LF	\$1,054	\$880,090	\$109,616	731	LF	\$1,054	\$770,474	\$0
Tunneling	681	LF	\$2,232	\$1,519,992	650	LF	\$2,323	\$1,509,950	-\$10,042	780	LF	\$2,232	\$1,740,960	\$220,968	745	LF	\$2,232	\$1,662,840	\$142,848	681	LF	\$2,232	\$1,519,992	\$0
Tunnel Pit Additional Depth	0	LF	\$25,000	\$0	0	LF	\$25,000	\$0	\$0	0	LF	\$25,000	\$0	\$0	6	LF	\$25,000	\$150,000	\$150,000	0	LF	\$25,000	\$0	\$0
Easement (25' wide)	20,000	SF	\$30	\$600,000	25,000	SF	\$30	\$750,000	\$150,000	22,750	SF	\$30	\$682,500	\$82,500	22,125	SF	\$30	\$663,750	\$63,750	20,000	SF	\$30	\$600,000	\$0
Temporary Construction Easement (15' wide)	12,000	SF	\$4	\$48,000	15,000	SF	\$4	\$60,000	\$12,000	13,650	SF	\$4	\$54,600	\$6,600	13,275	SF	\$4	\$53,100	\$5,100	12,000	SF	\$4	\$48,000	\$0
Utility Relocations (Relocate 20" Water)	0	LF	\$125	\$0	2,500	LF	\$440	\$1,100,000	\$1,100,000	0	LF	\$125	\$0	\$0	0	LF	\$125	\$0	\$0	0	LF	\$125	\$0	\$0
Utility Relocations (Relocate 8" Water)	0	LF	\$125	\$0	0	LF	\$125	\$0	\$0	900	LF	\$125	\$112,500	\$112,500	900	LF	\$125	\$112,500	\$112,500	0	LF	\$125	\$0	\$0
Increased Capital Cost at Pump Station	0	EA	\$15,557	\$0	4	EA	\$15,557	\$62,228	\$62,228	4	EA	\$15,557	\$62,228	\$62,228	0	EA	\$15,557	\$0	\$0	0	EA	\$15,557	\$0	\$0
Street / Parking Paving (Slurry Seal)	0	SF	\$0.00	\$0	114,725	SF	\$2	\$229,450	\$229,450	90,000	SF	\$2	\$180,000	\$180,000	0	SF	\$2.00	\$0	\$0	0	SF	\$2.00	\$0	\$0
ADA Curb Ramps	0	EA	\$0	\$0	16	EA	\$3,500.00	\$56,000	\$56,000	0	EA	\$3,500	\$0	\$0	0	EA	\$3,500	\$0	\$0	0	EA	\$3,500	\$0	\$0
Asphalt Parking Lot Restoration (4* thick)	0	SY	\$0	\$0	0	SY	\$0	\$0	\$0	742	SY	\$41	\$30,422	\$30,422	0	SY	\$41	\$0	\$0	0	SY	\$41	\$0	\$0
Additional Shoring	0	LS	\$0	\$0	0	LS	\$0	\$0	\$0	0	LS	\$0	\$0	\$0	1	LS	\$25,000	\$25,000	\$25,000	1	LS	\$25,000	\$25,000	\$25,000
Retaining Wall Construction	0	SF	\$65	\$0	0	SF	\$65	\$0	\$0	0	SF	\$65	\$0	\$0	2,500	SF	\$65	\$162,500	\$162,500	4,500	SF	\$65	\$292,500	\$292,500
Slope grading/Private restoration of surface improvements restoration (6*AC, curb&gutter, driveway apron)	0	LF	\$0	\$0	0	LF	\$0	\$0	\$0	0	LF	\$0	\$0	\$0	700	LF	\$180	\$126,000	\$126,000	300	LF	\$180	\$54,000	\$54,000
Landscape	0	LS	\$50,000	\$0	0	LS	\$50,000	\$0	\$0	0	LS	\$50,000	\$0	\$0	1	LS	\$50,000	\$50,000	\$50,000	1	LS	\$0	\$0	\$0
Storm Drain Improvements	0	LS	\$114,000	\$0	0	LS	\$114,000	\$0	\$0	0	LS	\$114,000	\$0	\$0	1	LS	\$114,000	\$114,000	\$114,000	1	LS	\$0	\$0	\$0
Total				\$2,938,466				\$10,091,628	\$7,153,162				\$3,719,058	\$780,592				\$3,999,780	\$1,061,314				\$3,309,966	\$371,500

\* Not represented in these costs : Alternatives 1 and 2 will incur an increase in energy cost of approximately \$92,000 annually.

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#### Conclusions

Alternative 1, the realignment within Scripps Lake Drive, is the most challenging with the highest added construction cost estimated at over \$7 million. This alignment contains roadways that are congested with many existing utilities. The alignment within Scripps Lake Drive would require the relocation of other utilities to make space for the Pure Water's 48" pipeline. Due to the utility congestion within Meanley Drive, it would require easements from 3 additional property owners. Other difficulties with this alignment would be the need to trench and tunnel adjacent to critical utilities such as the County Water Authority's Aqueduct. Finally, based on maps of geological formations, dense volcanic formations are identified in this area which would require blasting methods to remove.

Alternative 2 is within the parking lot owned by KBS Horizon. This alternative would result in an increase in annual energy cost, will require parking and pavement restoration, utility relocation, easement acquisition, a tunneling receiving pit in close proximity to an existing building, an increase in construction cost estimated at \$780k and the need to come to agreement with KBS Horizon.

Alternative 3 is within the property owned by KBS Horizon, it is an alternative that results in no significant increase in energy cost and is located within a landscaped area within this property's unbuildable set back area. The disadvantages is that it would require additional grading, retaining walls, utility relocation, a permanent access for operations and maintenance staff, is estimated to be a \$1 million increase to construct and the need to come to agreement with KBS Horizon.

Alternative 4 remains within the property owned by Murphy Development, it is the alternative that results in no significant increase in energy cost, it has the smallest increase in construction cost estimated at \$370k, it is located within the unbuildable set back area, and the analysis indicates that it would not restrict the design of the property owner's proposed development. The disadvantage is that it would require additional grading, retaining walls, a permanent access for operations and maintenance staff and the need to come to agreement with Murphy Development.



**Attachment A** 

**Miramar Pipeline Alternatives Analysis Report** 



# FINAL

### Task 8: Miramar Pipeline / Pump Station (SA04)

### **Miramar Pipeline Alternatives Analysis Report**

#### **Prepared For:**

City of San Diego Public Utilities Department San Diego, California September 17, 2015

#### **Prepared By:**

MWH Americas, Inc.

and

Brown and Caldwell



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### List of Acronyms & Abbreviations

ACRONYM	DEFINITION
Blvd	Boulevard
CFS	Cubic Feet per Second
Ct	Court
Dr	Drive
HDPE	High Density Polyethylene
HGL	Hydraulic Grade Line
MCAS	Marine Corps Air Station
MGD	Million Gallons per Day
MTS	San Diego Metropolitan Transit System
MWTP	Miramar Water Treatment Plant
NC	North City
NCAWPF	North City Advanced Water Purification Facility
NCPS	North City Pump Station (NCAWPF Effluent)
O&M	Operations & Maintenance
Rd	Road
READ	City of San Diego's Real Estate Assets Department
ROW	Right-of-Way
RWS	Recycled Water Study
WA	San Diego County Water Authority



### 1. Executive Summary

The proposed Miramar pipeline will convey 30 MGD (46.4 CFS), on an annual average basis, of highly purified water from the North City Advanced Water Purification Facility (NCAWPF) to Miramar Lake. Delivery to the lake via the pipeline will be achieved using the proposed North City Pump Station (NCPS), which will serve as the NCAWPF's only effluent pump station. This analysis presents the three (3) alignment alternatives proposed for the Miramar pipeline, along with the evaluation performed to select the preferred alignment. Provided herein is a summary describing the three (3) proposed alignment alternatives, the results of the evaluation with rankings, and a recommendation for advancing this task (Task 8) to the 10% Design Phase.

Applicable data points were collected within a pre-determined study area, as defined in the Refinement of Recycled Water Study (RWS), which is shown in **Figure 1.** Data points collected included, but were not limited to, environmentally-sensitive areas, geotechnical conditions, current land use, permitting requirements, planned capital projects, rights-of-way and real property acquisition, and current traffic conditions. All data points were evaluated relative to their potential to impact a pipeline's constructability, schedule, and cost. Furthermore, a broader consideration was given to each data point's potential to impact the goals and objectives of the Pure Water Program. The three (3) principle alignment alternatives developed using this data were named:

- 1. Alternative A North Alignment
- 2. Alternative B Central Alignment
- 3. Alternative C South Alignment

These three alternatives are shown on Figure 2.

Once the principle three alternatives were established, an evaluation matrix was developed and specifically tailored to this project. The matrix accounts for the risks associated with constructing each alternative relative to its anticipated 50-year life cycle cost. The objective was to compare the alternatives side-by-side in a manner that would highlight the alignment with the best risk-to-cost balance. To achieve this objective, the alternatives were numerically ranked against each other across a range of project specific criteria. These criteria were organized into five (5) major risk categories:

- 1. Alignment Characteristics
- 2. Schedule and Coordination
- 3. Pipeline Operation and Maintenance
- 4. Constructability
- 5. Cost

Each category produced a score that quantified the comparative importance of the risks across all categories. The objective of assessing the comparative importance was to focus on: 1) construction feasibility, 2) impact to the Program's schedule, and 3) total cost to the City. Ultimately, the final score highlighted the preferred alignment via "the lowest score wins" method. **Table 1** provides a summary of the ranking results for each of the above five (5) categories, as well as the total scores awarded to each alternative.

The results of the evaluation matrix ultimately identified <u>Alternative B – Central Alignment</u> as the preferred alignment. Additional detail is provided under **Appendix A** – Evaluation Matrix and Supporting Maps, as well as **Appendix B** – Conceptual Cost Estimates.

It is recommended that the above mentioned alignment advance into the 10% design phase with additional consideration given to the I-15 crossing at Miramar Road. A 10% design will be completed for this alignment and submitted to the City along with a 10% Engineering Design Report for use by the final pipeline designer.





### 2. Alignment Alternatives Development

The three alternative alignments evaluated herein were developed based on the conceptual alignment presented in the Refinement of Recycled Water Study (RWS) Technical Memorandum No. 7 (Brown and Caldwell, July 2015). The technical memorandum studied an area bounded by Interstate 805 (I-805) to the west, Mira Mesa Blvd and Scripps Ranch Blvd to the north, MCAS Miramar and the Navy Operations Center to the south, and the existing WA easements and Miramar Lake's eastern banks to the east. The study area described above is shown in **Figure 1**.

The above mentioned study area served as the boundary for which data was collected and alignment alternatives were further developed under this task (Task 8). The work performed under this analysis was intended to: 1) determine the feasibility of constructing the conceptual alignment identified in the RWS, and 2) explore other viable alternatives within the predefined study area. The result was the identification of three (3) principle alignment alternatives, each with various options for deviations. For reference, the routes reviewed and ultimately rejected during this analysis are discussed in **Appendix C.** Considerations used in developing the three (3) principle alternate alignments are provided below:

### General

The proposed alignments primarily include one (1) 48-inch pipeline. Smaller pipe sizes were proposed in areas that required special accommodations for construction and operational flexibilities. The pipeline would be located entirely within the City of San Diego's city limits, and is intended to convey highly purified water from the NCPS to Miramar Lake with no service connections or redundancies along the way. All three alternative alignments assumed discharge into Miramar Lake via a submerged HDPE pipeline. Highly purified water delivered by this pipeline may need to be blended with imported WA water upon discharge into Miramar Lake. The lake's water would then be drawn and treated by the Miramar Water Treatment Plant (MWTP) for potable use. The option to bypass the lake and connect directly to the MWTP was evaluated but not considered feasible at this time due to current regulatory restrictions. Although strategies for blending imported water with highly purified water did not factor into this analysis, the options for blending, and their impact on the MWTP itself, are being evaluated under Task Order No. 4.

Construction methods for the pipeline are anticipated to be primarily open-cut, with the exception of a tunnel crossing at Interstate 15 (I-15). Welded steel pipe (WSP), with a minimum ¼" wall thickness, pursuant to Health Department standards, was the assumed material of choice for establishing material costs, hydraulic losses, anticipated trench widths, and installation rates for construction. The construction schedule for this project (Task 8) is a critical factor to the successful completion of the Pure Water Program.

Pursuant to the Program's current cooperative agreement, the delivery of purified water to the reservoir(s) must begin by December 31, 2023; however, the Pure Water Program has identified several opportunities for accelerating the schedule, thereby putting the updated completion of the Miramar pipeline at July 1, 2021. Starting on January 1, 2016, this deadline allows 72 months (6 years) for final designer ramp-up, final design, property acquisition, contractor procurement, final regulatory approval, and construction and commissioning. Specifically, the budgeted schedule would allow:

- 4 months for final designer ramp-up
- 18 months for final design
- 8 months for property acquisition
- 8 months for contractor procurement
- 6 months to obtain final regulatory approvals





• 28 months for construction and commissioning

For the three principle alignments evaluated, the timeframe for construction was estimated to be between 18 and 26 months.

#### NCPS to Black Mountain Rd

Based on the Task 8 analysis, coupled with the RWS's findings, Miramar Rd was selected as the preferred route between the NCPS and I-15. All three alignment alternatives share the Miramar Rd corridor leading to I-15 as a common route. The alignment starts at the NCPS and follows Eastgate Mall to Miramar Rd and then turns east. The alignment then follows Miramar Rd to the intersection with Black Mountain Rd and then travels south along the ROW along Miramar Rd. Two key challenges identified along this route were: 1) crossing the MTS railroad ravine at the San Diego Metropolitan Transit Authority Railroad Crossing and the Burlington Northern Santa Fe Railway Crossing, and 2) the MCAS Miramar access gate at Mitscher Way. Other east-west corridors that were considered and ultimately rejected, such as Mira Mesa Blvd, are discussed in **Figure C-1**.

### I-15 Crossing

A key constraint for defining alternative alignments east of I-15 was the location of the I-15 crossings. Factors for evaluating the crossing at I-15 included total tunnel length, surrounding property ownership, staging-area availability and access, ease of construction and permitting, and impact on overall alignment length. These elements helped determine the potential crossing locations, which ultimately established the alternative alignments east of I-15.

Alternatives for crossing I-15 were sent to Caltrans for review. A meeting with Caltrans was held on August 26, 2015 to obtain their feedback (see Appendix D for presentation and meeting minutes). Below is a description of the preferred crossings with feedback provided by Caltrans:

- Mira Mesa Blvd: This crossing location was chosen to access the northern corridor to Miramar Lake, which
  is identified as Alternative A North Alignment. The preferred option is to open-cut or tunnel within the Mira
  Mesa ROW directly under the existing I-15 overpass. Caltrans noted that obtaining approval to go under
  the overpass on a street with such high traffic volumes would be an extremely difficult and lengthy process.
  An additional option available for crossing I-15 at Mira Mesa Blvd includes tunneling from adjacent private
  properties as well as Caltrans ROWs. Key challenges to be faced with this option include: permitting, high
  traffic volumes, crossing the existing 96-inch WA pipeline, and mitigating impacts to the existing bridge
  structure or I-15 on/off ramps. In general, this crossing was Caltrans' least favorite of the preferred options
  and could thereby be difficult to secure their approval.
- Via Excelencia: This crossing location, which is associated with Alternative B Central Alignment, was originally proposed in the RWS conceptual alignment. Of the three options, this location presents the shortest tunneling length. However, permanent utility easements, limited construction access, and utility conflicts on both sides of I-15 could potentially increase the overall tunnel length. Cost and scheduled impacts associated with securing the easements and avoiding utilities factored heavily into the feasibility of this crossing's location. Staging is available in the private parking lot on the east side of I-15, but truck traffic and maneuverability within the parking area will be difficult during regular business hours. No alternate method for crossing I-15 is available at this location if the two private property easements cannot be secured. Caltrans took no exception to this crossing so long as their utility assets were avoided.



• **Miramar Rd:** This crossing location is the preferred option for crossing I-15, and is represented as part of Alternative C – South Alignment. The alignment would tunnel from property formally owned by the County of San Diego on the east side of I-15 to a receiving pit in a public ROW cul-de-sac on the west side. The tunneling length is the second shortest Caltrans ROW crossing, with staging available on both sides of the interstate away from heavily trafficked streets. Key challenges to this crossing include a skewed crossing of the interstate and the greatest tunnel depth of all options considered. Caltrans took no exception to this crossing so long as the skewed angle of the tunnel was less than 30-degrees off-perpendicular in relation to the interstate.

#### **Black Mountain Rd to Miramar Lake**

Three viable alternatives were identified east of I-15 to Miramar Lake. These alignment alternatives are shown in **Figure 2** and were analyzed in greater detail using the Alignment Alternatives Evaluation Matrix discussed below in Sections 3 and 4. Additionally, options for deviations from the principal three alternatives are presented in **Figure C-1**. Below is a description of each alternative alignment from the point where they diverge off of the Miramar Rd corridor up to Miramar Lake.

• Alternative A – North Alignment: This alignment continues north along Black Mountain Rd to Mira Mesa Blvd and turns east. Following Mira Mesa Blvd, the alignment will cross under the interstate. The alignment turns north at Scripps Ranch Blvd and follows the ROW for approximately 3,500 feet before turning south towards the lake. The pipeline will then drop across undeveloped City-owned parkland down into the Lake via a new subaqueous discharge pipeline. It should be noted that, unlike Alternatives B and C, if blending with imported WA water is needed, Alternative A will require the addition of a new WA turn-out on the north side of the lake. This turn-out would increase the total construction cost by approximately \$3-5 million and require additional coordination with the WA. The turn-out described above was not a major factor in this evaluation because blending requirements were unknown at the time of this analysis.

This alignment was chosen to utilize the wide public ROWs identified along the route. Key challenges associated with this alternative include: 1) difficulties in obtaining a Caltrans encroachment permit; 2) the higher HGL required due to the existing topography; 3) anticipated energy dissipation due to the elevated HGL; and 4) environmental impact mitigation within the parkland area north of the lake. Optional deviations for the North Alignment (see **Figure C-1**) include use of Activity Rd to bypass the MCAS main access gate.

• Alternative B – Central Alignment: The conceptual draft of this alignment was originally presented in the RWS. Revisions to the conceptual RWS alignment were required for both constructability and to reduce life-cycle O&M costs (see Figure C-1). These deviations were required in order to 1) avoid heavy utility congestion along Scripps Ranch Blvd and Scripps Lake Dr., and 2) to reduce the dynamic head required to discharge into the lake. The result was an alignment that is approximately two miles shorter than the conceptual alignment shown in the RWS.

At Black Mountain Rd, this alignment will turn north and then east on Carroll Centre Rd, then south on Via Pasar, and then east on Via Excelencia. It is here where two (2) permanent private property easements, one on each side of I-15, will be required to cross the interstate. From the cul-de-sac at Old Grove Rd, located east of I-15, the alignment generally continues northeast along Business Park Avenue, turns east on Carroll Canyon Rd, then north along Scripps Ranch Blvd. At Scripps Ranch Ct, the alignment turns east and continues out of the public ROW. This portion of the alignment will require additional permanent easements along three (3) City properties and four (4) private properties (i.e. 6 private properties in total for entire alignment). The alignment continues northeast along these proposed easements to Scripps Lake Dr, where the alignment crosses under the Roadway and onto the MWTP site. From there the alignment has the option to connect to the lake via a new subaqueous discharge pipeline, or turn east to tap the existing



MWTP's 108" Overflow pipe. Key challenges include construction schedule, required easement acquisition, and trenchless crossing of the I-15 through two (2) private property easements.

Alternative C – South Alignment: At Black Mountain Rd, this alignment continues east towards I-15 along Miramar Rd. The alignment turns north at Kearny Mesa Rd and enters a cul-de-sac, where it then crosses I-15. Access pits for the tunnel will be located in the cul-de-sac to the west, and the County ROW to the east. The alignment continues east of I-15 on Pomerado Rd, north along Scripps Ranch Blvd, east on Aviary Dr, and north on Red Cedar Dr. to Scripps Lake Dr. The alignment then crosses Scripps Lake Dr onto the MWTP site. From here it has the option to turn west and then north to connect to a new subaqueous discharge pipeline, or to turn east to tap the existing MWTP's 108" Overflow pipe.

In general, business and residential properties are developed along this alignment and are adjacent to 60-100 foot ROWs. An easement over school district property is anticipated along Red Cedar Dr. to avoid heavy utility congestion. Additionally, WA easement crossings and relocation of existing utilities along the narrow 60-foot corridors are also anticipated. Options for avoiding these conflicts are shown in **Figure C-1**. Key challenges include mitigating impacts to the residential community, scheduling work around the school's calendar, and utility conflicts/relocations in narrow 60-foot ROWs.

In summary, three (3) viable alignment alternatives were identified. These alignment alternatives, as shown in **Figure 2**, were evaluated in greater detail using the Alignment Alternatives Evaluation Matrix discussed below. Additionally, options for deviating from the three (3) principle alternatives are discussed in **Appendix C** and are shown in **Figure C-1**.

### 3. Scoring and Evaluation of Alignments

In order to objectively evaluate each alternative, various criteria were defined to assess potential impacts to the project's constructability, schedule, and costs. Criteria were developed based on available data and input from the City's staff. Ultimately, a total of 24 individual evaluation criteria were defined to quantitatively and qualitatively account for the foreseeable impacts associated with each alignment alternative. Rankings for each of these criteria were established based on desktop studies performed in topic areas including, but not limited to: environmental, geotechnical, traffic, permits, real property, construction methods, and O&M. Results of each desktop study were summarized and illustrated on the maps found in **Appendix A**.

Then, all the criteria were organized into five major categories, which included: 1) Alignment Characteristics, 2) Schedule and Coordination, 3) Pipeline Operation and Maintenance, 4) Constructability, and 5) Cost. The purpose of these categories was to organize the criteria in a manner that would highlight the comparative importance of each category relative to the risk associated with successfully implementing each alternative. The intent was to assign, in order of priority, risks associated with: 1) construction feasibility, 2) impact to the program's schedule, and 3) total cost to the City.

To provide additional objectivity to this evaluation, a non-weighted ranking methodology was used for all criteria. This method, as approved by the City's staff, directly ranked each alternative against the others. This method used a 1 to 3 ranking, which was applied using the follow logic:

- Rank 1 = First Choice; least impacted by criteria (Best)
- Rank 2 = Second Choice; moderately impacted by criteria (Better)
- Rank 3 = Third Choice; most impacted by criteria (Good)





Once a ranking was assigned for each of the 24 criterion, all ranking values were tallied and a total score was assigned. The lowest score would be identified as the preferred alignment alternative that best balanced risks and costs. A summary of the evaluation ranking results, including the preferred alignment (highlighted in green), is provided in **Table 1**.

Table 1- Alternatives Alignment Ranking Summary							
Criterion/Alignment	Alternative A "North"	Alternative B "Central"	Alternative C "South"				
1. Subtotal – Alignment Characteristics	14	10	11				
2. Subtotal – Schedule and Coordination	10	8	12				
3. Subtotal – Pipeline Operation & Maintenance	6	5	7				
4. Subtotal – Constructability	14	15	11				
5. Subtotal – Cost	7	3	3				
Total Ranking Score	51	41	44				
Final Ranking	3	1	2				

The results of this evaluation ultimately identified <u>Alternative B – Central Alignment</u> as the preferred alignment. For more detail pertaining to the ranking of each criterion see **Appendix A** – Evaluation Matrix and Supporting Materials. Cost tables showing estimated construction, annual O&M, and 50-year life cycle costs are included in **Appendix B**.

### 4. Evaluation Findings

In addition to highlighting the preferred alignment based on this risk/cost matrix, the above evaluation also provided organized findings relative to the pros and cons associated with each alignment alternative. **Table 2** below summarizes the high-level findings resulting from the evaluation matrix.

Table 2- Summary of Findings							
Finding/Alignment	Alternative A "North"	Alternative B "Central"	Alternative C "South"				
Principal Pro(s)	<ul> <li>Entirely within City</li> </ul>	<ul> <li>Least direct impact to</li> </ul>	- Best I-15 Crossing				
1 IIICipai 1 10(3)	ROW/public land	residential areas	<ul> <li>Land acquisition optional</li> </ul>				
		<ul> <li>Most private land</li> </ul>					
Principal Con(s)	<ul> <li>Most traffic congestion</li> </ul>	acquisition	<ul> <li>Greatest direct impact to</li> </ul>				
Fincipal Con(s)	<ul> <li>Highest pumping costs</li> </ul>	- Tunnel is in private	residential areas				
		property					
Construction Schedule	6 months float	10 months float	2 months float				
Construction Cost	All alte	ernatives are within 5% of each	other				
	15% more than least cost		3% more than least cost				
50-yr Life Cycle Cost	alternative (\$37 Million	Least cost alternative	alternative (\$7 Million more				
	<u>more</u> over 50 Years)		over 50 Years)				
Greatest Risk To	Cost and Schedule	Constructability	Schedule				

The above summary of findings is briefly discussed below in additional detail for reference.

 Alternative A – North Alignment is located entirely on public streets and, on the final approach to Miramar Lake, in a City-owned parkland with no need to acquire private lands or private property easements. This will assist in expediting the overall schedule; however, high traffic volumes and utility congestion slow the overall production rate during construction. The topography encountered by approaching Miramar Lake from the north ultimately requires an HGL that is approximately 100-feet higher



than the other two alternatives. This elevated HGL drives up the energy required to deliver purified water to Miramar Lake. In turn, this elevated HGL requires more pumping energy, which increases the annual O&M costs. Although the cost to construct this alternative is similar to the others, the 50-year life cycle cost of the elevated HGL is approximately 15% higher than in Alternatives B and C.

- Alternative B Central Alignment presents great opportunities as it relates to mitigating impacts to traffic, local area residents, and ease of construction. However, the feasibility to construct Alternative B hinges on the successful acquisition of easements along six (6) private properties and three (3) City-owned properties. Specifically, two (2) of the private properties easements are required for the I-15 tunnel crossing, and the other four (4) private properties easements are required for the approach to the MWTP site. Using the private property easements for the approach to the MWTP site avoids the utility congestion at the Scripps Lake Dr. and Scripps Ranch Blvd intersection. The utility congestions in this area ultimately lead to extremely difficult construction conditions down Scripps Lake Dr. en-route to the Lake. Without an easement through all eight properties unique to this alignment, Alternative B will not be constructible. Therefore, it is recommended that actions be taken during the 10% design, and the final design, to mitigate the risks that these properties present on the project's viability.
- Alternative C South Alignment has the greatest risk of not meeting the Program's schedule goal of July 1, 2021. Allowing only two (2) months of estimated float, construction operations would essentially need to be performed with no delays. This narrow window for success presented too great of a risk to the Program. Additionally, Alternative C has the potential to create the greatest upset to the local area residents. This is based on the foreseeable construction operation impacts to traffic along Pomerado Rd, access to public parks and schools, as well as its disturbance to neighborhoods with streets containing narrow ROWs that are congested with utilities. Alternative C does not require land acquisition, but there are two (2) properties which have been identified as optional for acquisition in order to mitigate some of these disturbances. Although this option is feasible without acquiring easements, construction will be slow and disruptive. This condition is anticipated to generate several complaints, especially as utility relocation adjacent to the school progresses. However, it should be noted that Alternative C has the most favorable I-15 crossing. This crossing presents the Contractor with the best working conditions and thereby mitigates some of the most substantial constructability, schedule, and cost risks on this project. This benefit was accounted for in the evaluation matrix and is considered in the recommendations below.

### 5. Recommendations

The previous sections summarized the results of this evaluation, and ultimately identified a preferred pipeline alignment. The following is a list of recommended actions for moving Task 8 forward:

- 1. Advance 10% Design using <u>Alternative B Central Alignment</u>.
- 2. Explore using the I-15 crossing at Miramar Rd instead of Via Excelencia to mitigate construction related risks and costs.
- 3. Discussion easements with the Real Estate Assets Department (READ) relative to the preferred options for crossing I-15.
- 4. Complete 10% Design using the I-15 crossing with the greatest probability of success relative to READ's input.
- 5. Re-engage Caltrans with a more complete 10% Design of the proposed crossing of I-15 for concurrence.



MIRAMAR PIPELINE / PUMP STATION (SA04)

### Figures







### Appendix A: Evaluation Matrix and Supporting Maps

#### TABLE A-1: ALIGNMENT ALTERNATIVES EVALUATION MATRIX Task 8 - Miramar Pipeline and Pump station

Ranking Priority: 1 - First Choice; least impacted by criteria (Best) 2 - Second Choice; moderately impacted by criteria (Better) 3 - Third Choice; most impacted by criteria (Good)

The following table evaluates three alignment alternatives (see Figure 2) for the delivery of highly purified water from the NCPS to Miramar Lake.										
Cat.	Parameter/Condition	Criteria Definition		Alternative A: North*		Alternative B: Central*	Beat	Alternative C: South*		
			Rank	48" Nominal Diameter w/ reduction to 30"	Rank	Description	Rank	Description 48" Nominal Diameter w/ 42" in congested		
	1a. Approximate Alignment Length (LF)			downstream of high point.		48" Nominal Diameter		corridor		
	NCPS to Miramar Reservoir (Miles)		3	8.5 Miles	1	7.5 Miles	2	8.1 Miles		
	Length of Open-Cut Trench (LF)			43,800 LF		39,100 LF		41,750 LF		
	1b. Static Hydraulic Profile			1,200 Ei		100 Ei		130 El		
	Elevation NCPS - Discharge			370 FT		370 FT		370 FT		
	High Point Elev. (Static head)	See Annendix A.		815 FT		720 FT		725 FT		
	Total Dynamic Head (TDH)	Figures A-1. A-2. and A-3	3	510 FT	1	410 FT	1	410 FT		
52	Pumping pressures & operating ranges			Low End: 14,000 gpm @ 210 psi		Low End: 14,000 gpm @ 165 psi		Low End: 14,000 gpm @ 165 psi High End: 22,600 gpm @ 175 psi		
SING	Installed motor HP			1.500 HP 3 pumps plus 1		1.250 HP 3 pumps plus 1		1.250 HP 3 pumps plus 1		
NCTE N							_	South side of Miramar Lake with MWTP		
HAR	1c. Discharge Location		3	North side of Miramar Lake with no MWTP Connection option	1	South side of Miramar Lake with MWTP Connection option	2	Connection option. May require a segment		
Ē								parallel to Scripps Lake Dr		
BW	1d. Estimated Construction Duration	Considerations:								
nev	(Field Construction)	80-120 LF/day per crew	2	22 Months using	1	18 Months using	3	26 Months using		
1. A		1 Crew(s) per shift		100 LF/day per crew		110 LF/day per crew		80 LF/day per crew		
		1 Shift(s) per working day		Estimated Float = 6 Months		Estimated Float = 10 Months		Estimated Float = 2 Months		
	1e. Additional Time for Land Acquisition	Considerations:								
		Commercial	1	0 Months	3	9 Months	2	6 Months		
		City / Public								
		Percentage of total nineline length requiring				15% in Permitted Caltrans BOW and		3% in Permitted Caltrans ROW and Permanent		
	1f. Total Length of Easements	a permitted or permanent easement.	2	5% in Permitted Caltrans ROW	3	Permanent Easements	1	Easements		
		CATEGORY 1 - SUBTOTALS	14		10					
	2.a Coordination Requirements:	Ranking for 2.a =	2		1		3			
		Subtotal of i - ix below =	17		16		21			
		- Number of permits		See Appendix A. Figure A-4:						
	i) General Permitting	- Difficulty / LOE of permits	2	~ 1 additional permit (high risk)	3	See Appendix A, Figure A-4:	1	See Appendix A, Figure A-4:		
		See Appendix A. Figure A-4		~ 2 additional permit (med risk)		2 additional permit (high risk)		± auditional permit (filgh fisk)		
	ii) Department of Health Services Approval	- Ability to meet offset requirements w/o mitigation / relocation	1	~ large ROW widths (95% +/- > 100 FT) to	2	~ mixed ROW widths (84% +/- > 80 ft.) to	2	~ mixed ROW widths with narrow 60 ROW (74% +/, > 80 ft 24% +/ 60 ft ) Mitigation		
	in department of nearth bervices Approval	- available ROW	1	meet separation requirements	2	meet separation requirements	3	for reduce separation anticipated.		
				Least preferred by Caltrans:		Preferred by Caltrans:				
		Free of Free It Assessed		~ I-15 crossing at Mira Mesa Blvd with		~ I-15 crossing will require private property		Preferred by Caltrans:		
		- Ease of Permit Approval		Caltrans approval for open cut or trenchless		easements on both sides to cross.		~ I-15 crossing will be in public ROW and		
	ii) Caltrans I-15 Crossing Permitting	- Impact to tunneling ops	3	underpass	1	~ No impact to access ramps or freeway	1	optional, not required.		
		- Results from meeting w/ Caltrans held on		~ required structural review of bridge		traffic		~ minimal interference with access ramps		
		8/26/2015.		~ alignment crosses access ramps		properties		and traffic		
				~ Alternative options available if required to		~ no alternatives to revise crossing location		~ alternatives to revise crossing location		
				revise crossing						
	iii) Railroad Crossing	- Existing MOU		Two railroad crossings, both on Miramar and		Two railroad crossings, both on Miramar and		Two railroad crossings, both on Miramar and		
	~ City's Metropolitan Transit System (MTS) ~ BMFS rail crossing at grade	- Need to obtain new MOU.	1	are comment to all alternatives.	1	are comment to all alternatives.	1	are comment to all alternatives.		
	Bill o fail aloosing at glade					-		Three military entrances crossings:		
	iv) MCAS Miramar and Naval Operations	- Avoid impacts to access	1	Two military entrance crossing:	1	Two military entrance crossing:	3	~ North and West entrance to MCAS		
				- Norun & West entrance to MCAS		- North & West entrance to MCAS		~ Naval observatory		
	v) Contaminated Sites	- Crossing areas of known contamination	2	8 additional sites	1	6 additional sites	3	12 additional sites; concentrated in		
		- Planned CIP						GALINANG		
	ul) On the Device the	- Pavement Moratoriums	2	~ 4 CIPs	_	~ 4 CIPs		~ 1 CIPs		
	vi) Capital Projects		3	~ 3 Moratoriums	2	~ 0 Moratoriums	1	~ 2 Moratoriums		
		See Appendix A, Figure A-5								
		Community engagement		~ large group, broad impact to non-resident		~ large group, broad impact to commuters.		~ larger resident group, specific impact in		
-	vii) Public/Community Relations Coordination	- Likelihood of community complaints	2	commuters and business.	1	schools and businesses.	3	residential areas and elementary school		
101		management						access		
DIN										
BO	viii) Litility Coordination	Number of conflicts/utility congestion     Service Discustions	1	See 4 d. Utility Conflicts	2	See 4 d - Utility Conflicts	2	See 4 d . Utility Conflicts		
8 8	vin) ounty coordination	- Relative difficulty for conflict negotiations	-	See 4.g - builty connets	-	See 4.g - Ounty Connicts	-	See 4.g - Ourly Connicts		
ILLE										
E S						Multiple crossing and parallel pipelines along		54" and 84" at Red Cedar Drive. Parallel		
S. SC	ix) SDCWA Crossing	- Easement access	1	96" at I-15 and Mira Mesa Blvd and	2	Scripps Ranch Blvd and Scripps Lake Dr.	3	Construction Red Cedar Dr. Requires narrow		
		- connicting utilities		Lasement crossing at Scripps Ranch Blvd.		Includes 66", 72", 84" and others		restricted school access		
		- Special Permit Requirements								
	2.b Environmental Permits	- ACP Handling/Hauling/Disposal	3	See 4.a - Environmental	2	See 4.a - Environmental	1	See 4.a - Environmental		
		- Difficulty of obtaining permits								
		Discustion to community ( husing						Residential areas and Elementary Schools		
		Impacts to residents		Mostly commercial with minor impacts to		Disruption during construction primarily in		restricting noise and schedule:		
		- Home owner group coordination		residential:		the form of noise and traffic disruptions:		~ more residential		
	2.c Local Communities/Schedule Impacts	- Fire/police stations coordination	2	~ major business & commercial shopping	1	~ Predominantly commercial.	3	~ Construction widening of Pomerado has		
		- Alternate traffic/emergency routing		~ Fire Station 44 on Black mountain Road		~ No residential.		previously failed to pass based on public		
		- Ingress Egress via construction area		~ Elementary, Middle School and College		No impact to schedule.		impact. (Anticipate 9 am to 3 pm		
								construction restrictions)		
-										
		Dealer and Dealer		~ Float ~6 months				~ Float ~2 months		
	2.d Duration of Construction and Ability to Meet July	28 months total available for constr	2	~ Primary delay risks are traffic control,	1	~ Float ~10 months	2	~ Primary delay risks are property acquisition,		
	2021 Deadline	- Can deadline be met accounting for risk	-	limited construction operations, and utility	-	~ Primary delay risk is property acquisition	3	narrow ROWs, limited construction		
				connicts				operations, and utility conflicts		
						See 1.e - Additional Time for Land		See 1.e - Additional Time for Land		
		- Public vs private property availability				Acquisition:		Acquisition:		
		- Establish easements		See 1.e - Additional Time for Land		~ Lake Access via City owned property		~ Required, one (1) easement through San		
	2.e Real Property Acquisition for Permanent	- Purchase Property		Acquisition:	2	~ Required, six (6) easements through	2	Diego School Board property		
	Easement/ROW	- Legal risks that may impact schedule	-	~ All in ROW or City Property	3	~ Required, one (1) easement through San	4	~ Optional, one (1) private property parcel		
		See Annendix A Elduro A C		~ No easement acquisition in non-city		Diego School Board property		purchase. Unused and inaccessible property,		
		See Appendix A, Figure A-0		property anticipated		~ Easements required for tunnel, which		~ Easements required for tunnel, which		
						presents significant risk to project		presents significant risk to project		
		ATEODOV 9. EURTOTAL E	10		0		12			
		UNICOUNT 2- SUBIUIALS	10		0		14			



#### TABLE A-1: ALIGNMENT ALTERNATIVES EVALUATION MATRIX Task 8 - Miramar Pipeline and Pump station

*The fo	The following table evaluates three alignment alternatives (see Figure 2) for the delivery of highly purified water from the NCPS to Miramar Lake.									
Cat.	Parameter/Condition	Criteria Definition	Rank	Description	Rank	Description	Rank	Description		
	1a. Approximate Alignment Length (LF)			48" Nominal Diameter w/ reduction to 30" downstream of high point		48" Nominal Diameter		48" Nominal Diameter w/ 42" in congested		
	3a. O&M Considerations	Ranking for 3.a =	3	downstream of high point.	1		2	contor		
3. PIPELINE OPERATION & MAINTEMINCE		Subtotal of i - iv below =	9		6		8			
	<ul> <li>i) Access to Manways, vaults, valves, low points and air valves/Siting of annurtenances</li> </ul>		3	All appurtenances are located in trafficked	1	Most appurtenances are located out of trafficked areas	1	Mixed; appurtenances are located in both trafficked and non-trafficked areas		
	ii) Access to Tunnel and shafts	Ability to repair/access/ depth of crossing	1	In CALTRANS ROW and shallow	3	Within private property, with varying slopes	2	All in public ROW, but is the deepest crossing		
	IIII Terffle Ornheit Orfebrung Assess	based on topography Crew access and safety performing	-			and grades	0			
	III) Tramic Control - Sarety and Access	maintenance	3	See 4.c - frame control	1	See 4.c - frame control	2	See 4.c - frame Control		
	iv) Pipeline Appurtenance maintenance at High/low points	- Total high/low points which may require appurtenances - Appurtenances (i.e. air valves, blow off structures) requiring maintenance	2	Least number of high and low points. Has highest highpoint which requires energy dissipation.	1	Local topographic high and low points	3	Most local topographic high and low points		
	3b. O&M Risk (consequence of failure)	- Limited access - Difficult to repair	2	- Maintenance and access at I-15 underpass. - Greatest impact to businesses - Greatest effect on traffic	1	<ul> <li>Maintenance and access on private property required at I-15 crossing,</li> <li>Alignment crosses 100 Yr, flood plain at Black Mountain Road.</li> </ul>	3	Maintenance and access on private property if utility congestion requires use of easements at Red Cedar Dr. and Awary Dr. Alignment parallels flood plain at Miramar Rd and Pomerado Rd east of Black Mountain Rd. Longest 1:3 Crossing Greatest impacts to residents		
	3c. Operating Permit Requirements	<ul> <li>Identify permits required</li> <li>Flag long lead permit items</li> <li>Access to special special properties, e.g.</li> <li>ESLs</li> </ul>	1	Entire alignment is in public ROW or City owned property	3	Alignment has the most private property easements. Most critical segment, the tunnel under I-15, is on private property	2	Some private property easements, and a school district easement		
		CATEGORY 3 - SUBTOTALS	6		5		7			
4. CONSTRUCTION IT	4a. Environmental Resources	- ESLs - Archeological - Paleontological - Known areas of potential contamination - Wetlands and waterways <u>See Appendix A, Figure A-7</u>	3	~ 20 additional points ~ Dechlor facility will be in park	2	~ 19 additional points ~ Dechlor facility will on MWTP site.	1	- 17 additional points - Dechlor facility will on MWTP site.		
	4b. Geotechnical	Poor and loose materials in Flood Plain     Suitable for backfill material     Corrosive soils     Seismic fault line areas     construction on side slopes     See Appendix A, Figure A-8	3	~ Unsuitable for backfill (longest LF)I, similar haulage distances ~ Cemente hard to excavate ~ Some metavolcanic rock by Dam	2	<ul> <li>Unsuitable for backfill, similar haulage distances</li> <li>Some metavolcanic rock by MWTP</li> </ul>	1	- Unsuitable for backfill, similar haulage distances - Mostly all stadium conglomerate		
	4c. Traffic Control	Pipe laying production rate     Need for vertical shoring     Open cut feasibility     Trenchless need     Schedule impacts     See Appendix A, Figure A-2	3	<ul> <li>Highest Traffic Count = 175,307</li> <li>Highest traffic volumes. Mira Mesa contains the highest traffic counts of all alignments w/ extensive traffic control</li> <li>Traffic disruptions impacts commercial office spaces, high density shopping, and schools w/ detours and lane closures</li> </ul>	1	<ul> <li>Highest Traffic Count = 27,759</li> <li>Most of alignment, aside from Miramar, does not run along major throughways</li> <li>Traffic diruptions mostly impact commercial office spaces w/ detours and lane closures</li> </ul>	2	<ul> <li>Highest Traffic Count = 135,027</li> <li>Pomerado presents high traffic counts and is a narrow road.</li> <li>Traffic disruptions impacts residential w/ road closures and detours to construct</li> </ul>		
	4.d Tunneling (anticipated permitted Xinga)	- Length - Setup and staging area available - Depth of crossing	2	-Tunneling operations and staging in public parcel - Staging in ROW w/ controlled access - Longest casing and Caltrans ROW crossing ' Moderate access depth	3	~ Tunneling operations, staging, and approaches on Private Property w/ limited access - Shallowest access depth Safety risks to public	1	- Excellent staging & access - Varying grades and slopes - Deepest and longest tunnel - All in public ROW or public land		
	4. e Clearing Grubbing/Tree Removal	- Extent of clearing and grubbing - Requires removal of mature trees	1	~ Limited clearing required in alignment to access Lake. (3%)	2	~ Clearing of side slopes behind commercial properties and City held property to construct. (13%)	2	~ Clearing on Pomerado Rd in undeveloped road ROW and possible removal of trees at school to avoid utility relocation on Red Cedar Dr. (13%)		
	4.f Construction Methods	- Fessibility to construct - ROW and TCE availability - (long/short lead)	1	<ul> <li>Open cut construction for majority of alignment with open cut or trenchless crossing of 1-15 w/ Caltrans approval</li> <li>Large ROW for contractor staging</li> <li>Approach to lake is most difficult of the options.</li> </ul>	3	Open cut construction for majority of alignment with trenchless crossing of I-15. Large ROW and PE contractor staging Private property trenching and tunneling Requires the most clearing of trees, side slope and *off-road* construction.	2	<ul> <li>Open cut construction for majority of alignment with trenchless crossing of I-15.</li> <li>Narrow residential ROW on Pomerado and in Residential Area.</li> </ul>		
	4.g Utility Conflicts	- Number of conflicts/utility congestion - Service Discruptions - Meath and asterly risks - Deflect alignment or relocate utility	1	<ul> <li>Buried utilities will be crossed and paraleled in the pipeline alignment with heavy congestion in Mira Mesa Dr.</li> <li>Mix of commercial and industrial service connections. Limited disruption.</li> <li>Options exist to use alternate routes or tunnel in wide ROW to avoid utilities.</li> </ul>	2	<ul> <li>Buried utilities will be crossed and paralleled in the corridor. Heaviest congestion of all three options seen on the Carroll Carryon, Scripps Ranch Bivd and Scripps Lake corridors. Revised to avoid.</li> <li>Mix of commercial and industrial service connections. Limited disruptions.</li> </ul>	2	Buried utilities will be crossed and paralleled in the corridor with heavy congestion in Red Cedar Dr. and Aviary Dr. (60° R0W). -Mix of commercial, and residential services Service disrytonia in residential areas. - Utility relocation or parallel trench w/ reduced offset may be required to construct. Relocation of 20° ACP or PE on school property required to construct.		
		CATEGORY 4 - SUBTOTALS	14	\$109,000,000	15	\$100,000,000	11	\$105,000,000		
	5a. Total Project Cost	Provide Construction     Pump station construction     Posta	1	\$109,000,000 \$32	1	\$100,000,000 \$31 20%/+E0% Enterth Doctor - 11	1	\$105,000,000 \$32		
5. COST	5b. Total Annual O&M Cost	- sort costs - Electrical Cost - Routine Maintenance - Repair and replacement of appurtenances	3	یں + 50% + 50% באנוומנא Esumate Hange = \$ to \$ \$5,200,000 / year	1	یں +50% באנווזמונא Esumate Kange = \$ to \$ \$4,400,000 / year	1	-50%/+50% Esumate Kange = \$ to \$ \$4,500,000 / year		
	5c. Life Cycle Net Present Value (50-yr)	NPV on a 50-yr life cycle     Discount rate of 4%     Pump replacement every 20 yrs.	3	\$278,000,000	1	\$241,000,000	1	\$248,000,000		
		CATEGORY 5 - SUBTOTALS	7		3		3			

TOTAL = 51 FINAL RANK = 3

41 1

44 2






















Appendix B: Conceptual Cost Estimates

1. Detailed Cost Estimate

TABLE B-1: ALTERNATIVE A - NORTH ALIGNMENT																		
	SCENARIO DA	TA								CAPI	FAL COST					AN	NUAL COST	
			Capacity						Ur	nit Cost Multip	iers	Final			O&M	Annual O&M	Annual Elect	Total Annual
Item	Description	Used	Average/Duty	Peak/Installed	Unit	Diameter	Unit Cost	Unit	Site <sup>2</sup>	Pressure <sup>3</sup>	Misc <sup>4</sup>	Unit Cost	Quantity	Cost	%	Cost	Cost	Cost
PUMP STATION																		
NCPS	NCWRP Pump Station	3,991	4,500	6,000	hp	NA	\$3,215	hp	1.0	1.0	1.0	\$3,215	6,000	\$19,290,337	2.5%	\$482,258	\$4,432,149	\$4,914,408
		3,991	4,500	6,000	hp						PUMP STATION SUBTOTAL = \$19,290,33 PUMP STATION SOFT COSTS = \$13,503,23 PUMP STATION TOTAL = \$32,793,53			\$19,290,337 \$13,503,236 \$32,793,573		\$482,258	\$4,432,149	\$4,914,408
PIPELINE	New One Oak Diseline is Deed					10	0070				4.0	6070	10.110	600 F04 000	4.000	0005 040		
Miramar Pipe	New Open-Cut Pipeline in Road		0.0	0.0	mga	48	3672	LF	1.0	1.0	1.0	\$672	42,440	\$28,524,002	1.0%	\$285,240		
Miramar Pipe Dressure Deducing Station	New Open-Cut Pipeline in Road		0.0	0.0	mga	30	\$420	LF	1.0	1.0	1.0	\$420	4,701	\$1,974,500	1.0%	\$19,746		
Tranchlass Crossings	Interestate 15 Crossing		0.0	0.0	mad	19	\$2,517	10	1.0	1.0	1.0	\$2,517	000	\$00,000	1.0%	\$22.654		
Discharge Structure	Discharge Structure		0.0	0.0	mad	40 NA	\$610.000	19	1.0	1.0	1.0	\$610,000	1	\$610,000	1.0%	\$6 100		
FOOTNOTES	Discriarge Structure		0.0	0.0	nigu	110	\$010,000	20	1.0	1.0	1.0	PIPELINE SI	IBTOTAL =	\$33 453 976	1.070	\$334 540	\$0	\$334 540
Note this is a Class 4 Cost Estima	te with a -30%/+50% accuracy according to th	ne Associatio	on of Cost Enginee	ring (AACE) Interna	tional co	st estimate cla	assification system	1.				PIPELINE SOF	T COSTS =	\$23,417,783				
1 For pump stations. "Used" is actua	al (not rated) HP where HP = (flow.gpm * head	d. ft) / (3960*	efficiency), "Avera	ge" is installed duty	motor H	P. "Peak" is to	tal installed motor	HP.				PIPELIN	E TOTAL =	\$56.871.759				
2 Site multiplier is to account for uni	que site expenses. 1.0 = greenfield, 1.5 = exis	sting facility																
3 Pressure multiplier accounts for in	creased HP cost. 1.0 = open-cut to 200-psi, 1	.3 = open-c	ut to 260-psi									SCENARIO SL	JBTOTAL =	\$52,744,313		\$816,798	\$4,432,149	\$5,248,947
4 Misc. allows for additional unit price adjustment.									S	CENARIO SOF	r costs =	\$36,921,019						
5 Escalation of construction costs to	mid-point of construction.										S	CENARIO ESC.	ALATION <sup>5</sup> =	\$19,426,254				
												SCENARI	0 TOTAL =	\$109,091,586				

CAPITAL COST         Annual Cost           Item         Description         Lag         Capacity <sup>1</sup> Item         Unit Cost Multipliers         Final         O&M         Annual Cost         Cost         Cost         Annual Cost         Cost <thcost< th="">         Cost         <thcost< th=""></thcost<></thcost<>	TABLE B-2: ALTERNATIVE B - CENTRAL ALIGNMENT																		
SCENARIO DATA         ANVIAL COST         CAPAPENTAL COST         COST <th colspa<="" th=""><th colspan="12"></th></th>	<th colspan="12"></th>																		
Image: Product Stratuc         Capacity?         Unit         Diserce for the second strate of the second strate strate of the	SCENARIO DATA							CAPITAL COST ANNUAL COST											
Item         Description         Used         Average/Duty         Peak/Installed         Unit         Diameter         Unit         Site <sup>2</sup> Pressure <sup>3</sup> Misc <sup>4</sup> Unit Cost         Quantity         Cost         %         Cost				Capacity <sup>1</sup>					1	U	nit Cost Multipl	iers	Final			O&M	Annual O&M	Annual Elect	Total Annual
PUMP STATION         NCPS         NCWR Pump Station         3,209         3,750         5,000         hp         NA         \$3,408         hp         1.0         1.0         53,408         5,000         \$17,037,652         2.5%         \$425,949         \$3,674,428         \$4,100,377           PIPELNE	Item	Description	Used	Average/Duty	Peak/Installed	Unit	Diameter	Unit Cost	Unit	Site <sup>2</sup>	Pressure <sup>3</sup>	Misc <sup>4</sup>	Unit Cost	Quantity	Cost	%	Cost	Cost	Cost
NCPS         NCWRP Pump Station         3.309         3.750         5.000         hp         NA         \$3.408         hp         1.0         1.0         1.0         \$3.408         500.00         \$170.378.22         \$425.949         \$3.674.428         \$4.100.377           PUMP STATION SOFT COSTS =         3.309         3.750         5.000         hp         New Station         PUMP STATION SOFT COSTS =         \$11.926.566         \$425.949         \$3.674.428         \$4.100.377           PPELNE         Memorar Pige         New Open-Cut Pipeline in Road         0.0         mgd         48         \$2.517         1.0         1.0         \$572         42.330         \$2.845.840         1.0%         \$2.391.34           Discharge Structure         0.0         0.0         mgd         NA         \$510.000         1.0         \$572         42.330         \$2.845.840         1.0%         \$2.391.34           Discharge Structure         0.0         0.0         mgd         NA         \$510.000         1.0         \$572         42.330         \$2.845.840         1.0%         \$2.391.3           Tenchlose Cost Estimate with a 30%+26%         0.0         mgd         NA         \$510.000         1.0         \$511.00         1.0         \$511.00         1.0%	PUMP STATION																		
Bit Pump         Status         Statu	NCPS	NCWRP Pump Station	3 309	3 750	5 000	hp	NA	\$3.408	hp	1.0	10	10	\$3 408	5 000	\$17 037 952	2.5%	\$425 949	\$3 674 428	\$4 100 377
PUMP STATION SOFT COSTS = PUMP STATION TOTAL =         \$11,926,566           PPELNE           Mismar Pipe Mismar Pipe Trenchless Crossings Interstate 15 Crossing Discharge Structure         0.0         0.0         mgd         48         \$872         LF         1.0         1.0         \$572         42,330         \$228,445,840         1.0%         \$28,94,458           Discharge Structure         0.0         0.0         mgd         48         \$52,717         F0.10         1.0         \$572         42,330         \$228,445,840         1.0%         \$23,913         528,913         528,913         528,913         500,000         1.0%         \$51,47,10%         \$51,47,10%         \$51,47,10%         \$51,47,11%         \$50,000         1.0%         \$51,47,11%         \$50,000         1.0%         \$51,44,71%         \$50,000         \$51,44,71%         \$50,000         \$51,44,71%         \$51,4			3.309	3,750	5.000	hp						PUN	VP STATION SU	JBTOTAL =	\$17.037.952		\$425,949	\$3.674.428	\$4,100,377
PUMP STATION TOTAL =         \$28,964,518           PUMP STATION TOTAL =         \$28,964,518           PIPELINE           Miranar Pipe Trenchless Crossing         New Open-Cut Pipeline in Road         0.0         0.0         mgd         48         \$672         LF         1.0         1.0         \$672         42.330         \$28,445,840         1.0%         \$284,458           Discharge Structure         0.0         0.0         mgd         A8         \$25,17         LF         1.0         1.0         \$672         42.330         \$28,445,840         1.0%         \$52,913         Discharge Structure         0.0         0.0         mgd         A8         \$25,17         LF         1.0         1.0         1.0         \$610,000         1.0%         \$62,913,044         1.0%         \$63,913           FOOTNOTES         PIPELINE SUBTOTAL =         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471         \$514,471 <td></td> <td>PUMP</td> <td>STATION SOF</td> <td>T COSTS =</td> <td>\$11,926,566</td> <td></td> <td></td> <td></td> <td></td>												PUMP	STATION SOF	T COSTS =	\$11,926,566				
PPELNE           Miramar Pipe         New Open-Cut Pipeline in Road         0.0         mgd         48         \$672         LF         1.0         1.0         \$572         42.330         \$28.445.840         1.0%         \$28.445.840           Trenchises Crossings         Interstate 15 Crossing         0.0         0.0         mgd         48         \$2.517         LF         1.0         1.0         \$28.445.840         1.0%         \$23.91.264         1.0%         \$23.91.264         1.0%         \$23.91.264         1.0%         \$23.91.264         1.0%         \$23.91.264         1.0%         \$23.91.264         1.0%         \$23.91.264         1.0%         \$23.91.264         1.0%         \$23.91.264         1.0%         \$23.91.264         1.0%         \$23.91.264         1.0%         \$50.000         1.0%         \$51.47.110         \$50.000         1.0%         \$51.47.110         \$50.000         1.0%         \$51.47.110         \$50.000         1.0%         \$51.47.110         \$50.000         1.0%         \$51.47.110         \$50.000         1.0%         \$51.47.110         \$50.000         1.0%         \$51.47.110         \$50.000         1.0%         \$51.47.110         \$50.000         1.0%         \$51.47.110         \$50.000         \$51.47.110         \$50.000         \$51.47.110<													PUMP STATIC	N TOTAL =	\$28,964,518				
PipeLine Minimari Pipe Trenchless Crossing         New Open-Cut Pipeline in Road         0.0         0.0         mgd         48         \$672         LF         1.0         1.0         \$672         2.3.30         \$528,458.40         1.0%         \$284,458           Trenchless Crossing         Interstate 15 Crossing         0.0         0.0         mgd         48         \$25,17         LF         1.0         1.0         \$5672         \$2.3.30         \$22,845,840         1.0%         \$23,913           Discharge Structure         0.0         0.0         mgd         NA         \$510,000         1         \$510,400         \$510,400         \$510,400         \$510,400         \$510,400         \$510,400         <																			
Miramar Pipe         New Open-Cut Pipeline in Road         0.0         0.0         mgd         45         \$572         LF         1.0         1.0         \$572         42.330         \$22,445,404         1.0%         \$22,445,404         1.0%         \$22,445,404         1.0%         \$22,445,404         1.0%         \$22,445,404         1.0%         \$22,445,404         1.0%         \$22,445,404         1.0%         \$22,311,26         1.0%         \$22,311,26         1.0%         \$22,311,26         1.0%         \$22,311,26         1.0%         \$23,311,27         1.0%         \$23,311,27         1.0%         \$23,311,27         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         \$23,311,26         1.0%         1.0%         1.0%         \$23,311,26         1.0%         1.0%         1.0%         \$23,311,47,10%         \$23,14,47,11         \$0         \$31,44,711         \$0         \$31,44,711         \$0         \$31,44,711         \$31,44,711         \$31,44,711         \$31,44,711 <t< td=""><td>PIPELINE</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>	PIPELINE																		
Trenchess Crossings         Interstate 15 Crossing         0.0         0.0         mgd         48         \$2,517         LF         1.0         1.0         \$2,317         50         \$2,3124         1.0%         \$5,010           Discharge Structure         0.0         0.0         mgd         N48         \$51,070         LF         1.0         1.0         1.0         \$510,000         1         \$510,000	Miramar Pipe	New Open-Cut Pipeline in Road		0.0	0.0	mgd	48	\$672	LF	1.0	1.0	1.0	\$672	42,330	\$28,445,840	1.0%	\$284,458		
Discharge Structure         0.0         0.0         mpd         NA         \$610,000         LS         1.0         1.0         \$610,000         1.0%         \$610,000         \$610,000	Trenchless Crossings	Interstate 15 Crossing		0.0	0.0	mgd	48	\$2,517	LF	1.0	1.0	1.0	\$2,517	950	\$2,391,264	1.0%	\$23,913		
FOOTNOTES         PIPELINE SUBTOR1 =         \$314,471         \$0         \$314,471           Note this is a class 4 Cost Estimate with a -30%/-50% accuracy according to the Association of Cost Engineering (AACE) International cost estimate classification system.         PIPELINE SUBTOR1 =         \$314,471         \$0         \$314,471           1         For pump stations, 'Used' is actual (not rated) HP where HP = (low-gpm' head, 1)/ (390°/efficiency). 'Average' is installed duly motor HP. 'Peak' is total installed motor HP.         PIPELINE SUBTOR1 =         \$314,471         \$0         \$314,471           2         Stem multiple is accounts for increased HP cost.         1.0 = operine-cit to 2800-rpsi.         PIPELINE SUBTOR1 =         \$314,471         \$344,47	Discharge Structure	Discharge Structure		0.0	0.0	mgd	NA	\$610,000	LS	1.0	1.0	1.0	\$610,000	1	\$610,000	1.0%	\$6,100		
Note this is a Class 4 Cost Estimate with a 30%/50% accuracy according to the Association of Cost Engineering (AACE) International cost estimate classification system.         PIPELINE SOFT COSTS = 522,170,208.54           1 For purp statul (not tack) Phyter PH = 16 (mog system).         Sign (and system).         Pipeline Society (and system).           2 Site multiplier is to account for unique site expenses. 1.0 = generitied (1.5 = existing facility)         Site multiplier is to account for unique site expenses. 1.0 = generitied (1.5 = existing facility)           3 Pressure multiplier accounts for unique site expenses. 1.0 = generitied (1.5 = existing facility)         SCENARIO SUBTOTAL = 548,485,066         \$740,420         \$3,674,428         \$4,414,848           Misc. allows for additional unit price adjustment.         SCENARIO SOFT COSTS = \$34,006,775         \$34,646,755	FOOTNOTES												PIPELINE SU	JBTOTAL =	\$31,447,104		\$314,471	\$0	\$314,471
1         For pump stations, Used''s actual (not rates) H* where H* = (low-gpm* nead, 1) (/g80*/emcency). "Average' is installed duly motor H*. "Peak" is total installed motor H*.         PI+ELINE. IDIAL = \$53,671,433           2         Site multipler is obcounts for increased H* cost. 1.0 = genering duly motor H*. "Peak" is total installed motor H*.         PI+ELINE. IDIAL = \$53,671,433           3         Pressure multipler accounts for increased H* cost. 1.0 = open-cut to 280-psi.         SCEMARIO SQFT COSTS = \$34,080,775           4         Here. allows read/open units         SCEMARIO SQFT COSTS = \$34,080,775         \$34,714,848	Note this is a Class 4 Cost Estimation	ate with a -30%/+50% accuracy according to the	he Associatio	on of Cost Enginee	ring (AACE) Interna	ational co	ost estimate cla	assification system	n.				PIPELINE SOF	T COSTS =	\$22,170,208.54				
2         Site multiplier is to account for unique site expenses. 10 = greenfield, 1.5 = existing facility           3         Pressure multiplier accounts for increased HP occi. 10 = open-cut to 200-psi. 1.3 = open-cut to 200-psi.           4         Misc. allows for additional unit price adjustment.           5         Carefaction enterprise mathematics           6         Carefaction enterprise mathematics           6         Carefaction enterprise mathematics	<ol> <li>For pump stations, "Used" is actual</li> </ol>	al (not rated) HP where HP = (flow,gpm * hea	d, ft) / (3960*	efficiency). "Avera	ge" is installed duty	motor H	IP. "Peak" is to	otal installed moto	r HP.				PIPELIN	E TOTAL =	\$53,617,313				
3         Pressure multiplier accounts for increased HP cost. 1.0 = open-cut to 200-psi.         SCENARIO SUB IO Increased HP cost. 1.0 = open-cut to 200-psi.           4         Misc. allows for additional unit price adjustment.         SCENARIO SOFT COSTS = \$334,086,775         \$34,442,850,775           5         Creating in a construction control of construction.         SCENARIO SOFT COSTS = \$34,086,775         \$34,042,050	2 Site multiplier is to account for uni	ique site expenses. 1.0 = greenfield, 1.5 = exi	sting facility																
14     Miss, allows for adoutinal unit price adjustment.     SEENARIO SOFT COSTS = \$34,000 //5       2     Examplifier     SEENARIO SOFT COSTS = \$34,000 //5	3 Pressure multiplier accounts for increased HP cost. 1.0 = open-cut to 200-psi Pressure multiplier accounts for increased HP cost. 1.0 = open-cut to 200-psi									SCENARIO SU	JBIOIAL =	\$48,485,056		\$740,420	\$3,674,428	\$4,414,848			
CCENIADIO ECCALATION <sup>2</sup> - \$17,001,502	4 INISC. allows for additional unit price	ce adjustment.										S	CENARIO SOF	rcosis=	\$34,096,775				
3 Estatation or construction costs to mid-point or construction.	5 Escalation of construction costs to	o mid-point of construction.										S	CENARIO ESC	ALATION° =	\$17,891,593				

Brown---Caldwell

9/15/2015

TABLE B-3: ALTERNATIVE C - SOUTH ALIGNMENT																		
	SCENARIO DA	ТА					CAPITAL COST ANNUAL COST											
			Capacity <sup>1</sup>						U	nit Cost Multipl	iers	Final			O&M	Annual O&M	Annual Elect	Total Annual
Item	Description	Used	Average/Duty	Peak/Installed	Unit	Diameter	Unit Cost	Unit	Site <sup>2</sup>	Pressure <sup>3</sup>	Misc <sup>4</sup>	Unit Cost	Quantity	Cost	%	Cost	Cost	Cost
DUMD STATION																		
NCDS	NCW/PP Pump Station	3 362	3 750	5 000	hn	NA	\$3.409	hn	1.0	1.0	1.0	\$3.409	5 000	\$17,027,052	2.5%	\$425.040	\$2 722 271	\$4 150 320
Nor 5	Nowid Fully Station	3,362	3 750	5 000	hp	in A	30,400	np	1.0	1.0	PUI	VP STATION SL	IBTOTAL =	\$17,037,952	2.370	\$425,949	\$3,733,371	\$4 159 320
											PUMP STATION SOFT COSTS = \$11,926,566							
												PUMP STATIO	N TOTAL =	\$28,964,518				
PIPELINE																		
Miramar Pipe	New Open-Cut Pipeline in Road		0.0	0.0	mgd	48	\$672	LF	1.0	1.0	1.0	\$672	41,639	\$27,981,408	1.0%	\$279,814		
Miramar Pipe	New Open-Cut Pipeline in Road		0.0	0.0	mgd	42	\$588	LF	1.0	1.0	1.0	\$588	3,450	\$2,028,593	1.0%	\$20,286		
Trenchless Crossings	Interstate 15 Crossing		0.0	0.0	mgd	48	\$2,517	LF	1.0	1.0	1.0	\$2,517	1,000	\$2,517,120	1.0%	\$25,171		
Demolition	Demo and Dispose Ex. ACP		0.0	0.0	mgd	NA	\$137,000	LS	1.0	1.0	1.0	\$137,000	1	\$137,000	0.0%	\$0		
Discharge Structure	Discharge Structure		0.0	0.0	mgd	NA	\$610,000	LS	1.0	1.0	1.0	\$610,000	1	\$610,000	1.0%	\$6,100		
FOOTNOTES												PIPELINE SU	JBTOTAL =	\$33,274,121		\$331,371	\$0	\$331,371
Note this is a Class 4 Cost Estimation	te with a -30%/+50% accuracy according to th	e Associatio	on of Cost Enginee	ering (AACE) Interna	ational co	st estimate cla	assification system	n.				PIPELINE SOF	T COSTS =	\$24,290,109				
<ol> <li>For pump stations, "Used" is actual</li> </ol>	al (not rated) HP where HP = (flow,gpm * head	i, ft) / (3960*	efficiency). "Avera	ge" is installed duty	motor H	P. "Peak" is to	tal installed motor	r HP.				PIPELIN	E TOTAL =	\$57,564,230				
2 Site multiplier is to account for uni	que site expenses. 1.0 = greenfield, 1.5 = exis	ting facility																
3 Pressure multiplier accounts for in	creased HP cost. 1.0 = open-cut to 200-psi, 1.	.3 = open-c	ut to 260-psi									SCENARIO SL	JBTOTAL =	\$50,312,073		\$757,320	\$3,733,371	\$4,490,691
4 Misc. allows for additional unit price adjustment.								S	CENARIO SOF	r costs =	\$36,216,675							
5 Escalation of construction costs to	care to millionist of construction							S	CENARIO ESCA	ALATION <sup>5</sup> =	\$18 746 704							
S Estalation of construction costs to microstruction.								SCENARI	D TOTAL =	\$105 275 453								

#### Life Cycle Analysis (LCA)

IABLE B-4: ALTERNA					
Description	Cost	Year	FV	NPV	LCA Unit C
					PUMP STA
Scenario Total <sup>1</sup>	\$109,091,586	0	\$109,091,586	\$109,091,586	
Pump Station Subtotal <sup>2</sup>	\$19,290,337				
Pump & Motor Replacement <sup>3</sup>	\$2,352,462	20	\$3,854,782	\$1,759,272	ESCALATIO
Pump & Motor Replacement <sup>3</sup>	\$2,352,462	40	\$6,316,509	\$1,315,660	Escalation
Total Annual Costs, Reoccurring Years 1 to 50	\$5,248,947		\$511,690,216	\$185,571,205	0&M
	Total Net Preser	nt Value =		\$278,311,469	
TABLE B-5: ALTERNA		TRAL /	ALIGNMEN	NT	SOFT COST
Description	Cost	rear	FV	INPV	
Scenario Total <sup>1</sup>	\$100,473,424	0	\$100,473,424	\$100,473,424	
Pump Station Subtotal <sup>2</sup>	\$17,037,952				
Pump & Motor Replacement <sup>3</sup>	\$2,077,783	20	\$3,404,689	\$1,553,855	
Pump & Motor Replacement <sup>3</sup>	\$2.077.783	40	\$5.578.979	\$1,162,040	LIFE CYCLE
Total Annual Costs, Reoccurring Years 1 to 50	\$4,414,848		\$430,378,584	\$156,082,470	
	Total Net Preser	nt Value =		\$241,380,196	
					Footnotes
					1 Scenario to

TABLE B-6: ALTERNATIVE C - SOUTH ALIGNMENT									
Description	Cost	Year	FV	NPV					
Scenario Total <sup>1</sup>	\$105,275,453	0	\$105,275,453	\$105,275,45					
Pump Station Subtotal <sup>2</sup>	\$17,037,952								
Pump & Motor Replacement <sup>3</sup>	\$2,077,783	20	\$3,404,689	\$1,553,85					
Pump & Motor Replacement <sup>3</sup>	\$2,077,783	40	\$5,578,979	\$1,162,04					
Total Annual Costs, Reoccurring Years 1 to 50	\$4,490,691		\$437,772,078	\$158,763,81					
Total Net Present Value =									

LCA Unit Costs and Assumptions (See "Unit Cost" tab in Appendix C Table 3 complete listing)						
PUMP STATIONS	_					
Total Pump & Motor Cost / Pump Station Cost (%) = 8.1%						
Motor Cost / Pump Cost (%) = 36.6%						
ESCALATION						
Escalation from Oct 2014 to Oct 2019 (mid-point of construction), 4% APY = 21.7%						
0&M						
Pump Station O&M (%) = 2.5%						
Pipeline O&M (%) = 1.0%						
Average Electricity cost (\$/KW-hr) = \$0.17						
SOFT COSTS						
Total Soft Cost, New Pump Station (%) = 70.0%						
Total Soft Cost, Pump Replacement (%) = 50.0%						
Total Soft Cost, North Pipeline (%) = 70.0%						
Total Soft Cost, Central Pipeline (%) = 70.5%						
Total Soft Cost, South Pipeline (%) = 73.0%						
LIFE CYCLE ANALYSIS						
LCA Duration (years) = 50						
LCA Escalation / Inflation Rate, APY (%) = 2.5%						
LCA Discount Rate, APY (%) = 4.0%						
Footnotes						
<ol> <li>Scenario total is complete construction cost w/ soft costs (engineering, contingency, etc.)</li> </ol>						
and has been escalated to the mid-point of construction.						
2 Pump station construction only. Basis of replacement calculation. Not directly in NPV.						
3 Replacement value = (8.1% of Pump Station Subtotal) x (1.5 soft cost markup)						

 $FV_{Ordinary Annuky} = C * \left[ \frac{(1+i)^n - 1}{i} \right]$ 



## Appendix C: Corridor Study and Alternatives Development



## Appendix C: Corridor Study and Alternatives Development

## **Alignment Alternative Deviations Considered**

Several corridors were reviewed when developing alignment alternatives between the NCPS and Miramar Lake. Options for deviating from the principle three (3) alignment alternatives, which are shown in Figure 2, included: 1) Mira Mesa Blvd, 2) an existing sewer easement paralleling the southern boundary of the Carrol Canyon Rock Quarry, and 3) existing WA easements. These options are briefly described below and shown on Figure C-1:

- Mira Mesa Blvd: Starting at the NCPS, this alignment proceeds north to Mira Mesa Blvd and travels east. The ROW along Mira Mesa is 80-120 feet wide with high-density residential areas and commercial shopping districts. This route was eliminated based on the disturbance to the adjacent residential communities and the resulting impact on the project's schedule. Significant schedule impacts are anticipated due to heavy daytime traffic counts identified within high-density residential areas and commercial shopping districts. As a result, it was believed that daily construction activities would also be restricted and night-time construction operations would be impermissible adjacent to the residential dwellings. Furthermore, replacement of the existing median along Mira Mesa, which includes a raised median and landscaping, was considered unfavorable.
- Rock Quarry Alignment: Starting at the NCPS, this alignment follows the initial route along Miramar Rd. At Camino Santa Fe, it turns north to join an existing sewer easement going east. This option was eliminated due to the limited width of the existing easement (20-30 feet wide), which currently houses the Carroll Canyon Trunk Sewer at a depth of 20-feet below grade. Other factors included: 1) the easement is in a 100-year floodway that would require special Army Corps permits, 2) O&M crews will have limited access to the easement in the future, and 3) existing rock quarry buffer zones and future plans for development limit the likelihood of expanding the existing easement width.
- WA Easements: A north-south WA easement located to the east of Aviary Dr was considered as a possible approach route to the MWTP. This option was eliminated based on language in the WA's standard easement leasing contracts. The language precludes the WA from co-leasing or sharing their easements with any others agencies. In order to use a portion of the WA's easement, an agency such the City would have to negotiate with the WA to relinguish the portion of the easement desired back to the original property owner. At that time, the City would then enter into negotiations with the original property owner to establish a new agreement for the easement. This process posed a substantial risk to the schedule, and therefore was not pursued any further.

## I-15 Crossings

A total of seven (7) potential I-15 crossing locations were identified. These crossing locations are briefly described below and are shown on presentation included in this appendix:

- 1. Mira Lee Way: This crossing location was eliminated, as it is the furthest to the north and extends the overall pipeline length by approximately 10%. The tunnel would cross using two public ROW streets; Mira Lee Way and Erma Rd. Both ROWs are narrow and offer limited space for tunneling operations. Additionally, the Erma Rd ROW is located in a residential area, so 24-hour tunneling operations are anticipated to be impermissible.
- 2. Mira Mesa Blvd (preferred): This crossing location was chosen as a preferred option for accessing the northern corridor to Miramar Lake, which is identified as Alternative A – North Alignment. The preferred option is to open cut or tunnel within the Mira Mesa ROW directly under the existing I-15 overpass. Caltrans noted that obtaining approval to go under the overpass on a street with such high



traffic volumes would be an extremely difficult and lengthy process. Additional options available for crossing I-15 at Mira Mesa Blvd included tunneling from adjacent private properties and Caltrans ROWs. These options also proved to be difficult with earning Caltrans' approval due to high traffic volumes. Key challenges to be faced with this option include permitting, high-traffic volumes, crossing the existing 96-inch WA pipeline, and mitigating impacts to the existing bridge structure or I-15 on/off ramps. In general, this crossing was Caltrans' least favorite of the preferred options.

- 3. **Maya Linda Rd**: This crossing location was reviewed as a viable option due to the public ROW to the west, and the San Diego Unified School District site located to the east. This site was eliminated due to the limited staging area on the east side in order to maintain access to the residential housing, as well as the heavy utility congestion along both Treena Street and Scripps Lake Dr. This site was also eliminated due to a strongly sloped embankment on the east side requiring the deepest access pits and tunneling bore paths of all the available options.
- 4. **Gold Coast Dr**: This crossing location was eliminated due to limited staging areas, property access, and private ownership. The east side of the interstate is privately owned, which would require an easement. Using Gold Coast Dr on the western side of the interstate would block the only access to the adjacent apartment complex.
- 5. **Carroll Canyon Rd**: This crossing location was eliminated due to limited staging areas and potential utility conflicts. There are a number of existing water and recycled water lines that currently cross I-15 at this same location. The conflicts with the existing utility crossings, as well as the apartment complexes abutting the ROW, presents substantial risk to construction operations and overall project schedule.
- 6. Via Excelencia (preferred): This crossing location, which is associated with Alternative B Central Alignment, was originally proposed in the RWS's conceptual alignment. Of the three options, this location presents the shortest tunneling length. However, permanent utility easements, limited construction access, and utility conflicts on both sides of I-15 could potentially increase the overall tunnel length. Cost and scheduled impacts associated with securing the easements and avoiding utilities factored heavily in the feasibility of this crossing's location. Staging is available in the private parking lot on the east side of I-15, but truck traffic and maneuverability within the parking area will be difficult during regular business hours. No alternative for crossing I-15 is available at this location if the two private property easements cannot be secured. Caltrans took no exception to this crossing so long as their utility assets were avoided.
- 7. **Miramar Rd (preferred)**: This crossing location is the preferred option for crossing I-15, and is represented as part of Alternative C South Alignment. The alignment would tunnel from property formally owned by the County on the east side of I-15, to a receiving pit in a public ROW cul-de-sac on the west side. The tunneling length is the second shortest Caltrans ROW crossing, with staging available on both sides of the interstate away from heavily trafficked streets. Key challenges to this crossing include a skewed crossing of the interstate, and the greatest depth of all options considered. Caltrans took no exception to this crossing so long as the skewed angle of the tunnel was less than 30-degrees off-perpendicular to the interstate.

For reference, Appendix D includes the slides used to present the above I-15 crossings to Caltrans during the August 26, 2015 project coordination meeting, as well as the corresponding meeting minutes.





# Appendix D: Caltrans Coordination Meeting Minutes and Presentation



## Task 8 – Miramar Pipeline & Pump Station City of San Diego and Caltrans Coordination

#### **Meeting Information**

Date: August 26, 2015

Location: Caltrans District 11, Planning Department, 4050 Taylor Street MS-240, San Diego, CA 92110

Start/End Time: 2:30 p.m. - 4:00 p.m.

#### Meeting Purpose/Objectives

Purpose: Coordinate interstate crossing, as well as the use of Caltrans lands, with Caltrans.

**Objectives:** 

- 1. Introduce the Pure Water Program's objectives, and the Task 8 Miramar Pipeline Project
- 2. Obtain input on the various options for crossing I-15, as well as using I-805 land for parking.

#### **Meeting Participants**

PUD – Public Utilities Dept.

Ann Fox, Caltrans – Permit Division Jacob Armstrong, Caltrans – Planning Division Michael Pedersen, Catrans – Utilities Division Roy Abboud, Caltrans - Planning Division Amer Barhoumi, City of San Diego, PUD Anthony Van, City of San Diego, PUD JP Semper, Brown and Caldwell (BC) Victor Occiano, Brown and Caldwell

Sign-in Sheet Attached

#### **Meeting Minutes**

The following meeting Minutes are supplemental to the information presented in Attachment A and the notes shown in Attachment B.

• PUD introduced the objectives of the City's Pure Water Program. BC provided a summary of the Task 8 – Miramar Pipeline and Pump Station. Caltrans requested confirmation that no above-ground facilities, such as a pump station, would permanently reside in Caltrans ROW. BC confirmed.



- Caltrans noted that a successful EIR will be key to this project's success, especially when crossing the interstate. Caltrans requested that special attention be given to how Caltrans is referred to in the EIR, and that proper coordination with Caltrans remain ongoing and frequent throughout the EIR process.
- BC presented the seven (7) potential locations identified for crossing I-15. BC then discussed the three (3) preferred options; 1) Mira Mesa Blvd, 2) Via Excelencia, and 3) Miramar Rd.
- <u>Mira Mesa Blvd</u>: Caltrans noted this option as being the least favorable due to high traffic volumes. As a result of traffic volumes, open cutting would be extremely difficult to permit, any impact to on/off ramps would be impermissible, and obtaining approvals would most likely present a schedule impact. This option would only be acceptable if tunneling operations were set back into adjacent properties away from the trafficked streets.
- <u>Via Excelencia</u>: Caltrans noted this as being a favorable option. They requested that BC obtain Caltrans' GIS to design around existing Caltrans storm drains in the area. Caltrans noted that obtaining easements will be important to the crossings success, and therefore starting the process early will be key.
- <u>Miramar Rd</u>: Caltrans noted this as a favorable option. Caltrans noted that for skewed crossings of the interstate the angle of the crossing must be less than 30-degrees from perpendicular. Caltrans requested the BC check the crossing angle, and report back with their findings. Caltrans noted that obtaining easements will be important to the crossings success, and therefore starting the process early will be key.
- PUD also introduced the potential need of utilizing Caltrans land for a parking lot in Caltrans' ROW. PUD shared conceptual drawings showing auxiliary parking, intended to serve the future full-scale Advanced Water Purification Facility, which would be located on the east side of I-805 and north of Eastgate Mall. Caltrans noted that they would have to check with the geotechnical engineers if this land is required for the slope stability of I-805. If not, or if measures can be taken to protect the slope's stability, Caltrans would be willing to relinquish the land to PUD. Caltrans noted that in order to relinquish land the property must first go through a valuation process. Caltrans mentioned that a land agreement to exchange the land for recycled water could be an option.

#### Action Items

- BC to request Caltrans GIS for the subject area.
- BC to check if proposed X-ing at Miramar Road is less than the 30-degree off perpendicular.
- Caltrans to confirm with geotechnical engineers if land next to I-805 is required for slope stability.

#### **Decision Items**

• Caltrans confirmed that the Mira Mesa crossing is the least desirable of the option present, and would be the hardest to permit.

Attachment A – Slide Presentation

Attachment B – Meeting Sign In Sheet

# Potential I-15 Crossings

1. Mira Lee Way

PUBLIC UTILITIES

- 2. Mira Mesa Blvd
- 3. Maya Linda Rd
- 4. Gold Coast Dr
- 5. Carroll Canyon Rd

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- 6. Via Excelencia
- 7. Miramar Rd





# 1. Mira Lee Way

- Property
  - ROW/Private

#### Distance

- 400'
- Adds 4,800 LF
   of pipeline





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# 2. Mira Mesa Blvd

- Property
  - Caltrans
- Distance
  - 2A: 450'
  - 2B: 1200'
  - 2C: 450'
  - 2D: 650'





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# 3. Maya Linda Rd

- Property
  - West side: ROW
  - East side: SDUSD/ROW
- Distance
  - To brush: 450'
  - To cul-de-sac:





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## 4. Gold Coast Dr

- Property
  - West side: ROW
  - East side: Private
- Distance
  - 500'





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# 5. Carroll Canyon

• Property

PUBLIC UTILITIES

- West side: Private
- East side: Private
- Distance
  - 600'



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## 6. Via Excelencia

- Property
  - West side: ROW
  - East side: Private
- Distance
  - To parking lot: 700'





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# 7. Miramar Rd



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## **Caltrans Coordination Meeting Minutes**

Date/Time: August 26, 2015 2:30 p.m. - 4:00 p.m

#### Attendees:

Organization / Department	Phone Number	Email	
City of S.D. Public U	hilitics 858-292-649	2 AVANOSANDIEGO. 6	vo.
11 11 11	858-292-6364	abarhouni ( sanding	12.6
Brown & Caldwell	619-206-0227	sosember@bruncald.	por
CALTERNS-PER	MIT 619 688 3276	ann, fox 2 dot.ca	.90
caltrans	619 638 6968	Roy. Abboud & dot. cd.	190
Cartrans	619-688-6900	Jacob amstranged	A.
CALTERADE OTILITE	\$ 619 6698 7911	Cedon eagas.	g
	Organization / Department City of S.D. Public W """" Brown & Caldwell """" CALTEMNS - PER CO I HANS CAITEMAS OTHER CALTEMAS OTHER CALTEMAS	Organization / Department       Phone Number         City of S.DPublic Unitivic BS8-292-649         ''       ''         Brown & Caldwell       858-292-649         ''       ''         Brown & Caldwell       858-292-6364         ''       ''         ''       ''         Gl9-203-8077         ''       ''         ''       ''         ''       ''         ''       ''         Gl9-206-0227         CALTEMAS       619 629 6768         CALTEANS       019-057-0900         CALTEANS       019-058 7900	Organization / Phone Number Email Department Email City of S.D. Public Unititie 858-292-6492 AVAN@ SANDieGO. ( """" 858-292-6492 AVAN@ SANDieGO. ( Brown & Caldwell 858-292-6492 AVAN@ SANDieGO. ( Brown & Caldwell 858-292-6492 AVAN@ SANDieGO. ( Brown & Caldwell 858-292-6492 AVAN@ SANDieGO. ( Second and Caldwell 858-292-6492 AVAN@ SANDieGO. ( Cauter of Caldwell 858-292-6492 AVAN@ Sandie Caldwell 90 Cauter of Caldwell 858-292-6492 Avan@ Sandie Caldwell 90 Cauter of Caldwell 858-292-6492 Avan@ Sandie Caldwell 90 Cauter of Caldwell 858-292-6492 Avan@ Sandie Caldwell 90 Caldwell 90 Ca

Meeting Purpose: Miramar Pipeline pre-design

Comments:

· Review of preferred alternatives for utilities crossing

Action Items:

- · Cheek culvert at Via Exelencia.
- · Miramar Road 74, verify angle of crossing and texception needed.

· Edsement for palkking > Relinquishment of ROW or decertification



**Attachment B** 

10% HDR Technical Memo #3



#### **Technical Memorandum No. 3**

Meanley Drive to Lake Miramar Pipeline Alignment Analysis

To: Jeff Soriano and Wendy Gamboa Date: August 1, 2016 From: Kathy Haynes/HDR Copy: Doug Biglen/HDR, Dean Gipson/HDR

#### **Objective:**

To determine the most cost effective alignment alternative for the North City Conveyance System from Meanley Drive to Lake Miramar.

#### Introduction:

The purpose of the North City Conveyance System is to convey purified water from the proposed Advanced Water Purification Facility (AWPF) to the Miramar Reservoir. A dechlorination facility is a required element of the system. The optimal location for the dechlorination facility is at the existing Reclaimed Water Tank located at the end of Meanley Drive. With the de-chlorination facility located at the Reclaimed water tank a preferred alignment will need to be selected from Meanley Drive to Miramar Reservoir.

#### Alternative Analysis:

At the roundabout at the easterly terminus of Meanley Drive, three alternative alignments were identified; a description of each Alignment is noted below. Attachment A provides a Map of the Alignments.

- Alignment 1 Is the eastern-most alignment and proceeds east past Meanley Drive onto APN 319-170-23-00 which consists of three multi-story office buildings and associated parking.
- Alignment 2 Is the center alignment and proceeds northeast past Meanley Drive onto APN 319-170-22-00 which is a rough graded parcel part of the Scripps Ranch Business Park Development.
- Alignment 3 Is the western-most alignment and proceeds north outside of the Meanley Drive Right-of-Way along an existing City of San Diego Utility Easement at APN 319-170-33-00 to the Scripps Ranch Public Library Property before heading north east to the Miramar Reservoir.

A rough order magnitude Opinion of Probable Construction Cost (OPCC) was developed to compare the three alternatives along with a review of the alignment alternative's potential advantages and disadvantages, as summarized in the Tables 1, 2 and 3, below. Costs have been rounded to the nearest \$1,000 to conform with the level of magnitude for this estimate.



#### Table 1 - Alternative 1

ALTERNATIVE 1 – APN 319-170-23-00							
<u>Advantages</u>	<u>Disadvantages</u>						
No Utility Conflicts	<ul> <li>Paving Required - Additional costs associated with pavement of parking lot.</li> </ul>						
<ul> <li>No Retaining Wall Required</li> </ul>	<ul> <li>Construction will impact businesses – Pavement construction may impact schedule by adding up to 68 days.</li> </ul>						
	<ul> <li>Temporary and Permanent Easements Required</li> </ul>						

## Alternative 1 Total Cost = \$5,828,000

#### Table 2 - Alternative 2

# Advantages Disadvantages Advantages • Temporary and Permanent Easements Required • No Retaining Wall Required • Passes Through Undeveloped Land

- Additional Pavement Unnecessary.
- Minimal Community Disruption/Impact
- Minimal Landscaping Issues
- Lowest Overall Cost

#### Alternative 2 Total Cost = \$3,982,000

#### Table 3 - Alternative 3

#### ALTERNATIVE 3 - APN 319-170-33-00 **Advantages Disadvantages** • City Owned Property • Multiple Utility Conflicts including a Water Main. • Retaining Wall Required, 550ft long 6ft high - May impact construction schedule by adding up to 110 days. • Passes Through Library and Miramar Lake Parking Lot- High Community impact. • Heavy Tree/Brush landscaped Area-Additional Landscaping Required. • Temporary and Permanent Easements Required. • Impact to Construction Schedule due to Retaining wall – May impact construction schedule up to 23 days. Additional Costs Associated with Concrete Path Required. • Highest Overall Cost

Alternative 3 Total Cost = \$5,001,000



\*OPCC's costs are provided as Attachment 2

#### **Recommendation:**

Alternative 2 -APN 319-170-22-00 Central Alignment has the least total cost at approximately \$3,982,000. Along with being the most cost effective option, Alternative 2 has the least impact on adjacent businesses and the community. Based on these considerations HDR recommends Alternative 2 – Central Alignment.

#### **Disclaimers:**

- Depending on Property Owner's willingness to provide an easement, Alternative 1 Eastern Alignment could be considered a viable option.
- OPCC were developed as a rough order magnitude intended for comparison use only.

#### Attachments:

- 1. Alignment Map
- 2. Opinion of Probable Construction Costs (OPPC's)



FJS

Attachment 1 – Alignment Map



Draft NCCS TM No 3 20160731\_AH

4 of 5

8/1/2016



#### Attachment 2 – Opinion of Probable Construction Costs

ALTERNATIVE NUMBER	1	2	3
Open Cut Trench Construction Length (ft)	1,832	775	743
Open Cut Trench Construction Unit Cost (\$/ft)	\$1,054	\$1,054	\$1,054
Open Cut Trench Construction Cost	\$1,930,928	\$816,850	\$783,122
Tunnel Launch Shaft Depth (ft)	92	92	92
Tunnel Launch Shaft Unit Cost (\$/ft)	\$7,000	\$7,000	\$7,000
Tunnel Launch Shaft Cost	\$644,000	\$644,000	\$644,000
Tunnel Receiving Shaft Depth (ft)	40	20	14
Tunnel Receiving Shaft Unit Cost (\$/ft)	\$3,000	\$3,000	\$3,000
Tunnel Receiving Shaft Construction Cost	\$120,000	\$60,000	\$42,000
Total Tunnel Construction Length (ft)	595	755	1,222
Tunnel Construction Unit Cost (\$/ft)	\$2,232	\$2,232	\$2,232
Tunnel Construction Cost	\$1,328,040	\$1,685,160	\$2,727,504
Construction Cost	\$2,092,040	\$2,389,160	\$3,413,504
6' High Retaining Wall Construction Length (ft)	0	0	550
6' High Retaining Wall Construction Cost (\$/ft)	\$301	\$301	\$301
Retaining Walls Cost	\$0	\$0	\$165,550
Asphalt Paving R&R, 4" thick (yd <sup>2</sup> )	1,573	0	146
Asphalt Paving R&R, 4" thick, Unit Cost (\$/yd <sup>2</sup> )	\$41	\$41	\$41
Total Asphalt Paving R&R, 4" thick Cost	\$64,493	\$0	\$5,986
PCC Sidewalk R&R, 4" thick, Area (yd <sup>2</sup> )	0	0	310
PCC Sidewalk R&R, 4" thick, Unit Cost (\$/yd <sup>2</sup> )	\$75	\$75	\$75
Total PCC Sidewalk R&R, 4" thick, Cost	\$0	\$0	\$23,250
Total Asphalt Paving R&R and PCC Sidewalk R&R Cost	\$64,493	\$0	\$29,236
Landscaping/Irrigation R&R Area (ft <sup>2</sup> )	0	0	1,400
Landscaping/Irrigation R&R Unit Cost (ft <sup>2</sup> )	\$0	\$0	\$12.25
Total Landscaping/Irrigation R&R cost	\$0	\$0	\$17,150
Permanent 30' Wide Waterline Easement Length (ft)	1,852	825	630
Permanent 30' Wide Waterline Easement Area (ft <sup>2</sup> )	55,560	24,750	18,900
Permanent 30' Wide Waterline Easement Unit Cost (\$/ft <sup>2</sup> )	\$30	\$30	\$30
Permanent 30' Wide Waterline Easement Cost	\$1,666,800	\$742,500	\$567,000
Temporary 10' Wide Construction Easement Length (ft)	1,852	825	630
Temporary 10' Wide Construction Easement Area (ft <sup>2</sup> )	18,520	8,250	6,300
Temporary 10' Wide Construction Easement Unit Cost (\$/ft <sup>2</sup> )	\$4	\$4	\$4
Temporary 10' Wide Construction Easement Cost	\$74,080	\$33,000	\$25,200
Total Permanent and Temporary Easement Cost	\$1,740,880	\$775,500	\$592,200
Total Construction and Easement Cost	\$5,828,341	\$3,981,510	\$5,000,762

PCC - Portland Cement Concrete R&R - Remove and Replace



**Attachment C** 

Scripps Ranch Technology Park Lot 3 Concept





Attachment D

PM Letter to Murphy Development – Evaluation of Additional Alignments


## THE CITY OF SAN DIEGO

# MEMORANDUM

DATE:	September 14, 2016
TO:	Scripps Ranch Technology Park/Murphy Development
FROM:	Jeff Soriano, Project Manager, Public Utilities Department, Pure Water
SUBJECT:	Pure Water Alternate Alignment Evaluation West of Lot 3

The design consultant initially looked into an alignment west of Lot 3. This alignment extended from Meanley Drive and followed along the walking path to the Scripps Ranch Library (bright green line in **Figure 1** below). From the library parking lot, pipeline installation in a northeasterly direction would continue in order to arrive at the final location at the Miramar reservoir. As you'll see in the image below, the alignment is significantly longer than the proposed alignment (teal line, **Figure 1**) shown from Lot 3, with many utilities to cross under and the dam and other facilities to pass alongside. Similarly, an alignment further west of this would impose an even longer path to the final destination with the same utilities, dam and facilities to contend with.

## FIGURE 1:



**Figure 2** shows the proposed pipeline alignment (seen as a red line) overlaid on the geology map, with the zoomed in view showing that the area to the west of Lot 3 is found within the "Metasedimentary and metavolcanic rock" formation, labeled "Mzu" (Green area). It was brought to the City's attention that this formation can range in compressive strength of up to 50,000 pounds per square inch, where special mechanical excavation techniques, including blasting would be required. Similarly, an alignment further west, starting from an area that Murphy Development staff refers to as Lot 13, would require an even longer stretch of pipeline within this same material. Trenching via "mechanical excavation techniques" or "blasting" is not possible. In regards to "mechanical excavation", a large enough excavation to install the 48-inch diameter pure water pipeline on Scripps Lake Drive is not possible due to the conflicts with a large number of existing utilities previously mentioned. In addition, "blasting" is not allowed because of the close proximity to the dam and the San Diego County Water Authority water aqueducts.

#### FIGURE 2:



 Qya, Qvop1, Qvop2, Qvop4, Qvop5, Qvop6, Qvop7, Qvop8: Young Alluvium and Paralic Deposits

 Tst: Stadium Conglomerate
 Tf: Friars Formation
 Tt: Torey Sandstone

 Tp: Pomerado Conglomerate
 Mzu: Metasedimentary and metavolcanic rocks

# September 14, 2016 Pure Water Alternate Alignment Evaluation West of Lot 3

In summary, any alignment crossing Scripps Lake Drive from the west of Lot 3 imposes the following issues and risks that deem these alignments impractical: 1. Existing utility conflicts along Scripps Lake Drive; 2. Required long stretches of tunneling, excavations, blasting methods through strong volcanic material and; 3. endangering the stability of SDCWA aqueducts, Miramar Dam and other existing utilities and facilities along Scripps Lake Drive. In conclusion, the crossing at Lot 3, as shown in **Figure 2** is just outside the "Mzu" material and is the shortest tunnel/excavation length crossing the utilities in Scripps Lake Drive. Based on these high risk and prohibitive construction methods, the City concludes that the crossing from Lot 3 is the most practical and constructible path.

Jeff Soriano Project Manager



**Attachment E** 

**Murphy Development Letter to City** 

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James G. Sandler • Direct (619) 615-6772 • Email jsandler@sllby.com

April 13, 2017

## VIA E-MAIL

Jeff Soriano Project Manager City of San Diego Public Works Department City of San Diego

Re: Scripps Ranch Technology Park/Miramar Pure Water Pipeline and Pump Station Project

Dear Mr. Soriano:

This firm represents Scripps Ranch Technology Park LLC (SRTP) with regard to the City's plans to construct the Miramar Pure Water Pipeline and Pump Station.

The Scripps Ranch Technology Park is a full vested, fully entitled development project. Entitlements date back to 1986, when a Planned Industrial Development Permit was obtained. The EIR for the project has been approved. Lots are graded, streets, curbs and sidewalks are in place. SRTP intends to fully develop the entire Park, including Lot 3. Lot 3 is the portion of SRTP's property which the City, as of August 2016, now wishes to encroach upon to construct the Miramar Pure Water Pipeline and Pump Station.

We understand that the City's most recent proposal is to seek to acquire a permanent easement and a temporary construction easement across SRTP's single point of access to Lot 3 of the SRTP project, effectively denying access to Lot 3 for the duration of the still unscheduled construction of the City's project. Without access to the property, and depending on when the City elects to take the easement, development of the property becomes unfeasible and/or the developed property becomes uninhabitable. In either scenario the City's action will constitute a full taking of a 6 acre property, the fair market value of which, **today**, before any buildings are constructed, may exceed \$11M.

We also understand that other alternatives are available to the City to fulfill the purposes for which acquisition of easements from SRTP is sought. None of the alternatives will render any property unbuildable or uninhabitable, as would be the effect of the City's current plans regarding SRTP Lot 3. For example, the City could choose a route involving NO taking or

April 13, 2017 Page 2

acquisition of private property by routing the project along public right of ways (Scripps Ranch Boulevard and Scripps Lake Drive).

If the City persists in its intention to take easements across the only ingress and egress to SRTP Lot 3, attempted exercise of the City's eminent domain powers will be necessary. Considering the available alternatives, the City will not be able to fulfill the requirements of California Code of Civil Procedure Section 1245.230 which provides in pertinent part that no Resolution of Necessity can issue, and no condemnation can proceed, unless:

"The proposed project is planned or located in the manner that will be most compatible with the greatest public good and the least private injury."

With these facts in mind, SRTP urges the City to reconsider its plans. SRTP is prepared to meet with City representatives to assist the City in considering alternatives to the current plan to take easements across the only ingress and egress to SRTP Lot 3. SRTP sees no possibility, however, of cooperation or compromise which would lead to agreement on any variation of the current City plans if Lot 3 is to be involved.

SRTP looks forward to the City's moving on from current plans and adoption of alternatives.

Your response may be made directly to Kaitlin Arduino at Murphy Development. I will appreciate your also forwarding a copy of this letter to the appropriate representative of the City Attorney's office; contact from the City Attorney's office should be made to me.

Thank you.

Very truly yours,

Junes A. Landler

James G. Sandler

cc: Scripps Technology Business Park



Attachment F

Email Exchange City to Mr. Wulfeck

Hello Wally,

I'm the Project Manager for the pipeline design and wanted to provide you a response in Keli's absence regarding our evaluation of the alternate alignments you asked about below.

The "yellow" alignment would need to be adjusted to first travel to the location of our dechlorination facility proposed on City property at the top of Meanly Drive and then back-track down Meanley Drive again to the location where the pipeline would enter the landscaped parcel. There is no space within Meanley Drive for two 48" pipeline alignments due to the many existing underground utilities within this street.

The "orange" alignment was found to have conflicts with an existing water main in the same location, would require heavy tree/landscape removal and construction of retaining walls.

In addition, for both suggested alignments, entering into the reservoir at the locations shown is not feasible because the pipe would need to penetrate through the dam. Finally, the tunnel crossing of Scripps Lake Drive would be difficult due to the dense volcanic geotechnical formation discovered west of our proposed alignment which would involve blasting operations to remove. Blasting around the CWA aqueducts and the dam is not advisable.

If there's any questions related to the design that you may have, please feel free to contact me.

Thanks,

## **Jeff Soriano** Project Manager City of San Diego Public Works Department

(858) 292-6336

From: <a href="http://www.ifeck@gmail.com">whwulfeck@gmail.com</a>] On Behalf Of Wally Wulfeck Sent: Wednesday, April 26, 2017 3:57 PM

To: Balo, Keli <<u>KBalo@sandiego.gov</u>>

Cc: Rob@itsthewinetalking.info <srilko4@aol.com>; Soriano, Jeff <JSoriano@sandiego.gov>; Lemons, Sarah <<u>SLemons@sandiego.gov</u>>; Genevieve Fong <<u>gfong@cookandschmid.com</u>> Subject: Re: Scripps Miramar Ranch Community Planning Meeting 5-4-17 North City Project

Thank you Keli, Could you please comment on why the alternative routes I've shown in yellow or orange on the attached picture were not chosen? These are what we asked for 6 months ago.

--Wally

# On Wed, Apr 26, 2017 at 3:22 PM, Balo, Keli <<u>KBalo@sandiego.gov</u>> wrote:

Please see sheets from the design plans that show the location of the tunnel at Scripps Ranch Technology Park. The pipeline is proposed along the eastern boundary of the parcel and the tunnel shaft would be located just south of Evans Pond.

I have also included a draft figure from the biology report that zooms out a bit so you can see the pipeline alignment in relation to the library.

If you have any other questions, please let me know! I look forward to coming to your meeting next week.

Keli

